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Earth Science Strategies Consulting, Inc. ▽

**2013 Supplemental Monitoring Well
Installation Report
Island County Solid Waste Complex
Coupeville, Washington**

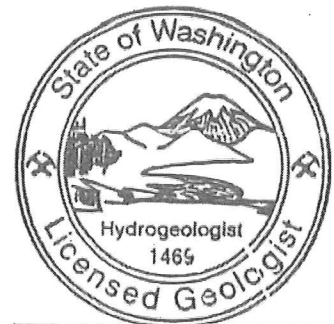
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for:

Island County Department of Public Works
Solid Waste Division

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1 INTRODUCTION

This report documents the drilling, installation, and hydraulic testing of four supplemental monitoring wells completed at the Island County Solid Waste Facility (ICSWF) located in Island County, Washington (Figure 1). This work, conducted between June and August 2013, was completed in partial fulfillment of the objectives outlined in the *“Work Plan for Further Assessment of Groundwater Contamination by Vinyl Chloride”* dated January 28, 2013, and specifically according to the *“Scope of Work and Field Procedures”* document dated April 24, 2013.

The objective of the work was to support the on-going evaluation of groundwater contamination by vinyl chloride (VC) at the site, specifically by establishing additional monitoring points necessary to better define the impacted area and evaluate plume dynamics (i.e. whether or not the VC plume is expanding, static, or shrinking). A secondary objective of the work was to provide additional aquifer property data and calibration points for future fate and transport modeling. This future additional evaluation work, when completed after two years of monitoring, aims to determine whether or not off-site migration of VC is possible and thus whether or not active remediation is necessary.

This report documents the drilling and construction of the new monitoring wells, presents the results of laboratory and field testing activities, and summarizes lithologic and groundwater flow information in the form of cross sections and a groundwater elevation map, respectively. Evaluations of the nature and extent of vinyl chloride and synthesis of this information into a revised conceptual model, as well as evaluations of plume stability and predicted contaminant fate and transport and the likelihood of off-site transport will be presented in future documents after long term monitoring data have been collected.

2 BOREHOLE ADVANCEMENT

LOCATIONS

The locations of the four new monitoring wells (E5D, E6D, E7D, and E8D) in relation to existing wells and other site features are shown on Figure 2. The locations were selected to provide upgradient (E6D), lateral (E7D and E8D) and downgradient (E5D) monitoring surrounding the well (E2D) where vinyl chloride has repeatedly been detected at the highest concentrations at the site. Monitoring well E6D was installed in the road and was completed with a flush-mount well monument adjacent to the west side of the scale house and directly east of the County's closed Municipal Solid Waste (MSW) landfill. Well E7D, also installed in the road and completed with a flush-mount monument, was installed approximately 275 feet north of the scale house, and 70 feet east of the roll-off container tipping walls, directly east of the County's closed Construction, Demolition, and Landclearing (CDL) landfill. Flush-mounted wells are bolted shut with a gasket to seal out precipitation and stormwater runoff. Well E8D was installed in a wooded area approximately 275 feet east of the scale house, and E5D was installed 450 feet northeast of the scale house at the edge of the trees. Monitoring wells E5D and E8D were both installed with locking "stick-up" well monuments, each of which were surrounded by protective bollards and painted bright yellow for visibility.

DRILLING OPERATIONS, SOIL SAMPLING, AND LOGGING

Drilling and well construction was completed by Cascade Drilling, LP (Woodinville, WA) at the direction Earth Science Strategies Consulting, Inc. (ESSCI). All of the work was directly supervised by either a licensed hydrogeologist, or a staff geologist who was under the direct supervision of (and in regular cellular telephone contact with) a licensed hydrogeologist.

Boreholes were advanced using the sonic drilling method which employs the use of high-frequency, resonant energy to advance a core barrel or casing into subsurface formations. The method is very fast, produces less investigation-derived waste than other methods, and produces nearly continuous relatively undisturbed core samples. At this site the sonic drill was used to advance a core barrel (either 6" or 8" diameter) up to 20' per "core run" depending on geologic conditions. After retrieving the core barrel from each "run", the cores were extracted for geologic logging and soil sampling, and a temporary steel casing was advanced to the point that the core barrel was advanced, and the hole cleaned out (again using a core barrel; however the sediments removed from this core run were disturbed and unsuitable for logging and sampling). The procedure was repeated until the desired depth was reached. Target depths were determined based on observed geologic materials. The drilling objective was to reach the same water bearing zone ("Aquifer 2") screened by monitoring well E2D. This zone is located beneath an approximate 50-ft thick semi-confining layer (generally extending from around 150 ft bls to approximately 200 ft bls). The specific depth and screen zone was selected based on generalized formation grain size (i.e. the well screen would be set adjacent to the most permeable materials within the target zone).

At this site, a conductor casing/telescopic procedure was used to prevent cross-contamination during drilling between the shallow (~145-150') water bearings zone ("Aquifer 1" or the perched zone) and the target zone at ~200-220'. A 10" diameter temporary casing was implemented for monitoring wells E5D and E6D; 8" diameter for E7D and E8D. These temporary casings were advanced approximately 5 feet into the semi-confining layer. At this point the casing was retracted approximately 1' and bentonite slurry was pushed out of the borehole to seal the casing into the confining layer. After the bentonite had set at least 12 hours, drilling proceeded using smaller diameter core barrels (4" or 6") and steel casing (8" for E5D and E6D; 6" for E7D and E8D) until the target depths were reached (between 200 – 220' below land surface).

All soil cores were extruded into plastic liners for inspection, logging, and sampling by the on-site geologist as appropriate. Boring logs are presented in Appendix A. Following visual description of the removed soil cores, soils from the saturated zones were checked for the presence of VOCs by placing soil collected from the core at 10' intervals into zip-top plastic bags (one for each interval), allowing the soil to equilibrate with the headspace air in the bag for approximately 15 minutes, and then evaluating the headspace air with a photoionization detector (PID). All PID readings were recorded on the boring logs and no samples were found to have significant PID values, although some minor detections were encountered at well E7D. All soil cuttings were placed in steel storage bins and transported to an appropriate area on ICSW property, as designated by ICSW staff. All drilling equipment was decontaminated between boreholes to minimize the potential for cross-contamination. Due to the nature of the drilling method, the drilling process did not generate contaminated water that would require special handling or disposal.

LABORATORY TESTING

Samples for laboratory analysis were collected from the cores extracted from the boreholes at the elevations of the monitoring well screens, and from select locations at E7D where PID readings were noted. Samples for VOC analysis were collected from the extruded cores using laboratory-provided disposable undisturbed core samplers and placed directly in laboratory-provided 40 ml vials containing methanol preservative. Other parameters (geotechnical) were sampled by the laboratory from undisturbed cores which were left in the plastic liners, secured for shipping, and delivered to the laboratory in a sample cooler. All samples for chemical analysis were packed in a cooler maintained at 4 degrees C and shipped to TestAmerica, a certified analytical laboratory. Analyses included:

- Volatile Organic Compounds
- Total Organic Carbon (TOC)
- Bulk Density
- Grain Size
- Porosity

The laboratory report from TestAmerica is provided in Appendix B.

Results of the laboratory geotechnical analyses are summarized in Table 1

Results of the chemical (TOC and VOC) analyses are summarized in Table 2. While some VOCs were detected in the soil samples, the concentrations may be considered minimal, and some of the detections are thought to be false positives/laboratory artifacts as evidenced by the detections of some constituents in the laboratory “blank” sample.

Table 3 shows hydraulic conductivity values determined using methods by Hazen, Shepherd, and Zhang & Brusseau using the grain-size distribution curves and percentages of gravel, sand, silt, and clay. These hydraulic conductivity values are considered complementary to the primary values determined by slug testing (discussed in the next section).

3 WELL COMPLETION AND TESTING

WELL CONSTRUCTION

Once the target depth was reached at each location, a groundwater monitoring well was installed. All monitoring wells were installed in accordance with WAC 173-160-420 (Minimum Standards for Construction and Maintenance of Wells; Resource Protection Wells) and according to industry standard practices for resource protection wells. Complete construction details are shown on the well logs and construction diagrams contained in Appendix A.

Monitoring wells E5D and E6D were completed as 4” diameter wells with “v-wrap” screens (for greater open area than machine-slotted) to facilitate possible future use as remediation wells (injection or extraction), should this be necessary. Monitoring wells E7D and E8D were completed as 2” diameter wells, also with “V-wrap” screens. All screens are 5’ long. Screen slot size and sandpack specifications were selected as was appropriate for the materials encountered in the screen zones. Schedule 80 PVC casing with centralizers were used for all monitoring wells. Following placement of the clean silica sand filter in the annular space between each well screen and the formation, the well was vibrated and/or surged (depending on the circumstances of each well) to ensure the filter pack was seated and there were no void spaces. The remainder of the annular space in each boring was filled with hydrated chip bentonite, pelletized bentonite, or bentonite grout, as conditions dictated while the steel casing was extracted. The PVC well casings (for wells E5D and E8D) terminate approximately 2.5 feet above the ground surface except. For the wells that are located in the road (locations E6D and E7D), the PVC casing terminates approximately 6” below the ground surface. Each well was completed at the ground surface with either flush-mount subsurface vaults for wells in traffic areas, or “stick-up” protectors surrounded by protective bollards in other areas depending on location. Well identification tags were affixed to the wells and permanent elevation reference points marked on the top of the PVC casings. Following construction, the wells were surveyed

for vertical and horizontal position by TMI Surveying, Inc. of Freeland, WA. A summary of the survey information and well construction data is provided in Table 4.

MONITORING WELL DEVELOPMENT

Each groundwater monitoring well was developed by ESSCI staff following installation to ensure good hydraulic connection between the well and the aquifer and to remove any solids that could potentially be mobilized during the sampling process. Sometimes a “skin” of clay and silt particles forms on the borehole wall as a result of drilling. The development process is designed to remove this skin to maximize hydraulic continuity, as well as to “set” the sand pack and remove fine-grained material from the area immediately adjacent to the well screen. Adequate development is necessary to ensure accurate water levels (fine soil particles can plug the well openings), the long-term viability (water production ability) of the wells, and the elimination of artifact turbidity in samples which can interfere with laboratory analyses. Well development was accomplished using surging and high volume pumping. Turbidity was measured and documented during the development process, and development proceeded until turbidity declined and stabilized to the maximum extent possible. All non-disposable well development equipment was decontaminated using a non-phosphate detergent (Alconox) wash and an initial clean tap water rinse followed by a final distilled water rinse. A summary of information collected during development is provided in Table 5.

HYDRAULIC CONDUCTIVITY TESTING

Slug tests were conducted on the new monitoring wells to measure hydraulic conductivity. The slug testing procedure involves instantaneously removing a “slug” of water from the well casing and measuring the rate of water level recovery. A variety of mathematical treatments (depending on geologic conditions and monitoring well configuration) may then applied to the data to develop an estimate of hydraulic conductivity. Given that the wells screens from these proposed monitoring wells were submerged (i.e., the top of the screen is below the static water level), the casing pressurizing method of slug testing was used to maximize the size of the “slug,” which increases the representativeness of the results. To implement the slug tests, a pressure transducer was installed in each well prior to testing. The well casing was then be pressurized with nitrogen to depress the static water level in the well to a point just above the well screen. After the desired displacement was achieved and the water level and casing pressure stabilized, the pressure was be instantaneously released from the well casing, allowing the water level to rise. The change in water level was recorded by the pressure transducer connected to a data logger. At least two slug tests were completed at each of the four groundwater monitoring wells. The pressure transducer was decontaminated between each well using the standard three-step process of Alconox detergent, followed by a tap water rinse, and finally a distilled water rinse. Table 6 presents a summary of the slug test results. Appendix C presents more detailed calculations of graphs of the well response during testing.

SAMPLING PUMP INSTALLATION

Following well development and slug testing, each new monitoring well was outfitted with a dedicated QED model T1250 (stainless-steel body, Teflon bladder) bladder pump with stainless steel inlet screen, Teflon-lined polyethylene tubing, well cap, and discharge adaptor. Pump intakes were set at the midpoint of each screen. Table 7 presents the details of each sampling pump installation.

4 CONDITIONS ENCOUNTERED

STRATIGRAPHY AND AQUIFER PROPERTIES

The geologic conditions encountered were generally consistent with those reported by previous investigations conducted at the Site. Following a thick (approximately 140') unsaturated zone composed of coarse sand and gravel, a semi-confining layer of gray, fine-grained sediments separates a discontinuous perched water-bearing zone (encountered at approximately 60' above msl, or approximately 140' bls) from a lower aquifer encountered at approximately 0' msl, or 200' bls. This sequence is illustrated in the cross-sections shown in Figures 3 and 4 (cross section locations are shown on Figure 2). No modifications to the existing geologic site conceptual model are suggested by the new borings, however the slug testing conducted showed a significantly lower hydraulic conductivity (0.8 – 10 ft/day) than had previously been suggested by grain size data (224 ft/day) for this aquifer. Because the slug testing is considered to be more reliable than the previous estimates from grain size data, this is considered a significant revision to the assumptions governing groundwater flow and transport.

GROUNDWATER FLOW

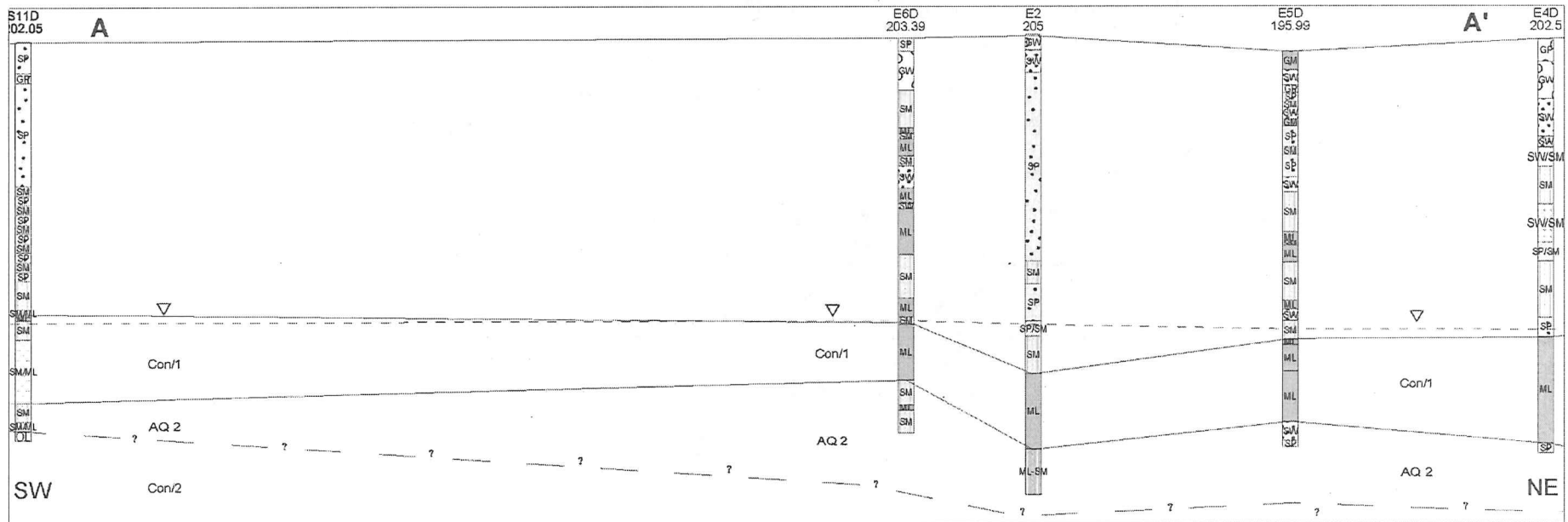
Depth to water measurements were collected in July 2013 from the four new monitoring wells in addition to existing monitoring wells E2D, E3D, E4D, and N3D and converted to water level elevations based on the surveyed top of casing elevations. Figure 5 shows the water level elevations, interpolated contours, and assumed groundwater flow direction. Consistent with previous investigations, the generalized groundwater flow direction was found to be from southwest to northeast, with a gradient of 0.004 ft/ft. Based on the revised hydraulic conductivity of 1-10 ft/day, and the measured porosity average of 33% (Table 1), the estimated groundwater flow velocity is between 4.4 – 44 ft/year. This is considerably slower than had been previously assumed due to the lower hydraulic conductivity and higher porosity (prior to the measurements collected by this investigation indicating a porosity of 33%, an assumption of 25% was used previously to estimate groundwater flow velocities).



Figure 1. Site Location



Figure 2. Monitoring Well and Cross Section Locations

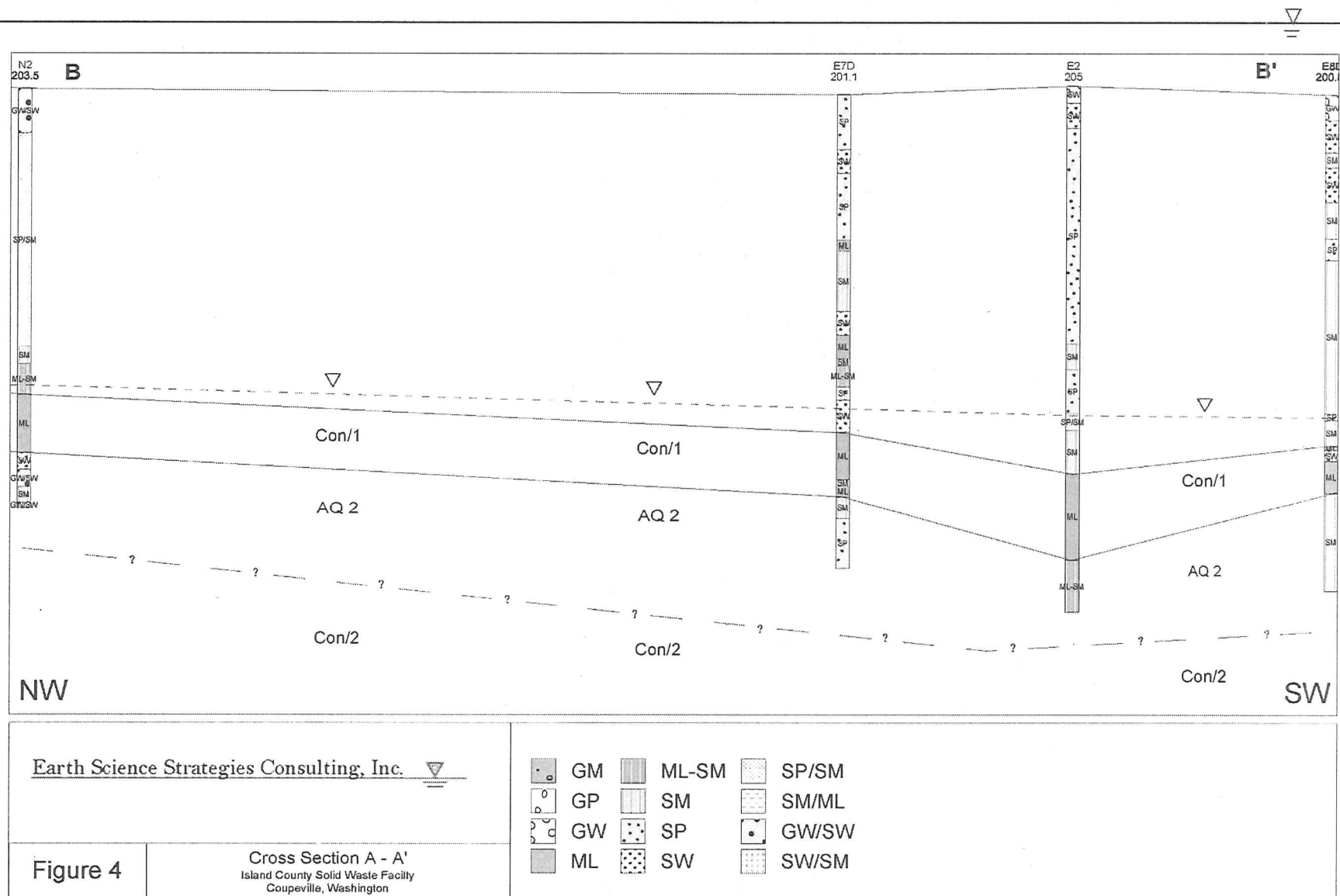


Earth Science Strategies Consulting, Inc. 

Figure 3

Cross Section A - A'
Island County Solid Waste Facility
Coupeville, Washington

	GM		ML-SM		SP/SM
	GP		SM		SM/ML
	GW		SP		GW/SW
	ML		SW		SW/SM





↗ Assumed groundwater flow direction

Figure 5. Groundwater Elevation Contour Map, July 2013



Table 1. Summary of Aquifer Sample Geotechnical Properties

Sample ID	Depth [ft]	% GRAVEL			% SAND			% Fines	Bulk Density [g/cm ³]	Porosity [%]	Soil Description
		cobbles and boulders	coarse	fine	coarse	medium	fine				
E5D	203	0.0	0.0	0.0	0.0	5.9	63.3	30.8	1.76	35.9	Gray, med SAND, little to no fines
E5D	208	0.0	0.0	27.1	14.6	21.8	19.6	16.9	2.29	17.8	Gray, silty, gravelly, coarse SAND
E6D	210	0.0	0.0	0.0	0.0	0.1	76.0	23.9	1.65	39.3	Gray, fine SAND
E7D	210	0.0	0.0	0.0	0.0	12.5	66.5	21.0	1.84	33.5	Brown, slightly silty, med SAND
E8D	212	0.0	0.0	0.0	0.0	0.0	87.5	12.5	1.58	41.3	Gray, silty fine SAND

MSL mean sea level

Table 2. Summary of Chemical Analyses

Sample Name	TOC [mg/Kg]	Methylene Chloride [ug/Kg]	Chloromethane [ug/Kg]	Bromomethane [ug/Kg]	Toluene [ug/Kg]	1,2,4-Trichlorobenzene [ug/Kg]	1,2,3-Trichlorobenzene [ug/Kg]	Methyl acetate [ug/Kg]	Acetone [ug/Kg]
5-203	<1230	43 J, H	<130	<130	<130	<130	<130	<130	<670
5-208	<1120	34 J, H	140 H, B	230 H, B	45 J, H	23 J, H, B	26 J, H, B	<120	<590
6-210	1890	<67	<67	<67	<67	<67	<67	<67	<330
7-20	NA	<54	47 J, H, B	67 H, B	<54	<54	<54	37 J, H	<270
7-40	NA	<49	31 J, H, B	47 J, H, B	<49	<49	<49	<54	<240
7-70	NA	<38	<38	<38	<38	<38	<38	<38	<190
7-160	NA	<64	18 J, H, B	<64	37 J, H	<64	<64	53 J, H	98 J, H
7-210	<1140	<66	<66	<66	<66	<66	<66	<66	<330
8-212	<1200	<58	61 H, B	85 H, B	<58	<58	<58	<54	<290
VHBKLM	NA	<48	44 J, H, B	65 H, B	<48	<48	<48	<48	<240

B compound was found in blank and sample

U indicates the analyte was analyzed for but not detected

* recovery or RPD exceeds control limits

J result is less than the RL but greater than or equal to the MDL and the concentration is an approximate value

H sample was prepped or analyzed beyond the specified holding time

NA not analyzed

Table 3. Hydraulic Conductivity Estimates from Grain Size Data

Sample ID	Depth [ft]	Grain Size Data						Calculated Hydraulic Conductivity [ft/day]				
		% Gravel	% Sand	% Silt	% Clay	D10 (mm)	D50 (mm)	Hazen H	Shepherd Ch	Zhang & Brusseau	Alyamani & Sen	Average
E5D	203	0	69.2	20.5	10.3	0.045	0.125	6.89	14.56	0.17	2.61	6.05
E5D	208	27.1	56	11.3	5.6	0.020	1.000	1.36	450.00	0.04	130.06	145.37
E6D	210	0	76.1	15.3	7.6	0.020	0.120	1.36	13.61	0.40	5.17	5.14
E7D	210	0	79	5.3	15.8	0.038	0.250	4.91	45.69	0.48	0.33	12.85
E8D	212	0	87.5	8.4	4.2	0.074	0.160	18.63	21.88	1.37	12.48	13.59

H - uniform highly sorted sand, C=1200

Ch- Channel

Table 4. Monitoring Well Survey Data and As-Built Measurements

Well Name	July 2013 Survey Coordinates ⁽¹⁾			Site Coordinates ⁽²⁾			July 2013 Event		TD [bgs]	TOS [bgs]	TOSE	BOS [bgs]	BOSE	SMPE
	Easting	Northing	TOCE	Easting	Northing	TOCE	DTW	WLE						
E5D	1198000	446406	202.32	10277	9758	198.56	146.57	51.99	211.30	206.30	-7.75	211.30	-12.75	-10.25
E6D	1197630	446155	206.73	9907	9507	202.97	149.6	53.37	209.25	204.25	-1.29	209.25	-6.29	-3.79
E7D	1197688	446421	204.26	9965	9773	200.50	147.56	52.94	209.50	204.50	-4.01	209.50	-9.01	-6.51
E8D	1197879	446171	206.89	10156	9522	203.13	150.83	52.30	214.60	209.60	-6.47	214.60	-11.48	-8.97

(1) As provided by TMI, Inc. - NAD-83/96 Horizontal and NAVD 88 Vertical

(2) Converted to local relative coordinates system and NAVD 1929 Vertical, as used in previous studies (by others)

TOCE - top of casing elevation (ft msl)

DTW - depth to water below top of casing (ft)

WLE - water level elevation (ft msl)

TD - total depth of boring (ft)

bgs - below ground surface

TOS - depth to top of screen (ft)

TOSE - top of screen elevation (ft msl)

BOS - depth to bottom of screen (ft)

BOSE - bottom of screen elevation (ft msl)

SMPE - screen midpoint elevation (ft msl)

Table 5. Well Development Information


Well ID	Depth to Water [ft]	Depth to Bottom [ft]	Length of Stick-up [ft]	Post-Development Turbidity [ntu]
E5D	146.54	211.30	1.57	30.1
E6D	149.00	209.25	-0.42	1.21
E7D	147.33	209.50	-0.60	171
E8D	150.44	214.60	2.28	28.8

Table 6. Hydraulic Conductivity Results From Slug Test Data

Test Name	Hvorslev Method			Bouwer & Rice Method			AVG [ft/day]
	K [ft/sec]	K [ft/day]	K [cm/s]	K [ft/sec]	K [ft/day]	K [cm/s]	
E5D 1	1.1E-04	9.5	3.4E-03	1.3E-04	11	3.9E-03	10.3
E5D 2	9.8E-05	8.5	3.0E-03	1.1E-04	9.4	3.3E-03	8.9
E5D 3	6.9E-05	6.0	2.1E-03	7.9E-05	6.8	2.4E-03	6.4
E6D 2	9.7E-06	0.8	3.0E-04	1.0E-05	0.88	3.1E-04	0.9
E6D 3	9.2E-06	0.8	2.8E-04	1.0E-05	0.89	3.1E-04	0.8
E7D 1	4.7E-05	4.0	1.4E-03	4.9E-05	4.2	1.5E-03	4.1
E7D 2	3.9E-05	3.4	1.2E-03	3.9E-05	3.4	1.2E-03	3.4
E8D 1	1.4E-05	1.2	4.2E-04	1.5E-05	1.3	4.6E-04	1.2
E8D 2	1.5E-05	1.3	4.6E-04	1.6E-05	1.4	4.9E-04	1.3
E8D 3	1.2E-05	1.1	3.7E-04	1.4E-05	1.2	4.2E-04	1.1

Appendix A

Boring Logs and Well Completion Diagrams

Earth Science Strategies Consulting, Inc. 

Started/Completed : 6/3/2013
 Drilling Method : Sonic Core
 Diameter : 10" to 150', 8" to 210'
 Sampling Device : Sonic Core
 Drilled By : Cascade Drillers, LP

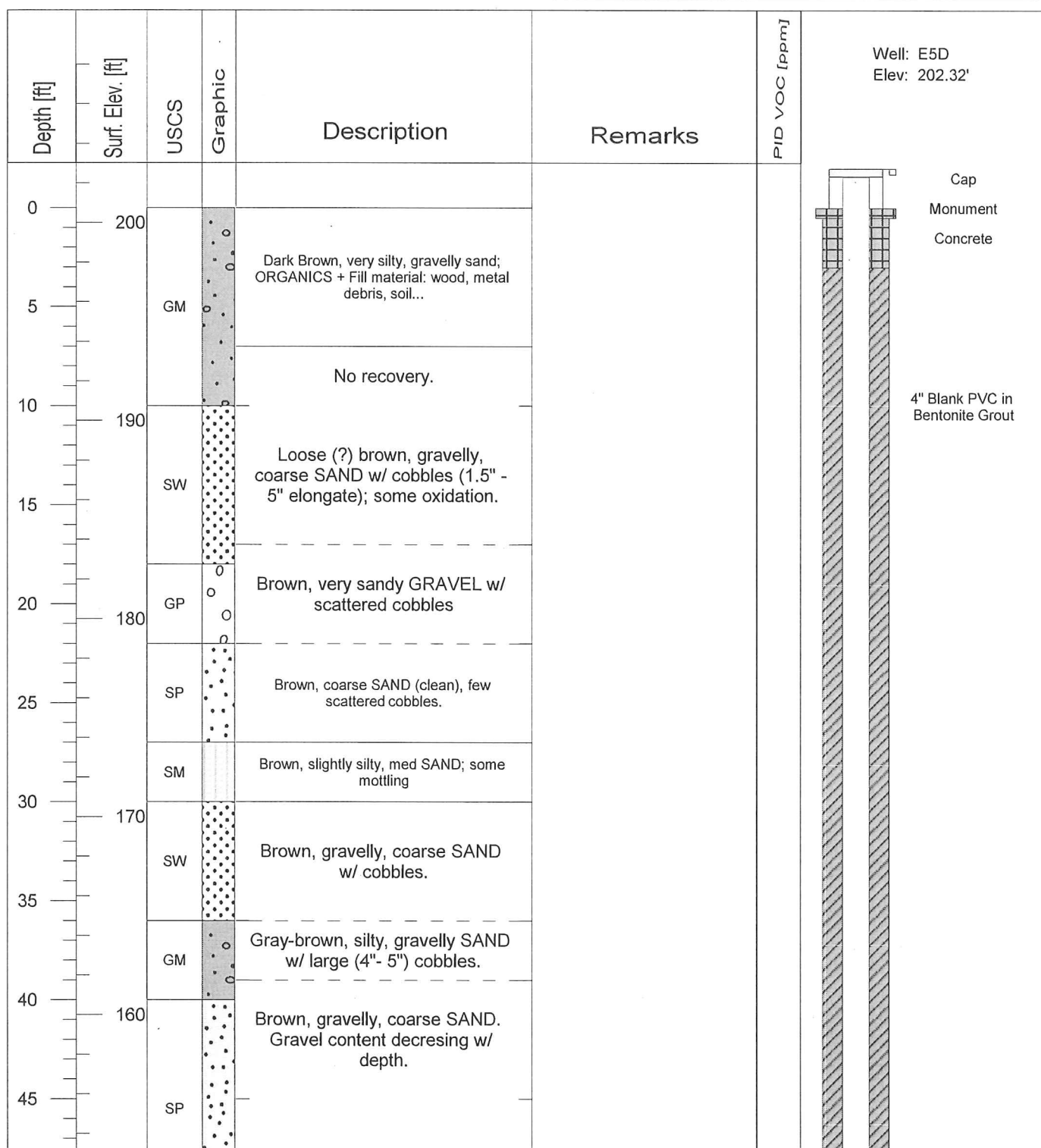
Monitoring Well: E5D

(Page: 1 of 4)

Island County Solid Waste Facility
 Coupeville, Washington

Total Depth : 210
 Logged By : Mark Varljen
 Reviewed By : Mark Varljen

Surveyed By : TMI
 Northing Coordinate : 9740.03
 Easting Coordinate : 10298.35
 TOC Elevation : 202.32
 Depth to Water : 146.54 bTOC



Started/Completed : 6/3/2013

Drilling Method : Sonic Core

Diameter : 10" to 150', 8" to 210'

Sampling Device : Sonic Core

Drilled By : Cascade Drillers, LP

Total Depth : 210

Logged By : Mark Varljen

Reviewed By : Mark Varljen

Monitoring Well: E5D

(Page: 2 of 4)

Island County Solid Waste Facility
Coupeville, Washington

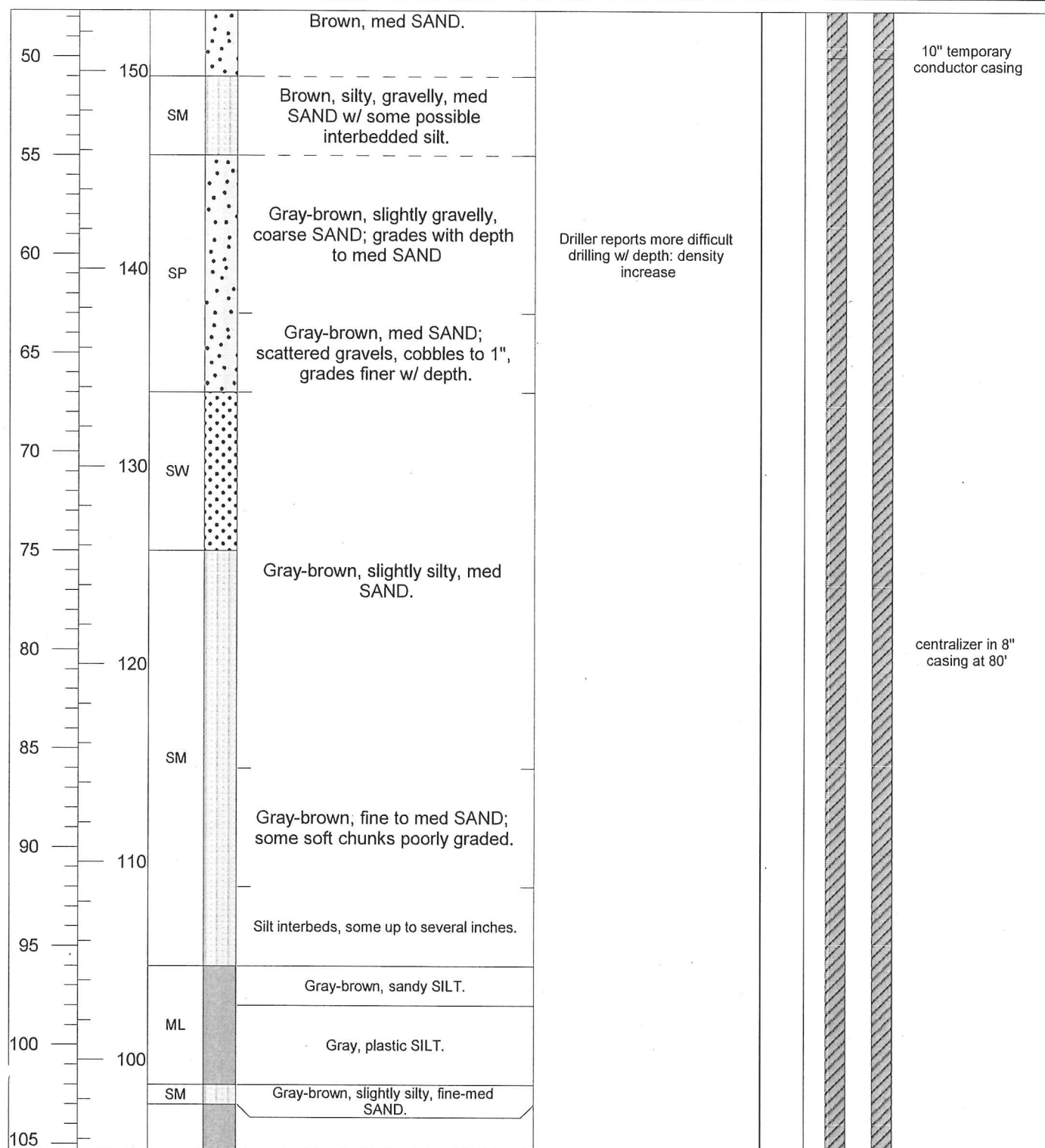
Surveyed By : TMI


Northing Coordinate : 9740.03

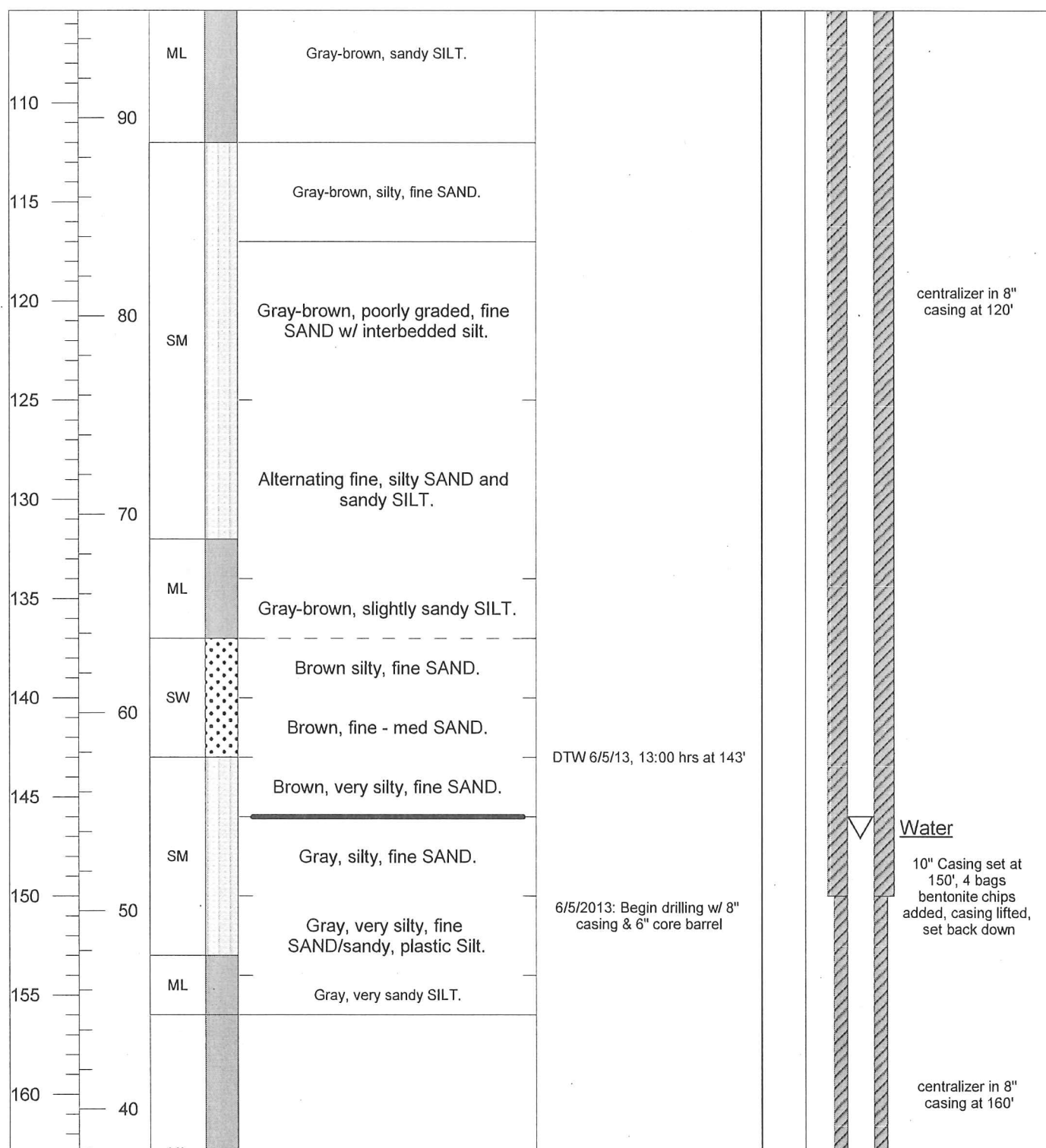
Easting Coordinate : 10298.35


TOC Elevation : 202.32

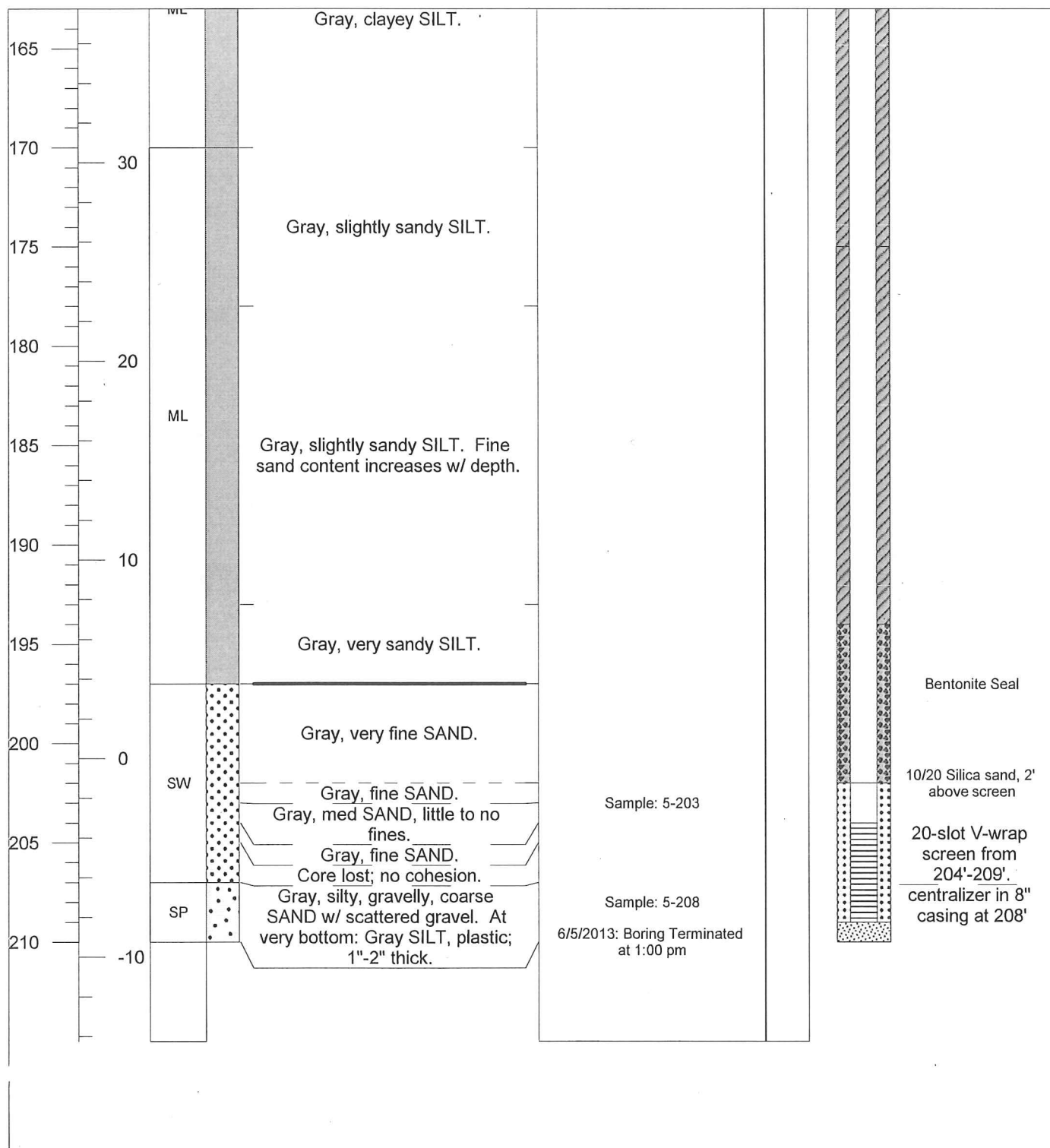
Depth to Water : 146.54 bTOC





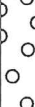
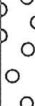
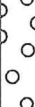
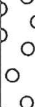
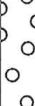







Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/3/2013	Monitoring Well: E5D (Page: 3 of 4)	
	Drilling Method : Sonic Core		
Island County Solid Waste Facility Coupeville, Washington	Diameter : 10" to 150', 8" to 210'	Surveyed By : TMI	
	Sampling Device : Sonic Core	Northing Coordinate : 9740.03	
	Drilled By : Cascade Drillers, LP	Easting Coordinate : 10298.35	
	Total Depth : 210	TOC Elevation : 202.32	
	Logged By : Mark Varljen	Depth to Water : 146.54 bTOC	
	Reviewed By : Mark Varljen		



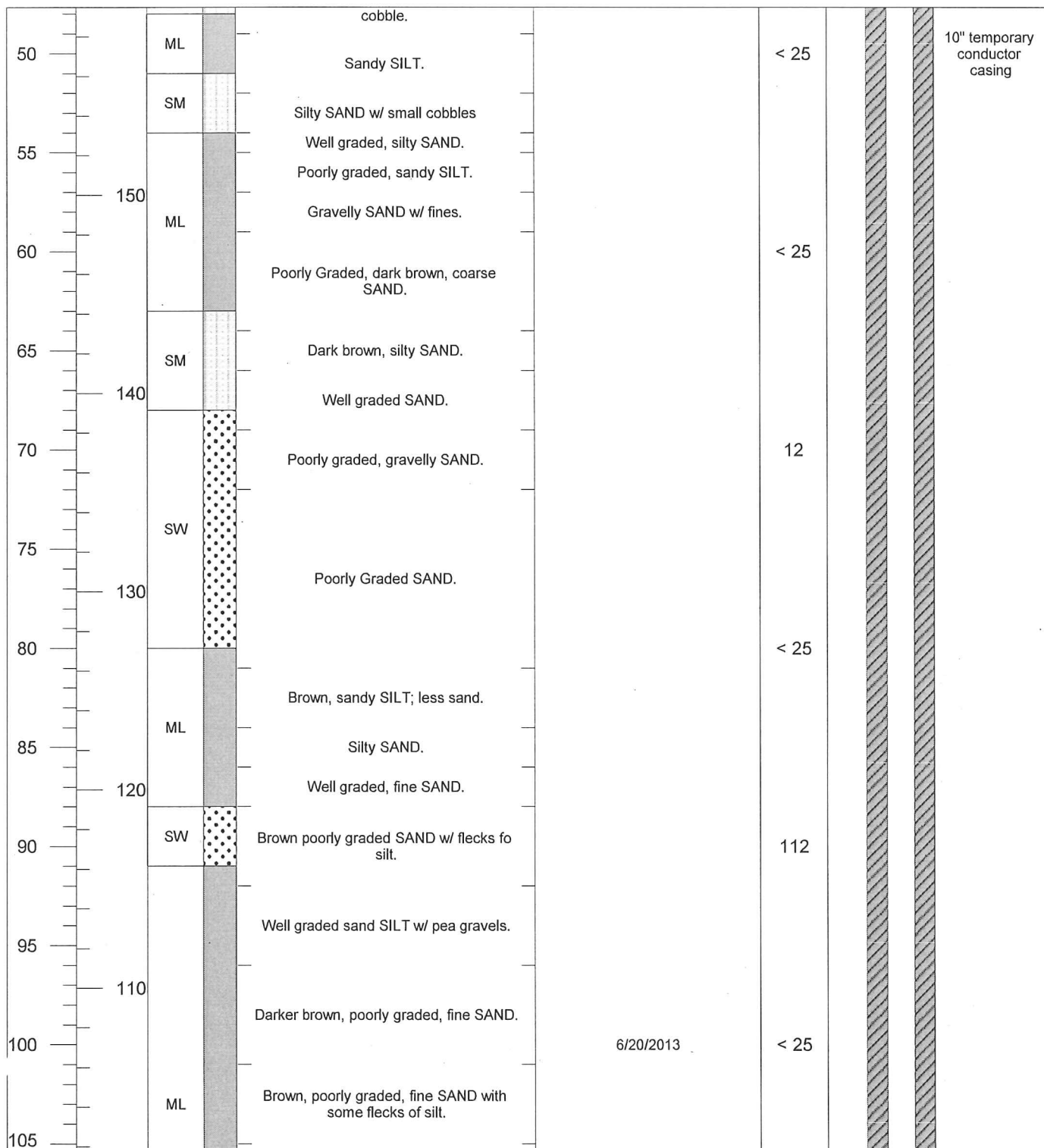
Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/3/2013 Drilling Method : Sonic Core Diameter : 10" to 150', 8" to 210' Sampling Device : Sonic Core Drilled By : Cascade Drillers, LP	Monitoring Well: E5D (Page: 4 of 4)
Island County Solid Waste Facility Coupeville, Washington	Total Depth : 210 Logged By : Mark Varljen Reviewed By : Mark Varljen	Surveyed By : TMI Northing Coordinate : 9740.03 Easting Coordinate : 10298.35 TOC Elevation : 202.32 Depth to Water : 146.54 bTOC




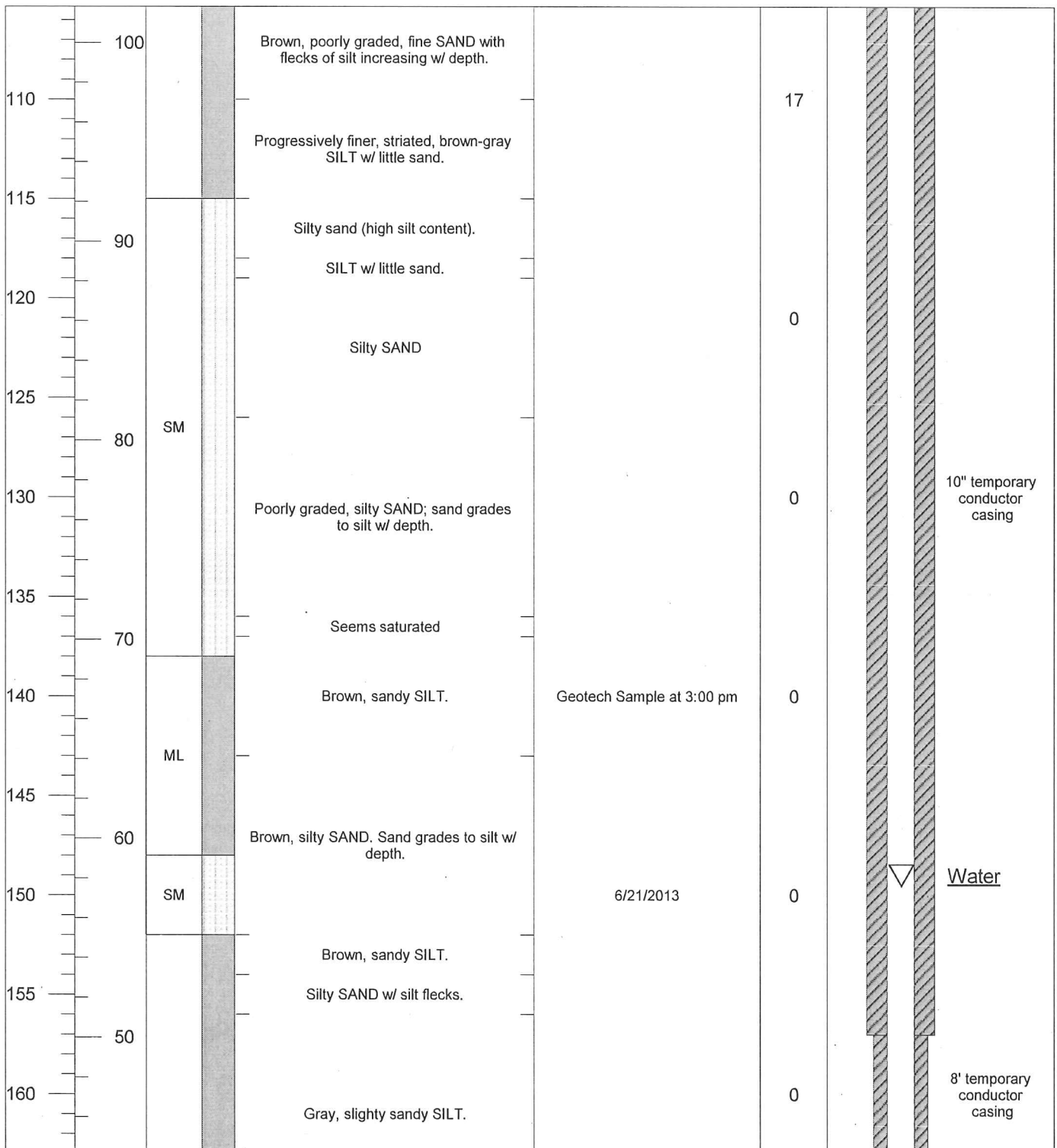
Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/13/2013	Monitoring Well: E6D (Page: 1 of 4)	
	Drilling Method : Sonic Core		
Island County Solid Waste Facility Coupeville, Washington	Diameter : 10" to 157", 8" to 210'	Surveyed By : TMI	
	Sampling Device : Sonic Core	Northing Coordinate : 9489.38	
	Drilled By : Cascade Drillers, LP	Easting Coordinate : 9927.61	
	Total Depth : 210	TOC Elevation : 206.73	
	Logged By : Allen Chartrand	Depth to Water : 149 bTOC	
	Reviewed By : Mark Varljen		


Depth [ft]	Surf. Elev. [ft]	USCS	Graphic	Description	Remarks	PID VOC [ppm]	
0				Brown-gray, coarse, gravelly SAND w/ small cobbles + loadfill debris			Well: E6D Elev: 206.73'
5		SP		Slightly finer, gravelly SAND.			
10	200			Brown, sandy GRAVEL.		< 25	4" Blank PVC and Bentonite Chips
15				Brown-gray, gravelly SAND w/ some fines			
20				Brown, gravelly, silty, SAND.			11:00 AM
25				Gray, gravelly, silty, SAND.		< 25	
30	190	GW		Slightly brownish gray, gravelly SAND.			
35				Brown, silty, fine SAND w/ cobble.			
40	180			Well graded, silty SAND w/ cobble.		< 25	Bentonite Grout
45				Well graded, silty SAND w/ cobble.			
50				Poorly graded, silty SAND.			
55	170	SM		Poorly graded, coarse SAND w/ some fines.		< 25	
60				Well graded, coarse SAND w/ some fines.			
65				Poorly graded sandy SILT.			
70	160			Brown/gray well graded silty SAND w/			

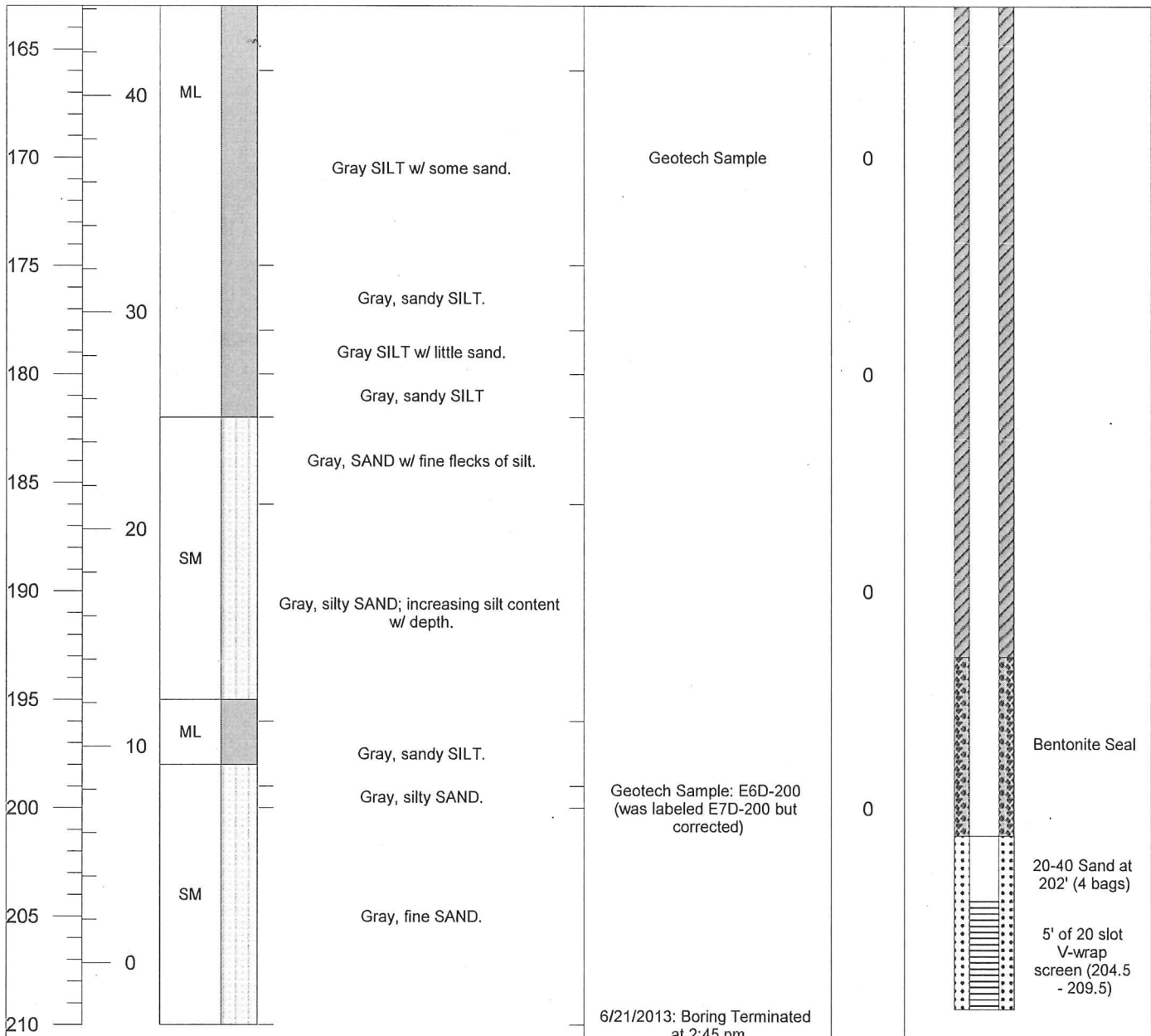
Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/13/2013	Monitoring Well: E6D (Page: 2 of 4)	
	Drilling Method : Sonic Core		
Island County Solid Waste Facility Coupeville, Washington	Diameter : 10" to 157", 8" to 210"	Surveyed By : TMI	
	Sampling Device : Sonic Core		
	Drilled By : Cascade Drillers, LP	Northing Coordinate : 9489.38	
	Total Depth : 210	Easting Coordinate : 9927.61	
	Logged By : Allen Chartrand	TOC Elevation : 206.73	
	Reviewed By : Mark Varljen	Depth to Water : 149 bTOC	




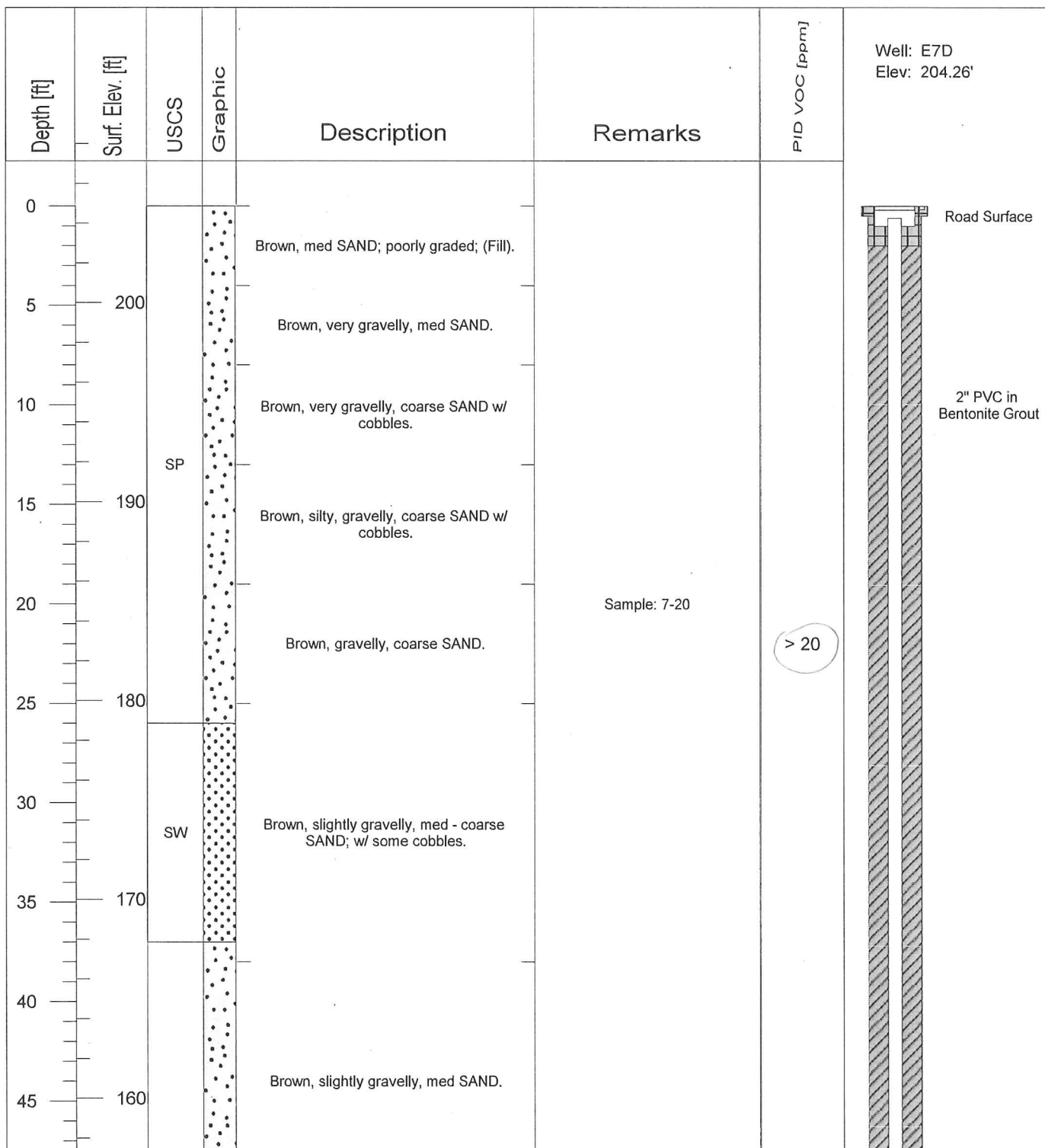
Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/13/2013 Drilling Method : Sonic Core Diameter : 10" to 157', 8" to 210" Sampling Device : Sonic Core Drilled By : Cascade Drillers, LP	Monitoring Well: E6D (Page: 3 of 4)
	Island County Solid Waste Facility Coupeville, Washington	Surveyed By : TMI Northing Coordinate : 9489.38 Easting Coordinate : 9927.61 TOC Elevation : 206.73 Depth to Water : 149 bTOC
	Total Depth : 210 Logged By : Allen Chartrand Reviewed By : Mark Varljen	




Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/13/2013	Monitoring Well: E6D (Page: 4 of 4)	
	Drilling Method : Sonic Core		
Island County Solid Waste Facility Coupeville, Washington	Diameter : 10" to 157', 8" to 210'	Surveyed By : TMI Northing Coordinate : 9489.38 Easting Coordinate : 9927.61 TOC Elevation : 206.73 Depth to Water : 149 bTOC	
	Sampling Device : Sonic Core		
	Drilled By : Cascade Drillers, LP		
	Total Depth : 210		
	Logged By : Allen Chartrand		
	Reviewed By : Mark Varljen		



Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/18/2013 Drilling Method : Sonic Core Diameter : 8" to 165', 6" to 220' Sampling Device : Sonic Core Drilled By : Cascade Drillers, LP	<h2>Monitoring Well: E7D</h2> (Page: 1 of 4)	
	Island County Solid Waste Facility Coupeville, Washington	Total Depth : 220 Logged By : Mark Varljen Reviewed By : Mark Varljen	Surveyed By : TMI Northing Coordinate : 9755.34 Easting Coordinate : 9986.01 TOC Elevation : 204.26 Depth to Water : 146.82 bTOC



Earth Science Strategies Consulting, Inc. 

Started/Completed : 6/18/2013
Drilling Method : Sonic Core
Diameter : 8" to 165', 6" to 220'
Sampling Device : Sonic Core
Drilled By : Cascade Drillers, LP

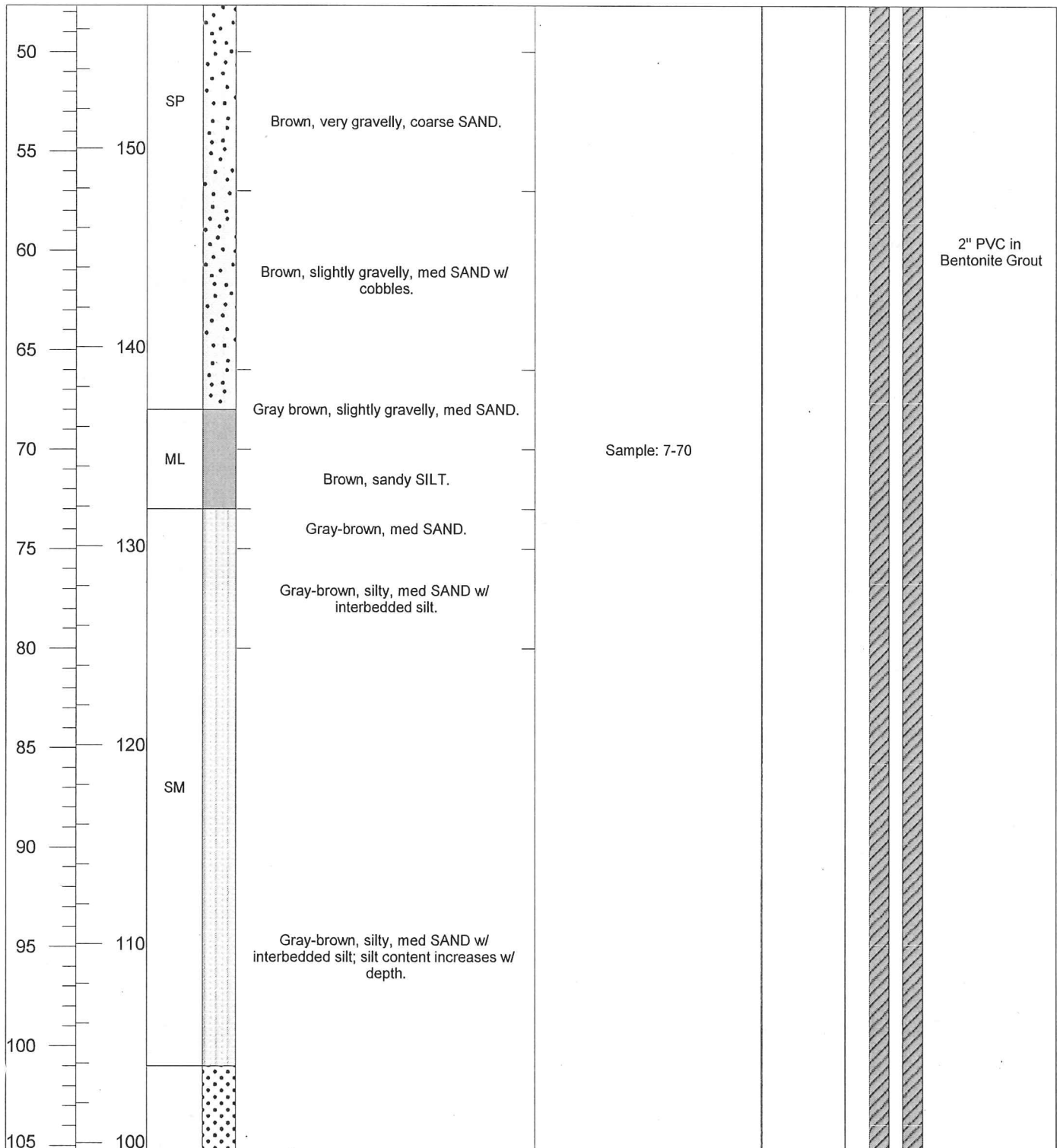
Monitoring Well: E7D


(Page: 2 of 4)

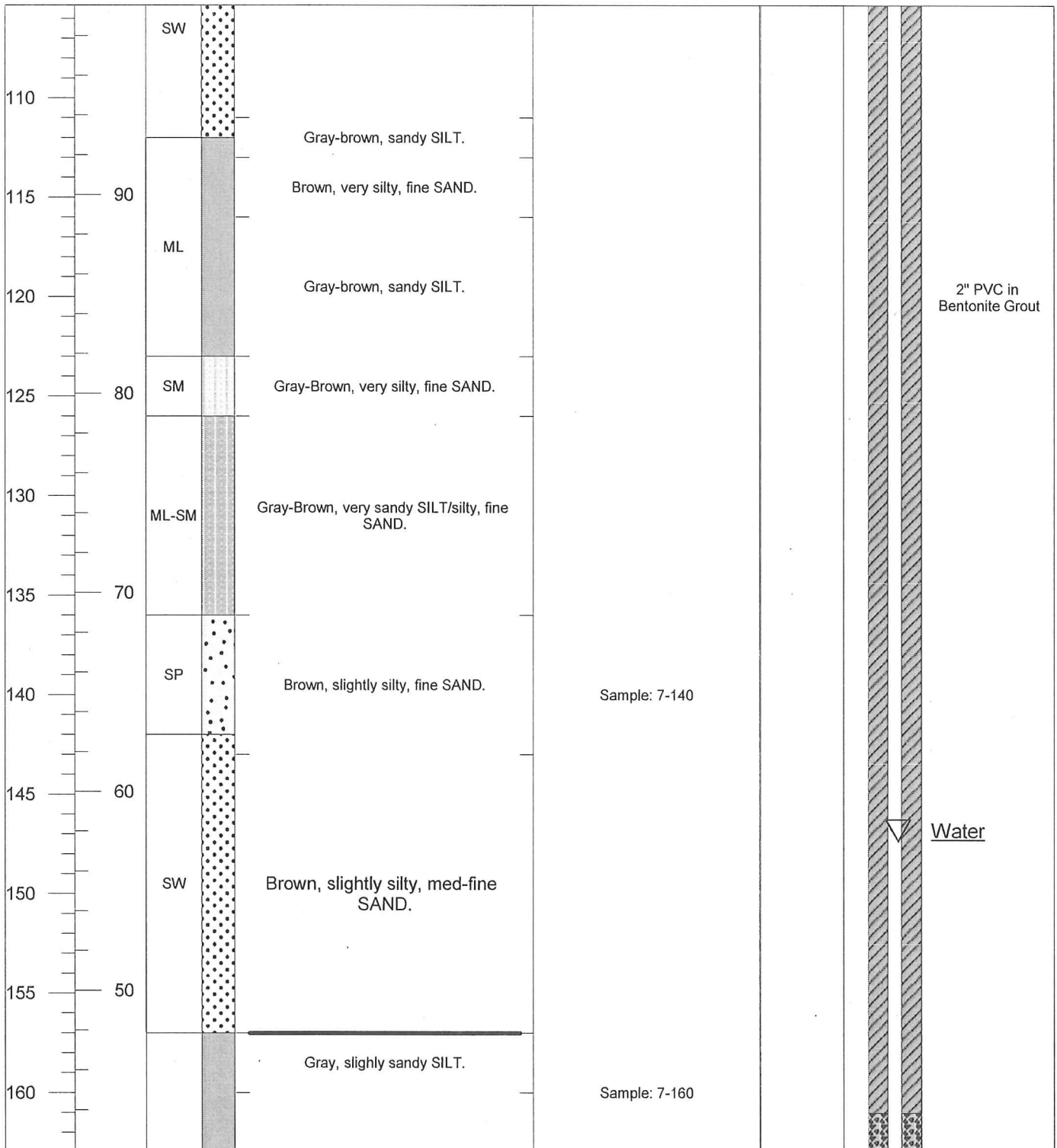
Island County Solid Waste Facility
Coupeville, Washington


Total Depth : 220
Logged By : Mark Varljen
Reviewed By : Mark Varljen

Surveyed By : TMI
Northing Coordinate : 9755.34
Easting Coordinate : 9986.01
TOC Elevation : 204.26
Depth to Water : 146.82 bTOC



Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/18/2013 Drilling Method : Sonic Core Diameter : 8" to 165', 6" to 220' Sampling Device : Sonic Core Drilled By : Cascade Drillers, LP	<h2>Monitoring Well: E7D</h2> (Page: 3 of 4)
	Island County Solid Waste Facility Coupeville, Washington	Total Depth : 220 Logged By : Mark Varljen Reviewed By : Mark Varljen



Earth Science Strategies Consulting, Inc. 

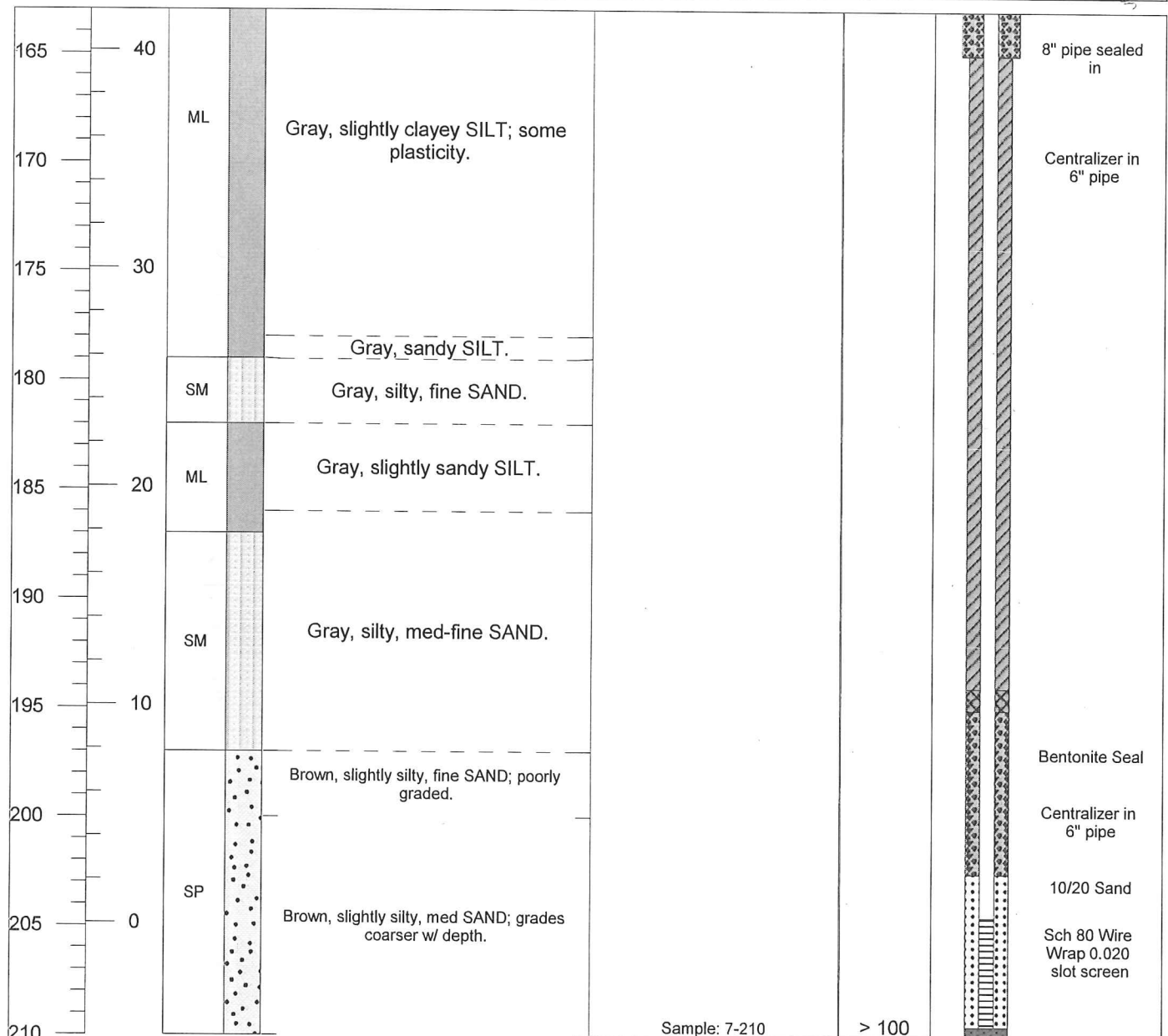
Started/Completed : 6/18/2013
Drilling Method : Sonic Core
Diameter : 8" to 165', 6" to 220'
Sampling Device : Sonic Core
Drilled By : Cascade Drillers, LP
Total Depth : 220
Logged By : Mark Varljen
Reviewed By : Mark Varljen


Monitoring Well: E7D

(Page: 4 of 4)

Island County Solid Waste Facility
Coupeville, Washington

Surveyed By : TMI
Northing Coordinate : 9755.34
Easting Coordinate : 9986.01
TOC Elevation : 204.26
Depth to Water : 146.82 bTOC



Earth Science Strategies Consulting, Inc. 

Started/Completed : 6/7/2013
 Drilling Method : Sonic Core
 Diameter : 8" to 165, 6" to 230
 Sampling Device : Sonic Core
 Drilled By : Cascade Drillers, LP

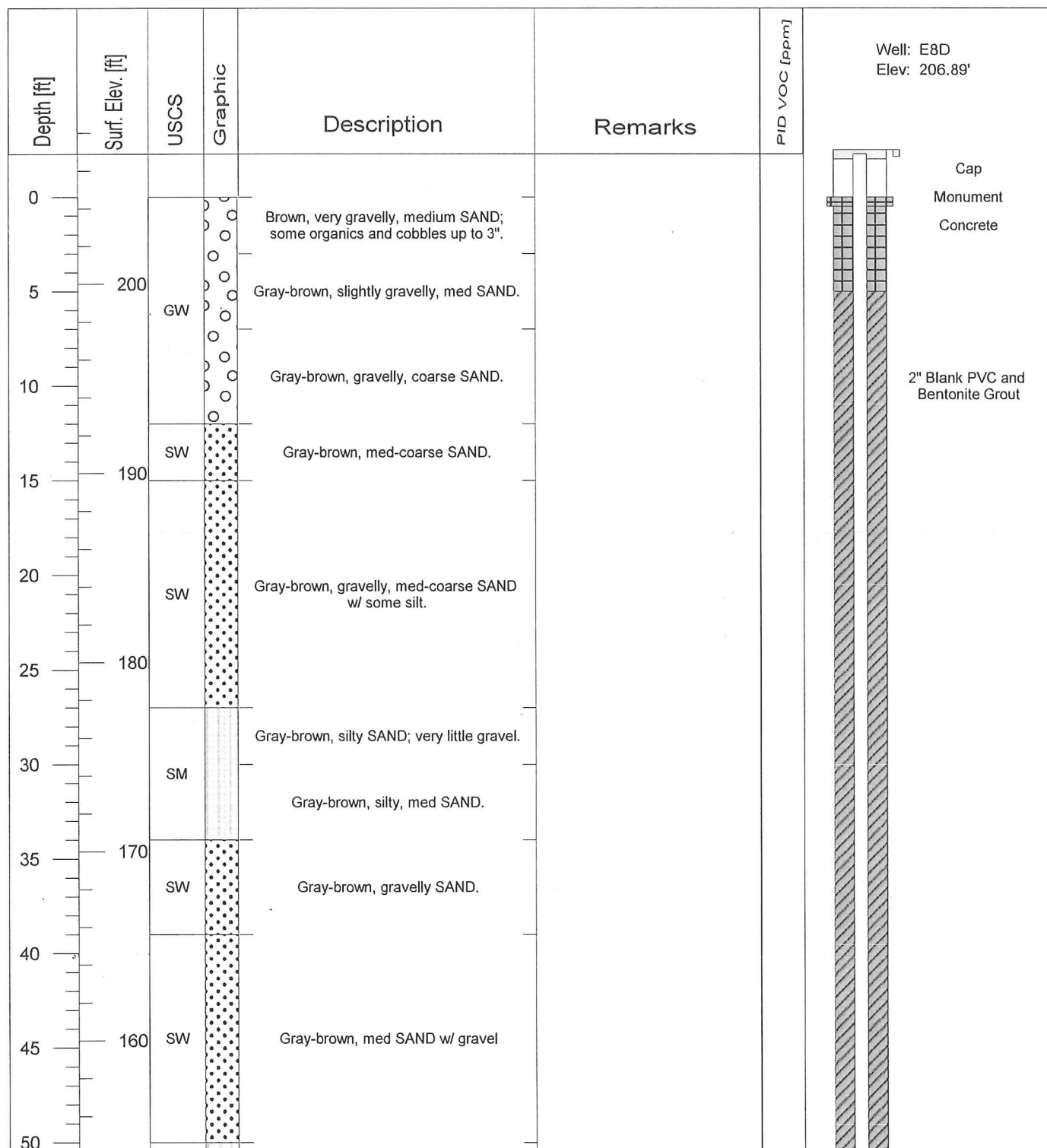
Monitoring Well: E8D


(Page: 1 of 4)

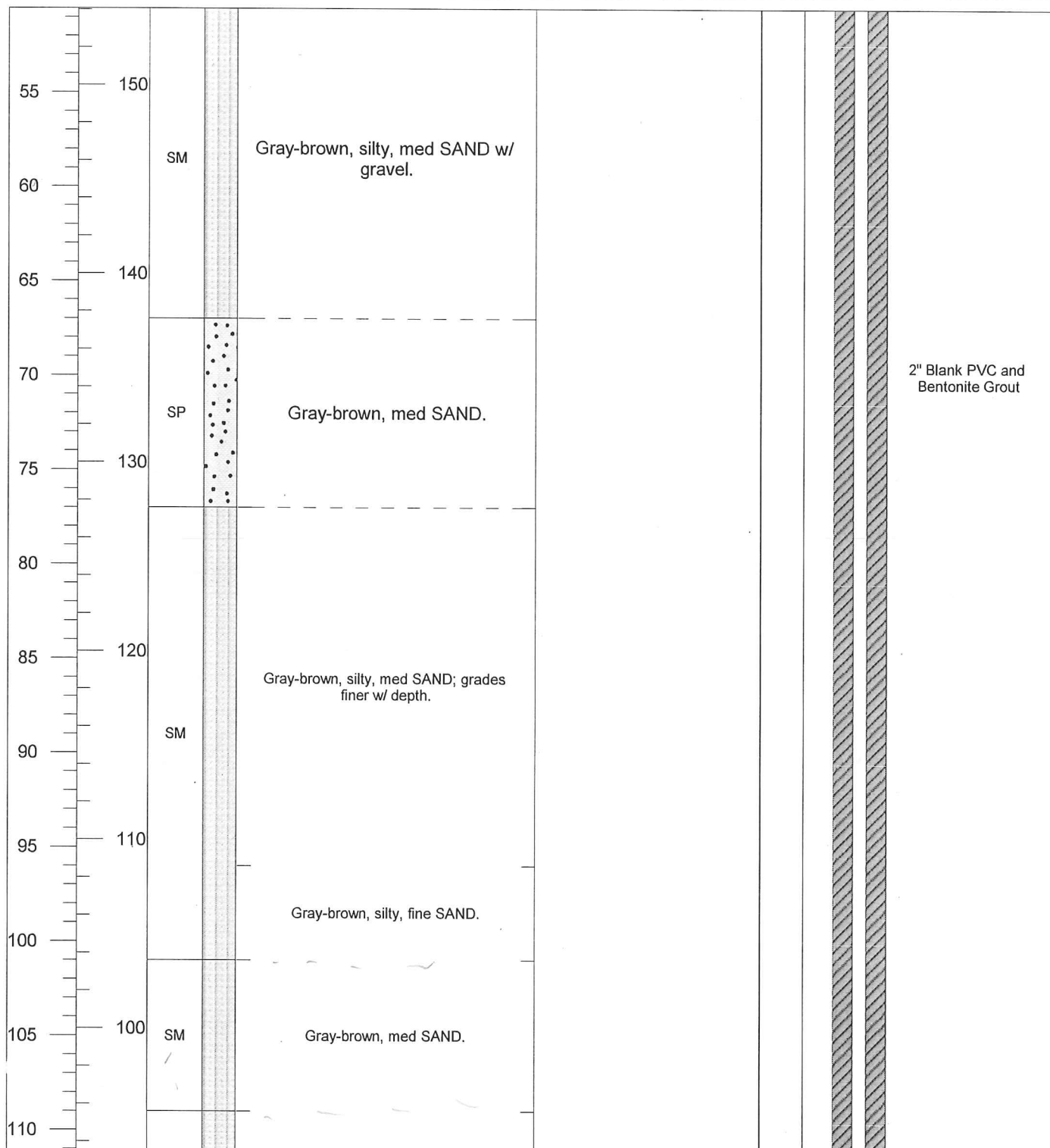
Island County Solid Waste Facility
 Coupeville, Washington


Total Depth : 230
 Logged By : Mark Varljen
 Reviewed By : Mark Varljen

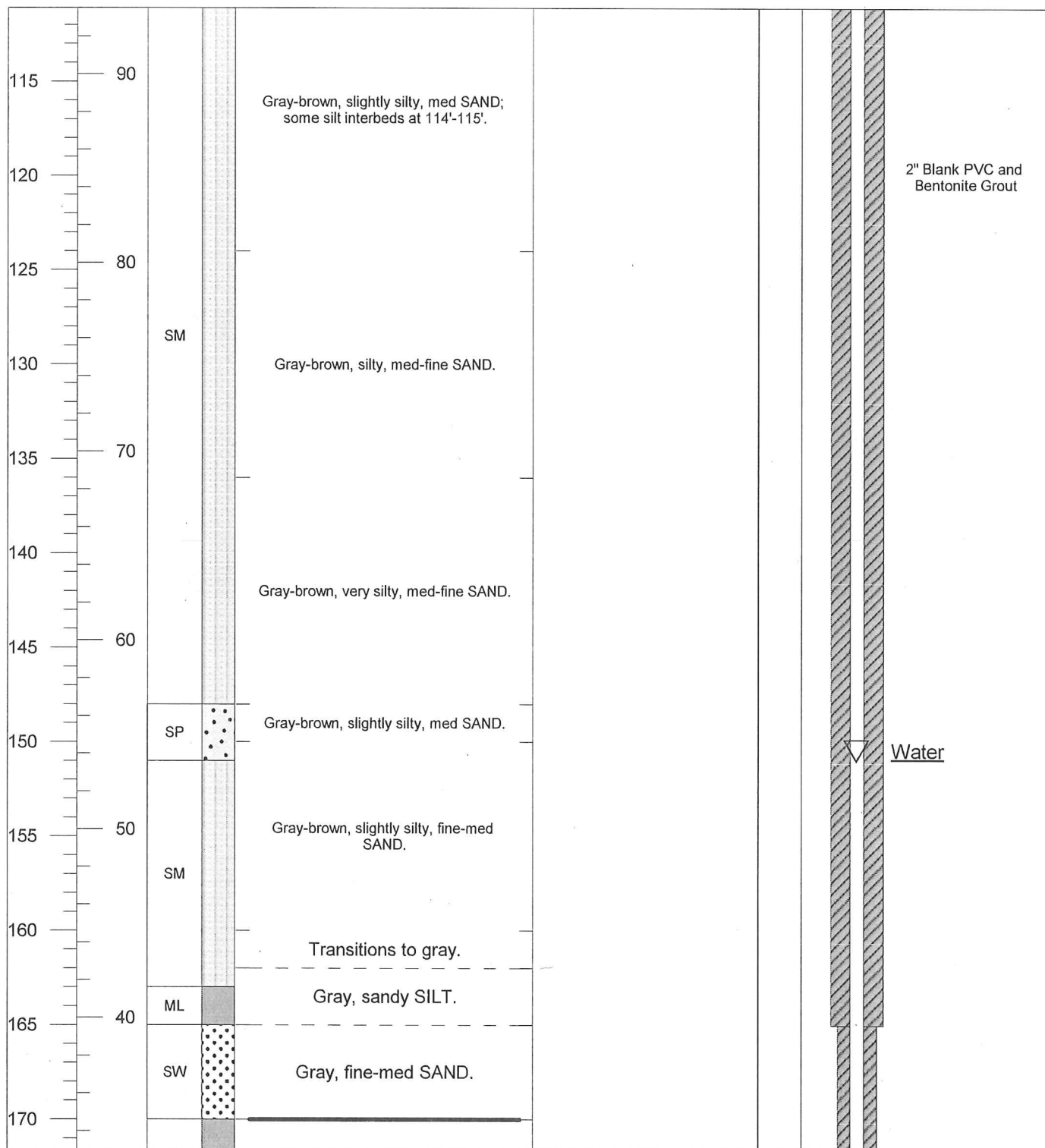
Surveyed By : TMI
 Northing Coordinate : 9504.91
 Easting Coordinate : 10176.71
 TOC Elevation : 206.89
 Depth to Water : 150.44 bTOC




Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/7/2013 Drilling Method : Sonic Core Diameter : 8" to 165, 6" to 230 Sampling Device : Sonic Core Drilled By : Cascade Drillers, LP	<h2 style="text-align: center;">Monitoring Well: E8D</h2> <p style="text-align: right;">(Page: 2 of 4)</p>
Island County Solid Waste Facility Coupeville, Washington	Total Depth : 230 Logged By : Mark Varljen Reviewed By : Mark Varljen	Surveyed By : TMI Northing Coordinate : 9504.91 Easting Coordinate : 10176.71 TOC Elevation : 206.89 Depth to Water : 150.44 bTOC



Earth Science Strategies Consulting, Inc. 	Started/Completed : 6/7/2013 Drilling Method : Sonic Core Diameter : 8" to 165, 6" to 230 Sampling Device : Sonic Core Drilled By : Cascade Drillers, LP	Monitoring Well: E8D (Page: 3 of 4)
Island County Solid Waste Facility Coupeville, Washington	Total Depth : 230 Logged By : Mark Varljen Reviewed By : Mark Varljen	Surveyed By : TMI Northing Coordinate : 9504.91 Easting Coordinate : 10176.71 TOC Elevation : 206.89 Depth to Water : 150.44 bTOC



Earth Science Strategies Consulting, Inc. 

Started/Completed : 6/7/2013

Drilling Method : Sonic Core

Diameter : 8" to 165, 6" to 230

Sampling Device : Sonic Core

Drilled By : Cascade Drillers, LP

Total Depth : 230

Logged By : Mark Varljen

Reviewed By : Mark Varljen

Monitoring Well: E8D

(Page: 4 of 4)

Island County Solid Waste Facility
Coupeville, Washington

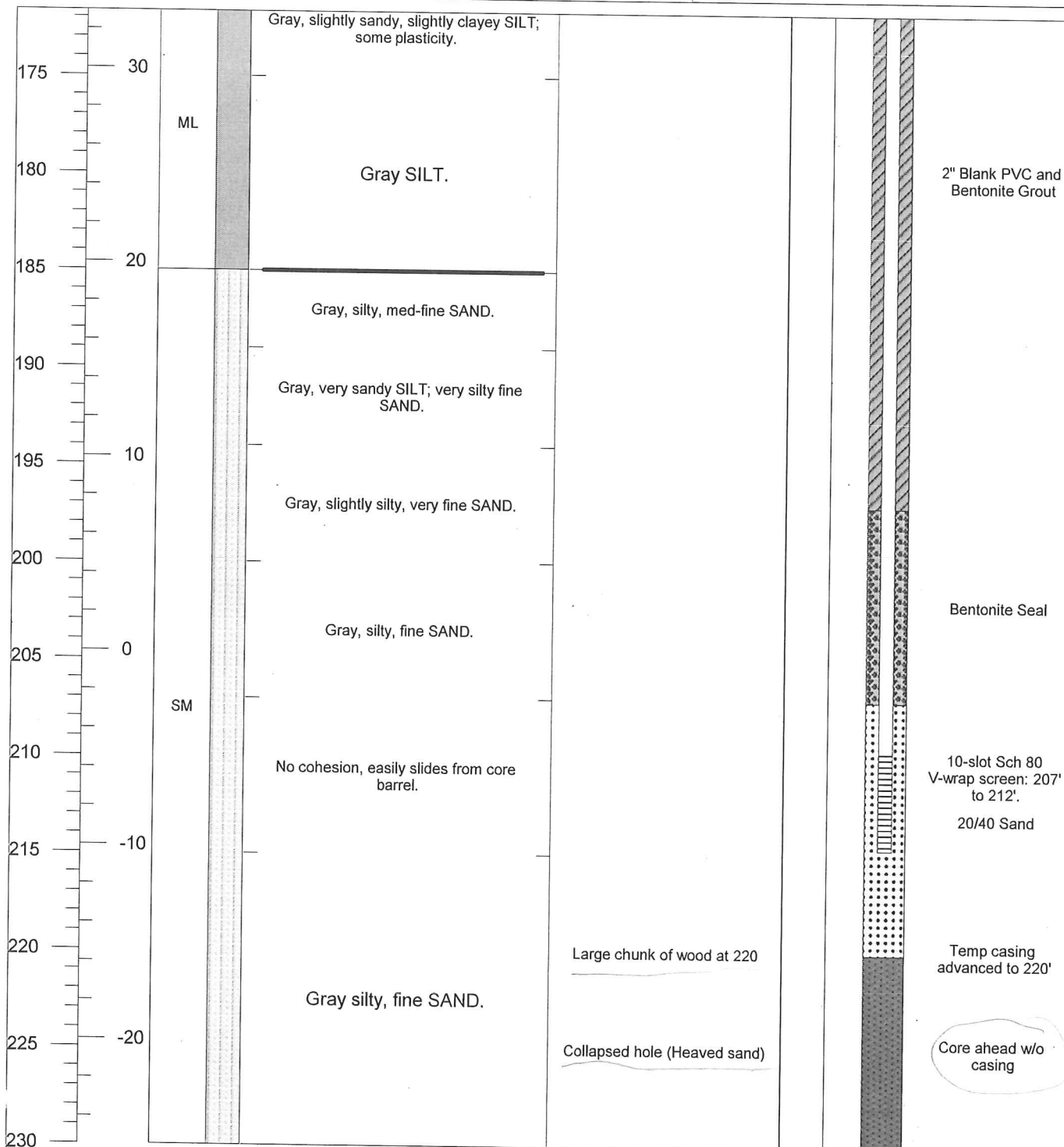
Surveyed By : TMI

Northing Coordinate : 9504.91

Easting Coordinate : 10176.71

TOC Elevation : 206.89

Depth to Water : 150.44 bTOC



Appendix B

Laboratory Data as Provided by TestAmerica

Appendix D

Slug Test Data and Calculations

MONITORING WELLS

WELL E7D

NORTHING (Y) 446421.132 FT

EASTING (X) 1197687.840 FT

ELEVATION (Z) 204.26 FT TOP OF PVC

WELL E6D

NORTHING (Y) 446155.193 FT

EASTING (X) 1197629.618 FT

ELEVATION (Z) 206.73 FT TOP OF PVC

WELL E8D

NORTHING (Y) 446170.638 FT

EASTING (X) 1197878.784 FT

ELEVATION (Z) 206.89 FT TOP OF PVC

WELL E5D

NORTHING (Y) 446405.944 FT

EASTING (X) 1198000.366 FT

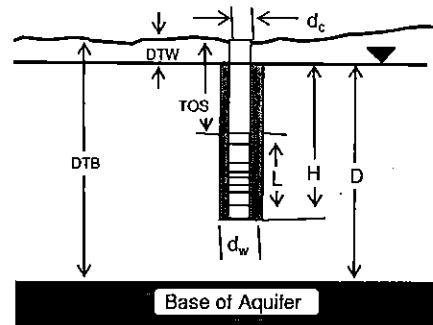
ELEVATION (Z) 202.32 FT TOP OF PVC

WELL ID: E5D Nitro 1

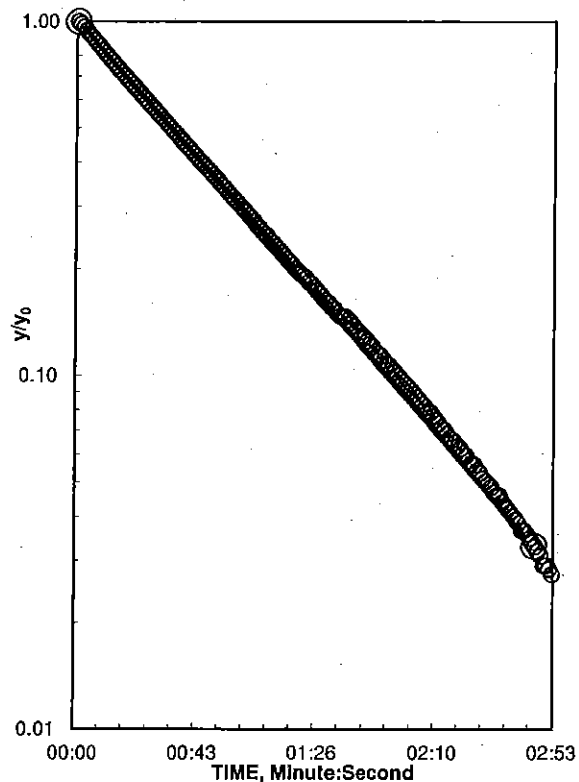
Local ID: E5D
Date: 8/20/2013
Time: 15:03

INPUT

Construction:	
Casing dia. (d_c)	4 Inch
Annulus dia. (d_w)	8 Inch
Screen Length (L)	5 Feet
Depths to:	
water level (DTW)	146.54 Feet
top of screen (TOS)	206.3 Feet
Base of Aquifer (DTB)	250 Feet
Annular Fill:	
across screen --	Medium Sand
above screen --	Bentonite
Aquifer Material --	
Fine Sand	



Adjust slope of line to estimate K



COMPUTED

L_{welled}	5 Feet
D =	103.46 Feet
H =	64.76 Feet
L/r_w	15.00
y_0 -DISPLACEMENT =	4.27 Feet
y_0 -SLUG =	4.27 Feet
From look-up table using L/r_w	
Partial penetrate A =	2.063
B =	0.312
$\ln(Re/r_w)$ =	2.247
Re =	3.15 Feet
Slope =	0.008952 \log_{10}/sec
$t_{90\%}$ recovery =	112 sec

Input is consistent.

K = 11 Feet/Day

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E5D Nitro 2

Local ID: E5D

Date: 8/20/2013

Time: 15:55

INPUT

Construction:

Casing dia. (d_c)	4 Inch
Annulus dia. (d_w)	8 Inch
Screen Length (L)	5 Feet

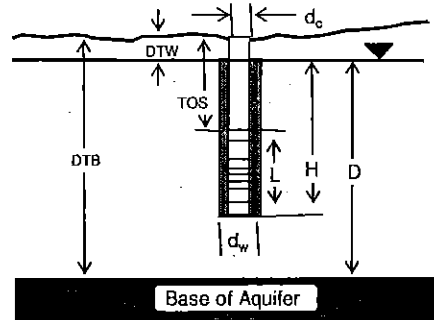
Depths to:

water level (DTW)	146.54 Feet
top of screen (TOS)	206.3 Feet
Base of Aquifer (DTB)	250 Feet

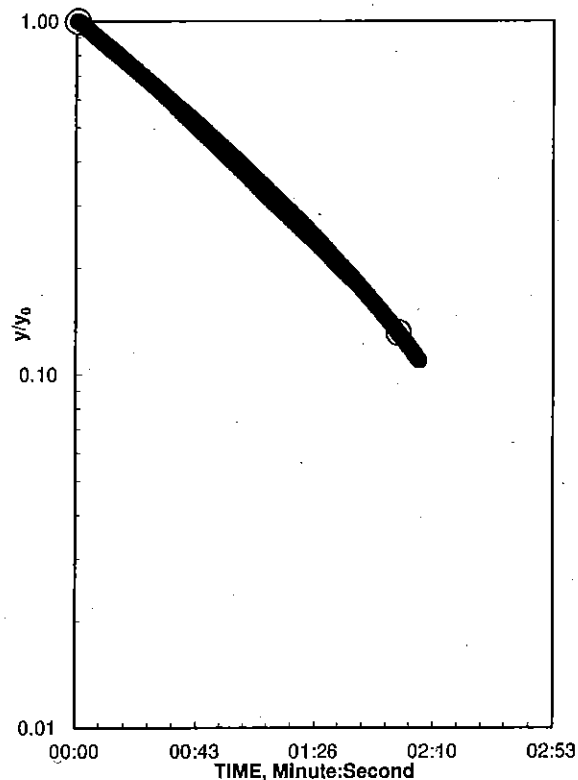
Annular Fill:

across screen -- Medium Sand
above screen -- Bentonite

Aquifer Material -- Fine Sand



Adjust slope of line to estimate K



COMPUTED

L_{welled} = 5 Feet

D = 103.46 Feet

H = 64.76 Feet

L/r_w = 15.00

y_0 -DISPLACEMENT = 20.14 Feet

y_0 -SLUG = 20.14 Feet

From look-up table using L/r_w

Partial penetrate A = 2.063

B = 0.312

$\ln(Re/r_w)$ = 2.247

Re = 3.15 Feet

Slope = 0.007577 \log_{10}/sec

$t_{90\%}$ recovery = 132 sec

Input is consistent.

K = 9.4 Feet/Day

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E5D Nitro 3

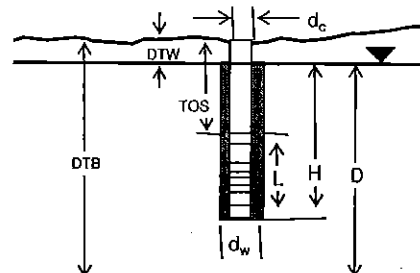
Local ID: E5D

Date: 8/20/2013

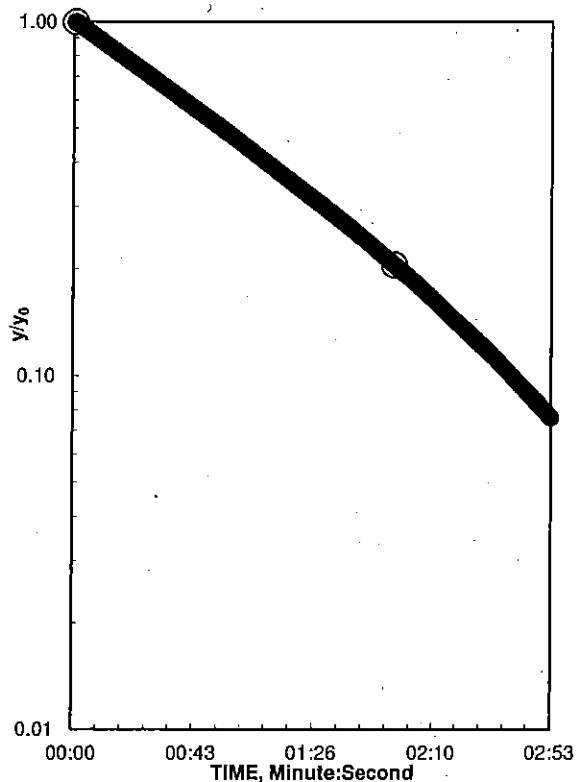
Time: 16:13

INPUT

Construction:	
Casing dia. (d_c)	4 Inch
Annulus dia. (d_w)	8 Inch
Screen Length (L)	5 Feet
Depths to:	
water level (DTW)	146.54 Feet
top of screen (TOS)	206.3 Feet
Base of Aquifer (DTB)	250 Feet
Annular Fill:	
across screen --	Medium Sand
above screen --	Bentonite
Aquifer Material --	
Fine Sand	



Adjust slope of line to estimate K



COMPUTED

L_{welled}	5 Feet
D =	103.46 Feet
H =	64.76 Feet
L/r_w	15.00
y_0 -DISPLACEMENT =	20.85 Feet
y_0 -SLUG =	20.85 Feet
From look-up table using L/r_w	
Partial penetrate A =	2.063
B =	0.312
$\ln(Re/r_w)$ =	2.247
Re =	3.15 Feet
Slope =	0.005985 \log_{10}/sec
$t_{90\%}$ recovery =	167 sec

Input is consistent.

K = 7.4 Feet/Day

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E6D Nitro 2

INPUT

Local ID: E6D
Date: 8/19/2013
Time: 15:13

Construction:

Casing dia. (d_c) 4 Inch
Annulus dia. (d_w) 8 Inch
Screen Length (L) 5 Feet

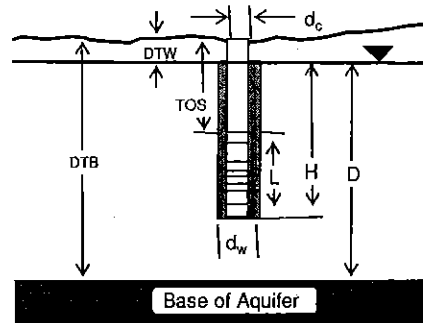
Depths to:

water level (DTW) 149 Feet
top of screen (TOS) 204.25 Feet
Base of Aquifer (DTB) 250 Feet

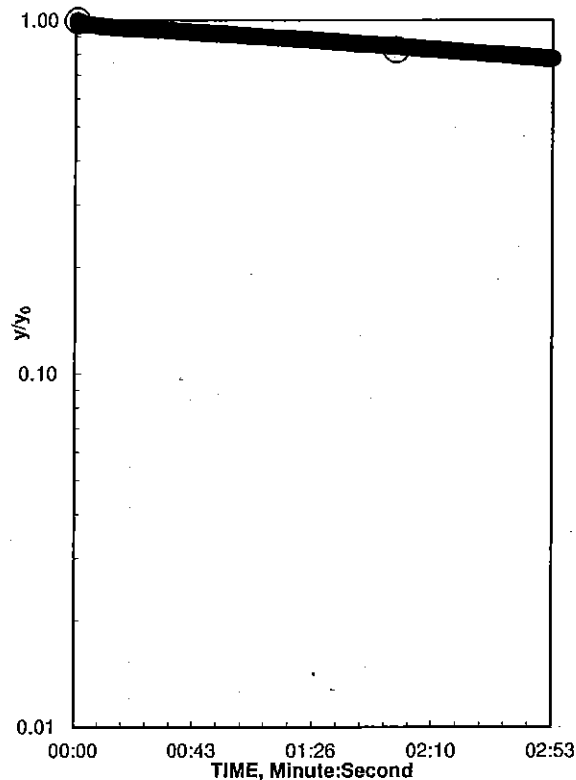
Annular Fill:

across screen -- Medium Sand
above screen -- Bentonite

Aquifer Material -- Fine Sand



Adjust slope of line to estimate K



COMPUTED

L_{wetted} 5 Feet
 $D =$ 101 Feet
 $H =$ 60.25 Feet
 $L/r_w =$ 15.00
 y_0 -DISPLACEMENT = 1.55 Feet
 y_0 -SLUG = 1.55 Feet

From look-up table using L/r_w

Partial penetrate A = 2.063
B = 0.312
 $\ln(Re/r_w) =$ 2.227
Re = 3.09 Feet

Slope = 0.000719 \log_{10}/sec
 $t_{90\%}$ recovery = 1391 sec

Input is consistent.

K = 0.88 Feet/Day

K= 0.88 is less than likely minimum of 3 for Fine Sand

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND.

WELL ID: E6D Nitro 3

Local ID: E6D

Date: 8/19/2013

Time: 15:46

INPUT

Construction:

Casing dia. (d_c) 4 Inch
Annulus dia. (d_w) 8 Inch
Screen Length (L) 5 Feet

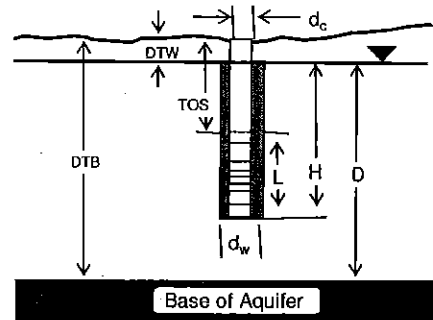
Depths to:

water level (DTW) 149 Feet
top of screen (TOS) 204.25 Feet
Base of Aquifer (DTB) 250 Feet

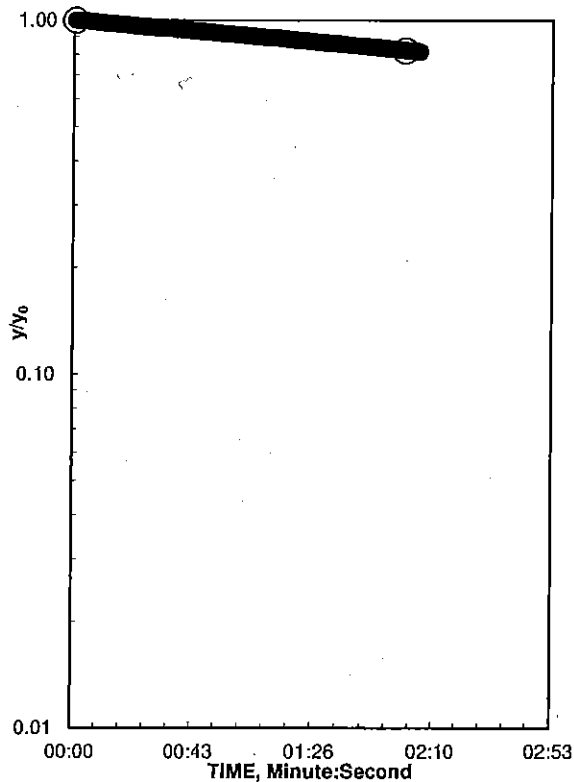
Annular Fill:

across screen -- Medium Sand
above screen -- Bentonite

Aquifer Material -- Fine Sand



Adjust slope of line to estimate K



COMPUTED

L_{wetted} 5 Feet

$D =$ 101 Feet

$H =$ 60.25 Feet

$L/r_w =$ 15.00

y_0 -DISPLACEMENT = 1.74 Feet

y_0 -SLUG = 1.74 Feet

From look-up table using L/r_w

Partial penetrate A = 2.063

B = 0.312

$\ln(Re/r_w) =$ 2.227

$Re =$ 3.09 Feet

Slope = 0.000722 \log_{10}/sec

$t_{90\%}$ recovery = 1385 sec

Input is consistent.

$K =$ 0.89 Feet/Day

$K = 0.89$ is less than likely minimum of 3 for Fine Sand

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E7D Nitro 1

Local ID: E7D

Date: 8/19/2013

Time: 12:32

INPUT

Construction:

Casing dia. (d_c) 2 Inch

Annulus dia. (d_w) 6 Inch

Screen Length (L) 5 Feet

Depths to:

water level (DTW) 146.82 Feet

top of screen (TOS) 204.5 Feet

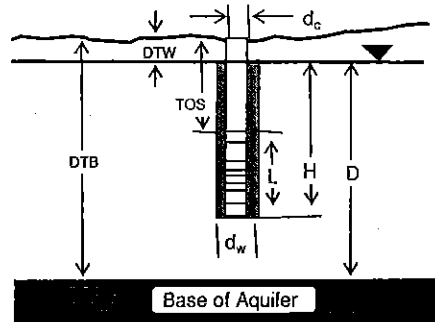
Base of Aquifer (DTB) 250 Feet

Annular Fill:

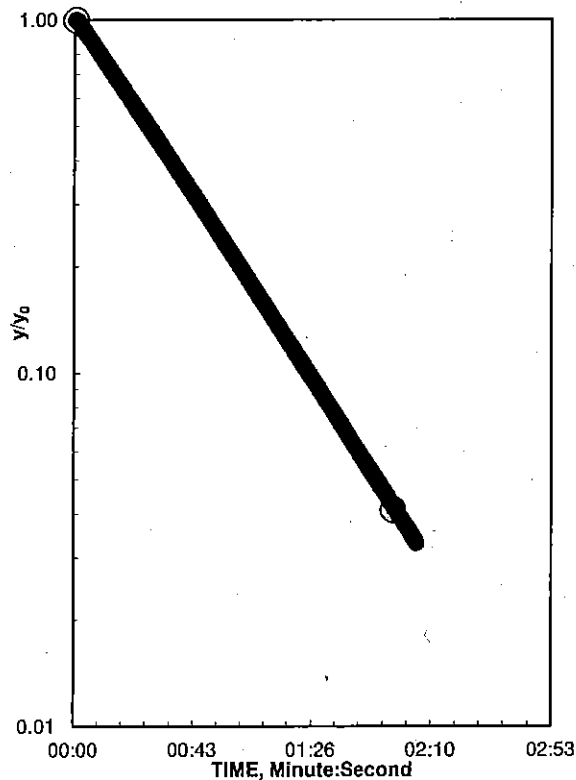
across screen -- Medium Sand

above screen -- Bentonite

Aquifer Material -- Fine Sand



Adjust slope of line to estimate K



COMPUTED

L_{wetted} 5 Feet

$D =$ 103.18 Feet

$H =$ 62.68 Feet

$L/r_w =$ 20.00

y_0 -DISPLACEMENT = 18.05 Feet

y_0 -SLUG = 18.05 Feet

From look-up table using L/r_w

Partial penetrate A = 2.202

B = 0.345

$\ln(Re/r_w) =$ 2.519

$Re =$ 3.10 Feet

Slope = 0.012011 \log_{10}/sec

$t_{90\%}$ recovery = 83 sec

Input is consistent.

$K =$ 4.2 Feet/Day

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E7D Nitro 2

Local ID: E7D

Date: 8/19/2013

Time: 12:45

INPUT

Construction:

Casing dia. (d_c) 2 Inch
Annulus dia. (d_w) 6 Inch
Screen Length (L) 5 Feet

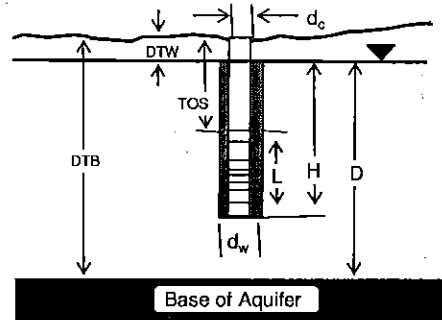
Depths to:

water level (DTW) 146.82 Feet
top of screen (TOS) 204.5 Feet
Base of Aquifer (DTB) 250 Feet

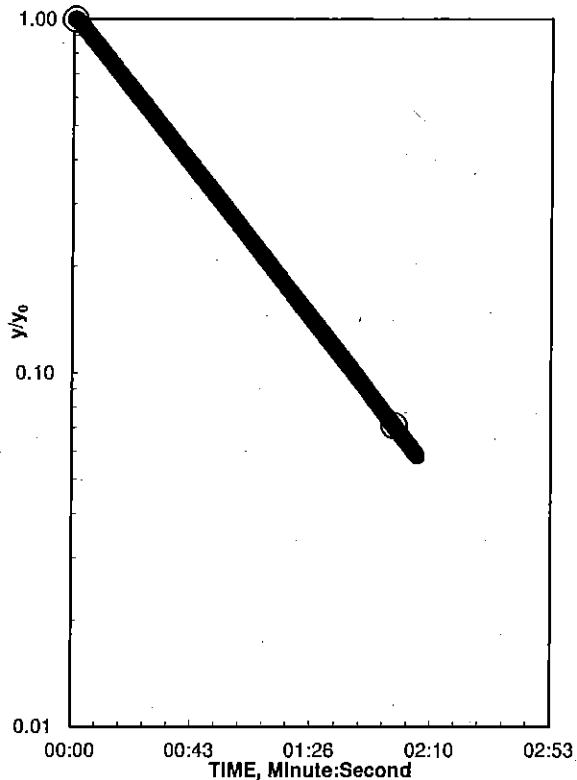
Annular Fill:

across screen -- Medium Sand
above screen -- Bentonite

Aquifer Material -- Fine Sand



Adjust slope of line to estimate K



COMPUTED

L_{wetted} 5 Feet

$D =$ 103.18 Feet

$H =$ 62.68 Feet

$L/r_w =$ 20.00

Y_0 -DISPLACEMENT = 20.47 Feet

Y_0 -SLUG = 20.47 Feet

From look-up table using L/r_w

Partial penetrate A = 2.202

B = 0.345

$\ln(Re/r_w) =$ 2.519

$Re =$ 3.10 Feet

Slope = 0.00992 \log_{10}/sec

$t_{90\% \text{ recovery}} =$ 101 sec

Input is consistent.

$K =$ 3.4 Feet/Day

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E8D Nitro 1

Local ID: E8D

Date: 8/20/2013

Time: 10:58

INPUT

Construction:

Casing dia. (d_c)	2 Inch
Annulus dia. (d_w)	6 Inch
Screen Length (L)	5 Feet

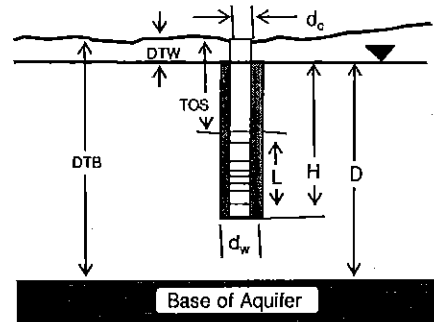
Depths to:

water level (DTW)	150.44 Feet
top of screen (TOS)	209.9 Feet
Base of Aquifer (DTB)	250 Feet

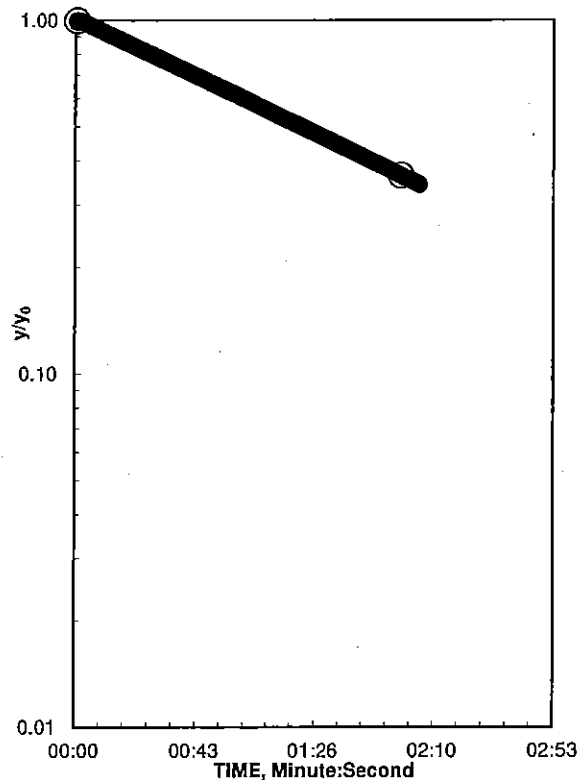
Annular Fill:

across screen -- Medium Sand
above screen -- Bentonite

Aquifer Material -- Fine Sand



Adjust slope of line to estimate K



COMPUTED

L_{wetted}	5 Feet
D =	99.56 Feet
H =	64.46 Feet
L/r_w	20.00
y_0 -DISPLACEMENT =	7.59 Feet
y_0 -SLUG =	7.60 Feet

From look-up table using L/r_w

Partial penetrate A =	2.202
B =	0.345
$\ln(Re/r_w)$ =	2.541
Re =	3.17 Feet

Slope = 0.003742 \log_{10}/sec
 $t_{90\% \text{ recovery}}$ = 267 sec

Input is consistent.

K = 1.3 Feet/Day

K= 1.3 is less than likely minimum of 3 for Fine Sand

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E8D Nitro 2

Local ID: E8D

Date: 8/20/2013

Time: 11:23

INPUT

Construction:

Casing dia. (d_c) 2 Inch
Annulus dia. (d_w) 6 Inch
Screen Length (L) 5 Feet

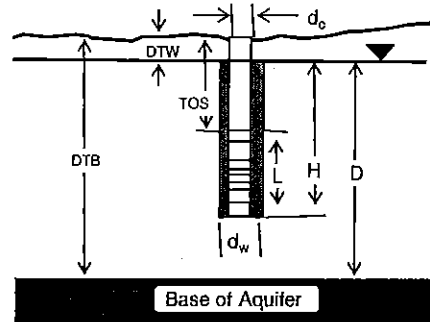
Depths to:

water level (DTW) 150.44 Feet
top of screen (TOS) 209.9 Feet
Base of Aquifer (DTB) 250 Feet

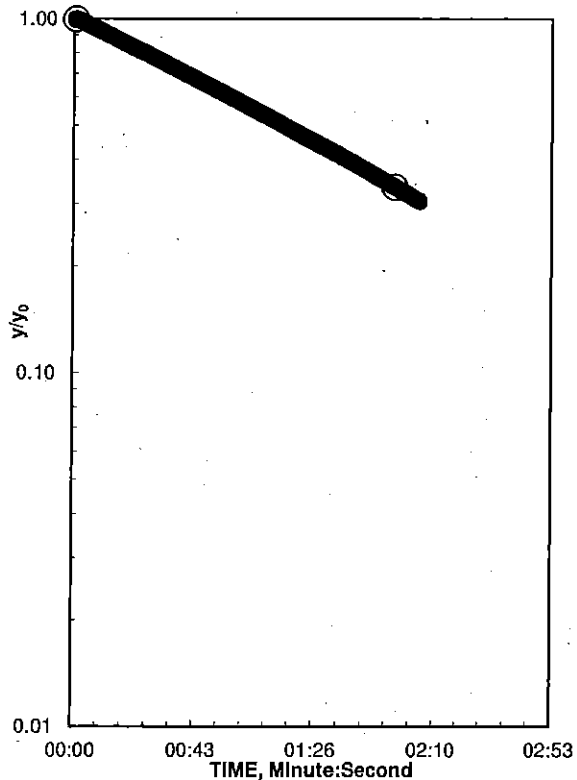
Annular Fill:

across screen -- Medium Sand
above screen -- Bentonite

Aquifer Material -- Fine Sand



Adjust slope of line to estimate K



COMPUTED

L_{welled} 5 Feet

$D =$ 99.56 Feet

$H =$ 64.46 Feet

$L/r_w =$ 20.00

y_0 -DISPLACEMENT = 9.56 Feet

y_0 -SLUG = 9.56 Feet

From look-up table using L/r_w

Partial penetrate $A =$ 2.202

$B =$ 0.345

$\ln(Re/r_w) =$ 2.541

$Re =$ 3.17 Feet

Slope = 0.00413 \log_{10}/sec

$t_{90\% \text{ recovery}} =$ 242 sec

Input is consistent.

$K =$ 1.4 Feet/Day

$K = 1.4$ is less than likely minimum of 3 for Fine Sand

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.

WELL ID: E8D Nitro 3

Local ID: E8D
Date: 8/20/2013
Time: 11:48

INPUT

Construction:

Casing dia. (d_c) 2 Inch
Annulus dia. (d_w) 6 Inch
Screen Length (L) 5 Feet

Depths to:

water level (DTW) 150.44 Feet
top of screen (TOS) 209.9 Feet
Base of Aquifer (DTB) 250 Feet

Annular Fill:

across screen -- Medium Sand
above screen -- Bentonite

Aquifer Material -- Fine Sand

COMPUTED

L_{wetted} 5 Feet
 $D =$ 99.56 Feet
 $H =$ 64.46 Feet
 $L/r_w =$ 20.00
 y_0 -DISPLACEMENT = 16.63 Feet
 y_0 -SLUG = 16.63 Feet

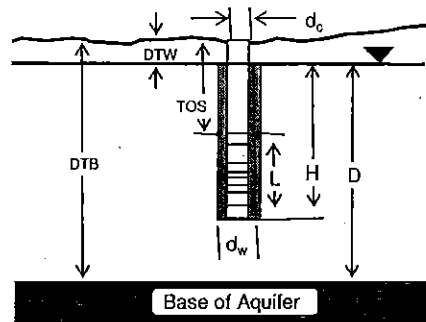
From look-up table using L/r_w

Partial penetrate A = 2.202
B = 0.345
 $\ln(Re/r_w) =$ 2.541
Re = 3.17 Feet

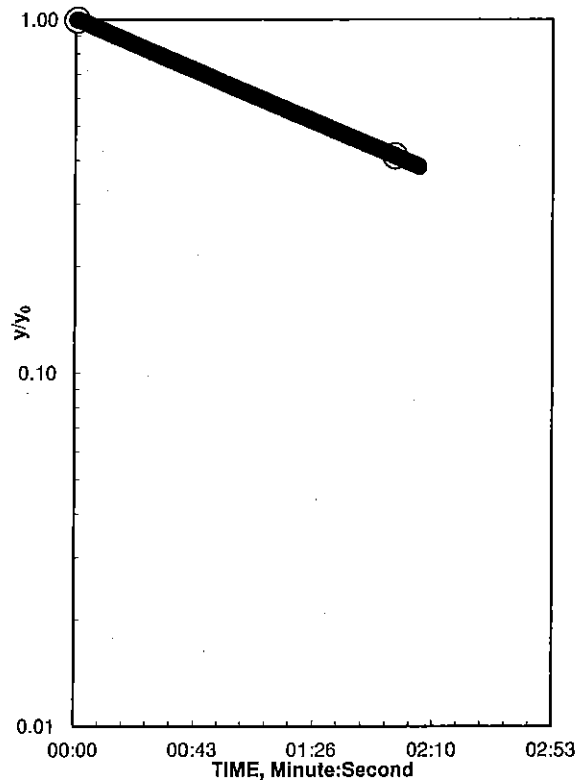
Slope = 0.003358 \log_{10} /sec
 $t_{90\%}$ recovery = 298 sec

Input is consistent.

K = 1.2 Feet/Day



Adjust slope of line to estimate K



K= 1.2 is less than likely minimum of 3 for Fine Sand

REMARKS:

Bouwer and Rice analysis of slug test, WRR 1976

Aquifer is a silty, fine SAND in the Puget Lowland.