

GENERAL NOTES & LEGEND

DESIGN CONSIDERATIONS

| | |
|------------------------|----------|
| BUILDING CODE: | 2015 IBC |
| FLOOR DESIGN | |
| RESIDENTIAL LIVE LOAD: | 40 PSF |
| RESIDENTIAL DEAD LOAD: | 35 PSF |
| STAIRS LIVE LOAD: | 100 PSF |
| STAIRS DEAD LOAD: | 20 PSF |

DRAWING NOTES & LEGEND

- FOR TYPICAL NOTES, STANDARD DETAILS, AND ABBREVIATIONS, SEE INSTALLATION COVERSHEET(S).
- ALL DIMENSIONS ARE FROM FACE-OF-STUD, FACE-OF-CONCRETE OR CENTER-OF COLUMN/BEAM UNLESS OTHERWISE NOTED

XX(##) - PRODUCT CALLOUT AND QUANTITY ON PLAN.
 "XX" - STRUCTURAL MEMBER TYPE CALLOUT
 "##" - QUANTITY OF STRUCTURAL MEMBERS IN BAY

RIM BOARD/FASCIA MATERIAL WILL BE SUPPLIED @ "STANDARD" 16'-0" LENGTHS AND AS EITHER LSL OR LVL

I-JOIST NOTES & LEGEND

- ALL I-JOISTS WILL BE SENT LONG TO BE FIELD TRIMMED

X - CONTINUOUS HANGER TYPE. SEE HANGER INFO.

THIRD PARTY PRODUCTS SOURCED BY REDBUILT

REDBUILT WILL PROVIDE GLULAM BEAMS WITH A 5000' RADIUS CAMBER. V8 BEAMS WILL NOT BE CAMBERED.

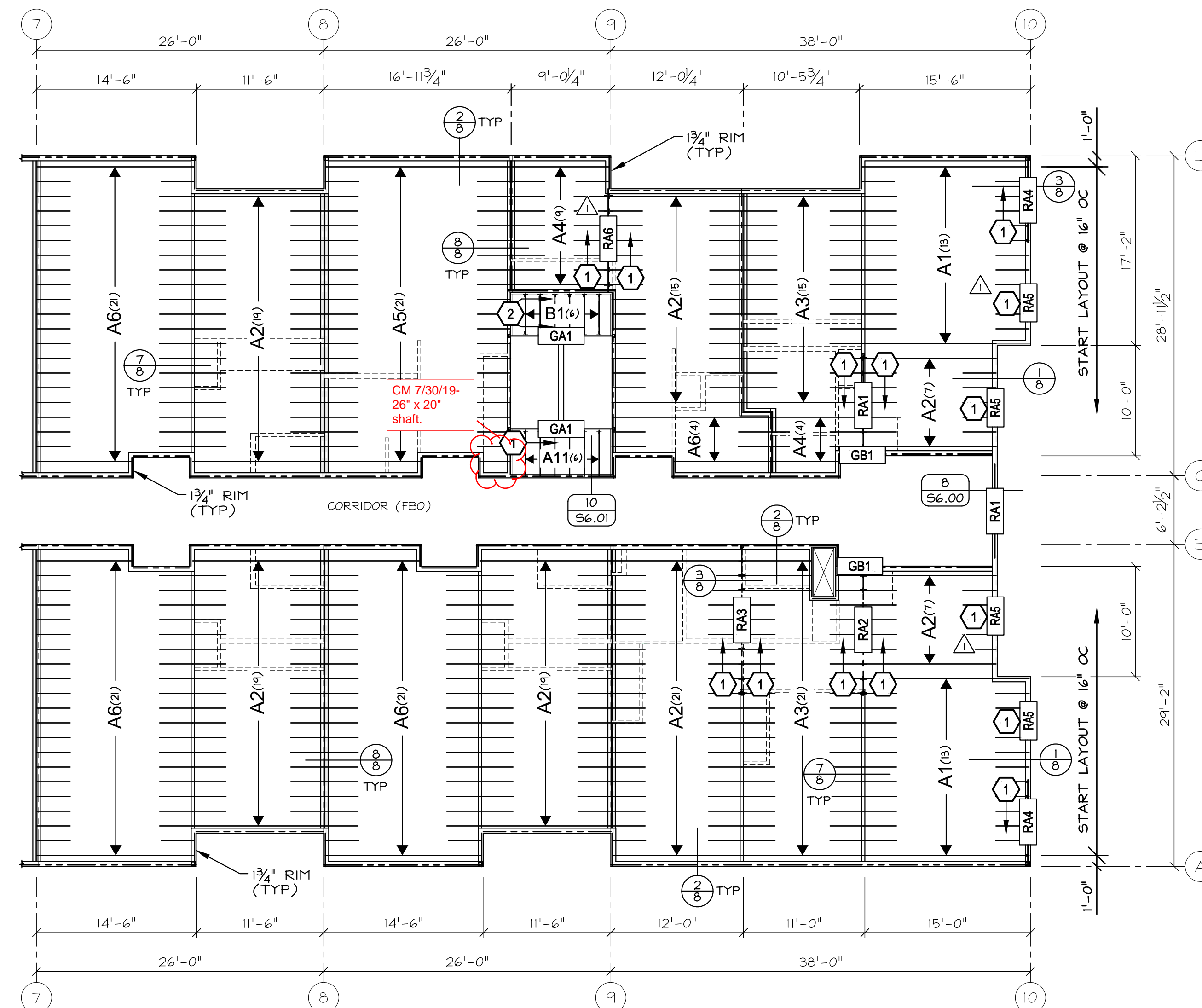
G? - LOCATION OF GLULAM BEAM. SEE MATERIAL LIST FOR MORE INFORMATION.

- ALL GLULAM BEAMS ARE SUPPLIED BASED ON THE SPECIFICATION SHOWN IN THE CONTRACT DOCUMENTS. REDBUILT SERVICES FOR THESE ITEMS ARE LIMITED TO PLACEMENT DRAWINGS ONLY. APPLICATION AND ADEQUACY REVIEW OF GLULAM BEAMS ARE THE SOLE RESPONSIBILITY OF THE DESIGN PROFESSIONAL OF RECORD AND ARE NOT PROVIDED BY REDBUILT.

RECTANGULAR SECTIONS

R? - LOCATION OF BEAM OR COLUMN BY RB. SEE MATERIAL LIST FOR MORE INFORMATION.

- ALL REDLAM LVL MATERIAL AND ASSOCIATED HARDWARE PROVIDED BY REDBUILT IS AS SPECIFIED ON THE CONTRACT DRAWINGS. SPECIFICATIONS AND SIZE HAVE NOT BEEN VERIFIED BY REDBUILT ENGINEERING UNLESS OTHERWISE NOTED.



**5TH FLOOR - GRIDS 7 TO 10
FRAMING LOCATION PLAN**
SCALE 1/8"=1'-0"

KEY NOTES

1. JOIST LAYOUT SHOWN DOES NOT ACCOUNT FOR CLEARANCE TO ANY FLOOR DRAINS. THE CONTRACTOR IS RESPONSIBLE FOR DRAIN LAYOUT. (NOTE: LIVING UNIT FLOOR JOISTS MAY BE OVERSPACED UP TO 2" FOR DRAIN CLEARANCE, THIS ALLOWS MOVING ANY ONE JOIST 4" MAX)

| Hangers | | | | | | |
|----------|------|---------------|---------|-------|--------|-------------------------|
| Quantity | Type | Model | Nailing | | | Web Stiffeners Required |
| | | | Top | Face | Member | |
| 100 | 1 | ITS1.81/11.88 | 4-NIO | 2-NIO | | |
| 12 | 2 | ITS1.81/9.5 | 4-NIO | 2-NIO | | |

- REFER TO CURRENT SIMPSON STRONG TIE® LITERATURE FOR HANGER SPECIFIC INSTALLATION INSTRUCTIONS.
- WEB STIFFENERS IF REQUIRED MUST BE ATTACHED BEFORE PLACING JOIST IN HANGER.

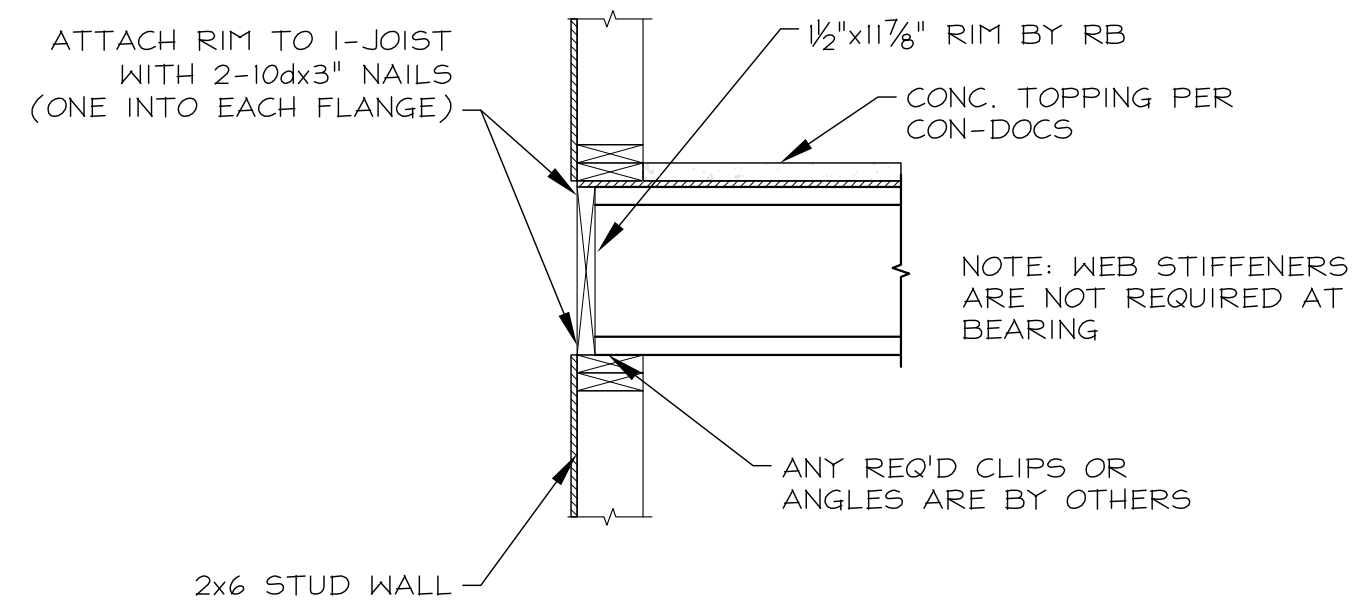
IN ACCORDANCE WITH IBC SECTION 1603.1, THE PRODUCTS IN THIS PACKAGE HAVE BEEN DESIGNED FOR ONLY THOSE LOADS SPECIFICALLY SHOWN ON THE CONSTRUCTION DOCUMENTS. THE CONSTRUCTION DOCUMENTS AND PLACEMENT DIAGRAMS HAVE NOT BEEN REVIEWED BY A REDBUILT ENGINEER. IF OTHER LOADS (WIND UPLIFT, SNOW DRIFT, BRACE LOADS, ETC.) ARE TO BE APPLIED, PLEASE PROVIDE THE MAGNITUDE AND LOCATION.

| PRODUCT TYPE CHART | |
|--|--------------------------------------|
| SEE MATERIAL LIST FOR MORE INFORMATION | |
| CALLOUT | MEMBER |
| A | 11 7/8" RED-145 JOIST |
| B | 9 1/2" RED-145 JOIST |
| GA | 5 1/8"x9" GLULAM BEAM (THIRD PARTY) |
| GB | 5 1/8"x12" GLULAM BEAM (THIRD PARTY) |
| RA | 5 1/4"x11 7/8" REDLAM BEAM |

PROJECT ASSUMPTIONS
 - ALL MISCELLANEOUS ITEMS (SPRINKLER LINES, SOFFITS, DUCTWORK, ELECTRICAL CONDUITS, ETC.) ARE ASSUMED TO BE INCLUDED IN THE UNIFORM DESIGN DEAD LOAD SHOWN, UNLESS SPECIFICALLY SHOWN OTHERWISE ON THESE DRAWINGS.
 - ALL OPENINGS (HATCHES, DUCTWORK, SKYLIGHTS, ETC.) ARE ASSUMED TO FIT BETWEEN REGULAR ON-CENTER SPACING AS SHOWN, UNLESS SPECIFICALLY SHOWN OTHERWISE ON THESE SHOP DRAWINGS.

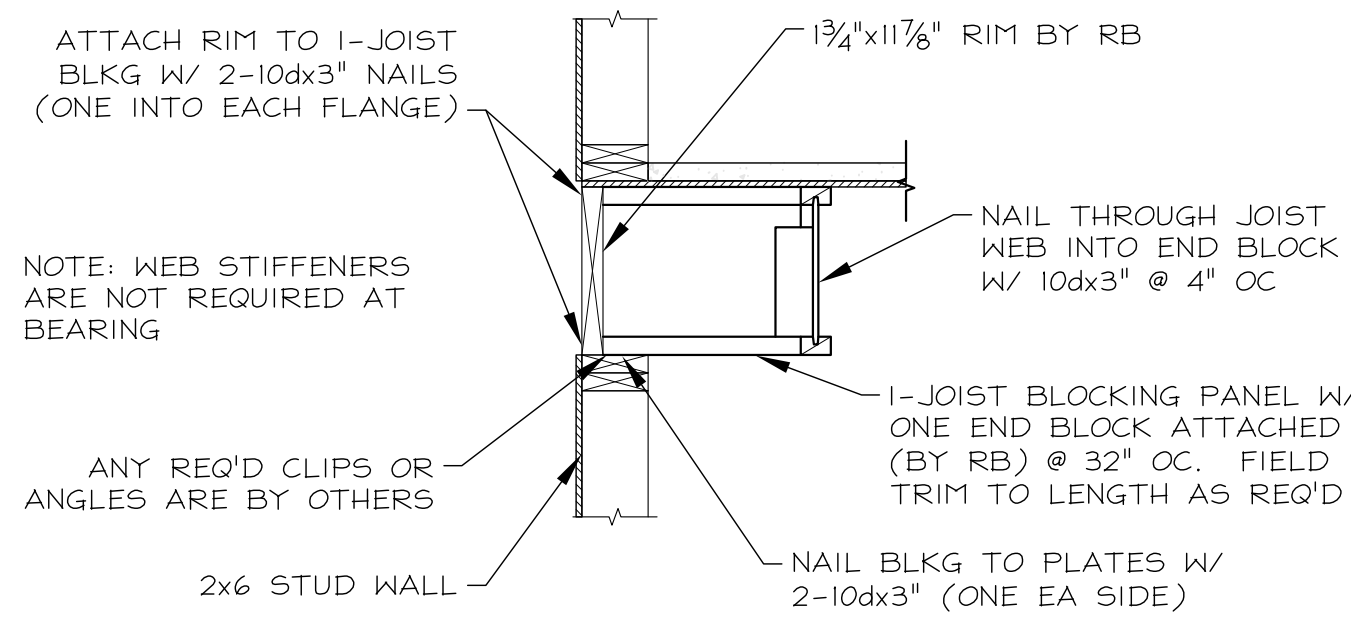
APPROVED FOR PRODUCTION

| | | | | | | |
|-----------------------------|---------|---------|--------------|---------|--------|--|
| △ | | | | | | |
| △ | JL | 5/24/19 | OFA Comments | | | |
| △ | BY | DATE | REMARKS | | | |
| Westman Mill Olympia, WA | | | | | | |
| DRAWN | DATE | CHECKED | DATE | ORDER # | SHEET | |
| JL | 2/26/19 | - | - | 110805 | 7 of 8 | |



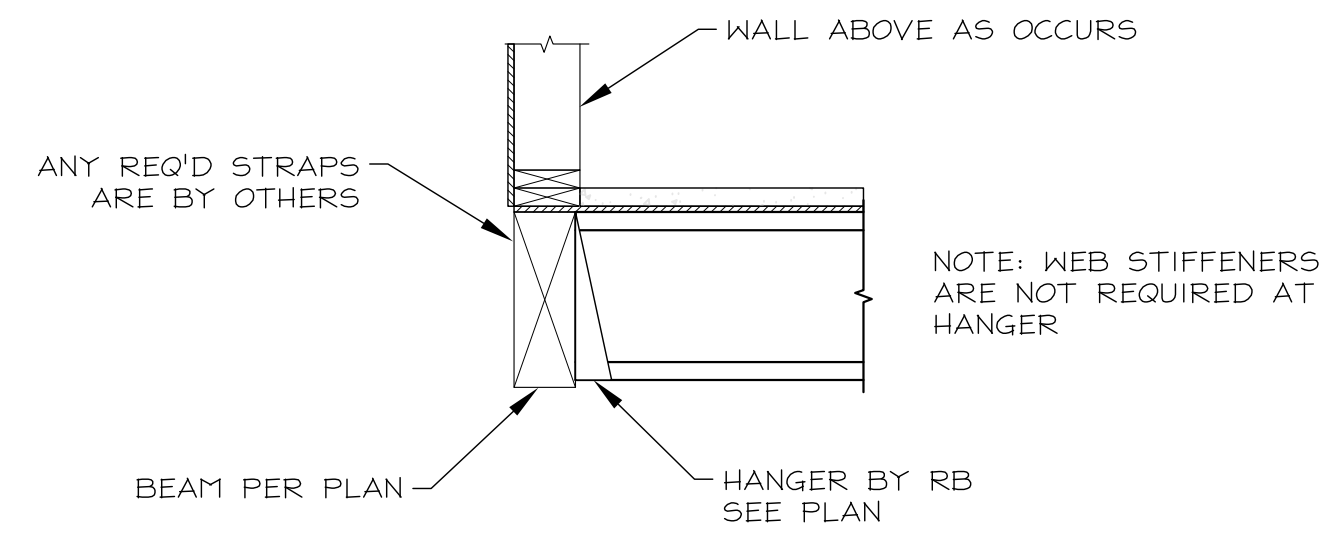
TYP @ GRIDS 1 & 10 AND JOISTS PERPENDICULAR TO SHAFTS (MECH, ELEVATOR & STAIRS)

REF. 1/56.00
1/8



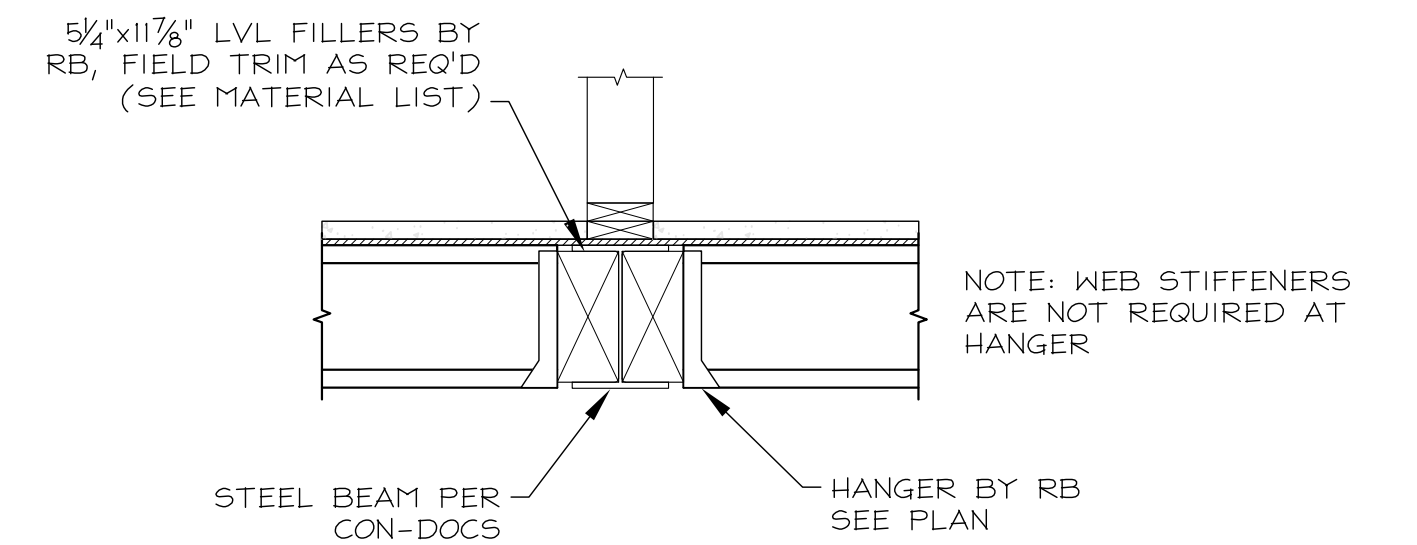
TYP @ GRIDS A & D, CORRIDOR WALLS, AND JOISTS PARALLEL TO SHAFTS (MECH, ELEVATOR & STAIRS)

REF. 2/56.00 & 9/56.00
2/8

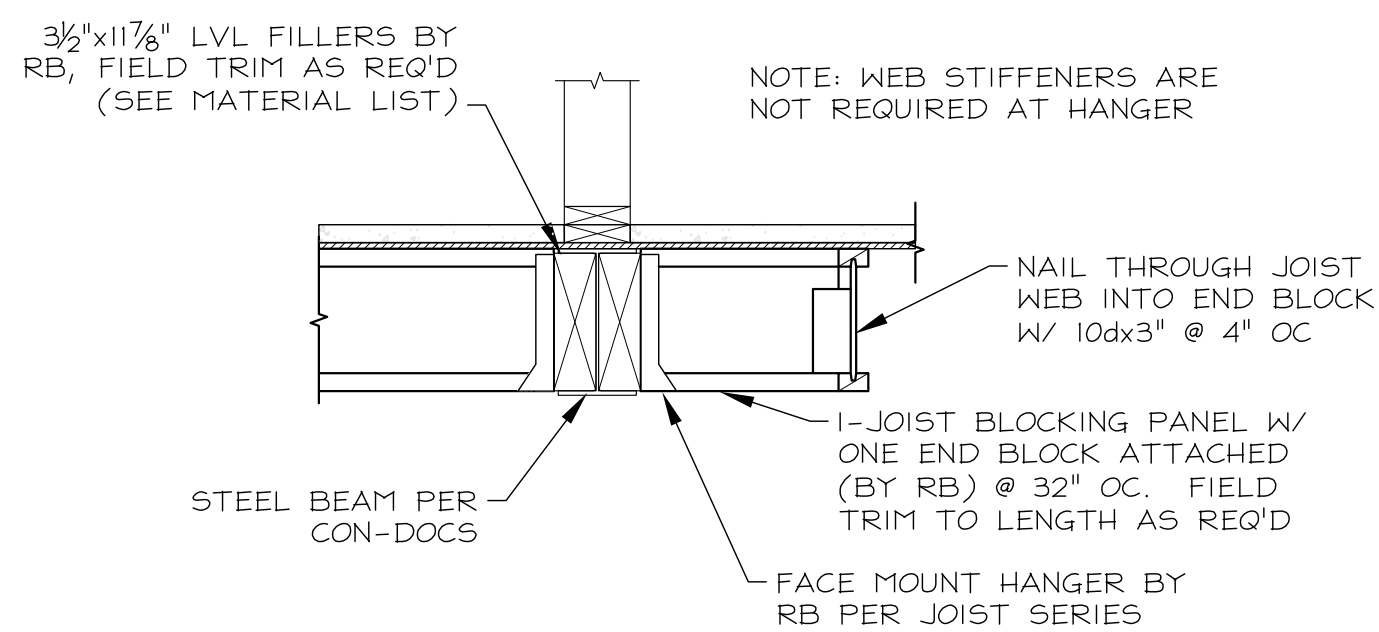


JOIST BOTH SIDES AS OCCURS

REF. 5/56.00 & 6/56.00
3/8

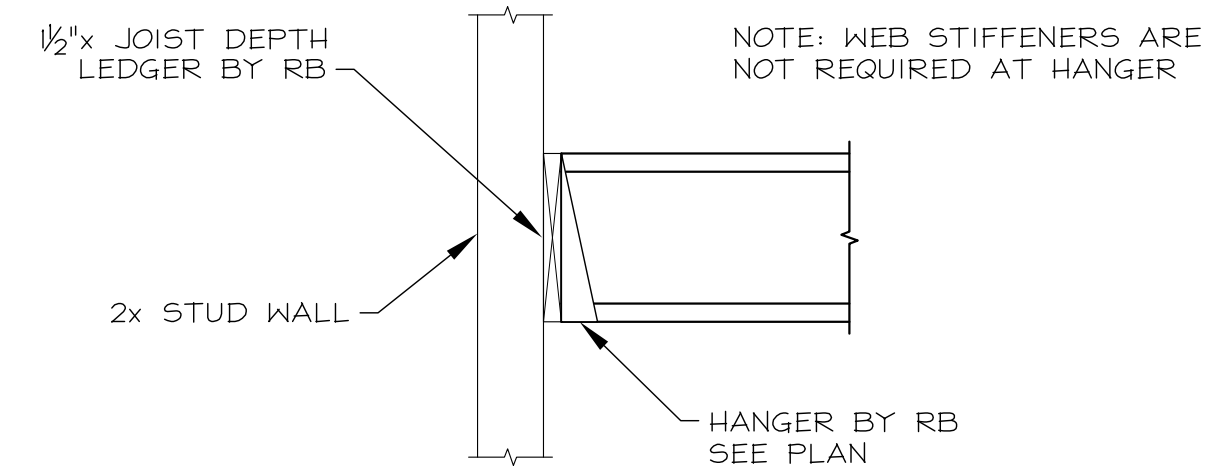


REF. 11/56.00
4/8



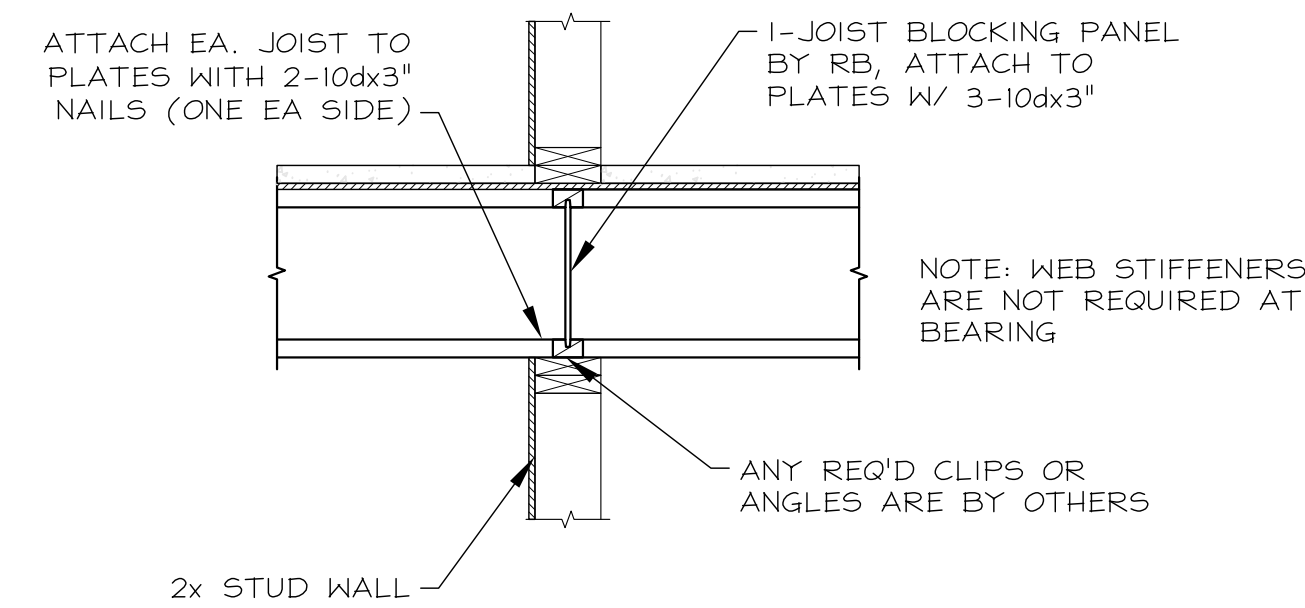
NOTE: BLOCKING PANELS AND/OR FLOOR JOIST CONFIGURATION VARIES, SEE PLAN.

REF. 12/56.00 & 13/56.00
5/8



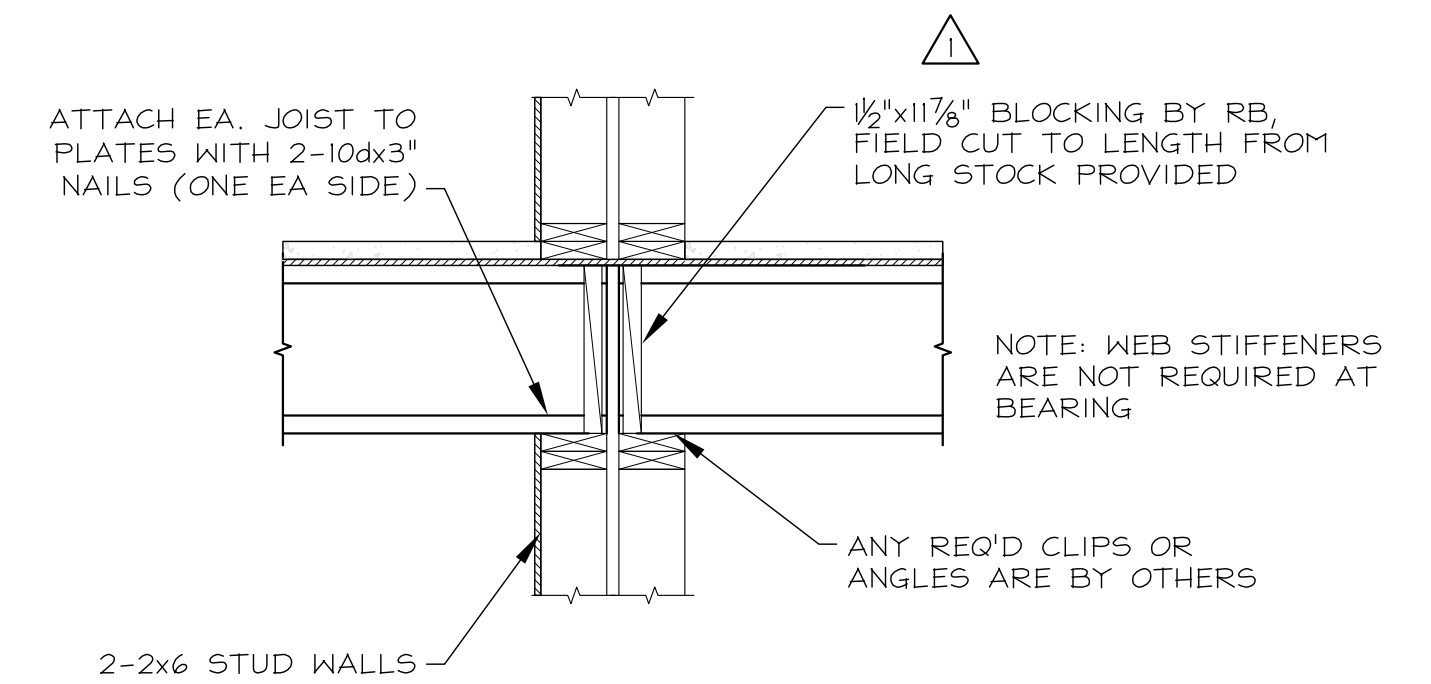
AT MID-LEVEL STAIR LANDINGS

REF. 3/56.01
6/8

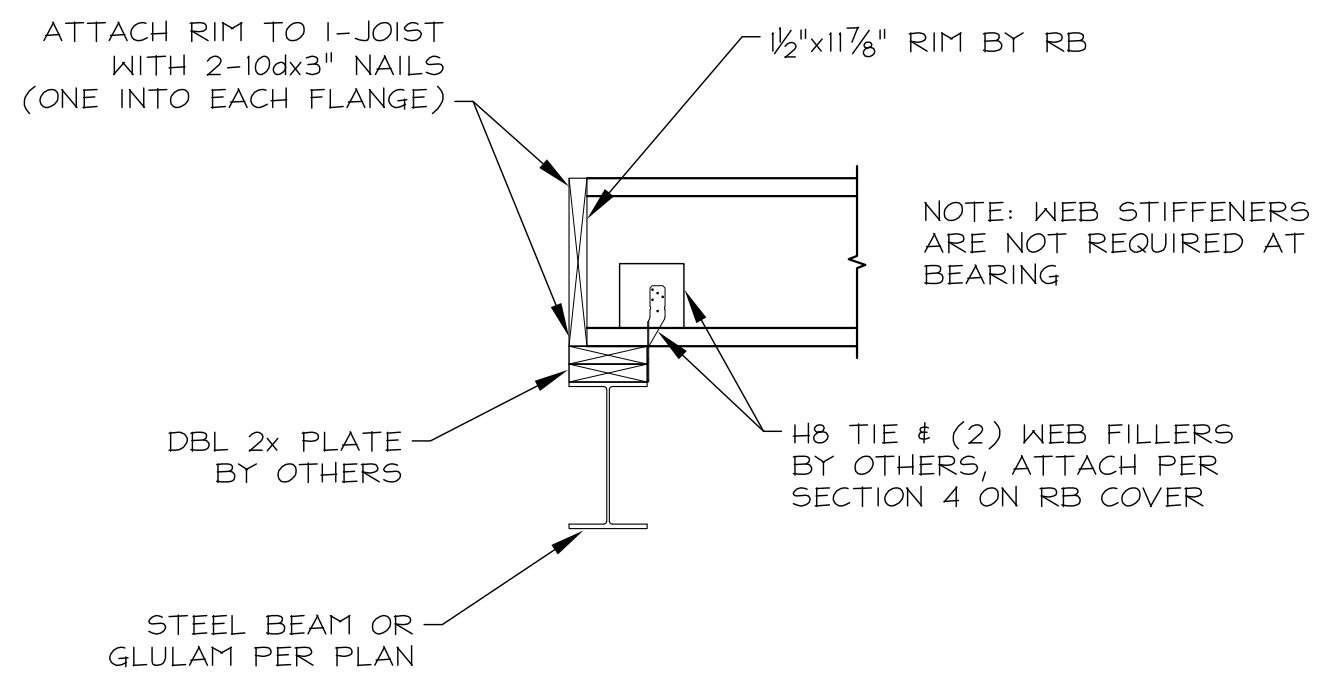


AT SIM CONDITION: 2 ROWS OF BLOCKING PANELS FOR DIFFERENT JOIST SERIES BUTTING ON WALL

REF. 4/56.00
7/8



REF. 3/56.00
8/8



REF. 1/56.02 & 2/56.02
9/8

- STRAPS, ANCHORS, CLIPS, AND OTHER HARDWARE NOT SHOWN ARE TO BE PROVIDED BY OTHERS. HARDWARE SHOWN IS TO BE PROVIDED BY OTHERS UNLESS MARKED 'BY RB.' REFER TO THE CONTRACT DOCUMENTS FOR HARDWARE SPECIFICATIONS AND INSTALLATION INSTRUCTIONS.

- SEE I-JOIST INSTALLATION SHEET FOR WEB STIFFENER NAILING.

- FOR BEAMS SUPPLIED BY OTHERS, SEE CONTRACT DOCUMENTS FOR SPECIFICATIONS AND OTHER INFORMATION NOT SHOWN HEREIN.

- FOR ATTACHMENT OF SPRINKLER LINES, MECHANICAL DUCTS, ETC... TO JOISTS OR TRUSSES, PLEASE SEE "SPRINKLER SYSTEM INSTALLATION GUIDELINES". IF ADDITIONAL COPIES ARE REQUIRED, PLEASE CONTACT REDBUILT OR GO ONLINE TO: [HTTP://WWW.REDBUILT.COM](http://www.redbuilt.com)

APPROVED FOR PRODUCTION

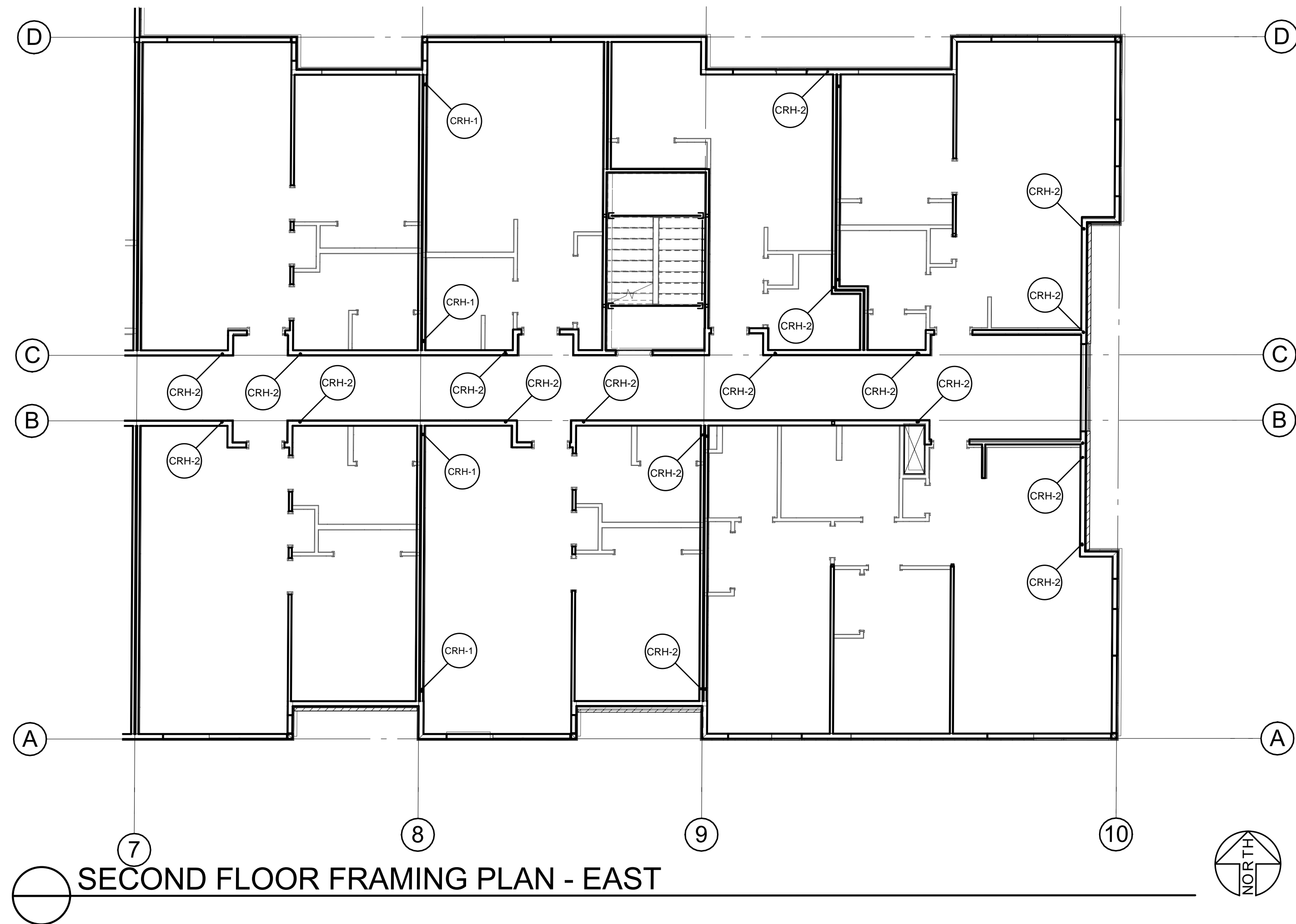
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|---|----|---------|----------|
| △ | | | |
| △ | JL | 5/23/19 | Detail 8 |
| △ | BY | DATE | REMARKS |

RedBUILT
Engineered Wood Products

Westman Mill
Olympia, WA

Details

| | | | | | |
|-------|---------|---------|------|---------|--------|
| DRAWN | DATE | CHECKED | DATE | ORDER # | SHEET |
| JL | 2/26/19 | - | - | 110805 | 8 OF 8 |




 Reviewed for Code Compliance
 Construction Permitting Only
Mark Balders
 Olympia
 Building Plans Examiner
 Community Planning & Development Department
 601 4th Ave East
 Olympia, WA 98501
 (360) 753-8248
 rbalders@ci.olympia.wa.us

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.


Conforms to Design Concept
 Conforms to Design Concept as noted
 Does Not Conform - Revise & Resubmit
 Checking is only for conformance with the design concept of the project. Contractor is responsible for dimensions to be confirmed and correlated at the job site; for information that pertains solely to the fabrication processes or to techniques of construction; and for the coordination of the work of all trades.
 By SJW Date 07/18/19
 PCS Structural Solutions
 101 SW Main Street, Suite 280
 Portland, OR 97204 503-232-3746

EARTHBOUND SYSTEM LOCATION LAYOUT SHEET

EARTHBOUND SYSTEM LAYOUT DISCLAIMER

THIS LAYOUT SHEET IS BASED ON HOLDOWN RUN LOCATIONS SHOWN ON STRUCTURAL PLAN SHEET "S2.02C". THE CONTRACTOR SHALL NOTIFY EARTHBOUND IMMEDIATELY IF REVISED PLANS THAT MAY AFFECT THE QUANTITY AND LOCATION OF RUNS SHOWN ARE ISSUED .

1. THIS LAYOUT SHEET IDENTIFIES EARTHBOUND SYSTEM RUNS FOR CONVENIENCE TO THE SYSTEM INSTALLER AND PROVIDED FOR REFERENCE ONLY. REFER TO THE CONSTRUCTION PLANS FOR ALL OTHER INFORMATION.
2. EARTHBOUND CORPORATION HAS NOT CONFIRMED AND IS NOT RESPONSIBLE FOR THE DESIGN, ENGINEERING, CALCULATIONS OR DERIVATION OF STRUCTURAL FORCES RELATED TO THE BUILDING. ANY DESIGN OR DEMAND LOAD INFORMATION USED OR SHOWN ON THESE DRAWINGS HAS BEEN TAKEN FROM THE CONSTRUCTION DOCUMENTS AND HAS NOT BEEN CONFIRMED BY EARTHBOUND.
3. THIS LAYOUT IS NOT FOR CONSTRUCTION WITHOUT SUBSTANTIATION OF REVIEW BY THE RESPONSIBLE ENGINEER OF RECORD AND THE GOVERNING BUILDING JURISDICTION.
4. THIS LAYOUT IS SPECIFIC TO THE EARTHBOUND SYSTEM AND IS NOT APPLICABLE TO OTHER TIEDOWN SYSTEMS.
5. REFER TO SHEETS SH-1, SH-2 FOR ADDITIONAL NOTES AND INFORMATION.

 "X" INDICATES EARTHBOUND SYSTEM HOLDOWN RUNS. ALL RUNS ALIGN WITH WALLS ABOVE.

THIS SHEET IS INTENDED FOR THE LOCATION OF THE EARTHBOUND HOLDOWNS ONLY REFER TO THE CONSTRUCTION PLANS FOR ALL OTHER INFORMATION.

| | |
|-----------------|---|
| PROJECT: | WESTMAN MILL 510 STATE AVE, OLYMPIA, WA 98501 |
| EB CORP JOB NO. | 19028 |
| DATE: | 05/15/19 |
| SHEET TITLE: | EARTHBOUND SEISMIC HOLDOWN SYSTEM LOCATION LAYOUT DRAWINGS. IAPMO ER-0429 |
| SHEET NO. | R1 |

| NO. | DATE | DESCRIPTION |
|-----|----------|---|
| 1 | 05/15/19 | REVISED PER STRUCTURAL PLANS DATED 03/19/19 |



http://www.holdown.com
 (800) 944-5669
 Fax: (360) 863-0722

EARTHBOUND CORPORATION
 17281 Tye Street SE
 Monroe, WA 98272

EARTHBOUND
 "Holdowns That Work"

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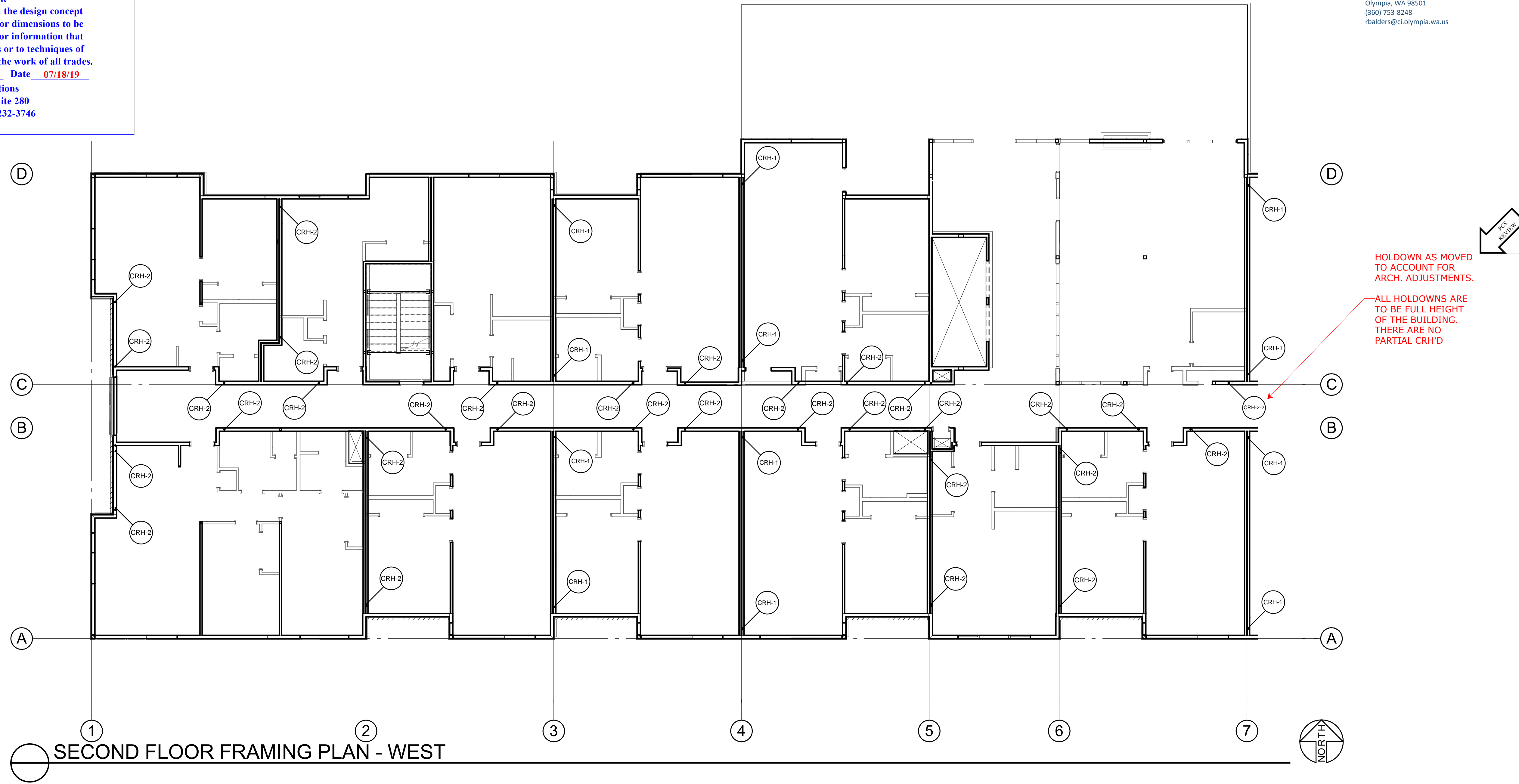
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By SJW Date 07/18/19

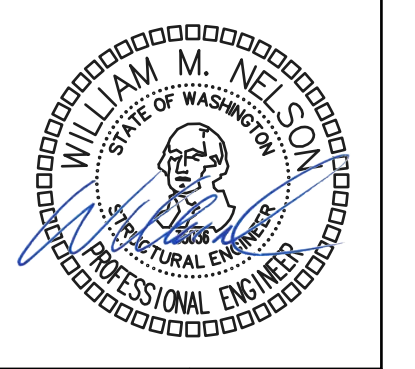
PCS Structural Solutions
 101 SW Main Street, Suite 280
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Olympia
 Building Plans Examiner
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 601 4th Ave East
 Olympia, WA 98501
 (360) 753-8248
 rbalders@ci.olympia.wa.us



SECOND FLOOR FRAMING PLAN - WEST

| NO. | DATE | DESCRIPTION |
|-----|----------|---|
| 1 | 05/15/19 | REVISED PER STRUCTURAL PLANS DATED 03/10/19 |



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WESTMAN MILL
 510 STATE AVE, OLYMPIA, WA 98501

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Holdowns That Work
 EARTHBOUND CORPORATION
 17281 Tye Street SE
 Monroe, WA 98272

EB CORP JOB NO.
19028

DATE:
 05/15/19

SHEET TITLE:
 EARTHBOUND SEISMIC
 HOLDDOWN SYSTEM LOCATION
 LAYOUT DRAWINGS.
 IAPMO ER-0429

SHEET NO.
R2

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**EARTHBOUND SYSTEM
 LOCATION LAYOUT SHEET**

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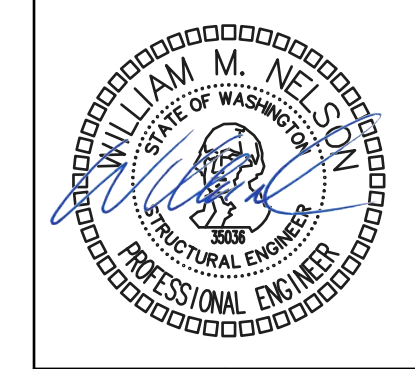
By SJW Date 07/18/19

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| NO. | DATE | DESCRIPTION |
|-----|----------|---|
| 1 | 05/15/19 | REVISED PER STRUCTURAL PLANS DATED 03/10/19 |



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PROJECT:
WESTMAN MILL
510 STATE AVE, OLYMPIA, WA 98501

EARTHBOUND
Holdowns That Work™

EARTHBOUND CORPORATION
 17261 Tye Street SE
 Monroe, WA 98272

**EARTHBOUND SYSTEM
 LOCATION LAYOUT SHEET**

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X

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| |
|---|
| EB CORP JOB NO. 19028 |
| DATE: 05/15/19 |
| SHEET TITLE: EARTHBOUND SEISMIC HOLDDOWN SYSTEM LOCATION LAYOUT DRAWINGS. IAPMO ER-0429 |
| SHEET NO. R3 |



**Earthbound System
Submittal Package for:**

**Westman Mill
510 State Ave
Olympia, WA 98501**

EB Corp. Job: 19028

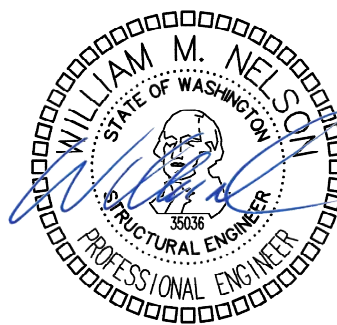
Rev A

May 15, 2019

Contains:

- 1) Run Type Analysis**
- 2) IAPMO ER0429**
- 3) Bridge and Nailing Calculations**

Prepared by:
Earthbound Corporation
17361 Tye Street SE
Monroe, WA 98272
800-944-5669
360-863-0724
<http://www.holdown.com>



Olympia

Building Plans Examiner
Community Planning & Development Department
601 4th Ave East
Olympia, WA 98501
(360) 753-8248
rbalders@ci.olympia.wa.us

Reviewed for Code Compliance
Construction Permitting Only

Rick Balders





DATE: May 15, 2019

PROJECT: Westman Mill
510 State Ave
Olympia, WA 98501

SUBJECT: Calculations Package

The following package is submitted for review:

- 1) Earthbound Run Analysis
- 2) IAPMO ER-0429 ("Earthbound Corporation")
- 3) Sample Bridge Calculations

Calculation Assumptions:

- 1) Earthbound Corporation took no part in the preparation or review of the project structural design and disclaims any liability regarding shear wall design, determination of holdown locations in the structure and/or magnitude of the holdown forces.
- 2) The specified uplift loads were based from the structural drawings.
- 3) Slackjack Devices are referenced from IAPMO ER-0429. This report is included in this submittal.
- 4) All full height member and compression capacities are referenced back to the required posting per structural plans. Wood Buckling and compression perpendicular to grain calculations are not required by Earthbound.

EARTHBOUND CORPORATION

17361 Tye Street, SE
Monroe, WA 98272

| | | |
|-----------|--------------|--|
| Project # | 19028 | CALCULATION SUMMARY OF ASSIGNED RUN TYPES |
| Project: | Westman Mill | |
| Date: | 5/15/2019 | |

| | | | | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| A | B | C | D | E | F | G | H | I | J | K | L | M | N | O |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|

Plans Date: 3/19/2019

CALCULATION NOTES ON COLUMNS:

| | |
|----------|--|
| A | Tension load provided by the structural plans. |
| B | Floor to floor differential loads. |
| C | Collected differential loads, if tiedowns on floors are skipped, the loads are brought up to the next level. |
| D | Assigned rod loads may be increased over column A if tiedowns are skipped on lower stories. |
| E | Final rod selection in accordance to specified design code (ASD) |
| F | Earthbound Shrinkage Fastener assigned to differential load (Column C) and travel required. |
| G | Bearing plate assigned to differential loads of Column C. Actual size shown. |
| H | Bearing plate is sized for both net area perpendicular to wood grain and bending thickness |
| I | Rod elongation based on PL/AE where A = nominal area and E = 29,000,000 psi. |
| J | Column Not Used |
| K | Device looseness is stated values of travel increments per IAPMO ER-0429. |
| L | Device capacity as stated in IAPMO ER-0429. |
| M | Device deflection is linear ratio of (applied load / rated load capacity of shrinkage fastener) x (displacement at rated load capacity) (M = C / L * Device Deflection at Rated Load) |
| N | Deflection total of Columns I thru M. |
| O | Quick check results if total system elongation is greater than required by the plans or local jurisdiction. |

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

| | | | | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| A | B | C | D | E | F | G | H | I | J | K | L | M | N | O |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|

| RUN TYPE CRH-1 | | | | | | | | | | | | | | | | | | |
|-----------------------|----------------------|-------------------------------|------------------------------|----------------------------|----------------|-------------------------|-----------------------|-------------------------|----------------------|------------------|-------------|-----------------------|------------|------------------|-----------------|-------------------|-----------------|--------------|
| WOOD FLOOR LEVEL | UPLIFT TENSION LOADS | CALCULATED DIFFERENTIAL LOADS | COLLECTED DIFFERENTIAL LOADS | ASSIGNED ROD TENSION LOADS | FINAL ROD SIZE | SHRINKAGE FASTENER SIZE | EBOUND PLATE PART NO. | ACTUAL PLATE DIMENSIONS | RATED PLATE CAPACITY | DEFLECTION CHECK | | | | | | | | |
| | | | | | | | | | | THREADED ROD | | NOMINAL AREA | ROD ELONG. | DEVICE LOOSENESS | DEVICE CAPACITY | DEVICE DEFLECTION | SYSTEM EL/DEFL. | 0.200 LIMIT? |
| | | | | | | | | | | DIA. (IN) | LENGTH (FT) | | | | | | | |
| 5th | | | | | | | | | | | | | | | | | | |
| 4th | 620 # | 620 # | 620 # | 620 # | R5HS | SJA225 | P6 | 3.00 x 3.50 x 1/4 | 6,650 lbs | 0.625 in | 7.5 FT | 0.307 in ² | 0.006 in | 0.050 in | 7,730 # | 0.001 in | 0.058 in | OK |
| 3rd | 1,500 # | 880 # | 880 # | 1,500 # | R5HS | SJA225 | P6 | 3.00 x 3.50 x 1/4 | 6,650 lbs | 0.625 in | 10.3 FT | 0.307 in ² | 0.021 in | 0.050 in | 7,730 # | 0.002 in | 0.073 in | OK |
| 2nd | 6,800 # | 5,300 # | 5,300 # | 6,800 # | R5HS | SJA215 | P6 | 3.00 x 3.50 x 1/4 | 6,650 lbs | 0.625 in | 10.3 FT | 0.307 in ² | 0.095 in | 0.045 in | 7,360 # | 0.009 in | 0.149 in | OK |
| 1st | 17,470 # | 10,670 # | 10,670 # | 17,470 # | R8HS | SJA418 | P12 | 3.25 x 6 x 5/8 | 12,270 lbs | 1.000 in | 10.3 FT | 0.785 in ² | 0.095 in | 0.051 in | 14,000 # | 0.017 in | 0.163 in | OK |

| RUN TYPE CRH-2 | | | | | | | | | | | | | | | | | | |
|-----------------------|----------------------|-------------------------------|------------------------------|----------------------------|----------------|-------------------------|-----------------------|-------------------------|----------------------|------------------|-------------|-----------------------|------------|------------------|-----------------|-------------------|-----------------|--------------|
| WOOD FLOOR LEVEL | UPLIFT TENSION LOADS | CALCULATED DIFFERENTIAL LOADS | COLLECTED DIFFERENTIAL LOADS | ASSIGNED ROD TENSION LOADS | FINAL ROD SIZE | SHRINKAGE FASTENER SIZE | EBOUND PLATE PART NO. | ACTUAL PLATE DIMENSIONS | RATED PLATE CAPACITY | DEFLECTION CHECK | | | | | | | | |
| | | | | | | | | | | THREADED ROD | | NOMINAL AREA | ROD ELONG. | DEVICE LOOSENESS | DEVICE CAPACITY | DEVICE DEFLECTION | SYSTEM EL/DEFL. | 0.200 LIMIT? |
| | | | | | | | | | | DIA. (IN) | LENGTH (FT) | | | | | | | |
| 5th | | | | | | | | | | | | | | | | | | |
| 4th | 1,040 # | 1,040 # | 1,040 # | 1,040 # | R5HS | SJA225 | P6 | 3.00 x 3.50 x 1/4 | 6,650 lbs | 0.625 in | 7.5 FT | 0.307 in ² | 0.011 in | 0.050 in | 7,730 # | 0.002 in | 0.063 in | OK |
| 3rd | 7,620 # | 6,580 # | 6,580 # | 7,620 # | R5HS | SJA225 | P6 | 3.00 x 3.50 x 1/4 | 6,650 lbs | 0.625 in | 10.3 FT | 0.307 in ² | 0.106 in | 0.050 in | 7,730 # | 0.015 in | 0.172 in | OK |
| 2nd | 21,260 # | 13,640 # | 13,640 # | 21,260 # | R8HS | SJA418 | P14 | 3.25 x 7 x 3/4 | 14,460 lbs | 1.000 in | 10.3 FT | 0.785 in ² | 0.116 in | 0.051 in | 14,000 # | 0.021 in | 0.188 in | OK |
| 1st | 45,790 # | 24,530 # | 24,530 # | 45,790 # | R12HS | HJS7112 | P26-12 | 3.5 x 12.5 x 1 1/4 | 26,490 lbs | 1.500 in | 10.3 FT | 1.767 in ² | 0.111 in | 0.012 in | 39,190 # | 0.018 in | 0.141 in | OK |

EARTHBOUND CORPORATION

17361 Tye Street, SE
 Monroe, WA 98272

| | | |
|-----------|--------------|--|
| Project # | 19028 | CALCULATION SUMMARY OF ASSIGNED RUN TYPES |
| Project: | Westman Mill | |
| Date: | 5/15/2019 | |

| | | | | | | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| A | B | C | D | E | F | G | H | I | J | K | L | M | N | O |
|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|

| RUN TYPE CRH-2-2 | | | | | | | | | | | | | | | | | | | |
|------------------|----------------------|-------------------------------|------------------------------|----------------------------|----------------|-------------------------|-----------------------|-------------------------|----------------------|------------------|-------------|-----------------------|------------|------------------|-----------------|-------------------|-----------------|--------------|--|
| WOOD FLOOR LEVEL | UPLIFT TENSION LOADS | CALCULATED DIFFERENTIAL LOADS | COLLECTED DIFFERENTIAL LOADS | ASSIGNED ROD TENSION LOADS | FINAL ROD SIZE | SHRINKAGE FASTENER SIZE | EBOUND PLATE PART NO. | ACTUAL PLATE DIMENSIONS | RATED PLATE CAPACITY | DEFLECTION CHECK | | | | | | | | | |
| | | | | | | | | | | THREADED ROD | | NOMINAL AREA | ROD ELONG. | DEVICE LOOSENESS | DEVICE CAPACITY | DEVICE DEFLECTION | SYSTEM EL/DEFL. | 0.200 LIMIT? | |
| | | | | | | | | | | DIA. (IN) | LENGTH (FT) | | | | | | | | |
| 5th | | | | | | | | | | | | | | | | | | | |
| 4th | | | | | | | | | | | | | | | | | | | |
| 3rd | | | | | | | | | | | | | | | | | | | |
| 2nd | 21,260 # | 21,260 # | 21,260 # | 21,260 # | R8HS | HJS718 | P22 | 3.5 x 11 x 1 1/4 | 23,210 lbs | 1.000 in | 10.3 FT | 0.785 in ² | 0.116 in | 0.012 in | 39,190 # | 0.016 in | 0.143 in | OK | |
| 1st | 45,790 # | 24,530 # | 24,530 # | 45,790 # | R12HS | HJS7112 | P26-12 | 3.5 x 12.5 x 1 1/4 | 26,490 lbs | 1.500 in | 10.3 FT | 1.767 in ² | 0.111 in | 0.012 in | 39,190 # | 0.018 in | 0.141 in | OK | |

| RUN TYPE CRH-2-SB | | | | | | | | | | | | | | | | | | | |
|-------------------|----------------------|-------------------------------|------------------------------|----------------------------|----------------|-------------------------|-----------------------|-------------------------|----------------------|------------------|-------------|-----------------------|------------|------------------|-----------------|-------------------|-----------------|--------------|--|
| WOOD FLOOR LEVEL | UPLIFT TENSION LOADS | CALCULATED DIFFERENTIAL LOADS | COLLECTED DIFFERENTIAL LOADS | ASSIGNED ROD TENSION LOADS | FINAL ROD SIZE | SHRINKAGE FASTENER SIZE | EBOUND PLATE PART NO. | ACTUAL PLATE DIMENSIONS | RATED PLATE CAPACITY | DEFLECTION CHECK | | | | | | | | | |
| | | | | | | | | | | THREADED ROD | | NOMINAL AREA | ROD ELONG. | DEVICE LOOSENESS | DEVICE CAPACITY | DEVICE DEFLECTION | SYSTEM EL/DEFL. | 0.200 LIMIT? | |
| | | | | | | | | | | DIA. (IN) | LENGTH (FT) | | | | | | | | |
| 5th | | | | | | | | | | | | | | | | | | | |
| 4th | 1,040 # | 1,040 # | 1,040 # | 1,040 # | R5HS | SJA215 | P6 | 3.00 x 3.50 x 1/4 | 6,650 lbs | 0.625 in | 7.5 FT | 0.307 in ² | 0.101 in | 0.045 in | 7,360 # | 0.002 in | 0.057 in | OK | |
| 3rd | 7,620 # | 6,580 # | 6,580 # | 7,620 # | R5HS | SJA215 | P6 | 3.00 x 3.50 x 1/4 | 6,650 lbs | 0.625 in | 10.3 FT | 0.307 in ² | 0.106 in | 0.045 in | 7,360 # | 0.012 in | 0.163 in | OK | |
| 2nd | | | | | | | | | | 0.625 in | | 0.307 in ² | | | | | | | |
| 1st | | | | | | | | | | | | | | | | | | | |

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

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| | |
|------------|--------------|
| Project: | Westman Mill |
| Project #: | 19028 |
| Date: | 5/15/2019 |

EARTHBOUND THREADED ROD CAPACITIES (IBC 2015)

| ROD SIZE | ROD SIZE (INCHES) | ALLOWABLE TENSION LOAD IBC 2015 | ROD REMARKS | SLACKJACK SIZE |
|----------|-------------------|---------------------------------|-----------------------|---|
| R4 | 1/2" DIA. | 4,470 LBS | ASTM A 307 (UNC) | MJ100, MJ200 |
| R5 | 5/8" DIA. | 7,120 LBS | ASTM A 307 (UNC) | SJA 215, 225, 415, 425 |
| R6 | 3/4" DIA. | 10,540 LBS | ASTM A 307 (UNC) | SJA 216, 226, 416, 426, HJA 716, 726 |
| R7 | 7/8" DIA. | 14,540 LBS | ASTM A 307 (UNC) | SJA 217, 227, 417, 427, HJA 717, 727 |
| R8 | 1" DIA. | 19,080 LBS | ASTM A 307 (UNC) | SJA 218, 228, 418, 428, HJA 718, 728 |
| R9 | 1 1/8" DIA. | 24,040 LBS | ASTM A 307 (UNC) | SJA 219, 229, 419, 429, HJA 719, 729 |
| R10 | 1 1/4" DIA. | 30,530 LBS | ASTM A 307 (UNC) | SJA 4110, 4210, HJA 7110, 7210, HJS 4110, 4210 |
| R12 | 1 1/2" DIA. | 44,270 LBS | ASTM A 307 (UNC) | SJST 4112, 4212, HJA 7112, 7212, HJS 4112, 4212 |
| R14 | 1 3/4" DIA. | 59,830 LBS | ASTM A 307 (UNC) | ---- |
| R5M | 5/8" DIA. | 8,900 LBS | ASTM F1554 GR55 (UNC) | SJA 215, 225, 415, 425 |
| R6M | 3/4" DIA. | 13,170 LBS | ASTM F1554 GR55 (UNC) | SJA 216, 226, 416, 426, HJA 716, 726 |
| R7M | 7/8" DIA. | 18,180 LBS | ASTM F1554 GR55 (UNC) | SJA 217, 227, 417, 427, HJA 717, 727 |
| R8M | 1" DIA. | 23,850 LBS | ASTM F1554 GR55 (UNC) | SJA 218, 228, 418, 428, HJA 718, 728 |
| R9M | 1 1/8" DIA. | 30,050 LBS | ASTM F1554 GR55 (UNC) | SJA 219, 229, 419, 429, HJA 719, 729 |
| R10M | 1 1/4" DIA. | 38,160 LBS | ASTM F1554 GR55 (UNC) | SJA 4110, 4210, HJA 7110, 7210, HJS 4110, 4210 |
| R12M | 1 1/2" DIA. | 55,330 LBS | ASTM F1554 GR55 (UNC) | SJST 4112, 4212, HJA 7112, 7212, HJS 4112, 4212 |
| R14M | 1 3/4" DIA. | 74,790 LBS | ASTM F1554 GR55 (UNC) | ---- |

ASTM A449 THREADED ROD ASTM A 193-B7 THREADED ROD SLACKJACK SIZES

| R5HS | 5/8" DIA. | 14,240 LBS | 14,830 LBS | SJA 215, 225, 415, 425 |
|-------|-------------|-------------|-------------|---|
| R6HS | 3/4" DIA. | 21,070 LBS | 21,950 LBS | SJA 216, 226, 416, 426, HJA 716, 726 |
| R7HS | 7/8" DIA. | 29,090 LBS | 30,300 LBS | SJA 217, 227, 417, 427, HJA 717, 727 |
| R8HS | 1" DIA. | 38,160 LBS | 39,750 LBS | SJA 218, 228, 418, 428, HJA 718, 728 |
| R9HS | 1 1/8" DIA. | 42,080 LBS | 50,090 LBS | SJA 219, 229, 419, 429, HJA 719, 729 |
| R10HS | 1 1/4" DIA. | 53,420 LBS | 63,600 LBS | SJA 4110, 4210, HJA 7110, 7210, HJS 4110, 4210 |
| R12HS | 1 1/2" DIA. | 77,460 LBS | 92,220 LBS | SJST 4112, 4212, HJA 7112, 7212, HJS 4112, 4212 |
| R14HS | 1 3/4" DIA. | 104,710 LBS | 124,650 LBS | ---- |

EARTHBOUND BEARING PLATE CAPACITIES (DF)

| PLATE SIZE | DIFFERENTIAL LOAD | COLOR CODE | PLATE DIMENSIONS (INCHES) | | | PART NO. | ROD DIAMETER | ROD SIZE | WASHER COLOR |
|------------|-------------------|------------|---------------------------|--------|--------|-------------------------------|--------------|--------------|--------------|
| | | | WIDTH | LENGTH | THICK. | | | | |
| P6 | 6,650 LBS | GREEN | 3" | 3.5" | 1/4" | MJ100 or MJ200 | 1/2" | R4 | PURPLE |
| P8 | 8,470 LBS | BLACK | 3-1/4" | 4.25" | 1/4" | SJA 2x5 or 4x5 | 5/8" | R5 | BLACK |
| P10 | 10,510 LBS | BLUE | 3-1/4" | 5" | 3/8" | SJA 2x6 or 4x6, HJA 7x6 | 3/4" | R6 or R6HS | GRAY |
| P12 | 12,270 LBS | GRAY | 3-1/4" | 6" | 5/8" | SJA 2x7 or 4x7, HJA 7x7 | 7/8" | R7 or R7HS | BLUE |
| P14 | 14,460 LBS | RED | 3-1/4" | 7" | 3/4" | SJA 2x8 or 4x8, HJA 7x8 | 1" | R8 or R8HS | YELLOW |
| P18 | 18,840 LBS | YELLOW | 3-1/2" | 9" | 1" | SJA 2x9 or 4x9, HJA 7x9 | 1 1/8" | R9 or R9HS | WHITE |
| P20 | 21,020 LBS | BROWN | 3-1/2" | 10" | 1" | SJA 4x10, HJA 7x10, HJS 4x10 | 1 1/4" | R10 or R10HS | GREEN |
| P22 | 23,210 LBS | WHITE | 3-1/2" | 11" | 1 1/4" | SJST 4x12, HJA 7x12, HJS 4x12 | 1 1/2" | R12 or R12HS | RED |
| P24 | 24,310 LBS | GOLD | 3-1/2" | 11.5" | 1 1/4" | | | | |
| P26 | 26,490 LBS | ORANGE | 3-1/2" | 12.5" | 1 1/2" | | | | |

*x" = SLACKJACK TRAVEL IN "INCHES"

NOTES:

- TENSION LOAD WAS CALCULATED FROM THE FOLLOWING EQUATION:
 - ASTM A 307 Threaded Rod Capacities are F_u = 60,000 psi. UNC thread pitch
M Rods are based on ASTM F1554 GR55, F_u = 75 ksi.
HS Rods are based on ASTM A193 B7, F_u = 125 ksi or ASTM A449, F_u = 120 ksi, 105 ksi above R8HS
 - The IBC 2015 column is calculated in accordance to Section 1905 and ASCE 7-10.
- PLATE STEEL SHALL BE ASTM A36: F_u = 60,000 PSI.
- SUBSTITUTIONS OF DESIGNATED BEARING PLATE SHALL NOT BE PERMITTED, OBTAIN WRITTEN APPROVAL FROM THE ENGINEER.
- SLACKJACK SELECTION NOTES:
THE SIZES SHOWN ABOVE ARE FOR BOTH ONE AND TWO INCH TRAVEL ("SJA 4xy")
(x = TRAVEL HEIGHT IN INCHES, y = ROD SIZE). TWO INCH TRAVEL SLACKJACKS ARE REQUIRED ON FOURTH WOOD FRAME LEVELS AND HIGHER.
- THIS TABLE IS FOR REFERENCE OF FULL PRODUCT LINE, SOME ROD AND PLATE SIZES MAY NOT BE IN USE. THE ENGINEER OF RECORD SHALL REVIEW AND APPROVE CAPACITIES.

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



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SLACKJACK[®], HEAVYJACK[™] AND MINIJACK[®] SHRINKAGE COMPENSATION DEVICES

CSI Section:

06 05 23 Wood, Plastic and Composite Fastenings

1.0 RECOGNITION

SlackJack[®], HeavyJack[™] and MiniJack[®], manufactured by Earthbound Corporation, were evaluated for use as compression-controlled shrinkage compensation devices in wood-framed construction. The structural properties of the devices were evaluated for compliance with the following codes and regulations:

- 2018, 2015, 2012, 2009 and 2006 International Building Code[®] (IBC)
- 2018, 2015, 2012, 2009 and 2006 International Residential Code[®] (IRC)
- 2016 and 2013 California Building Code (CBC) – See attached Supplement
- 2016 and 2013 California Residential Code (CRC) – See attached Supplement
- 2017 Florida Building Code, Building (FBC, Building) – See attached Supplement
- 2017 Florida Building Code, Residential (FBC, Residential) – See attached Supplement
- 2017 City of Los Angeles Building Code (LABC) – See attached Supplement
- 2017 City of Los Angeles Residential Code (LARC) – See attached Supplement

2.0 LIMITATIONS

Use of the SlackJack, HeavyJack and MiniJack Shrinkage Compensation Devices recognized in this report are subject to the following limitations:

- 2.1 The SlackJack, HeavyJack and MiniJack devices shall be limited to installations in dry, interior locations.
- 2.2 Use of the SlackJack, HeavyJack and MiniJack devices in direct contact with fire-retardant or preservative-treated wood is beyond the scope of recognition of this report.
- 2.3 Where required, designs using these products shall be submitted to the building code official for review.

2.4 No increase in the allowable capacities shown in [Table 4](#) of this report shall be permitted.

2.5 The dead load is limited to the self-weight of the SlackJack, HeavyJack and MiniJack devices. Additional dead load on the devices is beyond the scope of this report.

2.6 The maximum offset tolerance shall be 1.33 degrees from vertical.

2.7 “When the devices are used in continuous rod systems that resist light-frame shear wall overturning forces, calculations shall be submitted to the code official confirming that the total vertical displacement, which would include steel rod elongation and the shrinkage compensating device deflection, is less than or equal to 0.20 inch (5 mm) for each story, or between restraints, whichever is more restrictive, using allowable stress design (ASD). Shear wall drift limit calculations shall consider the 0.20 inch (5 mm) vertical displacement limit. This 0.20-inch (5 mm) vertical displacement limit may be exceeded when it can be demonstrated that the shear wall story drift limit and the deformation compatibility requirements of IBC Section 1604.4 are met when considering all sources of vertical displacement.” (AC316)

2.8 Buildings constructed to the IRC shall have engineered designs performed on the elements of construction using this device as required in Section R301.1.3.

2.9 The shrinkage compensation devices recognized in this report are produced by Earthbound Corporation in Monroe, Washington.

3.0 PRODUCT USE INSTRUCTIONS

3.1 The Earthbound installation instructions, this evaluation report, and the applicable provisions of the building code shall be followed when installing this product. Where conflicts occur between these documents, the more restrictive provisions shall govern. The published installation instructions shall be available at the jobsite during construction for use by installers and for quality assurance.

3.2 Where required by the code official or other authority having jurisdiction, calculations based on applied loads to the device shall be provided by a registered design professional to show the basis for its selection. The calculations shall show the projected shrinkage, deflection, and settlement the device will compensate for, and the method of transferring the loads through the supports. The appropriate SlackJack, HeavyJack and MiniJack device shall be chosen based on the model and series characteristics. The Rated Shrinkage Capacity, Allowable



Compression Load, Deflection at Allowable Load and Device Average Travel and Seating Increment, Δ_R , for each model are shown in [Table 4](#) of this report.

3.3 The chosen SlackJack, HeavyJack or MiniJack device shall be slid over a threaded-rod or anchor bolt, over an approved bearing surface or plate. The appropriate color-coded swivel washer (listed in [Table 4](#) of this report), shall be installed as required by the manufacturer’s installation instructions with the flat side up. A hex nut shall be installed over the rod and hand-tightened.

To place the device into service, the pull clip shall be completely removed and placed on the device to show that the device has been activated.

Where necessary, the devices may be reset in the field. The manufacturer’s installation instructions shall be referenced for details.

3.4 SlackJacks may be substituted and interchangeable with HeavyJack devices per Table 3 of this report.

4.0 PRODUCT DESCRIPTION

4.1 SlackJack, HeavyJack and MiniJack Shrinkage Compensation Devices are spring-loaded, compression-controlled shrinkage compensating devices that are designed to work with threaded-rod or anchor bolt hold-down systems in wood-frame construction. The devices are cylindrical, high-strength structural connectors that enable axial compression travel along a bolted or sliding connection but to withstand movement due to tensile loads.

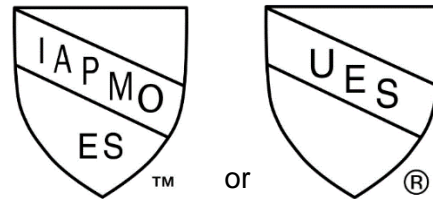
4.2 The materials used for the inner and outer cylinders, locking rings, springs, pull-clips, capture-rings and swivel washers, for the SlackJack, HeavyJack and MiniJack models are shown in [Table 1](#) of this report.

4.3 The model and series number describe the attributes of each shrinkage compensation device. The devices are color-coded to identify the specific model type, which varies based on strength and length of travel. The available travel lengths are: 1 inch, 1 ½ inches, 2 inches, and 3 inches (25.4 mm, 38.1 mm, 50.8 mm, and 76.2 mm). [Table 2](#) of this report describes the model and series notations. Color coding is also used to identify the size of the swivel washer for the SlackJack devices as shown in [Table 5](#) of this report.

5.0 IDENTIFICATION

A label shall be affixed on at least one of the following: product, packaging, installation instructions or descriptive literature. The label shall include the Earthbound name or trademark, the device model number, the IAPMO Uniform

ES Mark of Conformity and the Evaluation Report Number (ER-429) to identify the products recognized in this report. A die-stamp label may be used as a substitute for the label. Either Mark of Conformity may be used as shown below:



IAPMO UES ER-429

6.0 SUBSTANTIATING DATA

Data in accordance with ICC-ES Acceptance Criteria for Shrinkage Compensating Devices (AC316), dated June 2013 (Editorially Revised November 2017).

7.0 STATEMENT OF RECOGNITION

This report describes the results of research carried out by the IAPMO Uniform Evaluation Service on Earthbound’s SlackJack, HeavyJack and MiniJack Shrinkage Compensation Devices to assess their conformance to the codes and standards listed in Section 1.0 and serves as documentation of the product certification. Products are manufactured at the location noted in Section 2.9 of this report under a quality control program with periodic inspection under the supervision of IAPMO UES.

Brian Gerber, P.E., S.E.
Vice President, Technical Operations
Uniform Evaluation Service

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Vice President, Uniform Evaluation Service

GP Russ Chaney
CEO, The IAPMO Group

For additional information about this evaluation report please visit www.uniform-es.org or email us at info@uniform-es.org



TABLE 1 – Material Information for SlackJack, HeavyJack and MiniJack Devices

| Component | Material |
|--|---|
| Internal and Outer Cylinders for HeavyJack Models HJS and HJX | ASTM A513/5 520 DOM Steel |
| Internal and Outer Cylinders for SlackJack Models SJT and SJA, HeavyJack Model HJA and MiniJack Model MJ | ASTM B221-08 6061-T6511 Aluminum |
| Locking Components for all Models | ASTM A313 Stainless Steel |
| Springs and Pull Clips for all Models | ASTM A764 Galvanized Hard Drawn Spring Wire |
| Capture Rings for all Models | Plastic Injection Molded |
| Swivel Washers | ASTM A108 12L14 Steel |

TABLE 2 – Model and Series Notations

| Notation | |
|--|---|
| Model Type | MJ = MiniJack, HJ = HeavyJack SJ = SlackJack |
| First Number (Series Type) | 2= 2 Locking Rings 3= 3 Locking Rings 4= 4 Locking Rings 6= 6 Locking Rings 7=7 Locking Rings |
| Second Number (Rated Shrinkage Capacity) | 1 = 1 inch 1-5 = 1.5 inches 2 = 2 inches 3 = 3 inches |
| Third Number (Swivel Washer Code/Color) | See Table 5 |

Example Nomenclature: SlackJack model with a series number A420, the letter “A” indicates the model number, with the first number “4” which has 4 locking rings. The second number indicates the rated compensation capacity, which in this case is 2 inches and finally, the third number indicates the diameter of the rod and color code for the swivel washer. For example, if the rod is ½ inch in diameter, the swivel washer color will be purple.

TABLE 3 – HeavyJack Device Substitution for SlackJack Devices

HJA, HJS, HJX Series HeavyJacks are interchangeable and maybe substituted where SJA, SJS, and SJX devices are specified under the following conditions:

| SlackJack Devices | Allowed HeavyJack Substitution Devices | Conditions of Use |
|-------------------|--|-----------------------------|
| SJS-410 | HJA-710 | Loads up to 15,650 lbs |
| SJS-420 | HJA-720 | Loads up to 15,590 lbs |
| SJS-410 | HJS-410 | Any SJS-410 Load Conditions |
| SJS-420 | HJS-420 | Any SJS-420 Load Conditions |
| SJA-610 | HJA-710 | Loads up to 15,650 lbs |
| SJA-620 | HJA-720 | Loads up to 15,590 lbs |
| SJT-610 | HJA-710 | Loads up to 15,650 lbs |
| SJT-620 | HJA-720 | Loads up to 15,590 lbs |
| SJX-410 | HJA-710 | Loads up to 15,650 lbs |
| SJX-410 | HJS-710 | Any SJX-410 Load Conditions |
| SJX-410 | HJX-410 | Any SJX-410 Load Conditions |



TABLE 4 - Attributes for the SlackJack, HeavyJack and MiniJack Shrinkage Compensating Devices

| Model ³ & Series Number | Capture Ring Color Code | Additional Color Code | Nominal Dimensions (Inches) Figure 1 Shows Dimensions A, B And C | | | Rated Shrinkage Capacity (Inches) | Allowable Compression Load ^{1,2} (Lbs) | Δ_A Deflection at Allowable Load (Inch) ⁵ | Δ_R^5 (Inch) |
|------------------------------------|-------------------------|-----------------------|---|----------------------------|----------------------------|-----------------------------------|---|---|---------------------|
| | | | Height Before Activation (A) | Outer Diameter of Body (B) | Inner Diameter of Body (C) | | | | |
| MJ100 ⁴ | Red | N/A | 2.540 | 1.349 | 0.532 | 1.0 | 5,000 | 0.028 | 0.032 |
| MJ150 ⁴ | Green | N/A | 3.040 | 1.349 | 0.532 | 1.5 | 5,000 | 0.030 | 0.032 |
| MJ200 ⁴ | Orange | N/A | 3.540 | 1.349 | 0.532 | 2.0 | 4,900 | 0.033 | 0.031 |
| SJA-210 | Blue | N/A | 3.330 | 2.365 | 1.420 | 1.0 | 7,360 | 0.013 | 0.045 |
| SJA-21-50 | White | N/A | 3.830 | 2.365 | 1.420 | 1.5 | 8,000 | 0.018 | 0.047 |
| SJA-220 | Yellow | N/A | 4.330 | 2.365 | 1.420 | 2.0 | 7,730 | 0.018 | 0.050 |
| SJT-410 | Orange | N/A | 3.700 | 2.365 | 1.375 | 1.0 | 9,000 | 0.020 | 0.024 |
| SJT-420 | Black | N/A | 4.700 | 2.365 | 1.375 | 2.0 | 9,000 | 0.018 | 0.028 |
| SJA-410 | Red | N/A | 3.800 | 2.365 | 1.250 | 1.0 | 14,000 | 0.022 | 0.051 |
| SJA-420 | Green | N/A | 4.800 | 2.365 | 1.250 | 2.0 | 14,000 | 0.030 | 0.047 |
| SJA-430 | Brown | N/A | 5.800 | 2.365 | 1.250 | 3.0 | 14,750 | 0.033 | 0.046 |
| SJA-610 | Tan | N/A | 4.800 | 2.365 | 1.250 | 1.0 | 20,340 | 0.028 | 0.045 |
| SJA-620 | White | Blue ⁶ | 5.800 | 2.365 | 1.250 | 2.0 | 20,100 | 0.038 | 0.048 |
| HJA-710 | Purple | N/A | 4.730 | 2.500 | 1.625 | 1.0 | 15,650 | 0.015 | 0.015 |
| HJA-720 | Gray | N/A | 5.730 | 2.500 | 1.625 | 2.0 | 15,590 | 0.018 | 0.014 |
| HJS-410 ⁸ | Black | Blue ⁷ | 3.270 | 2.500 | 1.625 | 1.0 | 22,000 | 0.020 | 0.028 |
| HJS-420 ⁸ | Black | Yellow ⁷ | 4.270 | 2.500 | 1.625 | 2.0 | 22,000 | 0.026 | 0.028 |
| HJS-710 | Black | Green ⁷ | 4.730 | 2.500 | 1.625 | 1.0 | 39,190 | 0.029 | 0.012 |
| HJS-720 | Black | Orange ⁷ | 5.730 | 2.500 | 1.625 | 2.0 | 37,770 | 0.030 | 0.011 |
| HJX-410 ⁸ | Black | Gold ⁷ | 3.700 | 2.500 | 1.625 | 1.0 | 34,220 | 0.035 | 0.047 |

(Information in Table 4 and Notes 1-4 are from Earthbound QC (“QC”) Documentation and Test Data)

Notes to Table 4:

1. “Tabulated allowable loads are for the shrinkage compensating device only. The attached components (including anchors, tension rods, bearing plates, wood framing members, etc.) shall be designed to resist design loads in accordance with the applicable code.” (QC)
2. “No further increases to the tabulated allowable loads are permitted.” (QC)
3. “Model numbers beginning with an A or T are designed for use with W3 through W10 swivel washers capable of fitting over threaded rods or bolts having diameters ranging from 3/8 inch to 1¼ inches. Model numbers beginning with an S or X are designed for use with W10 or W12 swivel washers capable of fitting over threaded rods or bolts having diameters of 1 ¼ inches and 1 ½ inches respectively.” (QC)
4. “MJ100, MJ150 and MJ200 Series are for either 3/8 -inch or 1/2 -inch rod diameter. Requires Heavy Hex nut when installing on 3/8-inch-diameter rod.” (QC)
5. “The device Δ_R and Δ_A describe the total movement of the device at allowable load, Δ_T , and are additive. For design loads, P_D , less than the allowable load, P_A , the total movement of the device, Δ_T , is calculated as follows:

$$\Delta_T = \Delta_R + \Delta_A(P_D/P_A) \text{ (AC316)}$$



- 6. Additional color code for this series is a painted compression spring.
- 7. Additional color code for this series is a painted outer cylinder.
- 8. Earlier series of SlackJacks SJS-4xx and SJX-41x are relabeled as HJS-4xx and HJX-41x HeavyJacks respectively in this report.

Table 5 – Swivel Washer Color Code

| Number | Diameter of Rod | Color |
|--------|-----------------|--------|
| | 0=Base Model | |
| 3 | 3/8" | Orange |
| 4 | 1/2" | Purple |
| 5 | 5/8" | Black |
| 6 | 3/4" | Gray |
| 7 | 7/8" | Blue |
| 8 | 1" | Yellow |
| 9 | 1 1/8" | White |
| 10 | 1 1/4" | Green |
| 12 | 1 1/2" | Red |

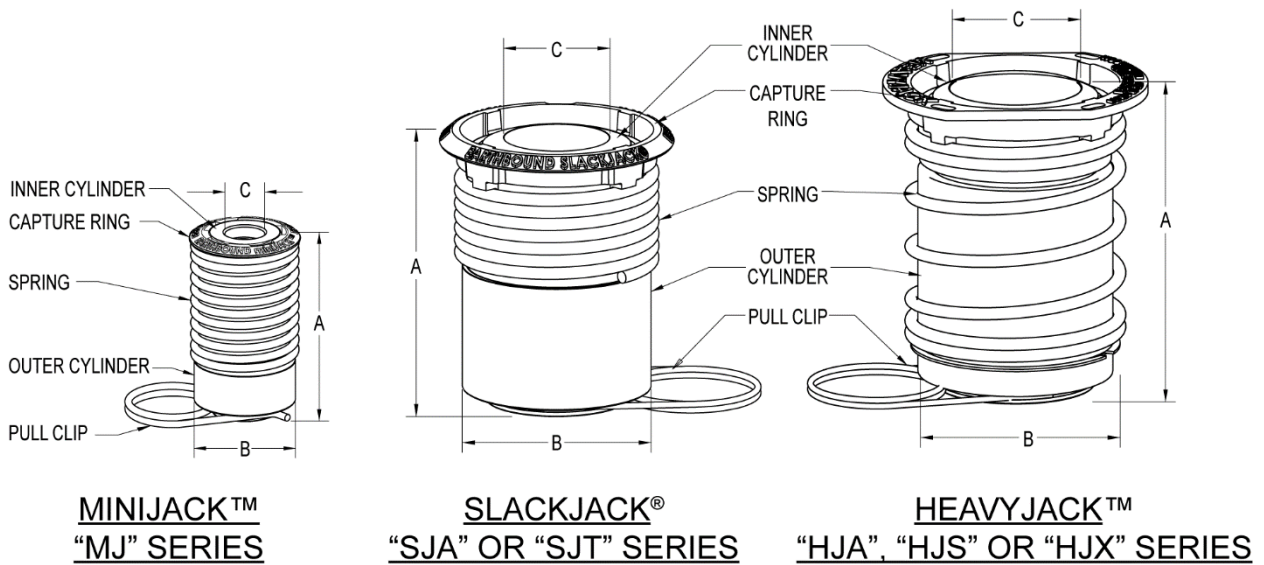


FIGURE 1 - SLACKJACK®, HEAVYJACK™ AND MINIJACK™ SHRINKAGE COMPENSATING DEVICES

| | | |
|--------------------|--------------|--|
| Project: | Westman Mill | WOOD COMPRESSION CALCULATION BASED ON IBC 2015 (NDS 2015) BASED ON NO. 2 DOUG FIR STUDS/POSTS ON DOUG FIR PLATES SEE SPECIFIC MEMBER HEIGHTS |
| Project #: | 19028 | |
| Date: | 5/15/2019 | |
| Struct Plans Date: | 3/19/2019 | |

POST COLUMN DESIGN STUD HEIGHT #1

| DOUG-FIR POSTS ON DOUG FIR FLOOR PLATES | | | | | | | | | | | | | | | | | | | | | |
|---|-------|---------------------|------------------------------------|----------------------------|------------------------------|------------------------------|-------------------|--|------|-----------------------|--------------------|-------------------------|-----------------------|-------------------|---|------------------------------------|--|---|--|---------------------------------|--------|
| A | B | C | D | E | F | G | H | I | J | K | L | M | N | O | P | Q | R | S | T | U | V |
| Size | Grade | Height h inch | PARALLEL F _{cl} psi | DURATION C _D | REPETITIVE C _r | SIZE FACT. C _F | M.O.E E ksi | PERPENDICULAR TO GRAIN F _{cp} psi C _b | | WIDTH b(d) inch | THICK t inch | E _{min} ksi | F _c psi | l _v /d | F _{ce} = (0.822*E _{min})/(l _v /d) ² | COLUMN FACTOR C _p | COL. STRESS F' _c psi | VERTICAL CAPACITY P _v lbs | BEARING CAPACITY P _b lbs | ALLOW. P _c lbs | Size |
| 2 x 4 | No. 2 | 107 | 1,350 | 1.60 | 1.00 | 1.15 | 1,600 | 625 | 1.00 | 3.50 | 1.50 | 584 | 2,484 | 30.70 | 509.68 | 0.196 | 486.03 | 2,552 | 3,281 | 2,552 | 2 x 4 |
| 3 x 4 | No. 2 | 107 | 1,350 | 1.60 | 1.00 | 1.15 | 1,600 | 625 | 1.00 | 3.50 | 2.50 | 584 | 2,484 | 30.70 | 509.68 | 0.196 | 486.03 | 4,253 | 5,469 | 4,253 | 3 x 4 |
| 4 x 4 | No. 1 | 107 | 1,500 | 1.60 | 1.00 | 1.15 | 1,700 | 625 | 1.00 | 3.50 | 3.50 | 621 | 2,760 | 30.70 | 541.53 | 0.188 | 517.63 | 6,341 | 7,656 | 6,341 | 4 x 4 |
| 4 x 6 | No. 1 | 107 | 1,500 | 1.60 | 1.00 | 1.10 | 1,700 | 625 | 1.00 | 3.50 | 5.50 | 621 | 2,640 | 30.70 | 541.53 | 0.196 | 516.41 | 9,941 | 12,031 | 9,941 | 4 x 6 |
| 4 x 8 | No. 1 | 107 | 1,500 | 1.60 | 1.00 | 1.05 | 1,700 | 625 | 1.00 | 3.50 | 7.25 | 621 | 2,520 | 30.70 | 541.53 | 0.204 | 515.07 | 13,070 | 15,859 | 13,070 | 4 x 8 |
| 4 x 10 | No. 1 | 107 | 1,500 | 1.60 | 1.00 | 1.00 | 1,700 | 625 | 1.00 | 3.50 | 9.25 | 621 | 2,400 | 30.70 | 541.53 | 0.214 | 513.57 | 16,627 | 20,234 | 16,627 | 4 x 10 |
| 4 x 12 | No. 1 | 107 | 1,500 | 1.60 | 1.00 | 1.00 | 1,700 | 625 | 1.00 | 3.50 | 11.25 | 621 | 2,400 | 30.70 | 541.53 | 0.214 | 513.57 | 20,222 | 24,609 | 20,222 | 4 x 12 |
| 2 x 6 | No. 2 | 107 | 1,350 | 1.60 | 1.00 | 1.00 | 1,600 | 625 | 1.00 | 5.50 | 1.50 | 584 | 2,160 | 19.54 | 1258.59 | 0.489 | 1056.36 | 8,715 | 5,156 | 5,156 | 2 x 6 |
| 3 x 6 | No. 2 | 107 | 1,350 | 1.60 | 1.00 | 1.05 | 1,600 | 625 | 1.00 | 5.50 | 2.50 | 584 | 2,268 | 19.54 | 1258.59 | 0.471 | 1068.32 | 14,689 | 8,594 | 8,594 | 3 x 6 |
| 6 x 4 | No. 1 | 107 | 1,500 | 1.60 | 1.00 | 1.10 | 1,700 | 625 | 1.00 | 5.50 | 3.50 | 621 | 2,640 | 19.54 | 1337.25 | 0.438 | 1156.80 | 22,268 | 12,031 | 12,031 | 6 x 4 |
| 6 x 6 | No. 1 | 107 | 1,000 | 1.60 | 1.00 | 1.10 | 1,600 | 625 | 1.00 | 5.50 | 5.50 | 584 | 1,760 | 19.54 | 1258.59 | 0.567 | 997.56 | 30,176 | 18,906 | 18,906 | 6 x 6 |
| 6 x 8 | No. 1 | 107 | 1,000 | 1.60 | 1.00 | 1.10 | 1,600 | 625 | 1.00 | 5.50 | 7.25 | 584 | 1,760 | 19.54 | 1258.59 | 0.567 | 997.56 | 39,778 | 24,922 | 24,922 | 6 x 8 |
| 6 x 10 | No. 1 | 107 | 1,000 | 1.60 | 1.00 | 1.10 | 1,600 | 625 | 1.00 | 5.50 | 9.25 | 584 | 1,760 | 19.54 | 1258.59 | 0.567 | 997.56 | 50,751 | 31,797 | 31,797 | 6 x 10 |
| 2 x 8 | No. 2 | 107 | 1,350 | 1.60 | 1.00 | 1.15 | 1,600 | 625 | 1.00 | 7.25 | 1.50 | 584 | 2,484 | 14.82 | 2186.93 | 0.645 | 1603.25 | 17,435 | 6,797 | 6,797 | 2 x 8 |
| 2 x 4 | No. 2 | 71 | 1,350 | 1.60 | 1.00 | 1.15 | 1,600 | 625 | 1.00 | 3.50 | 1.50 | 584 | 2,484 | 20.14 | 1184.16 | 0.417 | 1035.94 | 5,439 | 3,281 | 3,281 | 2 x 4 |
| 2 x 6 | No. 2 | 71 | 1,350 | 1.60 | 1.00 | 1.15 | 1,600 | 625 | 1.00 | 5.50 | 1.50 | 584 | 2,484 | 12.82 | 2924.15 | 0.744 | 1848.57 | 15,251 | 5,156 | 5,156 | 2 x 6 |

COLUMN NOTES

| | |
|-------|---|
| A | Nominal Member Size |
| B | Lumber Grade |
| C | Nominal Post Height - 4.5 inches for double 2x top plate and single 2x bottom plate. |
| D - J | Coefficients and parameters per NDS 2015 |
| K | Lumber depth (3.5" in 4" wall, 5.5" in 6" wall) |
| L | Lumber thickness parallel to wall direction |
| M | E _{min} = (2.03)(E)(2-2.645(CoVE))/2.66, CoVE = 0.25 for sawn lumber |
| N | F* _c = (F _{cl})(C _d)(C _r)(C _F) - (Columns D, E, F, G) |
| O | Slenderness Ratio: Effective Length / depth ratio: (Col. C / Col. K) |
| P | E _{min} = Euler critical buckling stress for columns F _{ce} = 08.22(E _{min}) ² /(l _v /d) ² . Equation H-2 in NDS 2015. |
| Q | Column Stability Factor C _p , Equation 3.7-2 in NDS 2015 |
| R | Column Stress F' _c = F* _c x C _p (Col. N x Col. Q). Equation H-2 in NDS 2015 |
| S | Final Buckling Capacity: Post Area x F' _c (Col. K x Col. L x Col. R) |
| T | Maximum Bearing Capacity Perpendicular to Grain Code Limitation: (Col. I x Col. K x Col. L). |
| U | Final Post Capacity, minimum value of Col. S or Col. T. |
| V | Nominal Member Size (Same as Col. A) |

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

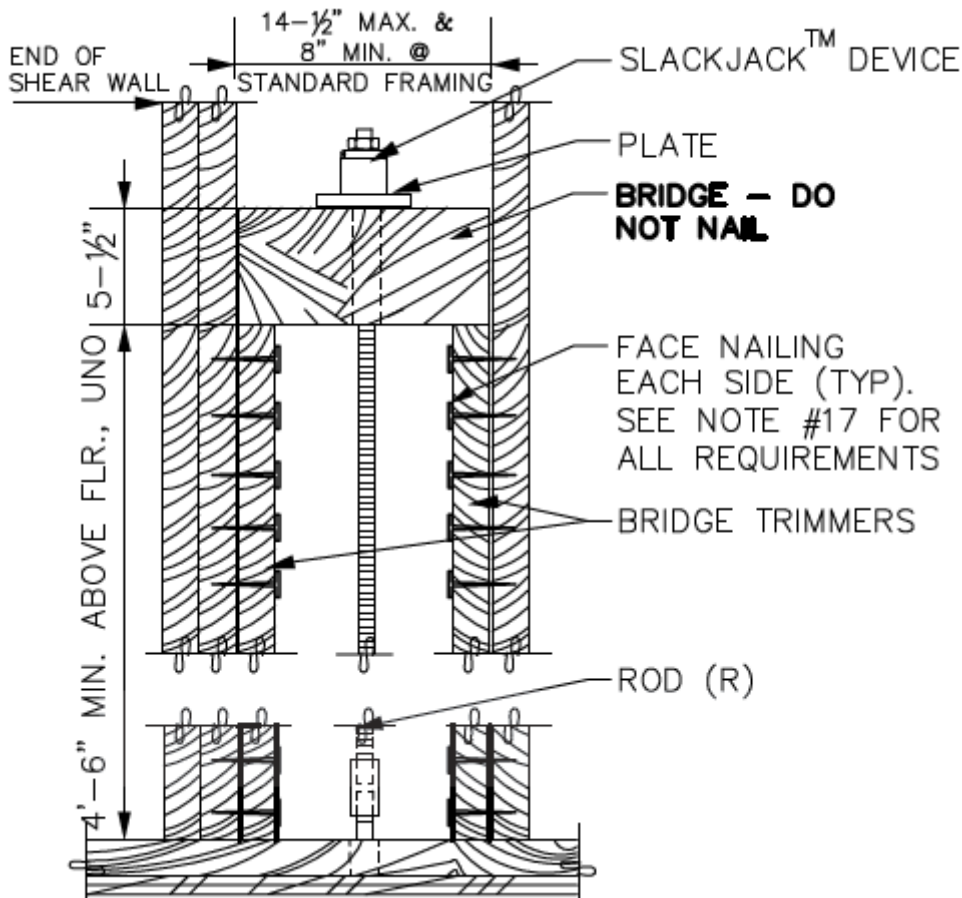
Earthbound System Compression Bridge and Support Trimmer Calculation

Summary of Compression Bridge Concept:

The "Compression Bridge" consists of 4x6 sawn lumber turned upright (or 6x6) and framed in the stud bay at each end of a shear wall. These bridges are utilized at the top floor of every holdown run in the Earthbound System. The 4x6 is framed in the wall similar to a header where it is supported by cripple studs. Depending on load, the minimum number of cripple studs is two, one at each end of the 4x6 bridge.

If the bearing load exceeds the maximum bearing pressure of the cripple studs perpendicular to grain of the floor plate, more studs can be added under the bridge. The outermost cripple studs are faced nailed two (2) nails on 4" o.c. to the adjacent full height stud members. The following example calculation will be based on 4'-6" tall cripple studs in performing the face nail calculation. The Earthbound System also recommends these cripple studs receive nailing for redundancy from the sheathing side equivalent to required plywood edge nailing schedules for the shear wall. For this calculation, we will use 4" o.c. plywood edge nailing.

The Slackjack or MiniJack Device is mounted at the top of the bridge resting on the Earthbound System bearing plates. The holdown load path is intended to transfer down via the bearing plate through the 4x6 bridge to the cripple studs with load transferring from the face nails into the full length studs.



Objective: Calculate lumber and nailing capacities to support holdown load under the Shrinkage Device and bearing plate. We are using face nailing and diaphragm nailing to transfer holdown load back into wall:

Define Variables and Coefficients:

From Table 10.3.1 2015 NDS (attached at end of calculation), the following are Adjustments Factors for Corrections applicable in calculating nail and spike capacities (under Dowel-Type Fasteners)

$C_D := 1.33$;Load Duration Factor = 1.33 for seismic $C_{D_Nails} := 1.60$;Load Duration Factor = 1.6 for Nails in 2015 NDS.
 $C_M := 1.0$;Wet Service Factor = 1.0 if dry conditions
 $C_t := 1.0$;Temperature Factor = 1.0 if below 100 degrees
 $C_{\Delta} := 1.0$;Geometry Factor = 1.0 unless you have a split ring or shear plate connection.
 $C_{eg} := 1.0$;End Grain Factor = 1.0 if non end grain nailing application
 $C_{di} := 1.0$;Diaphragm Factor = 1.0 if non diaphragm nailing application. 1.1 if diaphragm nailing
 $C_{tn} := 1.0$;Toenail Factor = 1.0 if non toe nailing application
 $Z := 93\text{ lbf}$;Box Nail Shear Design values from supplemental calculation from WWPA Wood-To-Wood Single Shear Connections is at the end of this document. WWPA = 148 lbs / 1.6 (Cd) = 93 lbf

Calculate Shear Value of one FACE nail based on above adjustment factors:

$$Z_{nail} := Z \times C_{D_Nails} \times C_M \times C_t \times C_{\Delta} \times C_{eg} \times C_{di} \times C_{tn} \quad Z_{nail} = 148.8 \text{ lbf}$$

Calculate Face Nailing Capacity based on two (2) nails on 4" o.c. rows and 4'-6" trimmer stud length:

$$\text{StudHeight} := 4.5\text{ft}$$

;Typically Trimmer Stud Height is set at mid floor or 1/2 full length studs used for wall framing

$$\text{NailRowSeperationOC} := 4.0\text{in}$$

$$\text{NumberNailRows} := \frac{\lceil \text{StudHeight} \times 12 \frac{\text{in}}{\text{ft}} \rceil}{\lceil \text{NailRowSeperationOC} \rceil} - 1$$

;We subtract one row of nails for spacing and clearance to the bridge.

$$\text{NumberNailRows} = 13$$

$$\text{NumberNailsPerRow} := 2$$

$$\text{NumberofRowArrays} := 2$$

$$\text{TotalFaceNailCapacity} := \text{NumberNailRows} \times \text{NumberNailsPerRow} \times \text{NumberofRowArrays} \times Z_{nail}$$

$$\text{TotalFaceNailCapacity} = 7440 \text{ lbf} \quad ;\text{This is (number of rows) x (2 nails per row) x (2 rows, one each side) x (nail capacity)}$$

Nailing Capacity Summary: The above calculation is based on the cripple stud height used and the spacing between rows of nails. Note that the Earthbound System does recommend sheathing nailing to match plywood edge nailing schedules. The additional transfer from shear panel nails is calculated later in this document.

Calculate Cripple Stud Bearing Capacity: This is a calculation of the maximum capacity per stud bearing perpendicular to grain on the bottom plates:

$$l_e := \text{StudHeight} \times \frac{12\text{in}}{1\text{ft}} \quad l_e = 54\text{in} \quad ;\text{Stud Height in inches}$$

$$d := 3.5\text{in} \quad ;\text{Stud Depth}$$

$$t := 1.5\text{in} \quad ;\text{Stud Thickness}$$

$$A := t \times d \quad ;\text{Stud Bearing Area}$$

$$A = 5.25\text{in}^2 \quad ;\text{Note the effective stud bearing area for a 2x6 cripple stud will be based on a 2x4 (1.5 in x 3.5 in) because we specify a 4x6 bridge turned upwards, the effective stud bearing area will be the same for a 2x4 or 2x6.}$$

$$F_{c_{\text{perp}}} := 625\text{psi} \quad ;\text{From 2015 NDS Table 4A Douglas Fir Stud Grade} = 625\text{ psi}$$

$$\text{StudBearingCapacity} := A \times F_{c_{\text{perp}}}$$

$$\text{StudBearingCapacity} = 3281\text{ lbf} \quad ;\text{NDS does not allow for Load Duration Factor greater than 1.0 to be applied to loads perpendicular to grain.}$$

Calculate Trimmer Stud Vertical (Column Buckling) Capacity: This is a calculation of the maximum buckling or column capacity per stud. Calculations are based from 2015 NDS:

$$F_{c_{\text{para}}} := 850\text{psi} \quad ;F_c (\text{parallel}): \text{ Douglas Fir (stud grade)} = 850\text{ psi}$$

$$C_D := 1.333 \quad ;\text{Load Duration Factor} = 1.33 \text{ for seismic}$$

$$C_T := 1.00 \quad ;\text{Repetitive Member Factor} = 1.00$$

$$C_F := 1.05 \quad ;\text{Size Factor} = 1.05, \text{ From 2015 NDS Table 4A for 2,3,4 inch width Stud Grade}$$

$$C_M := 1.0 \quad ;\text{Wet Service Factor} = 1.0, \text{ From 2015 NDS Table 4A, assume lumber is below 19\% moisture content}$$

$$E := 1400000\text{psi} \quad ;\text{Modulus of Elasticity of wood, From 2015 NDS Table 4A: Douglas Fir Stud. Grade} = 1,400,000\text{ psi}$$

$$c := 0.80 \quad ;c = 0.8 \text{ for sawn lumber}$$

$$\text{Adjusted_}F_{c_{\text{para}}} := F_{c_{\text{para}}} \times C_D \times C_F \times C_M \quad ;\text{Adjusted_}F_c(\text{para}) \text{ is also known as } F^*c \text{ from NDS 2015 reference. Mathcad cannot show the asterisk in "F*c".}$$

$$\text{Adjusted_}F_{c_{\text{para}}} = 1189.7\text{psi}$$

$$\text{CoV}_E := 0.25 \quad ;\text{Coefficient of variation of E, CoV(E)} = 0.25 \text{ for sawn lumber, 0.10 for glue lam lumber per NDS 2015 Appendix F.2.}$$

$$E_{min} := 1.03 \times E \times \frac{(1 - 1.645 \times CoV_E)}{1.66}$$

$$E_{min} = 511432.2 \text{ psi}$$

;Equation D-4 in NDS 2015. Determine reference modulus of elasticity where:

E = Modulus of Elasticity

1.03 = adjustment factor to convert E to pure bending basis.

Use 1.05 for structural glued laminated timber.

1.66 = factor of safety

CoV(E) = coefficient of variation in modulus of elasticity defined in NDS 2015 Appendix F.2.

Emin represents an approx 5% lower exclusion value on pure bending MOE, plus a factor of safety of 1.66.

$$F_{cE} := \frac{(0.822 \times E_{min})}{\left(\frac{L}{r}\right)^2} \quad F_{cE} = 1766 \text{ psi}$$

;Equation H-2 in NDS 2015: The 0.822 factor in Equation H-2 represents the Euler buckling coefficient for rectangular columns calculated as p²/12. MOE for beam and column stability Emin in H-2 represents an approximate 5% lower exclusion value on pure bending MOE, plus a 1.66 factor of safety (See NDS 2015 D.4.)

We effectively removed the 1.66 factor of safety from Equation H-2 by dividing Emin by 1.66 used in Equation D-4 above.

The maximum design value Fc in psi of cross-sectional area of individual members of columns shall be determined in accordance with the following formula. (Taken from 2015 NDS Equation 3.7-1).

$$F_c := \text{Adjusted_}F_{c\text{para}} \times \frac{\left(\frac{E_{min}}{E} + \frac{F_{cE}}{\text{Adjusted_}F_{c\text{para}}} \right)}{2} - \sqrt{\left(\frac{E_{min}}{E} + \frac{F_{cE}}{\text{Adjusted_}F_{c\text{para}}} \right)^2 - \frac{F_{cE}}{\text{Adjusted_}F_{c\text{para}}}}$$

$$F_c = 960.6 \text{ psi}$$

The vertical capacity of this column if design value Fc multiplied by the bearing area of the stud, in this case a 2x4:

$$d = 3.5 \text{ in} \quad t = 1.5 \text{ in}$$

$$\text{StudVerticalCapacity} := d \times F_c$$

$$\text{StudVerticalCapacity} = 5043 \text{ lbf}$$

$$\text{StudBearingCapacity} = 3281 \text{ lbf}$$

;From above bearing calculations, based on DF at Fc(perp)=625 psi.

The allowable stud capacity should be the lower value between Vertical Capacity and Bearing Capacity:

$$\text{AllowStudCapacity} := \text{if}(\text{StudBearingCapacity} > \text{StudVerticalCapacity}, \text{StudVerticalCapacity}, \text{StudBearingCapacity})$$

;The above formula is a conditional statement inherent to Mathcad to use the lower capacity as the final value.

$$\text{AllowStudCapacity} = 3281 \text{ lbf}$$

Calculate Shear Value of 10d nail assuming 1/2" Plywood:

$Z := 70 \text{ lbf}$;Box Nail Shear Design values from 2015 NDS Table 11Q (attached at end of calc) using 1/2" side member thickness 10d nails (x 3 inches nail length) and Douglas Fir Larch lumber.

$C_{di} := 1.1$;Cdi is 1.1 for diaphragm nailing.

$Z_{nail} := Z \times C_{D_Nails} \times C_M \times C_t \times C_{\Delta} \times C_{eg} \times C_{di} \times C_{tn}$;Incorporate all adjustment factors including C(D_Nails) = 1.60

$Z_{nail} = 123.2 \text{ lbf}$

Calculate Plywood Side Nailing Capacity based on 4" o.c. and 6'-0" trimmer stud length:

$\text{StudHeight} := 4.5 \text{ ft}$

$\text{NailRowSeperationOC} := 4.0 \text{ in}$

$$\text{NumberNailsPerStud} := \frac{\text{StudHeight} \times 2 \frac{\text{in}}{\text{ft}}}{\text{NailRowSeperationOC}} - 1$$

$\text{NumberNailsPerStud} = 12.5$

$\text{NumberStuds} := 4$

$\text{TotalPlywoodNailCapacity} := \text{NumberNailsPerStud} \times \text{NumberStuds} \times Z_{nail}$

$\text{TotalPlywoodNailCapacity} = 6160 \text{ lbf}$

$\text{TotalNailCapacity} := \text{TotalPlywoodNailCapacity} + \text{TotalFaceNailCapacity}$; See Page 2 for Face Nail Calc...

$\text{TotalNailCapacity} = 13600 \text{ lbf}$

Nailing Capacity Summary: The above calculation is based on the trimmer stud height used and the spacing between rows of nails. Note that the Earthbound System does require 4" o.c. sheathing nailing on all trimmer members and 4" o.c. face nailing of the outermost trimmer to the full height stud members.

Calculation Summary:

For a given bearing load underneath the take up device, the required amount of cripple studs under the bridge is an even number of multiples of the above Allowable Stud Capacity. We use even numbers to balance the supporting under the 4x6 or 6x6 bridge.

This bearing load may be greater than the actual uplift load required for the top floor due to assignment of loads upward in the building using the threaded rod system. The face nailing portion of the cripple studs need to exist to transfer shear load that occurs above the bridge back underneath the bridge and into the threaded rod. The loads required for panel nailing calculations will be the uplift load for that floor only. The additional studs required are to take up the bearing loads from loads transferred from below.

Examine Shear at Bridge:

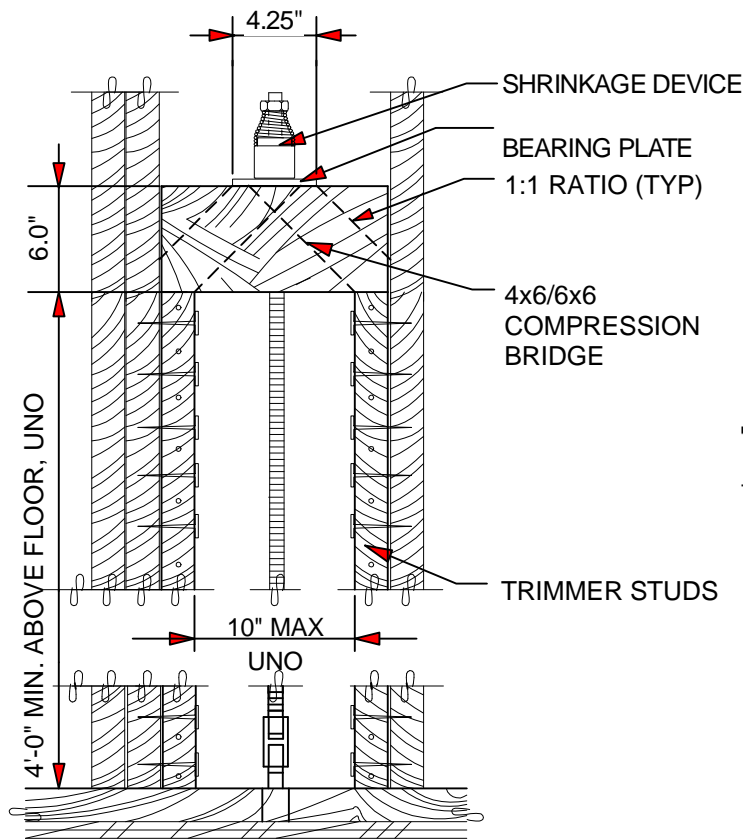
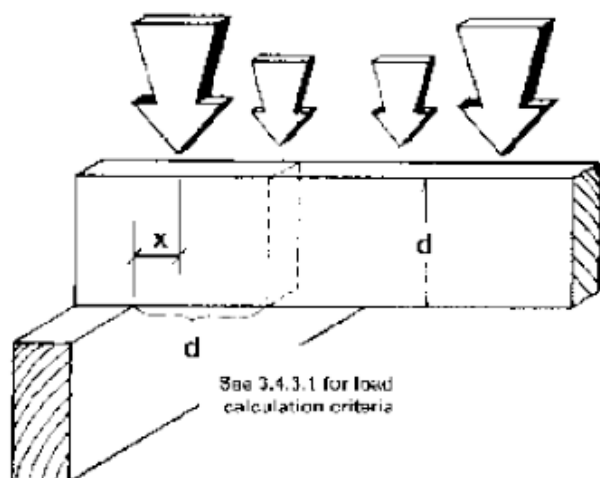
The figure at the left shows a worst case scenario of a bridge trimmer with our smallest plate (Part "P8" 3 1/4 x 4 1/4 x 3/8) and only two (2) trimmers total. By inspection we are within a 1:1 requirement per 2015 NDS 3.4.3 to ignore shear force. If the load under the shrinkage device exceeds 8,000 lbs, a larger bearing plate will be used. Depending on the lumber species (Hem vs Doug Fir) of the trimmers and floor plates, we would also increase the number of trimmer members under the bridge.

3.4.3 Shear Design

3.4.3.1 When calculating the shear force, V , in bending members:

- (a) For beams supported by full bearing on one surface and loads applied to the opposite surface, uniformly distributed loads within a distance from supports equal to the depth of the bending member, d , shall be permitted to be ignored. For beams supported by full bearing on one surface and loads applied to the opposite surface, concentrated loads within a distance, d , from supports shall be permitted to be multiplied by x/d where x is the distance from the beam support face to the load (see Figure 3C).

Figure 3C Shear at Supports



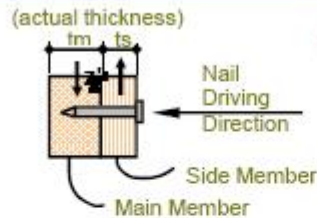


Wood-to-Wood Single Shear Connections

www.pe.org

Version: 2.0

Designed on: August 20, 2008



Developed by:
Forum Engineers



Main Member Species **Douglas Fir-Larch**
 Main Member Thickness (tm) **1 1/2**
 Side Member Species **Douglas Fir-Larch**
 Side Member Thickness (ts) **1 1/2**
 Loading Type **Seismic Load**
 End Grain Condition? **No**

Type of Connector

- Box Nail**
- Common Wire Nail
- Sinker Nail
- Bolt
- Wood Screw
- Lag Screw

| Connector Size | Length (in) | Shear Capacity | Controlling Mode |
|----------------|-------------|----------------|------------------|
| 10d x | 3 | = 148 lb each | MODE IV |



Open Detailed Calculation Sheet

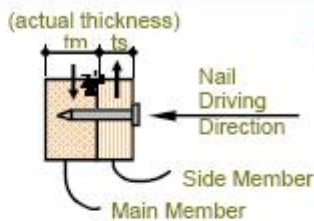


Wood-to-Wood Single Shear Connections

www.pe.org

Version: 2.0

Designed on: August 20, 2008



Developed by:
Forum Engineers



Main Member Species **Douglas Fir-Larch**
 Main Member Thickness (tm) **3 1/2**
 Side Member Species **Plywood (Str. 1 or Marine Gr.)**
 Side Member Thickness (ts) **7/16**
 Loading Type **Seismic Load**
 End Grain Condition? **No**

Type of Connector

- Box Nail**
- Common Wire Nail
- Sinker Nail
- Bolt
- Wood Screw
- Lag Screw

| Connector Size | Length (in) | Shear Capacity | Controlling Mode |
|----------------|-------------|----------------|------------------|
| 10d x | 3 | = 112 lb each | MODE IIIs |



Open Detailed Calculation Sheet

**NOTE: THIS CALCULATION MUST INCLUDE Cdl = 1.1 FOR DIAPHRAGM FACTOR.
 THEREFORE 112 LBS x 1.1 = 123 lbs PER SHEAR NAIL**

**WESTMAN MILL
EAST BAY LOT A
ROOF TRUSS LAYOUT**

ROOF TRUSS SUPPLY - (425) 481-0900
5910 - 234 ST. SE, WOODINVILLE, WA 98072
DESIGNER: DENNIS MOORE REVISED: 05/02/19
PATH: S:\DENNIS\LAY\W\WESTMAN\BLOB_1S

- 1) ALL TRUSSES TO BE SPACED 24" OC UNLESS NOTED OTHERWISE (U.N.O.)
- 2) PROVIDE FULL BEARING UNDER GIRDER TRUSSES.
- 3) BEARING BEAMS DESIGNED BY OTHERS.
- 4) 2X6 PARAPETS AS NOTED.
- 5) .125/12 (1/8") PITCH U.N.O..

Checking by PCS is only for performance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

Conforms to Design Concept
 Conforms to Design Concept as noted
 Does Not Conform - Revise & Resubmit

Checking is only for conformance with the design concept of the project. Contractor is responsible for dimensions to be confirmed and correlated at the job site; for information that pertains solely to the fabrication processes or to techniques of construction; and for the coordination of the work of all trades.

By: **SJW** Date: **07/17/2019**

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rbalders@ci.olympia.wa.us

CM 7/12/19-
TAS/PCS advise if any additional structural provisions are required for HVAC equipment at locations shown per A2.()

SHOP DRAWING / SUBMITTAL REVIEW

APPROVED APPROVED WITH CHANGES NOTED
 REVISE AND RESUBMIT REJECTED

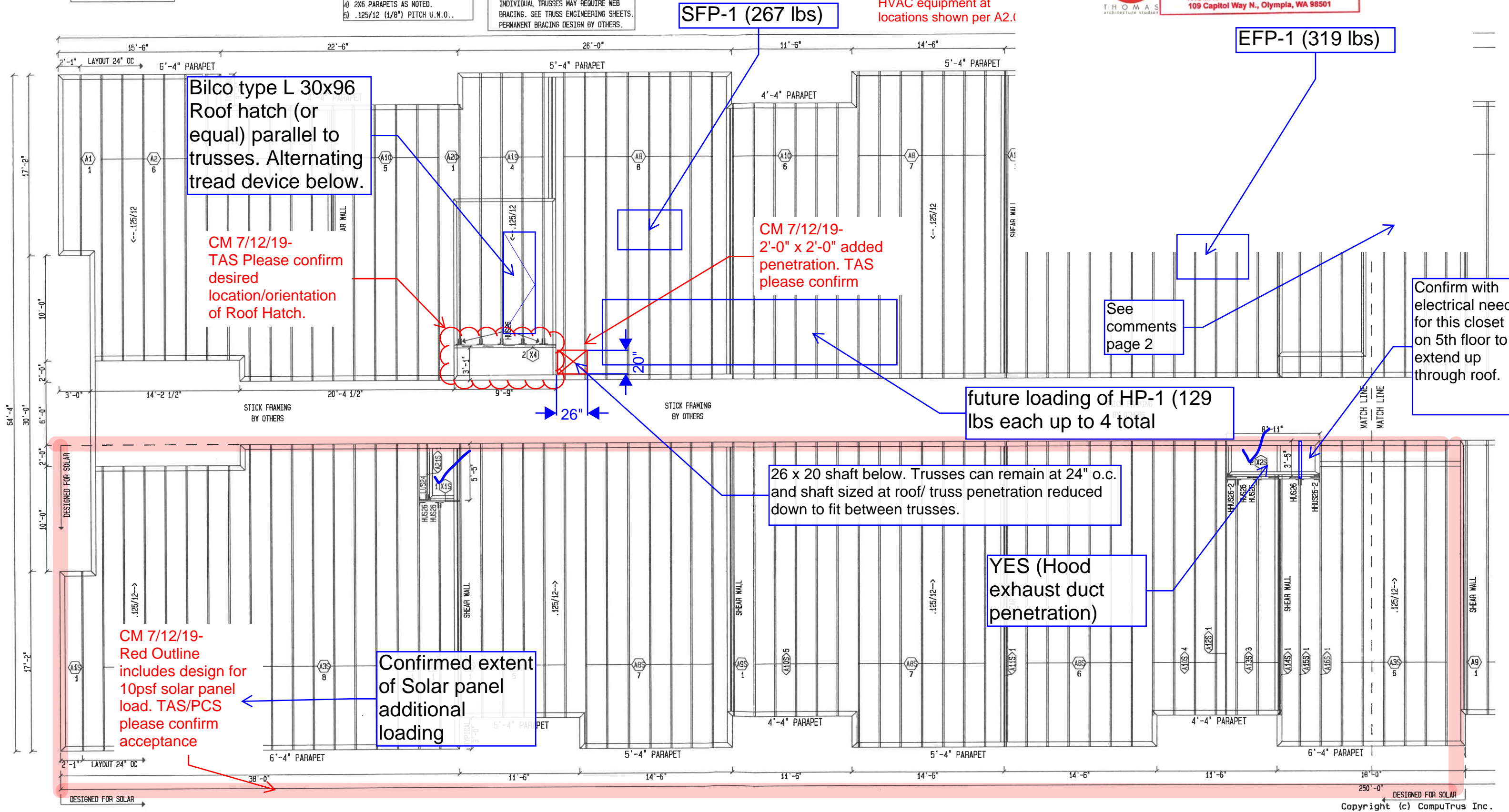
SUBMITTAL WAS REVIEWED FOR DESIGN CONFORMITY AND GENERAL CONFORMANCE TO CONTRACT DOCUMENTS ONLY. THE CONTRACTOR IS RESPONSIBLE FOR CONFIRMING AND CORRELATING DIMENSIONS AT JOB SITES FOR TOLERANCES, CLEARANCES, QUANTITIES, FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATION OF HIS WORK WITH OTHER TRADES AND FULL COMPLIANCE WITH THE CONTRACT DOCUMENTS.

BY: *[Signature]* DATE: **07/26/19**

THOMAS ARCHITECTURE STUDIO, INC.
109 Capitol Way N., Olympia, WA 98501



Refer to Mechanical M002 and M206 for rooftop equipment size and location





Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

Conforms to Design Concept
 Conforms to Design Concept as noted
 Does Not Conform - Revise & Resubmit
 Checking is only for conformance with the design concept of the project. Contractor is responsible for dimensions to be confirmed and correlated at the job site; for information that pertains solely to the fabrication processes or to techniques of construction; and for the coordination of the work of all trades.
 By SJW Date 07/17/2019
 PCS Structural Solutions
 101 SW Main Street, Suite 280
 Portland, OR 97204 503-232-3746

CONFIRMED - 2X8 FRAMING @ 24" O.C. PROVIDE DOUBLE 2X8'S AROUND THE MECHANICAL UNIT

22"x30" mechanical exhaust duct between 2x8's

Mechanical unit not on 2x8 roof. See page 1 for EPF-1 comments

SFP-1 (267 lbs)

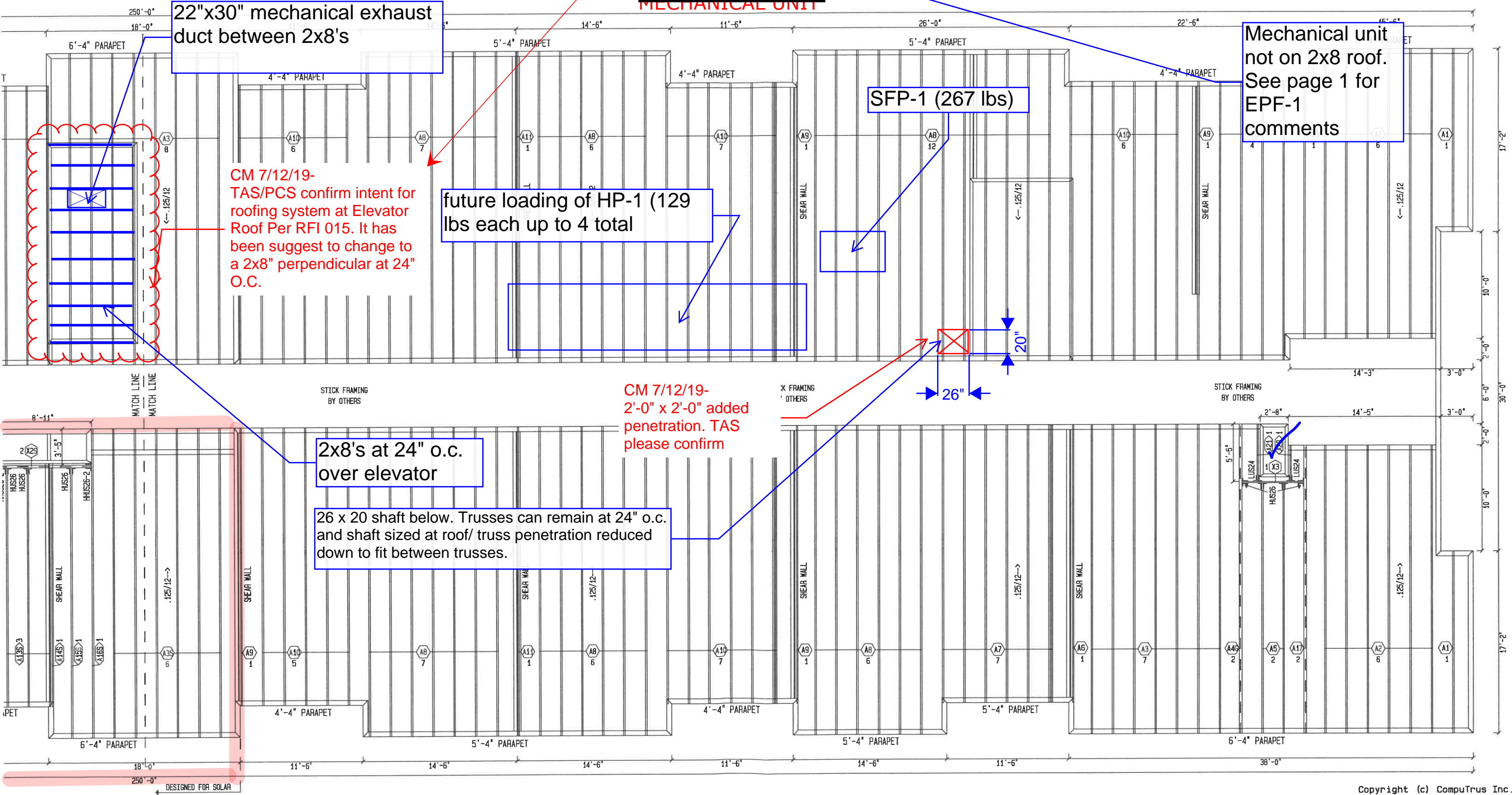
future loading of HP-1 (129 lbs each up to 4 total)

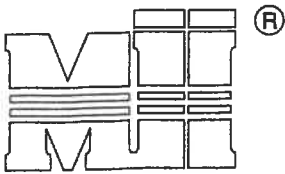
CM 7/12/19-TAS/PCS confirm intent for roofing system at Elevator Roof Per RFI 015. It has been suggest to change to a 2x8" perpendicular at 24" O.C.

CM 7/12/19-2'-0" x 2'-0" added penetration. TAS please confirm

2x8's at 24" o.c. over elevator

26 x 20 shaft below. Trusses can remain at 24" o.c. and shaft sized at roof/ truss penetration reduced down to fit between trusses.





MiTek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

| Nailing Pattern | | |
|-------------------|-----------|--------------|
| T-Brace size | Nail Size | Nail Spacing |
| 2x4 or 2x6 or 2x8 | 10d | 6" o.c. |

Note: Nail along entire length of T-Brace / I-Brace
(On Two-Ply's Nail to Both Plies)

| Brace Size for One-Ply Truss | | |
|--|---|---|
| Specified Continuous Rows of Lateral Bracing | | |
| Web Size | 1 | 2 |

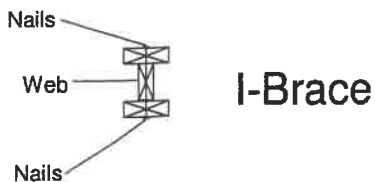
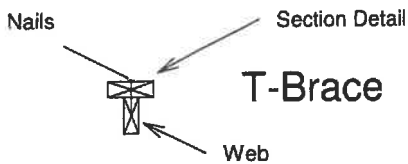
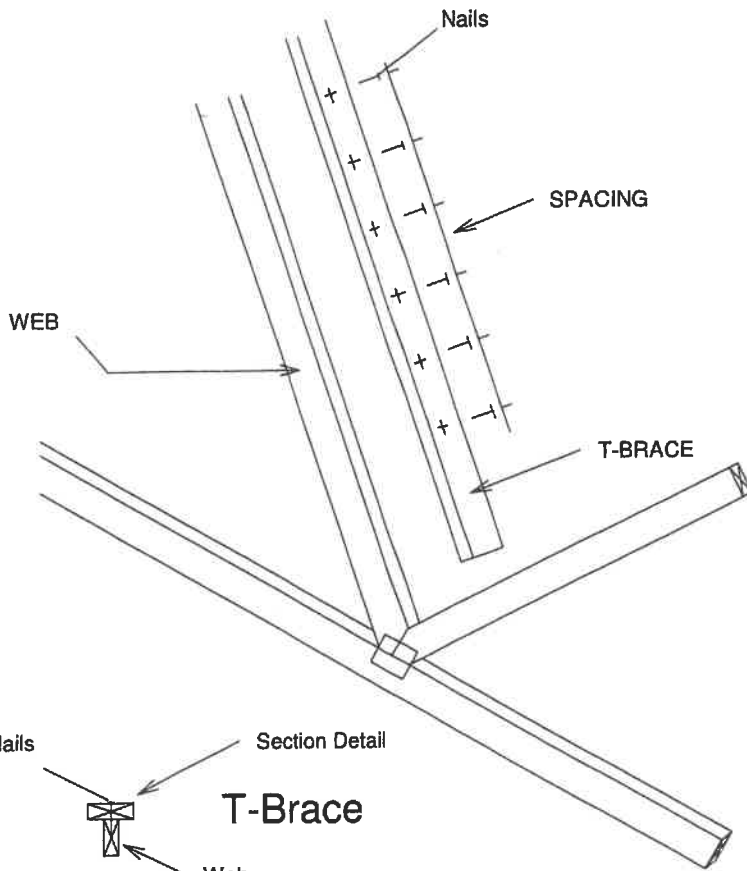
| | | |
|------------|-------------|-------------|
| 2x3 or 2x4 | 2x4 T-Brace | 2x4 I-Brace |
| 2x6 | 2x6 T-Brace | 2x6 I-Brace |
| 2x8 | 2x8 T-Brace | 2x8 I-Brace |

| Brace Size for Two-Ply Truss | | |
|--|---|---|
| Specified Continuous Rows of Lateral Bracing | | |
| Web Size | 1 | 2 |

| | | |
|------------|-------------|-------------|
| 2x3 or 2x4 | 2x4 T-Brace | 2x4 I-Brace |
| 2x6 | 2x6 T-Brace | 2x6 I-Brace |
| 2x8 | 2x8 T-Brace | 2x8 I-Brace |

T-Brace / I-Brace must be same species and grade (or better) as web member.

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x6 HF #2 A

TRUSS SPAN 17'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" o.c.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|----------|------------|-------------------|-----------|-----------------|-----------|
| 1- 2= | (-41) 33 | 6- 7= | (-36) 831 | 1- 5= | (0) 0 |
| 2- 3= | (-878) 23 | 7- 8= | (0) 1682 | 1- 6= | (0) 157 |
| 3- 4= | (-1633) 17 | 8-10= | (-24) 91 | 6- 2= | (-1464) 0 |
| | | | | 2- 7= | (0) 519 |
| | | | | 7- 3= | (-1001) 0 |
| | | | | 3- 8= | (-529) 87 |
| | | | | 4- 4= | (0) 1684 |
| | | | | 10- 4= | (-950) 35 |
| | | | | 4- 9= | (0) 0 |

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. |
|-------------------|--------------------|---------------------|----------|---------------------------|
| 0'- 0.0" | 0/ 1130V | -88/ 93H | 5.50" | 2.79 HF (405) |
| 17'- 2.0" | 0/ 1011V | 0/ 0H | 5.50" | 2.50 HF (405) |

Plateline to peak (L): 17-00-12
Plateline to peak (R): 3-05-15

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 102.29 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.069" @ 9'- 8.7" Allowed = 0.542"
MAX DL DEFL = -0.039" @ 9'- 8.7" Allowed = 0.812"
MAX TL DEFL = -0.108" @ 9'- 8.7" Allowed = 0.812"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

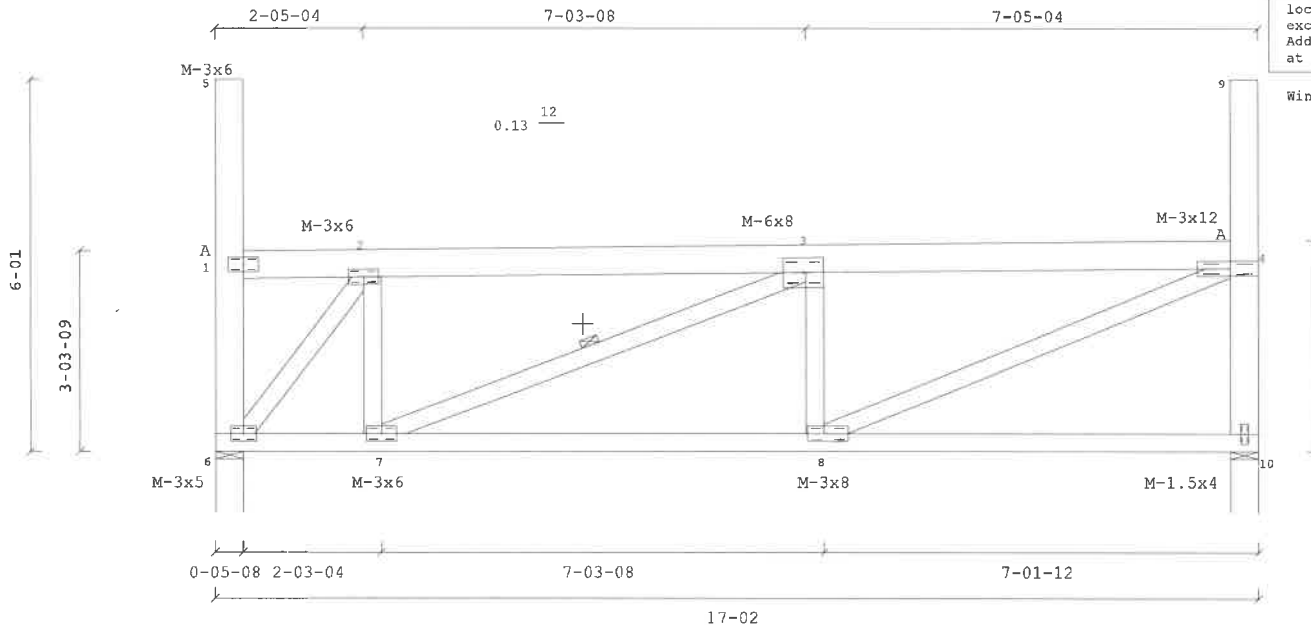
MAX HORIZ. LL DEFL = 0.016" @ 16'- 11.3"
MAX HORIZ. TL DEFL = 0.026" @ 16'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDL=7.2,BCDI=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, End vertical(s) are exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



JOB NAME: WESTMAN MILL - A1

Scale: 0.3310

Truss: A1

DATE: 1/24/2019
SEQ.: K5650383
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPUWTC in BCST, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF #2
HC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x6 HF #2 A;
2x4 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

Plateline to peak (L): 17-00-12
Plateline to peak (R): 3-05-15

TRUSS SPAN 17'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|--------------|----|-------------------|------|-----------------|-----|
| 1- 2=(-43) | 32 | 6- 7=(0) | 950 | 1- 5=(0) | 0 |
| 2- 3=(-1004) | 0 | 7- 8=(0) | 1932 | 1- 6=(0) | 168 |
| 3- 4=(-1875) | 0 | 8-10=(-14) | 109 | 6- 2=(-1675) | 0 |
| | | | | 2- 7=(0) | 574 |
| | | | | 4- 9=(0) | 0 |
| | | | | 7- 3=(-1137) | 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. | (SPECIES) |
|-------------------|--------------------|---------------------|----------|--------------------------|-----------|
| 0'- 0.0" | 0/ 1295V | -88/ 93H | 5.50" | 3.20 | HF (405) |
| 17'- 2.0" | 0/ 1171V | 0/ 0H | 5.50" | 2.89 | HF (405) |

Weight: 102.64 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.068" @ 9'- 8.7" Allowed = 0.542"
MAX DL DEFL = -0.054" @ 9'- 8.7" Allowed = 0.812"
MAX TL DEFL = -0.121" @ 9'- 8.7" Allowed = 0.812"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

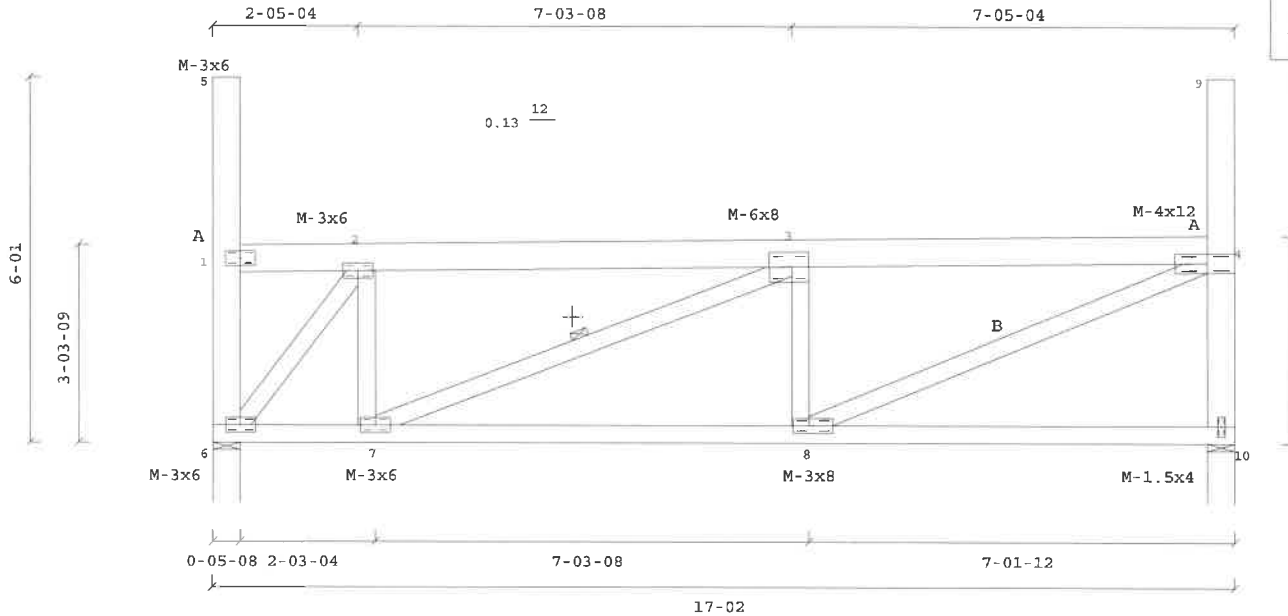
MAX HORIZ. LL DEPL = 0.016" @ 16'- 11.3"
MAX HORIZ. TL DEPL = 0.029" @ 16'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TC DL=13.2, BC DL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS (Dir), load duration factor=1.6, End vertical(s) are exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

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JOB NAME: WESTMAN MILL - A1S

Scale: 0.3243

Truss: A1S
DATE: 5/1/2019
SEQ.: K6059035
TRANS ID: LINK

WARNINGS:
1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TP/MTCA in BCSI, copies of which will be furnished upon request.
MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:
1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 27'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL (30.0)+DL (12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|--------------|---|-------------------|------|-----------------|------|
| 1- 2=(-2484) | 0 | 6- 7=(-45) | 85 | 6- 1=(-1610) | 15 |
| 2- 3=(-3950) | 0 | 7- 8={ 0} | 2609 | 1- 7={ 0} | 2774 |
| 3- 4=(-3950) | 0 | 8- 9={ 0} | 3067 | 7- 2=(-1262) | 75 |
| 4- 5=(-2962) | 1 | 9-11=(-23) | 105 | 2- 8={ 0} | 1589 |
| | | | | 3- 8={ -893} | 80 |
| | | | | 8- 4={ 0} | 1150 |
| | | | | 4- 9={(-1121) | 77 |
| | | | | 9- 5={ 0} | 3124 |
| | | | | 11- 5={(-1536) | 28 |
| | | | | 5-10={ 0} | 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. (SPECTES) |
|-------------------|--------------------|---------------------|----------|-------------------------------------|
| 0'- 0.0" | 0/ 1649V | -61/ 72H | 5.50" | 4.07 HF (405) |
| 27'- 2.0" | 0/ 1598V | 0/ 0H | 5.50" | 3.95 HF (405) |

Weight: 143.80 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.246" @ 12'- 7.0" Allowed = 0.875"
MAX DL DEFL = -0.151" @ 12'- 7.0" Allowed = 1.312"
MAX TL DEFL = -0.398" @ 12'- 7.0" Allowed = 1.312"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

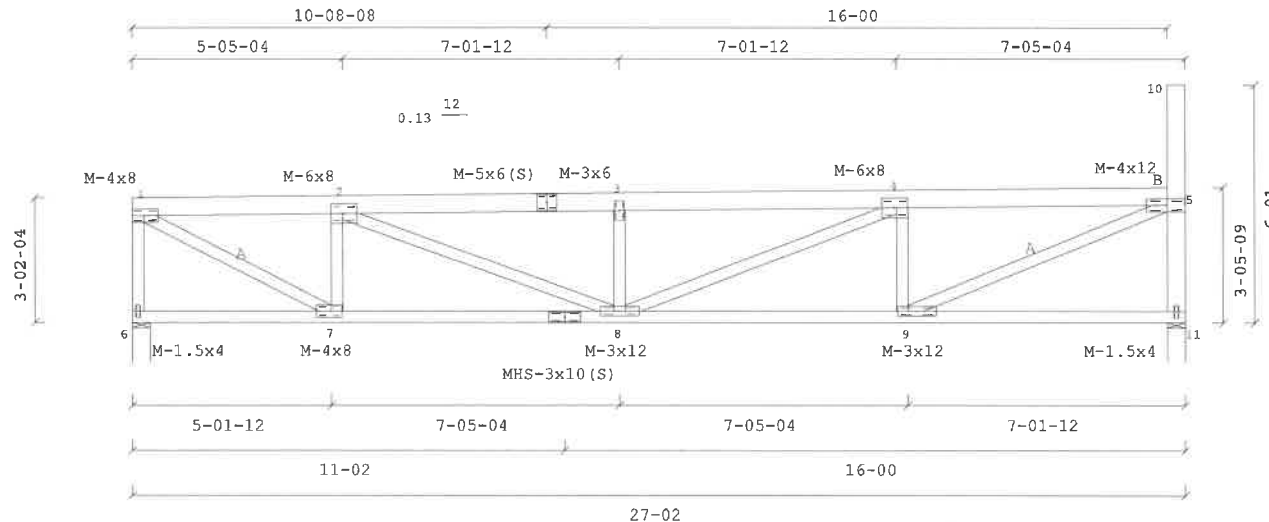
MAX HORIZ. LL DEFL = 0.044" @ 26'- 11.3"
MAX HORIZ. TL DEFL = 0.073" @ 26'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=7.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



JOB NAME: WESTMAN MILL - A2

Scale: 0.2113

Truss: A2

DATE: 1/24/2019
SEQ.: K5650384
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCA in BCS, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES. unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 27'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL (30.0)+DL (22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 |
|----------------|----------------|----------------|-----------------|
| 1- 2=(-2881) 0 | 6- 7=(-38) 99 | 6- 1=(-1881) 0 | 4- 9=(-1322) 0 |
| 2- 3=(-4581) 0 | 7- 8=(0) 3028 | 1- 7=(0) 3217 | 9- 5=(0) 3617 |
| 3- 4=(-4581) 0 | 8- 9=(0) 3559 | 7- 2=(-1486) 0 | 11- 5=(-1799) 0 |
| 4- 5=(-3435) 0 | 9-11=(-12) 127 | 2- 8=(0) 1816 | 5-10=(0) 0 |
| | | 3- 8=(-1037) 0 | |
| | | 8- 4=(0) 1300 | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. | (SPECIES) |
|-------------------|--------------------|---------------------|----------|--------------------------|-----------|
| 0'- 0.0" | 0/ 1920V | -61/ 72H | 5.50" | 4.74 | HF (405) |
| 27'- 2.0" | 0/ 1861V | 0/ 0H | 5.50" | 4.60 | HF (405) |

Weight: 144.90 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.246" @ 12'- 7.0" Allowed = 0.875"
MAX DL DEFL = -0.216" @ 12'- 7.0" Allowed = 1.312"
MAX TL DEFL = -0.462" @ 12'- 7.0" Allowed = 1.312"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.044" @ 26'- 11.3"
MAX HORIZ. TL DEFL = 0.085" @ 26'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

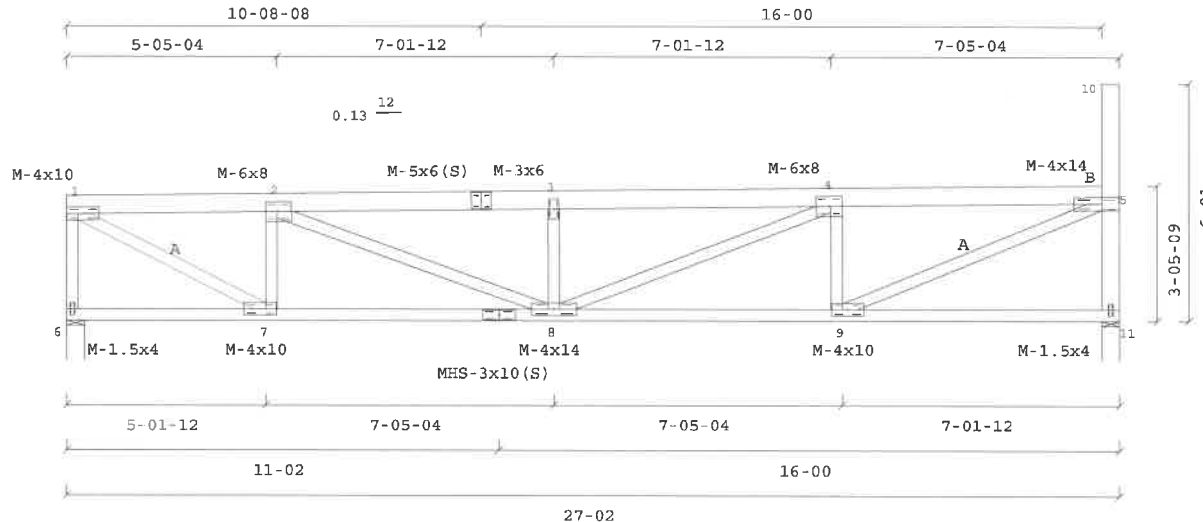
Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDF=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS (Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



May 2, 2019



JOB NAME: WESTMAN MILL - A2S

Scale: 0.2112

Truss: A2S

DATE: 5/1/2019
SEQ.: K6059036
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
 2. 2x4 compression web bracing must be installed where shown +.
 3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 6. This design is furnished subject to the limitations set forth by TPI/WTCA in BC51, copies of which will be furnished upon request.
- MITek USA, Inc./CompuTrus Software 7.6.8(11).E

GENERAL NOTES: unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(SC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 |
|----------------|----------------|-----------------|------------------|
| 1- 2=(-3429) 0 | 6- 7=(-46) 89 | 6- 1=(-1676) 26 | 4- 9=(-1232) 77 |
| 2- 3=(-4501) 0 | 7- 8=(0) 3548 | 1- 7=(0) 3607 | 9- 5=(0) 3397 |
| 3- 4=(-4500) 0 | 8- 9=(0) 3329 | 7- 2=(-1210) 89 | 11- 5=(-1645) 28 |
| 4- 5=(-3213) 0 | 9-11=(-23) 107 | 2- 8=(0) 1215 | 5-10=(0) 0 |
| | | 3- 8=(-874) 76 | |
| | | 8- 4=(0) 1449 | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. (SPECIES) |
|-------------------|--------------------|---------------------|----------|------------------------------------|
| 0'- 0.0" | 0/ 1736V | -61/ 72H | 5.50" | 4.29 HF (405) |
| 29'- 2.0" | 0/ 1708V | 0/ 0H | 5.50" | 4.22 HF (405) |

Weight: 152.56 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.322" @ 14'- 7.0" Allowed = 0.942"
MAX DL DEFL = -0.201" @ 14'- 7.0" Allowed = 1.413"
MAX TL DEFL = -0.523" @ 14'- 7.0" Allowed = 1.413"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

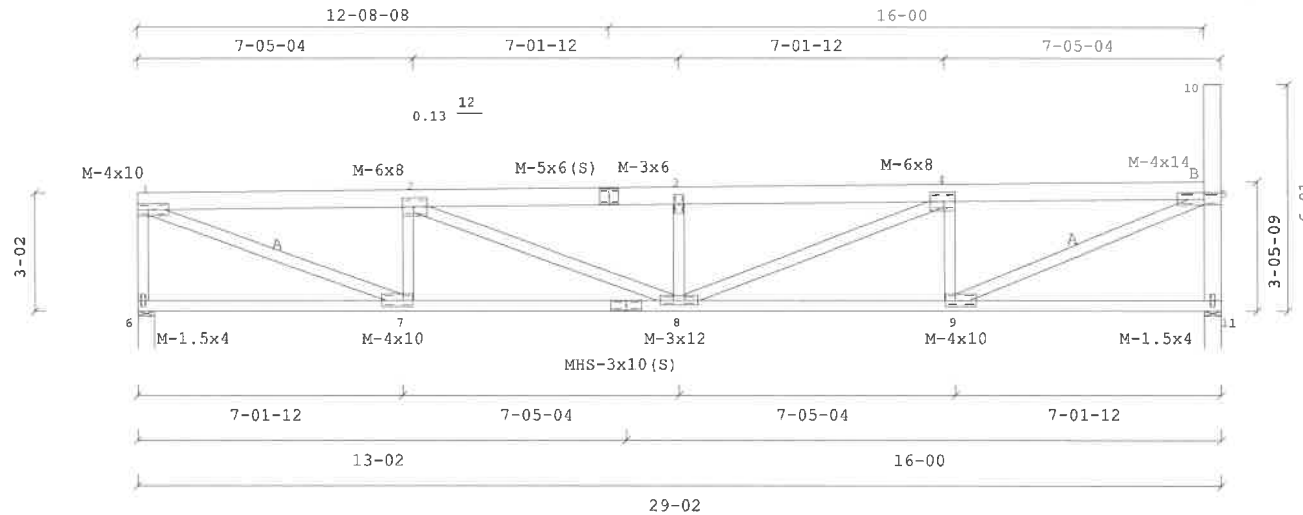
MAX HORIZ. LL DEFL = 0.054" @ 28'- 11.3"
MAX HORIZ. TL DEFL = 0.088" @ 28'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=7.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



JOB NAME: WESTMAN MILL - A3

Scale: 0.2025

Truss: A3

DATE: 1/24/2019
SEQ.: K5650385
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJWTC in BCS1, copies of which will be furnished upon request.

MiTek USA, Inc./CompuTrus Software 7.8.8(1L)-E

GENERAL NOTES: unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of steel in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MiTek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WBBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 |
|----------------|----------------|----------------|-----------------|
| 1- 2=(-3984) 0 | 6- 7=(-36) 104 | 6- 1=(-1967) 0 | 4- 9=(-1454) 0 |
| 2- 3=(-5227) 0 | 7- 8=(0) 4124 | 1- 7=(0) 4189 | 9- 5=(0) 3939 |
| 3- 4=(-5226) 0 | 8- 9=(0) 3869 | 7- 2=(-1432) 0 | 11- 5=(-1928) 0 |
| 4- 5=(-3732) 0 | 9-11=(-11) 129 | 2- 8=(0) 1376 | 5-10=(0) 0 |
| | | 3- 8=(-1013) 0 | |
| | | 8- 4=(0) 1650 | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. | (SPECIES) |
|-------------------|--------------------|---------------------|----------|---------------------------|-----------|
| 0'- 0.0" | 0/ 2027V | -61/ 72H | 5.50" | 5.00 | HF (405) |
| 29'- 2.0" | 0/ 1991V | 0/ 0H | 5.50" | 4.92 | HF (405) |

Weight: 153.41 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.322" @ 14'- 7.0" Allowed = 0.942"
MAX DL DEFL = -0.287" @ 14'- 7.0" Allowed = 1.413"
MAX TL DEFL = -0.608" @ 14'- 7.0" Allowed = 1.413"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

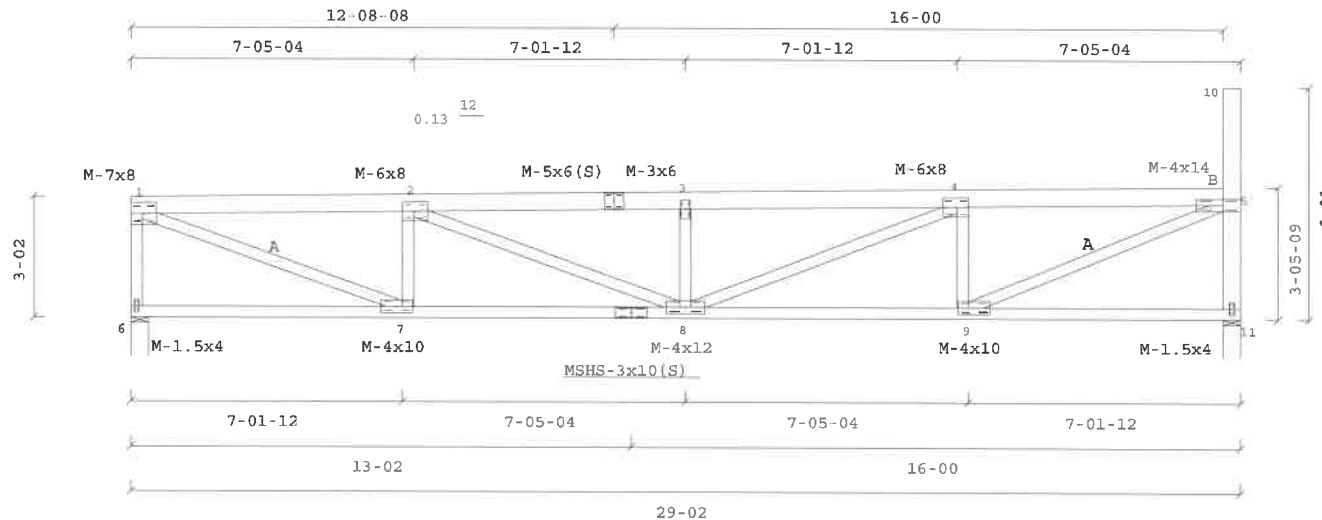
MAX HORIZ. LL DEFL = 0.054" @ 28'- 11.3"
MAX HORIZ. TL DEFL = 0.103" @ 28'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



JOB NAME: WESTMAN MILL - A3S

Scale: 0.2074

Truss: A3S

DATE: 5/1/2019
SEQ.: K6059037
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTC in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTruss Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



May 2,2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF SS
BC: 2x4 DF #1&BTR;
2x6 DF 2400F B1
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15 (Non-Rep)
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)
ADDL: BC CONC LL+DL= 1120.0 LBS @ 5'- 5.8"

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|--------------|---|-------------------|------|-----------------|---------------|
| 1- 2=(-3615) | 0 | 7- 8=(-30) | 157 | 7- 1=(-2978) | 0 |
| 2- 3=(-3616) | 0 | 8- 9=(0) | 5697 | 10- 5=(0) | 2226 |
| 3- 4=(-6246) | 0 | 9-10=(0) | 5658 | 1- 8=(0) | 4407 |
| 4- 5=(-6245) | 0 | 10-11=(0) | 4357 | 2- 8=(-590) | 0 |
| 5- 6=(-4200) | 0 | 11-12=(-9) | 135 | 0 | 11- 6=(0) |
| | | | | 0 | 12- 6=(-2136) |
| | | | | 0 | 0 |
| | | | | 0 | 0 |
| | | | | 3-10=(-67) | 817 |
| | | | | 4-10=(-1063) | 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. (SPECIES) |
|-------------------|--------------------|--------------------|----------|-------------------------------------|
| 0'- 0.0" | 0/ 2978V | -61/ 72H | 5.50" | 4.45 DF (670) |
| 29'- 2.0" | 0/ 2198V | 0/ 0H | 5.50" | 3.52 DF (625) |

Weight: 164.82 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.256" @ 14'- 7.0" Allowed = 0.942"
MAX DL DEFL = -0.322" @ 14'- 7.0" Allowed = 1.413"
MAX TL DEFL = -0.578" @ 14'- 7.0" Allowed = 1.413"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.043" @ 28'- 11.3"
MAX HORIZ. TL DEFL = 0.105" @ 28'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

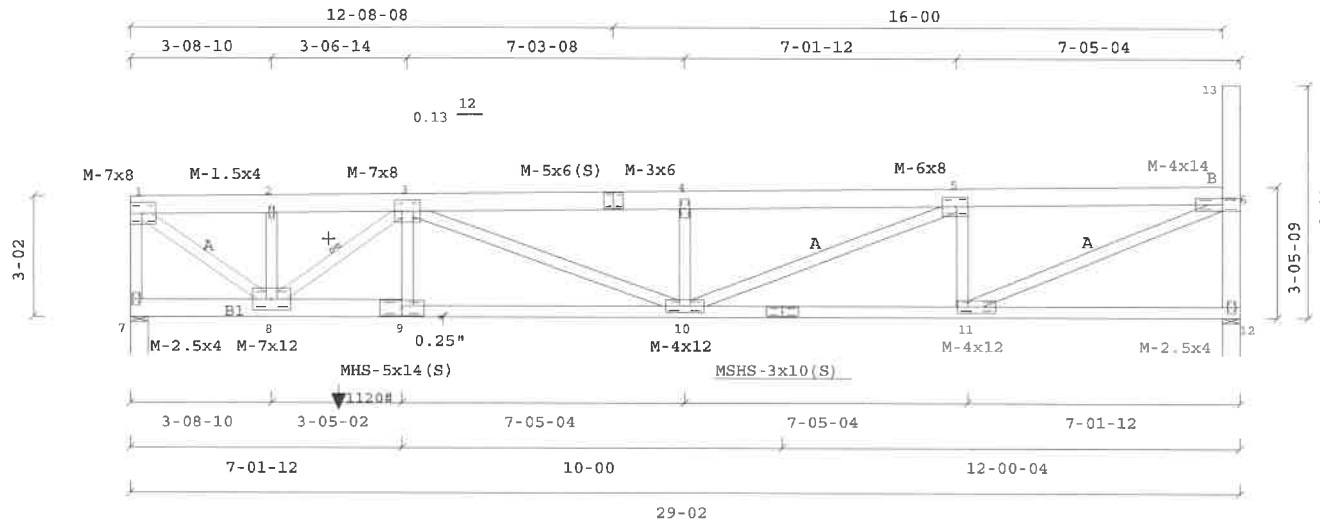
Wind: 110 mph, h=61.7ft, TC DL=13.2, BC DL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

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May 2, 2019



JOB NAME: WESTMAN MILL - A4S

Scale: 0.2074

Truss: A4S

DATE: 5/1/2019
SEQ.: K6059038
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
 2. 2x4 compression web bracing must be installed where shown +.
 3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 6. This design is furnished subject to the limitations set forth by TP/MTGA in BCSI, copies of which will be furnished upon request.
- MiTek USA, Inc./CompuTruss Software 7.6.8(11)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF #2;
BC: 2x4 HF #2;
2x6 HF SS B1
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15 (Non-Rep)
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)
ADDL: BC CONC LL+DL= 1695.0 LBS @ 5'- 5.8"

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|--------------|---|-------------------|------|-----------------|------|
| 1- 2=(-3948) | 0 | 7- 8=(-44) | 168 | 7- 1=(-3204) | 0 |
| 2- 3=(-3950) | 0 | 8- 9=(0) | 5958 | 1- 8=(0) | 4818 |
| 3- 4=(-6038) | 0 | 9-10=(0) | 5902 | 2- 8=(-528) | 32 |
| 4- 5=(-6038) | 0 | 10-11=(0) | 4070 | 8- 3=(-2656) | 0 |
| 5- 6=(-3922) | 0 | 11-12=(-22) | 114 | 9- 3=(0) | 1032 |
| | | | | 3-10=(-554) | 334 |
| | | | | 4-10=(-924) | 82 |
| | | | | 10- 5=(0) | 2314 |
| | | | | 5-11=(-1544) | 70 |
| | | | | 11- 6=(0) | 4166 |
| | | | | 12- 6=(-1958) | 22 |
| | | | | 6-13=(0) | 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. | SPECIES (SPECIES) |
|-------------------|--------------------|---------------------|----------|---------------------------|-------------------|
| 0'- 0.0" | 0/ 3156V | -61/ 72H | 5.50" | 7.79 | HF (405) |
| 29'- 2.0" | 0/ 2022V | 0/ 0H | 5.50" | 4.99 | HF (405) |

(2) complete trusses required.
Attach 2 ply with 3"x.131 DIA GUN nails staggered:
9" oc in 2 row(s) throughout 2x6 top chords,
9" oc in 2 row(s) throughout 2x6 bottom chords,
9" oc in 1 row(s) throughout 2x4 bottom chords,
9" oc in 1 row(s) throughout 2x4 webs,
9" oc in 2 row(s) throughout 2x6 webs.

Weight: 317.59 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.149" @ 14'- 7.0" Allowed = 0.942"
MAX DL DEFL = -0.179" @ 14'- 7.0" Allowed = 1.413"
MAX TL DEFL = -0.328" @ 14'- 7.0" Allowed = 1.413"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.029" @ 28'- 11.3"
MAX HORIZ. TL DEFL = 0.072" @ 28'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

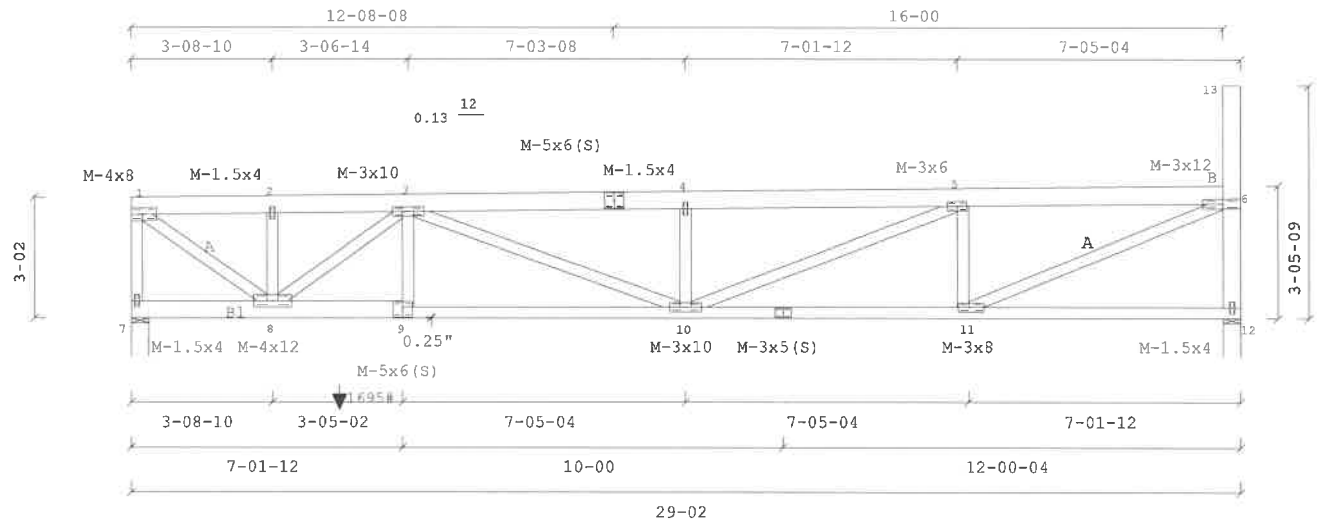
Wind: 110 mph, h=61.7ft, TCDF=7.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWERS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

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January 25, 2019



JOB NAME: WESTMAN MILL - A4G

Scale: 0.2073

Truss: A4G

DATE: 1/24/2019
SEQ.: K5650387
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCA in BCST, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 23'- 7.5"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL (30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|--------------|---|-------------------|------|-----------------|------|
| 1- 2={ -10} | 0 | 6- 7={-11} | 907 | 1- 6={ -14} | 265 |
| 2- 3={-2896} | 0 | 7- 8={-11} | 907 | 6- 2={-1974} | 0 |
| 3- 4={-2895} | 0 | 8- 9={ 0} | 2581 | 2- 7={ 0} | 269 |
| 4- 5={-2496} | 3 | 9-11={-24} | 100 | 2- 8={ 0} | 2243 |
| | | | | 3- 8={ -898} | 82 |
| | | | | 8- 4={ 0} | 566 |
| | | | | 4- 9={ -913} | 77 |
| | | | | 9- 5={ 0} | 2618 |
| | | | | 11- 5={-1331} | 29 |
| | | | | 5-10={ 0} | 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. (SPECIES) |
|-------------------|--------------------|---------------------|----------|------------------------------------|
| 0'- 0.0" | 0/ 1513V | -62/ 72H | 5.50" | 3.74 HF (405) |
| 23'- 7.5" | 0/ 1393V | 0/ 0H | 5.50" | 3.44 HF (405) |

Weight: 129.45 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.150" @ 9'- 0.5" Allowed = 0.757"
MAX DL DEFL = -0.090" @ 9'- 0.5" Allowed = 1.135"
MAX TL DEFL = -0.240" @ 9'- 0.5" Allowed = 1.135"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

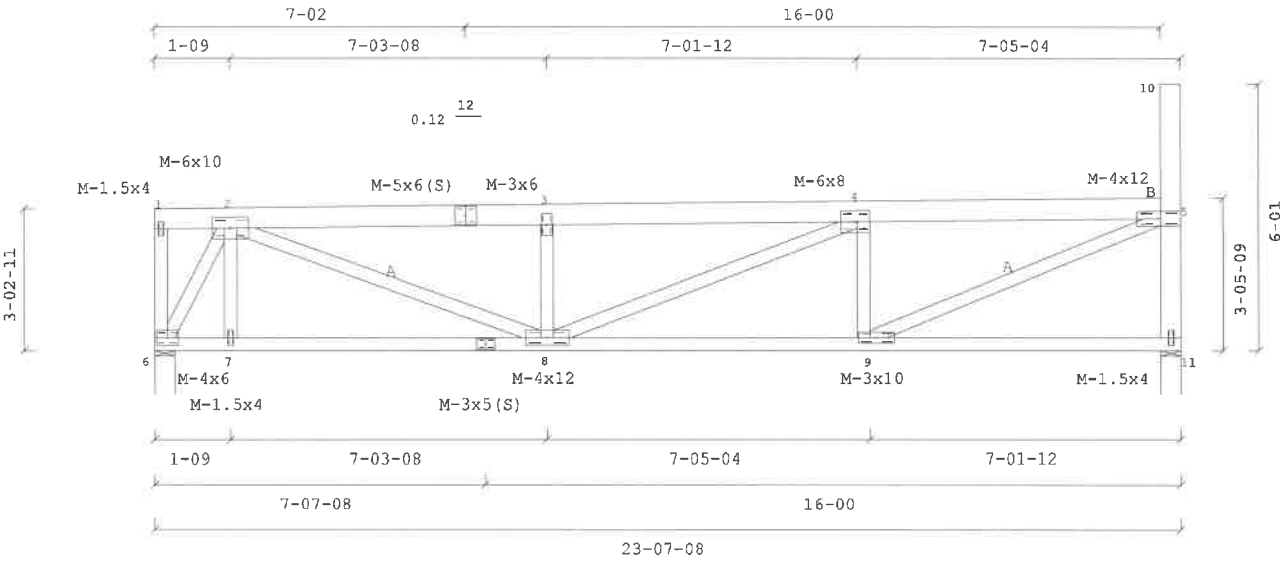
MAX HORIZ. LL DEFL = 0.029" @ 23'- 4.8"
MAX HORIZ. TL DEFL = 0.047" @ 23'- 4.8"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=7.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



JOB NAME: WESTMAN MILL - A5

Scale: 0.2367

| | |
|---|--|
| <p>Truss: A5</p> <p>DATE: 1/24/2019 SEQ.: K5650388 TRANS ID: LINK</p> | <p>WARNINGS:</p> <ol style="list-style-type: none"> 1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences. 2. 2x4 compression web bracing must be installed where shown +. 3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component. 5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components. 6. This design is furnished subject to the limitations set forth by TPJ/WTCA in BCSI, copies of which will be furnished upon request. <p>MITek USA, Inc./CompuTruss Software 7.5.8(1L)-E</p> <p>GENERAL NOTES, unless otherwise noted:</p> <ol style="list-style-type: none"> 1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer. 2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TO) and/or drywall(BC). 3. 2x Impact bridging or lateral bracing required where shown ++ 4. Installation of truss is the responsibility of the respective contractor. 5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use. 6. Design assumes full bearing at all supports shown. Shim or wedge if necessary. 7. Design assumes adequate drainage is provided. 8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines. 9. Digits indicate size of plate in inches. 10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek) |
|---|--|



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 23'- 7.5"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER FORCES | 4WR/DHF/Cq=1.00 |
|----------------|-------------------|-----------------|
| 1- 2=(-11) 0 | 6- 7=(0) 1042 | 1- 6=(0) 291 |
| 2- 3=(-3348) 0 | 7- 8=(0) 1042 | 4- 9=(-1077) 0 |
| 3- 4=(-3348) 0 | 8- 9=(0) 2985 | 6- 2=(-2269) 0 |
| 4- 5=(-2885) 0 | 9-11=(-12) 122 | 2- 7=(0) 270 |
| | | 11- 5=(-1557) 0 |
| | | 2- 8=(0) 2584 |
| | | 3- 8=(-1043) 0 |
| | | 5-10=(0) 0 |
| | | 8- 4=(0) 617 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. | (SPECIES) |
|-------------------|--------------------|---------------------|----------|---------------------------|-----------|
| 0'- 0.0" | 0/ 1750V | -62/ 72H | 5.50" | 4.32 | HF (405) |
| 23'- 7.5" | 0/ 1620V | 0/ 0H | 5.50" | 4.00 | HF (405) |

Weight: 129.68 lb

VERTICAL DEPLETION LIMITS: LL=L/360, TL=L/240
MAX LL DEPL = -0.150" @ 9'- 0.5" Allowed = 0.757"
MAX DL DEPL = -0.128" @ 9'- 0.5" Allowed = 1.135"
MAX TL DEPL = -0.278" @ 9'- 0.5" Allowed = 1.135"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEPL = 0.029" @ 23'- 4.8"
MAX HORIZ. TL DEPL = 0.054" @ 23'- 4.8"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

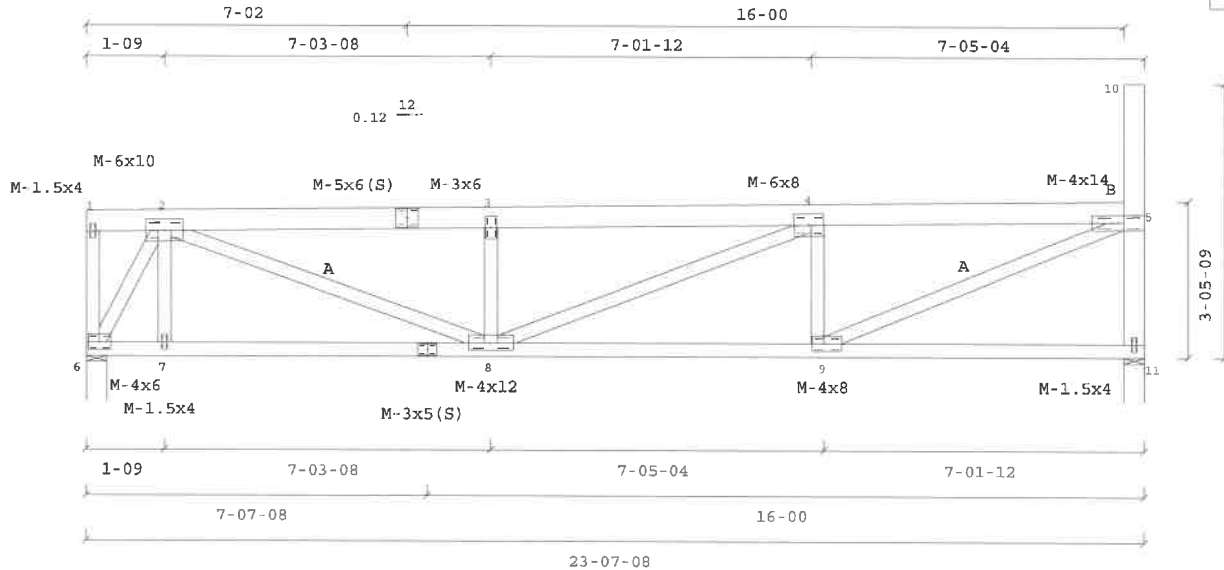
Wind: 110 mph, h=61.7ft, TCDF=13.2, RCDF=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), Load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



May 2, 2019



JOB NAME: WESTMAN MILL - A5S

Scale: 0.2441

Truss: A5S

DATE: 5/1/2019
SEQ.: K6059039
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TP/W/TCA in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTruss Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1968 (MITek)

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 44"OC. UON.

Unbalanced live loads have been
considered for this design.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 137.41 lb

COND. 2: Design checked for 50LBS sprinkler load
located at any point along top chord
except end panels and overhangs.
Additional 250LBS vertical load applied non-concurrently
at each sprinkler location.
COND. 3: 370.00 PLF SEISMIC LOAD.
SHEARWALL 0.00 to 26.17

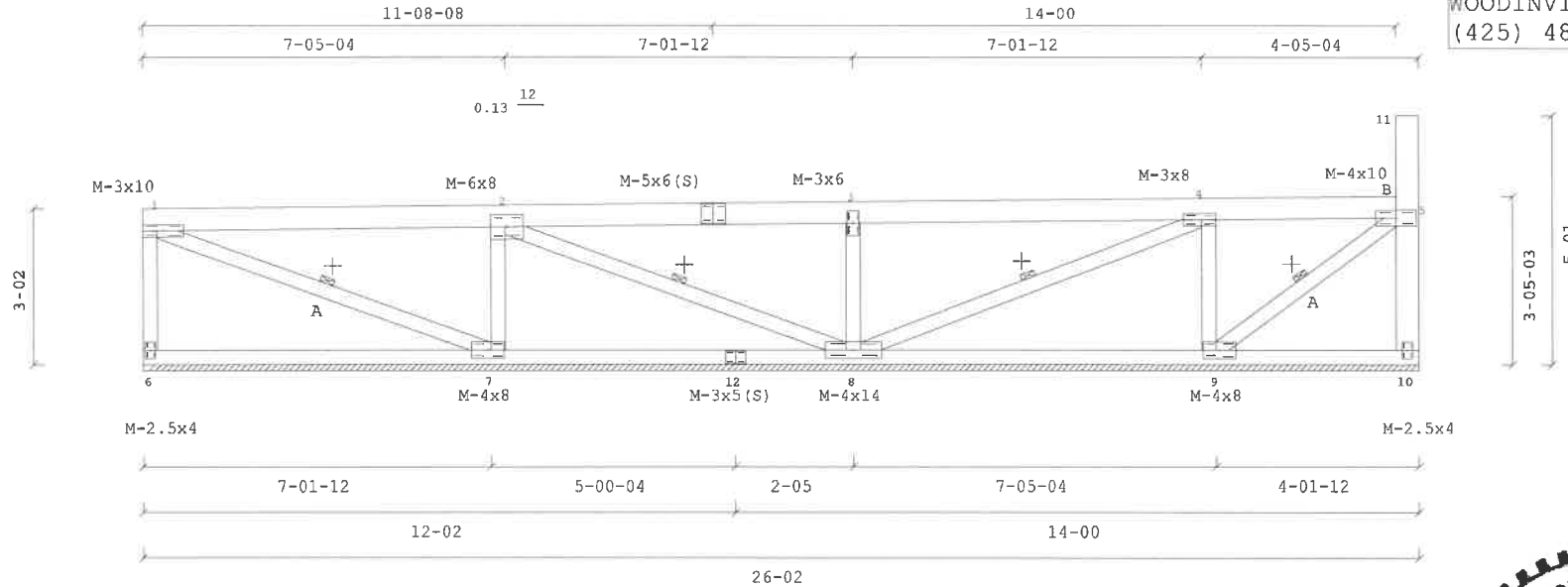
NOTE: TRUSS DESIGN ASSUMES UNIFORM
SHEAR TRANSFER

Wind: 110 mph, h=61.7ft, TCDF=7.2,BCDF=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
Only right end vertical is exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

Note:Truss design requires continuous
bearing wall for entire span UON.

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P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for
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concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A6

Scale: 0.2678

Truss: A6

DATE: 1/24/2019
SEQ.: K5650389
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCA in BCS, copies of which will be furnished upon request.

MITek USA, Inc./CompuTruss Software 7.6.8(1L)-E

GENERAL NOTES: unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TO) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HP #2
BC: 2x4 HP #2
WEBS: 2x4 HP STD/STUD;
2x4 HP #2 A;
2x6 HP #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 44"OC. UON.

Unbalanced live loads have been
considered for this design.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 138.19 lb

COND. 2: Design checked for 50LBS sprinkler load
located at any point along top chord
except end panels and overhangs.
Additional 250LBS vertical load applied non-concurrently
at each sprinkler location.
COND. 3: 370.00 PLF SEISMIC LOAD.
SHEARWALL 0.00 to 26.17

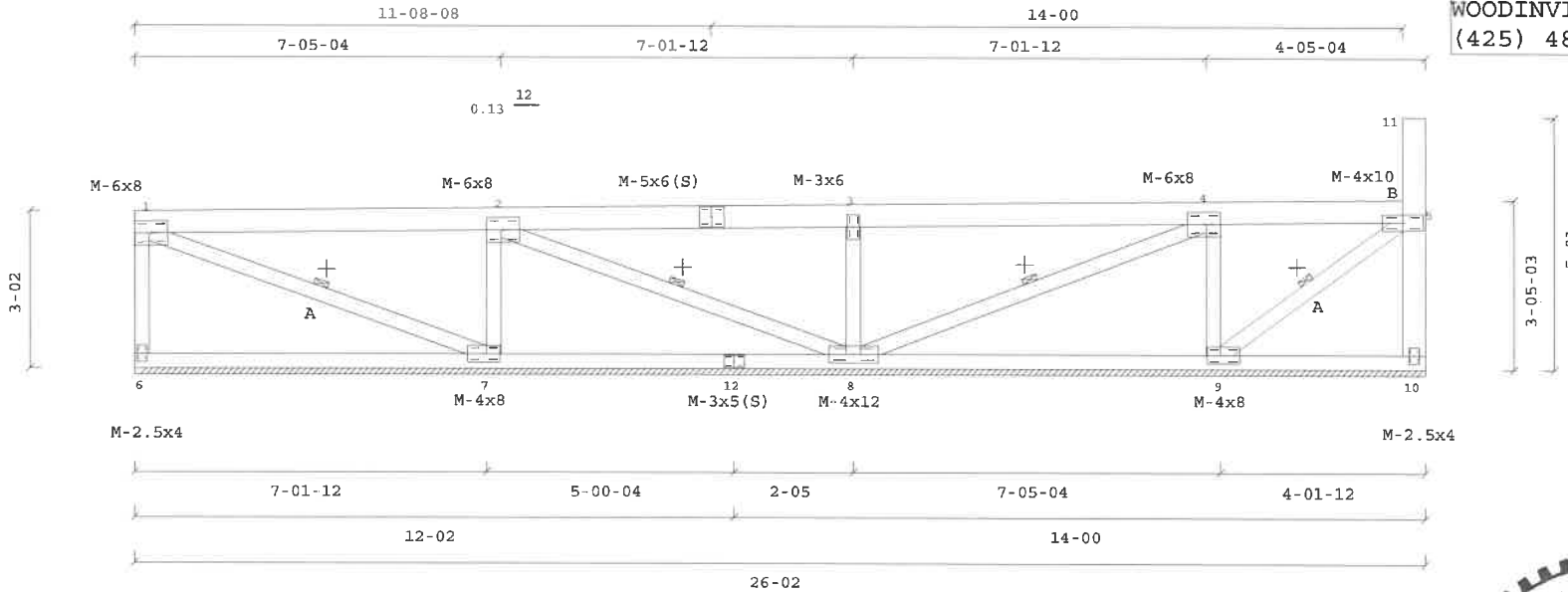
NOTE: TRUSS DESIGN ASSUMES UNIFORM
SHEAR TRANSFER

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDF=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
Only right end vertical is exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

Note:Truss design requires continuous
bearing wall for entire span UON.

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WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for
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concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A6S

Scale: 0.2699

| | |
|---|---|
| <p>Truss: A6S</p> <p>DATE: 5/1/2019 SEQ.: K6059040 TRANS ID: LINK</p> | <p>WARNINGS:</p> <ol style="list-style-type: none"> 1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences. 2. 2x4 compression web bracing must be installed where shown +. 3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component. 5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components. 6. This design is furnished subject to the limitations set forth by TPI/WTC in ECSI, copies of which will be furnished upon request. <p>MITek USA, Inc./CompuTruss Software 7.8.8(1L)-E</p> <p>GENERAL NOTES, unless otherwise noted:</p> <ol style="list-style-type: none"> 1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer. 2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC). 3. 2x Impact bridging or lateral bracing required where shown ++ 4. Installation of truss is the responsibility of the respective contractor. 5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use. 6. Design assumes full bearing at all supports shown. Shim or wedge if necessary. 7. Design assumes adequate drainage is provided. 8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines. 9. Digits indicate size of plate in inches. 10. For basic connector plate design values see ESR-1311, ESR-1088 (MITek) |
|---|---|



May 2,2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER FORCES | 4WR/DHF/Cq=1.00 |
|-----------------|-------------------|-----------------|
| 1- 2=(-3028) 0 | 6- 7=(-45) 82 | 6- 1=(-1511) 28 |
| 2- 3=(-3660) 0 | 7- 8=(0) 3130 | 1- 7=(0) 3183 |
| 3- 4=(-3659) 0 | 8- 9=(0) 2023 | 7- 2=(-1050) 90 |
| 4- 5=(-1895) 13 | 9-10=(-21) 34 | 2- 8=(0) 782 |
| | | 3- 8=(-885) 80 |
| | | 8- 4=(0) 1902 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. (SPECIES) |
|-------------------|--------------------|---------------------|----------|------------------------------------|
| 0'- 0.0" | 0/ 1571V | -61/ 71H | 5.50" | 3.88 HF (405) |
| 26'- 2.0" | 0/ 1578V | 0/ 0H | 5.50" | 3.90 HF (405) |

Weight: 137.99 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.219" @ 14'- 7.0" Allowed = 0.842"
MAX DL DEFL = -0.134" @ 14'- 7.0" Allowed = 1.263"
MAX TL DEFL = -0.354" @ 14'- 7.0" Allowed = 1.263"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

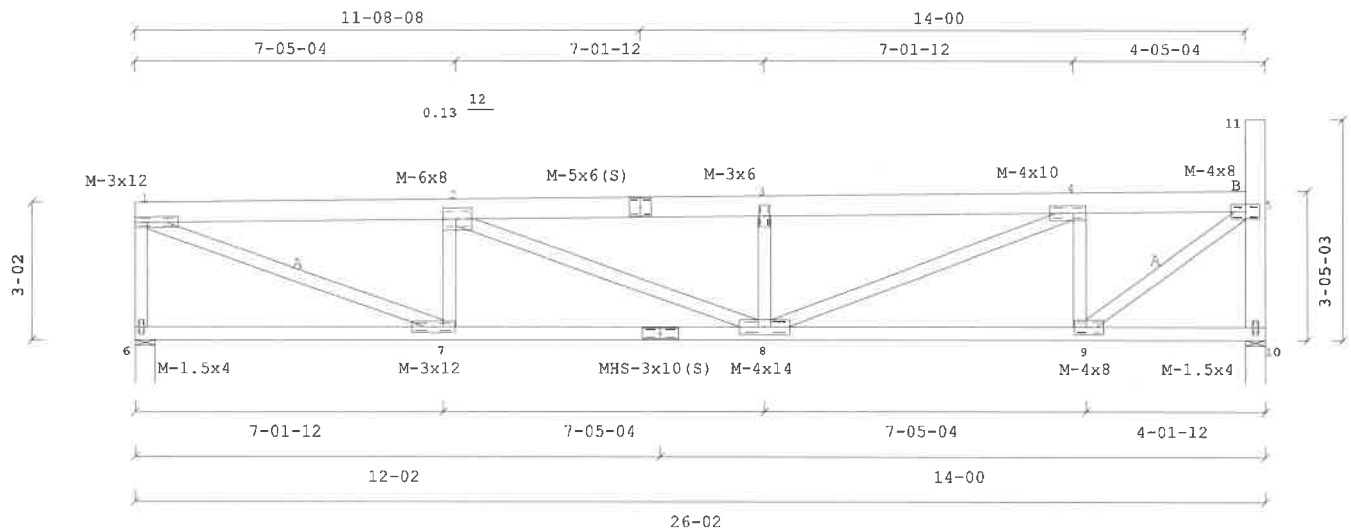
MAX HORIZ. LL DEFL = 0.040" @ 25'- 11.3"
MAX HORIZ. TL DEFL = 0.065" @ 25'- 11.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDL=7.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

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P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900



JOB NAME: WESTMAN MILL - A7

Scale: 0.2354

Truss: A7

DATE: 1/24/2019
SEQ.: K5650390
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJWTC in BCS, copies of which will be furnished upon request.

MiTek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES. unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' s.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MiTek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 |
|----------------|----------------|----------------|-----------------|
| 1- 2=(-3509) 0 | 6- 7=(-37) 95 | 6- 1=(-1772) 0 | 4- 9=(-1572) 0 |
| 2- 3=(-4242) 0 | 7- 8=(0) 3630 | 1- 7=(0) 3688 | 9- 5=(0) 2751 |
| 3- 4=(-4242) 0 | 8- 9=(0) 2342 | 7- 2=(-1242) 0 | 10- 5=(-1803) 0 |
| 4- 5=(-2193) 0 | 9-10=(-15) 40 | 2- 8=(0) 871 | 5-11=(0) 0 |
| | | 3- 8=(-1029) 0 | |
| | | 8- 4=(0) 2187 | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. (SPECIES) |
|-------------------|--------------------|---------------------|----------|-------------------------------------|
| 0'- 0.0" | 0/ 1832V | -61/ 71H | 5.50" | 4.52 HF (405) |
| 26'- 2.0" | 0/ 1831V | 0/ 0H | 5.50" | 4.52 HF (405) |

Weight: 138.9# lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.217" @ 14'- 7.0" Allowed = 0.842"
MAX DL DEFL = -0.189" @ 14'- 7.0" Allowed = 1.263"
MAX TL DEFL = -0.407" @ 14'- 7.0" Allowed = 1.263"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

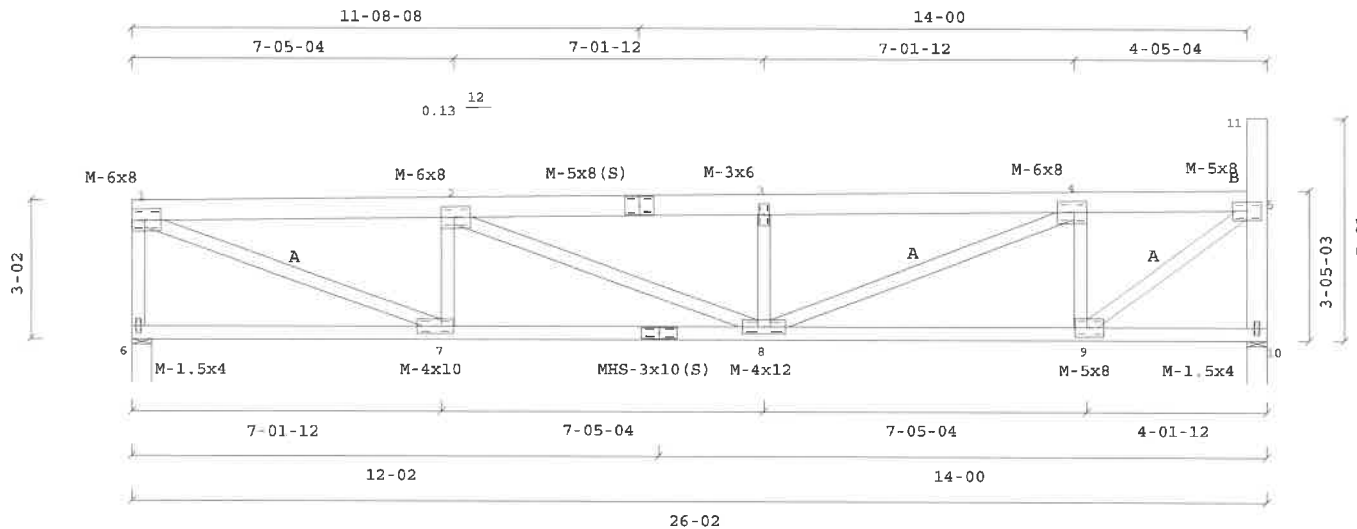
MAX HORIZ. LL DEFL = 0.040" @ 25'- 11.3"
MAX HORIZ. TL DEFL = 0.076" @ 25'- 11.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind. Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900



JOB NAME: WESTMAN MILL - A7S

Scale: 0.2363

Truss: A7S

DATE: 5/1/2019
SEQ.: K6059041
TRANS ID: LINK

WARNINGS:

- Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
 - 2x4 compression web bracing must be installed where shown +.
 - Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
 - No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 - CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 - This design is furnished subject to the limitations set forth by TPI/WTCA in BCSI, copies of which will be furnished upon request.
- MITek USA, Inc./CompuTruss Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

- This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
- Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
- 2x Impact bridging or lateral bracing required where shown + +
- Installation of truss is the responsibility of the respective contractor.
- Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
- Design assumes full bearing at all supports shown. Shim or wedge if necessary.
- Design assumes adequate drainage is provided.
- Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
- Digits indicate size of plate in inches.
- For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER FORCES | 4WR/DHF/Cq=1.00 |
|----------------|-------------------|-----------------|
| 1- 2=(-3429) 0 | 6- 7=(-46) 89 | 6- 1=(-1676) 26 |
| 2- 3=(-4501) 0 | 7- 8=(0) 3548 | 1- 7=(0) 3607 |
| 3- 4=(-4500) 0 | 8- 9=(0) 3329 | 7- 2=(-1210) 89 |
| 4- 5=(-3213) 0 | 9-11=(-23) 107 | 2- 8=(0) 1215 |
| | | 3- 8=(-874) 76 |
| | | 8- 4=(0) 1449 |
| | | 5-10=(0) 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. (SPECIES) |
|-------------------|--------------------|---------------------|----------|------------------------------------|
| 0'- 0.0" | 0/ 1736V | -61/ 72H | 5.50" | 4.29 HF (405) |
| 29'- 2.0" | 0/ 1708V | 0/ 0H | 5.50" | 4.22 HF (405) |

Weight: 150.73 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.322" @ 14'- 7.0" Allowed = 0.942"
MAX DL DEFL = -0.201" @ 14'- 7.0" Allowed = 1.412"
MAX TL DEFL = -0.523" @ 14'- 7.0" Allowed = 1.412"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

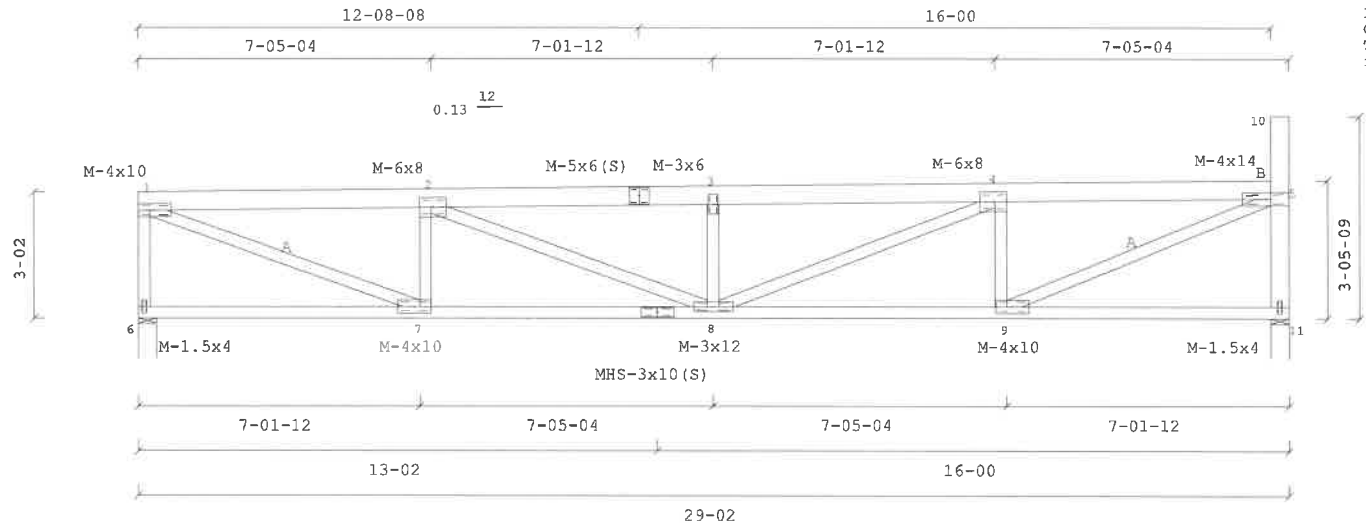
MAX HORIZ. LL DEFL = 0.054" @ 28'- 11.3"
MAX HORIZ. TL DEFL = 0.088" @ 28'- 11.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=7.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWERS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900



JOB NAME: WESTMAN MILL - A8

Scale: 0.2150

Truss: A8

DATE: 1/24/2019
SEQ.: K5650391
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJ/WTCA in BCSL copies of which will be furnished upon request.

MiTek USA, Inc./CompuTruss Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1886 (MiTek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HP #2
BC: 2x4 HP #2
WEBS: 2x4 HP STD/STUD,
2x4 HP #2 A,
2x6 HP #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|--------------|---|-------------------|------|-----------------|------|
| 1- 2=(-3984) | 0 | 6- 7=(-37) | 104 | 6- 1=(-1967) | 0 |
| 2- 3=(-5227) | 0 | 7- 8=(0) | 4124 | 1- 7=(0) | 4189 |
| 3- 4=(-5226) | 0 | 8- 9=(0) | 3869 | 7- 2=(-1432) | 0 |
| 4- 5=(-3732) | 0 | 9-11=(-10) | 129 | 2- 8=(0) | 1376 |
| | | | | 3- 8=(-1013) | 0 |
| | | | | 8- 4=(0) | 1650 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. | (SPECIES) |
|-------------------|--------------------|--------------------|----------|---------------------------|-----------|
| 0'- 0.0" | 0/ 2027V | -61/ 72H | 5.50" | 5.00 | HF (405) |
| 29'- 2.0" | 0/ 1991V | 0/ 0H | 5.50" | 4.92 | HF (405) |

Weight: 121.58 lb

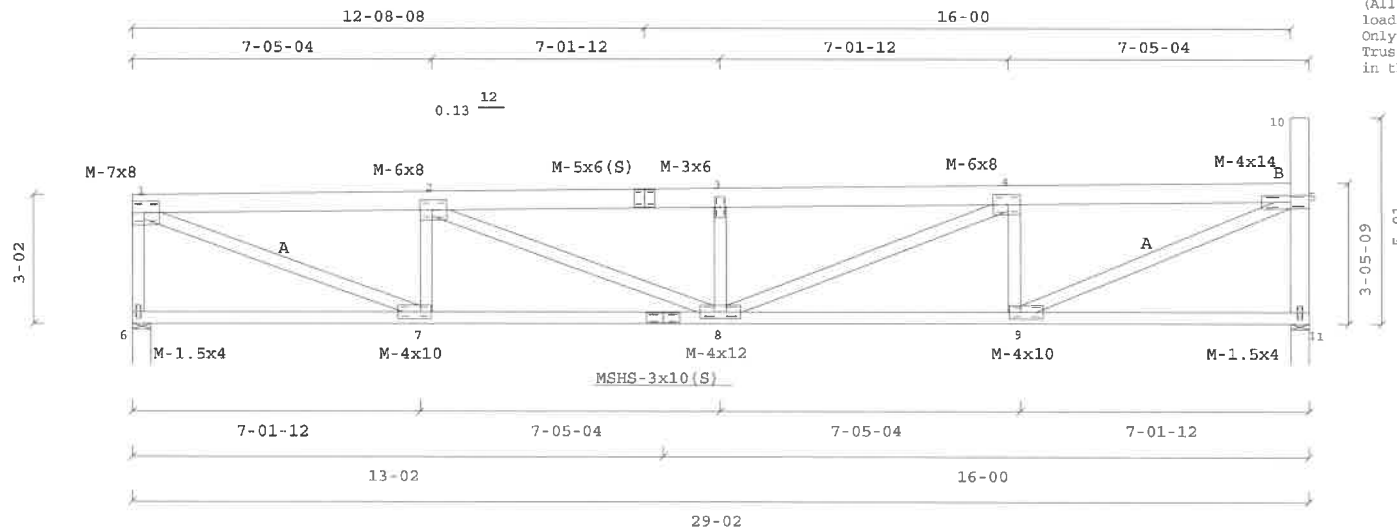
VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.322" @ 14'- 7.0" Allowed = 0.942"
MAX DL DEFL = -0.287" @ 14'- 7.0" Allowed = 1.412"
MAX TL DEFL = -0.608" @ 14'- 7.0" Allowed = 1.412"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.054" @ 28'- 11.3"
MAX HORIZ. TL DEFL = 0.103" @ 28'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TC_{DL}=13.2, BC_{DL}=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
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(425) 481-0900



JOB NAME: WESTMAN MILL - A8S

Scale: 0.2199

Truss: A8S

DATE: 5/1/2019
SEQ.: K6059042
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJ/WTCA in ECSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 7' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-combustive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1868 (MITek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 44"OC. UON.

Unbalanced live loads have been
considered for this design.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 135.58 lb

COND. 2: Design checked for 50LBS sprinkler load
located at any point along top chord
except end panels and overhangs.
Additional 250LBS vertical load applied non-concurrently
at each sprinkler location.
COND. 3: 370.00 PLF SEISMIC LOAD.
SHEARWALL 0.00 to 26.17

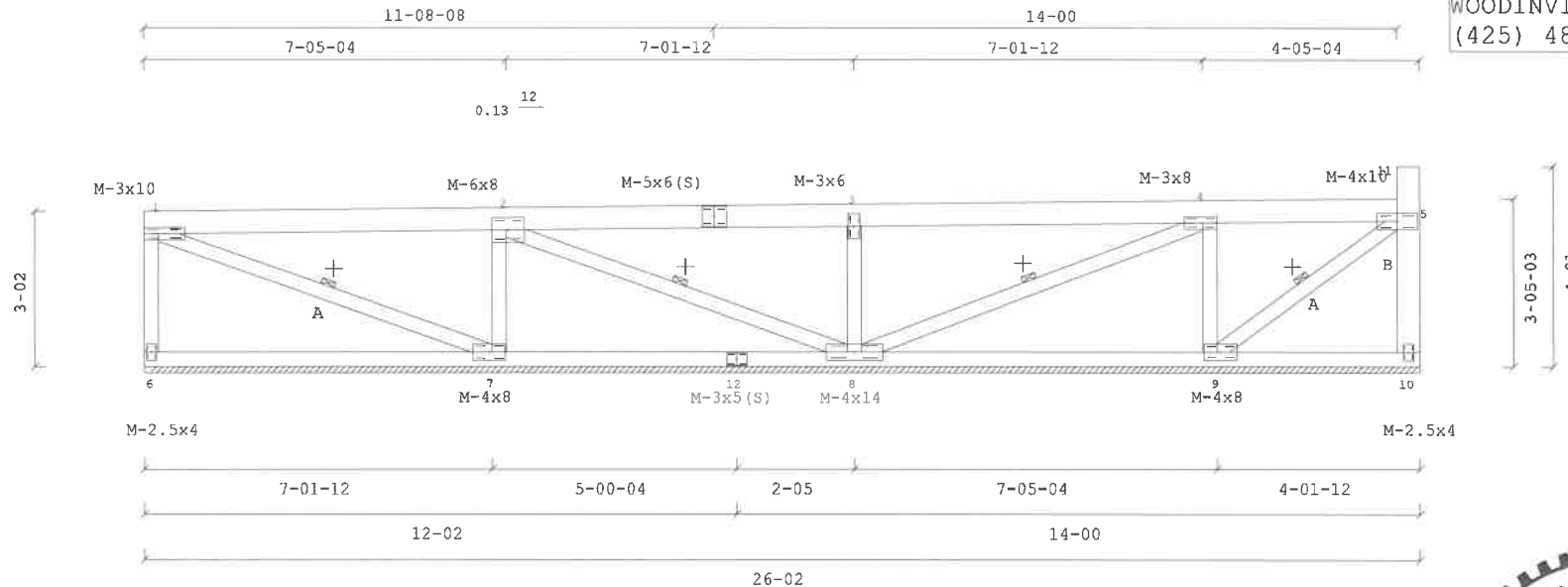
NOTE: TRUSS DESIGN ASSUMES UNIFORM
SHEAR TRANSFER

Wind: 110 mph, h=61.7ft, TCDF=7.2,BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
Only right end vertical is exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

Note:Truss design requires continuous
bearing wall for entire span UON.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for
conformance of design criteria and
concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A9

Scale: 0.2678

Truss: A9

DATE: 1/24/2019
SEQ.: K5650392
TRANS ID: LINK

WARNINGS:
1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJ/WTCA in BCS1, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:
1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced all 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 44"OC. UON.

Unbalanced live loads have been
considered for this design.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL (30.0)+DL (22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 136.36 lb

COND. 2: Design checked for 50LBS sprinkler load
located at any point along top chord
except end panels and overhangs.
Additional 250LBS vertical load applied non-concurrently
at each sprinkler location.
COND. 3: 370.00 PLF SEISMIC LOAD.
SHEARWALL 0.00 to 26.17

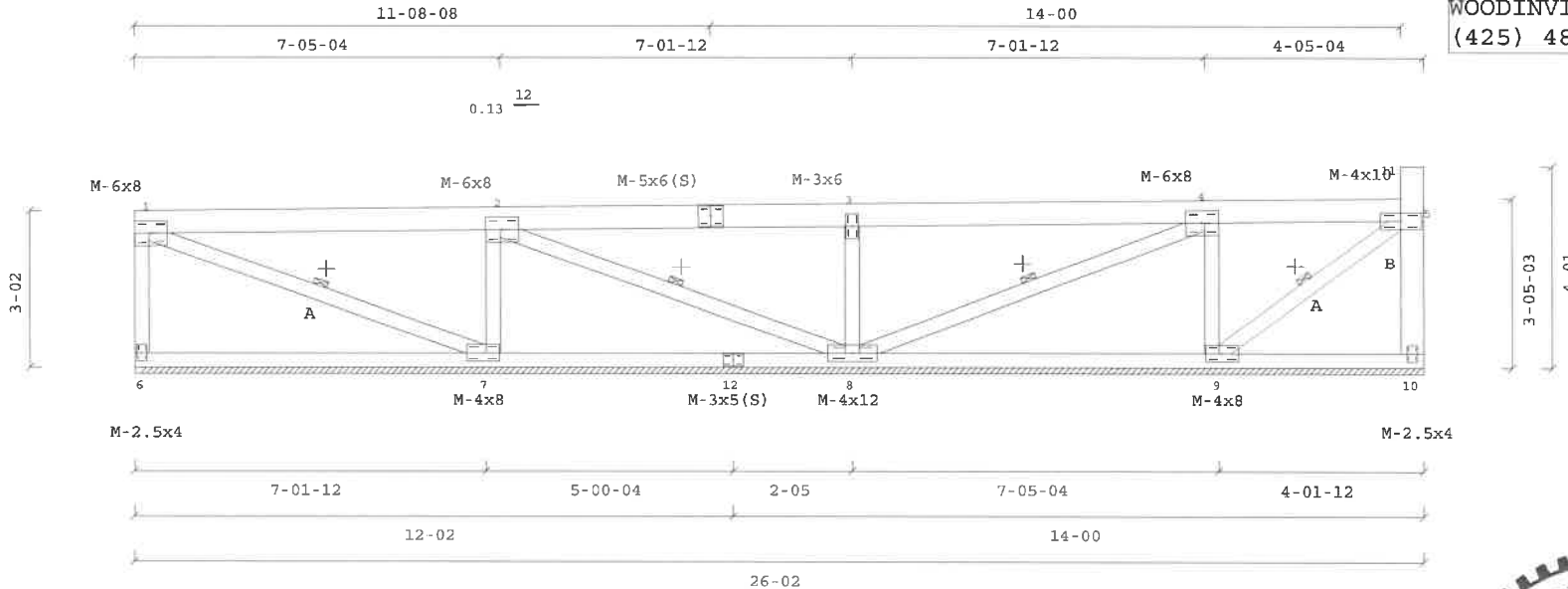
NOTE: TRUSS DESIGN ASSUMES UNIFORM
SHEAR TRANSFER

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
Only right end vertical is exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

Note:Truss design requires continuous
bearing wall for entire span UON.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for
conformance of design criteria and
concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A9S

Scale: 0.2699

Truss: A9S

DATE: 5/1/2019
SEQ.: K6059043
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TP/WTCA in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1986 (MITek)



May 2, 2019

This design prepared from computer input by
RT'S - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER FORCES | 4WR/DHF/Cq=1.00 |
|--------------|-------------------|-----------------|
| 1- 2=(-3028) | 0 | 6- 7=(-45) 82 |
| 2- 3=(-3660) | 0 | 7- 8=(0) 3130 |
| 3- 4=(-3659) | 0 | 8- 9=(0) 2023 |
| 4- 5=(-1895) | 13 | 9-10=(-21) 34 |
| | | 6- 1=(-1511) 28 |
| | | 1- 7=(0) 3183 |
| | | 7- 2=(-1050) 90 |
| | | 2- 8=(0) 782 |
| | | 3- 8=(-885) 80 |
| | | 8- 4=(0) 1902 |
| | | 4- 9=(-1343) 59 |
| | | 9- 5=(0) 2381 |
| | | 10- 5=(-1550) 9 |
| | | 5-11=(0) 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. | (SPECIES) |
|-------------------|--------------------|---------------------|----------|--------------------------|-----------|
| 0'- 0.0" | 0/ 1571V | -61/ 71H | 5.50" | 3.88 | HF (405) |
| 26'- 2.0" | 0/ 1578V | 0/ 0H | 5.50" | 3.90 | HF (405) |

Weight: 136.15 lb

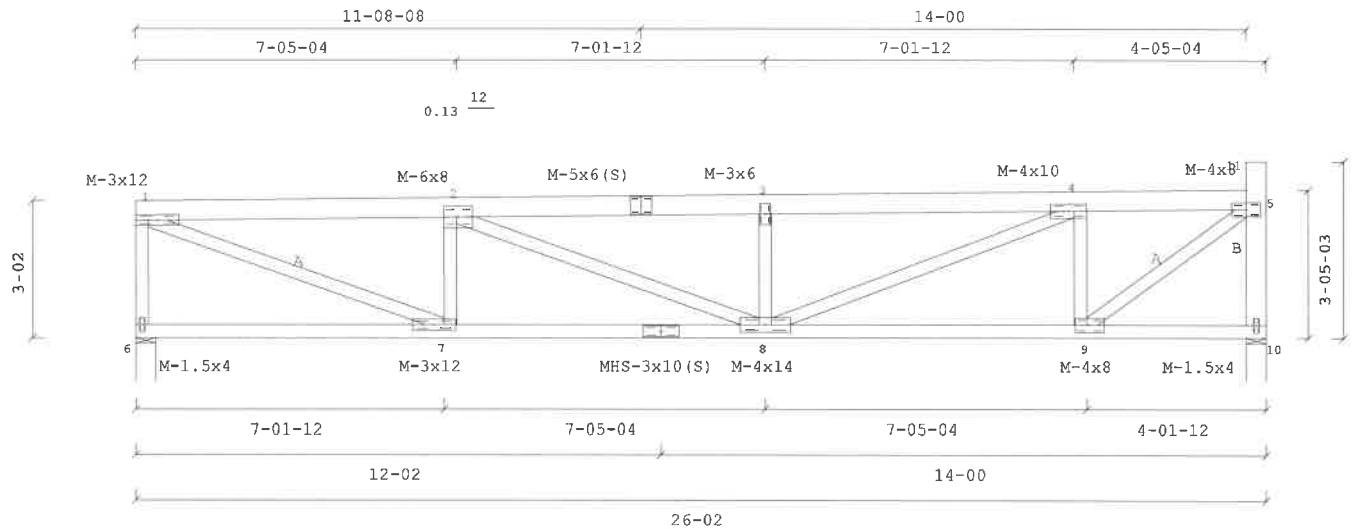
VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.213" @ 14'- 7.0" Allowed = 0.842"
MAX DL DEFL = -0.134" @ 14'- 7.0" Allowed = 1.263"
MAX TL DEFL = -0.354" @ 14'- 7.0" Allowed = 1.263"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.040" @ 25'- 11.3"
MAX HORIZ. TL DEFL = 0.065" @ 25'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TC DL=7.2, BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900



JOB NAME: WESTMAN MILL - A10

Scale: 0.2354

Truss: A10

DATE: 1/24/2019
SEQ.: K5650393
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCGA in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.8.8(1L)-E

GENERAL NOTES: unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 |
|----------------|----------------|----------------|-----------------|
| 1- 2=(-3509) 0 | 6- 7=(-37) 95 | 6- 1=(-1772) 0 | 4- 9=(-1572) 0 |
| 2- 3=(-4242) 0 | 7- 8=(0) 3630 | 1- 7=(0) 3688 | 9- 5=(0) 2751 |
| 3- 4=(-4242) 0 | 8- 9=(0) 2342 | 7- 2=(-1242) 0 | 10- 5=(-1803) 0 |
| 4- 5=(-2193) 0 | 9-10=(-15) 40 | 2- 8=(0) 871 | 5-11=(0) 0 |
| | | 3- 8=(-1029) 0 | |
| | | 8- 4=(0) 2187 | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. | (SPECIES) |
|-------------------|--------------------|---------------------|----------|---------------------------|-----------|
| 0'- 0.0" | 0/ 1832V | -61/ 71H | 5.50" | 4.52 | HF (405) |
| 26'- 2.0" | 0/ 1831V | 0/ 0H | 5.50" | 4.52 | HF (405) |

Weight: 137.12 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.217" @ 14'- 7.0" Allowed = 0.842"
MAX DL DEFL = -0.189" @ 14'- 7.0" Allowed = 1.263"
MAX TL DEFL = -0.407" @ 14'- 7.0" Allowed = 1.263"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

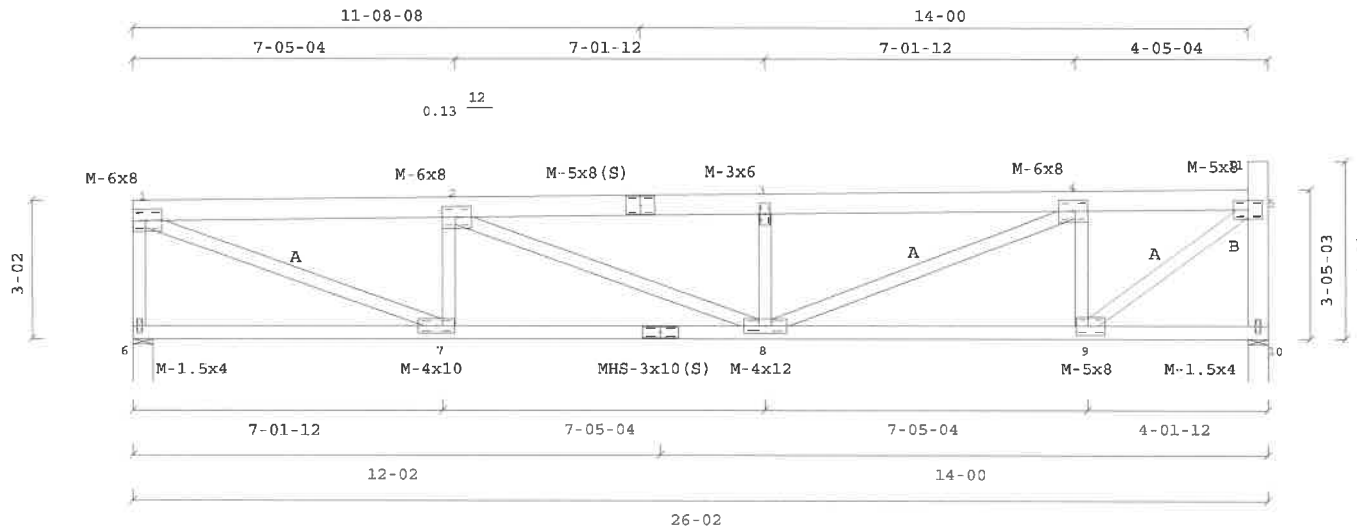
MAX HORIZ. LL DEFL = 0.040" @ 25'- 11.3"
MAX HORIZ. TL DEFL = 0.076" @ 25'- 11.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900



JOB NAME: WESTMAN MILL - A10S

Scale: 0.2363

Truss: A10S

DATE: 5/1/2019
SEQ.: K6059044
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCA in BCSI, copies of which will be furnished upon request.

MiTek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' s.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MiTek)



May 2, 2019

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 56"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 150.43 lb

COND. 2: Design checked for 50LBS sprinkler load
located at any point along top chord
except end panels and overhangs.
Additional 250LBS vertical load applied non-concurrently
at each sprinkler location.
COND. 3: 370.00 PLF SEISMIC LOAD.
SHEARWALL 0.00 to 29.17

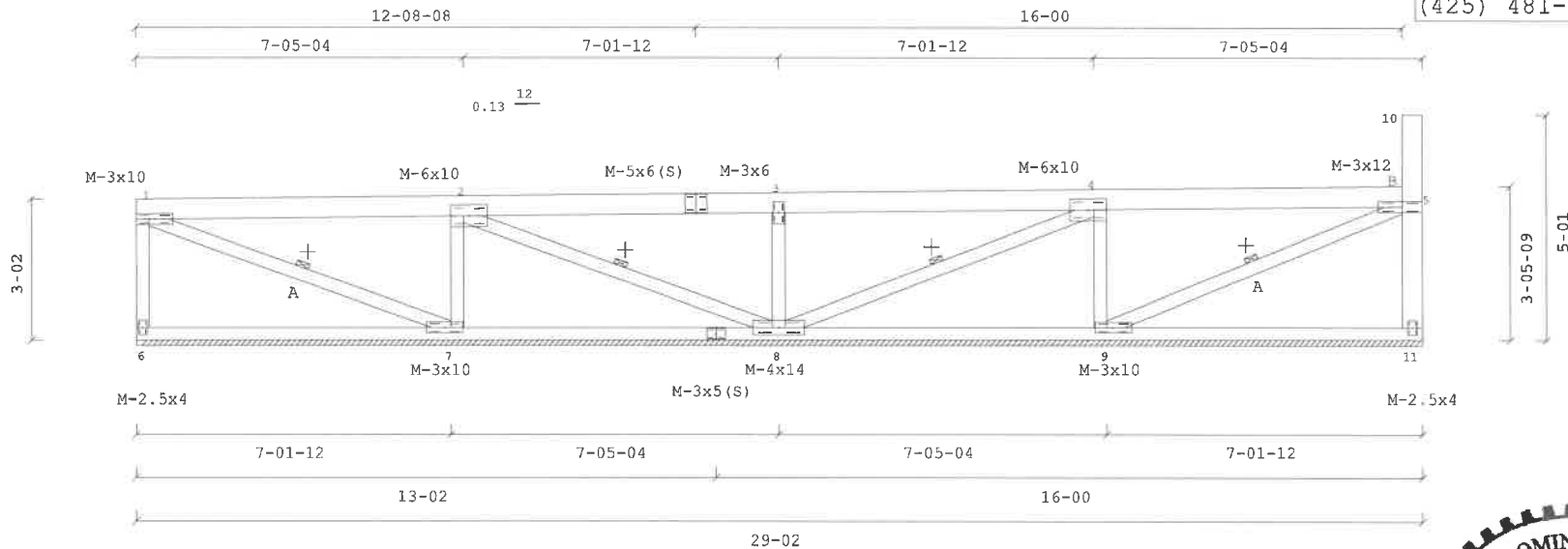
NOTE: TRUSS DESIGN ASSUMES UNIFORM
SHEAR TRANSFER

Wind: 110 mph, h=61.7ft, TCDL=7.2,BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
Only right end vertical is exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

Note:Truss design requires continuous
bearing wall for entire span UON.

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P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for
conformance of design criteria and
concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A11

Scale: 0.2484

Truss: A11

DATE: 1/24/2019
SEQ.: K5650394
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
 2. 2x4 compression web bracing must be installed where shown +.
 3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 6. This design is furnished subject to the limitations set forth by TPI/WTC in BCS, copies of which will be furnished upon request.
- MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++.
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

Weight: 150.84 lb

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 56"OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

COND. 2: Design checked for 50LBS sprinkler load
located at any point along top chord
except end panels and overhangs.
Additional 250LBS vertical load applied non-concurrently
at each sprinkler location.
COND. 3: 370.00 PLF SEISMIC LOAD.
SHEARWALL 0.00 to 29.17

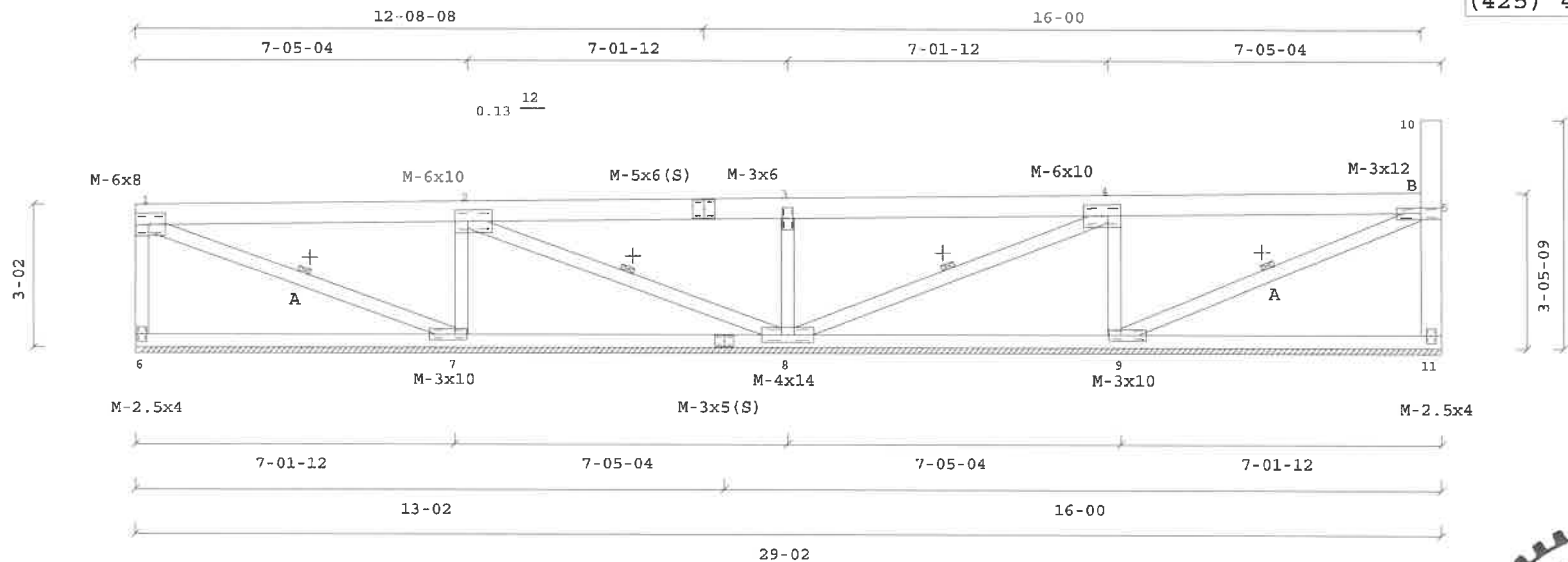
NOTE: TRUSS DESIGN ASSUMES UNIFORM
SHEAR TRANSFER

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDF=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWPRS(Dir),
load duration factor=1.6,
Only right end vertical is exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

Note:Truss design requires continuous
bearing wall for entire span UON.

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(425) 481-0900

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the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A11S

Scale: 0.2440

Truss: A11S

DATE: 5/1/2019
SEQ.: K6059045
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJW/TCA in BCSI, copies of which will be furnished upon request.

MiTek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MiTek)



May 2,2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF #2;
2x6 HF SS T1
BC: 2x4 DF #1&BTR;
2x4 HF #2 B3;
2x6 HF #2 B1
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 26'- 2.0"
LOAD DURATION INCREASE = 1.15 (Non-Rep)
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)
ADDL: BC CONC LL+DL= 1520.0 LBS @ 3'- 6.5"
BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER FORCES | 4WR/DHP/Cq=1.00 |
|----------------|-------------------|------------------|
| 1- 2=(-3735) 0 | 7- 8=(-29) 162 | 7- 1=(-3082) 0 |
| 2- 3=(-3735) 0 | 8- 9=(0) 4882 | 1- 8=(0) 4552 |
| 3- 4=(-5029) 0 | 9-10=(0) 4877 | 2- 8=(-566) 1 |
| 4- 5=(-5029) 0 | 10-11=(0) 2615 | 0- 3=(-1569) 0 |
| 5- 6=(-2447) 0 | 11-12=(-14) 42 | 9- 3=(0) 212 |
| | | 3-10=(-251) 373 |
| | | 4-10=(-1078) 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. (SPECIES) |
|-------------------|--------------------|--------------------|----------|------------------------------------|
| 0'- 0.0" | 0/ 3195V | -61/ 71H | 5.50" | 7.89 HF (405) |
| 26'- 2.0" | 0/ 2031V | 0/ 0H | 5.50" | 5.02 HF (405) |

Weight: 148.53 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.186" @ 14'- 7.0" Allowed = 0.842"
MAX DL DEFL = -0.231" @ 14'- 7.0" Allowed = 1.263"
MAX TL DEFL = -0.417" @ 14'- 7.0" Allowed = 1.263"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

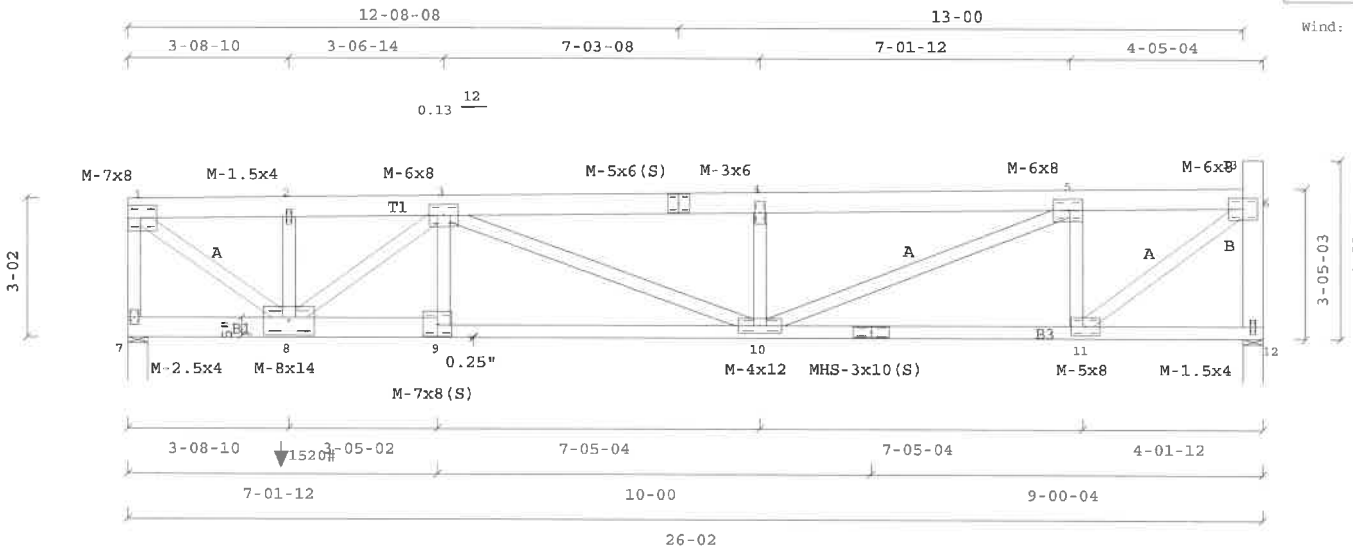
MAX HORIZ. LL DEFL = 0.039" @ 25'- 11.3"
MAX HORIZ. TL DEFL = 0.095" @ 25'- 11.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCCL=13.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWPRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

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(425) 481-0900



JOB NAME: WESTMAN MILL - A12S

Scale: 0.2364

Truss: A12S

DATE: 5/1/2019
SEQ.: K6059046
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TP/MTCA in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 22'- 6.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 | | | |
|---------------|---|-------------|--------|-----------------|------|----------------|------|
| 1- 2=(0) | 8 | 6- 7=(0) | 1788 | 1- 6=(-112) | 0 | 4- 9=(-1315) | 0 |
| 2- 3=(-3261) | 0 | 7- 8=(0) | 1788 | 6- 2=(-2362) | 0 | 9- 5=(0) | 2334 |
| 3- 4=(-3261) | 0 | 8- 9=(0) | 1987 | 2- 7=(0) | 241 | 10- 5=(-1557) | 0 |
| 4- 5=(-1862) | 0 | 9-10=(-17) | 37 | 2- 8=(0) | 1713 | 5-11=(0) | 0 |
| | | | | 3- 8=(-1068) | 0 | | |
| | | | | 8- 4=(0) | 1519 | | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. (SPECIES) |
|-------------------|--------------------|--------------------|----------|-------------------------------------|
| 0'- 0.0" | 0/ 1653V | -61/ 71H | 5.50" | 4.08 HF (405) |
| 22'- 6.0" | 0/ 1586V | 0/ 0H | 5.50" | 3.92 HF (405) |

Weight: 121.04 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.133" @ 10'- 11.0" Allowed = 0.719"
MAX DL DEFL = -0.111" @ 10'- 11.0" Allowed = 1.079"
MAX TL DEFL = -0.244" @ 10'- 11.0" Allowed = 1.079"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

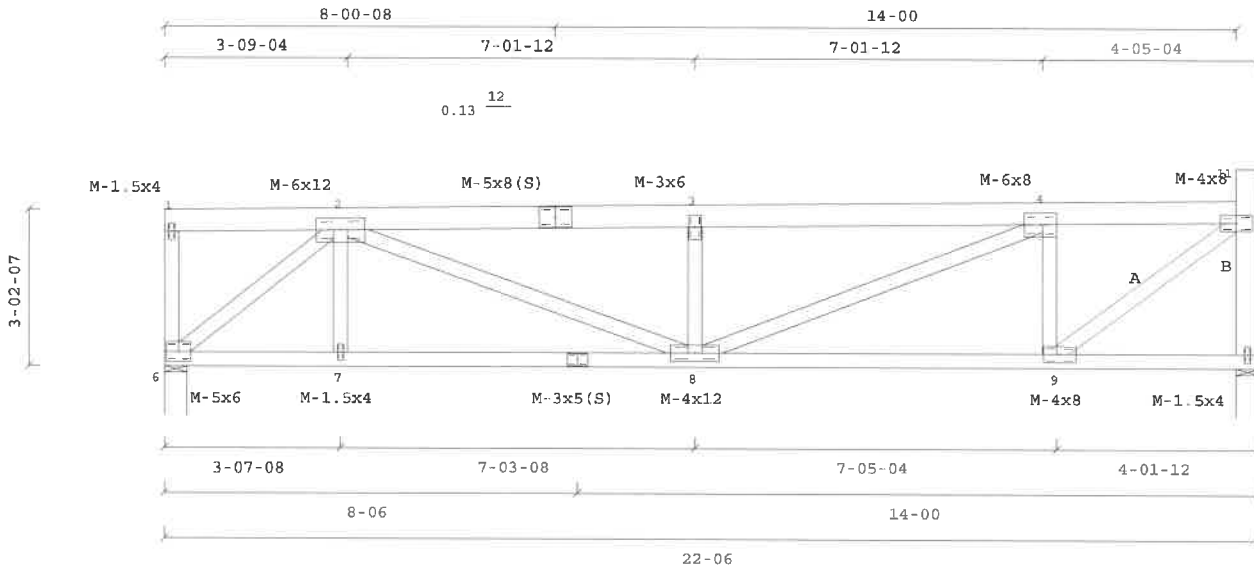
MAX HORIZ. LL DEFL = 0.030" @ 22'- 3.3"
MAX HORIZ. TL DEFL = 0.057" @ 22'- 3.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDF=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900



JOB NAME: WESTMAN MILL - A13S

Scale: 0.2650

Truss: A13S

DATE: 5/1/2019
SEQ.: K6059047
TRANS ID: LINK

WARNINGS:

- Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
 - 2x4 compression web bracing must be installed where shown +.
 - Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
 - No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 - CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 - This design is furnished subject to the limitations set forth by TP/NWCA in BCSI, copies of which will be furnished upon request.
- MITek USA, Inc./CompuTrus Software 7.8.8(1L)-E

GENERAL NOTES, unless otherwise noted:

- This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
- Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
- 2x Impact bridging or lateral bracing required where shown + +
- Installation of truss is the responsibility of the respective contractor.
- Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
- Design assumes full bearing at all supports shown. Shim or wedge if necessary.
- Design assumes adequate drainage is provided.
- Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
- Digits indicate size of plate in inches.
- For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



May 2, 2019

This design prepared from computer input by
RFS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 56"OC. UON.

TRUSS SPAN 22'- 6.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 120.75 lb

COND. 2: Design checked for 50LBS sprinkler load
located at any point along top chord
except end panels and overhangs.
Additional 250LBS vertical load applied non-concurrently
at each sprinkler location.
COND. 3: 370.00 PLF SEISMIC LOAD.
SHEARWALL 0.00 to 22.50

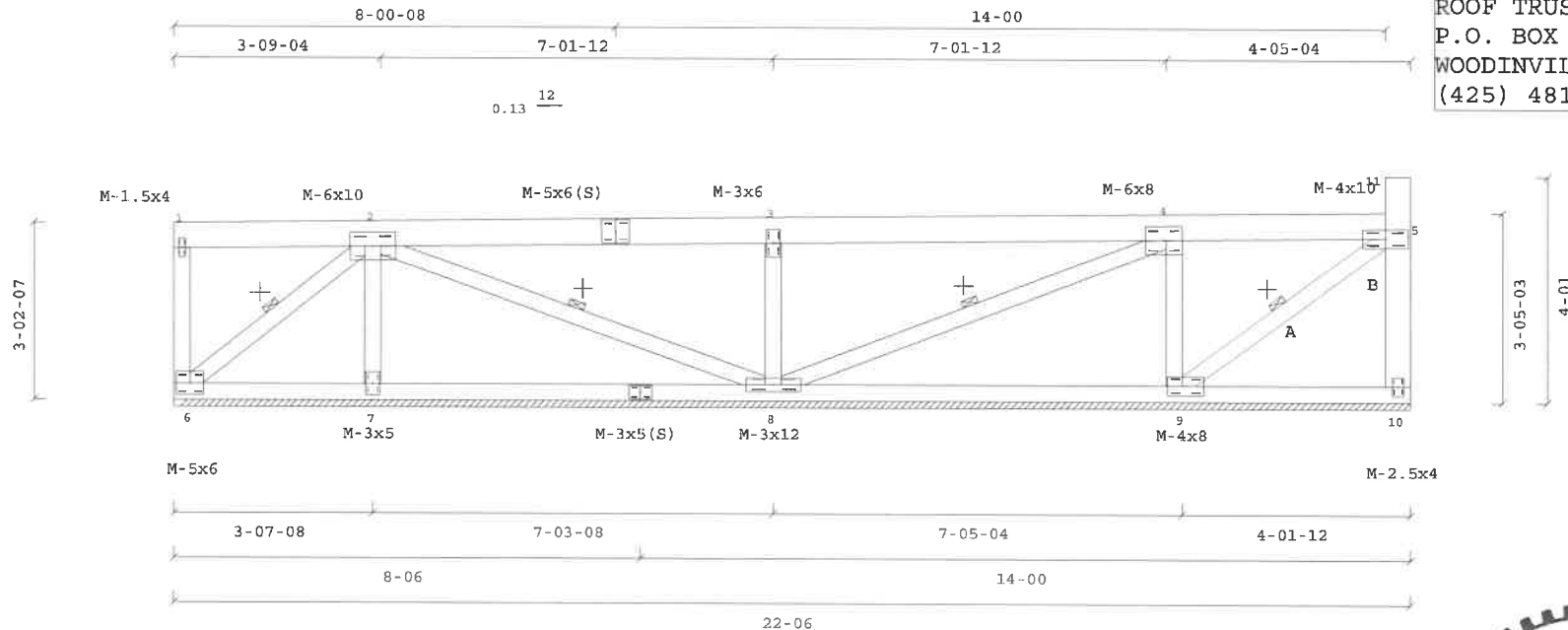
NOTE: TRUSS DESIGN ASSUMES UNIFORM
SHEAR TRANSFER

Wind: 110 mph, h=61.7ft, TCDF=13.2,BCDF=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
Only right end vertical is exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

Note:Truss design requires continuous
bearing wall for entire span UON.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

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concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A14S

Scale: 0.3038

Truss: A14S

DATE: 5/1/2019
SEQ.: K6059048
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCA in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR 1311, ESR-1988 (MITek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 25'- 6.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 |
|--------------|------------|---------------|-----------------|
| 1- 2=(-1994) | 0 | 6- 7=(-36) | 95 |
| 2- 3=(-4032) | 0 | 7- 8=(0) | 2151 |
| 3- 4=(-4031) | 0 | 8- 9=(0) | 3300 |
| 4- 5=(-3187) | 0 | 9-11=(-10) | 125 |
| | | 6- 1=(-1817) | 0 |
| | | 1- 7=(0) | 2546 |
| | | 7- 2=(-1572) | 0 |
| | | 2- 8=(0) | 2152 |
| | | 3- 8=(-1044) | 0 |
| | | 8- 4=(0) | 998 |
| | | 4- 9=(-1212) | 0 |
| | | 9- 5=(0) | 3348 |
| | | 11- 5=(-1690) | 0 |
| | | 5-10=(0) | 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. | (SPECIES) |
|-------------------|--------------------|--------------------|----------|---------------------------|-----------|
| 0'- 0.0" | 0/ 1832V | -62/ 72H | 5.50" | 4.52 | HF (405) |
| 25'- 6.0" | 0/ 1753V | 0/ 0H | 5.50" | 4.33 | HF (405) |

Weight: 137.75 lb

VERTICAL DEPLETION LIMITS: LL=L/360, TL=L/240
MAX LL DEPL = -0.196" @ 10'- 11.0" Allowed = 0.819"
MAX DL DEPL = -0.170" @ 10'- 11.0" Allowed = 1.229"
MAX TL DEPL = -0.366" @ 10'- 11.0" Allowed = 1.229"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

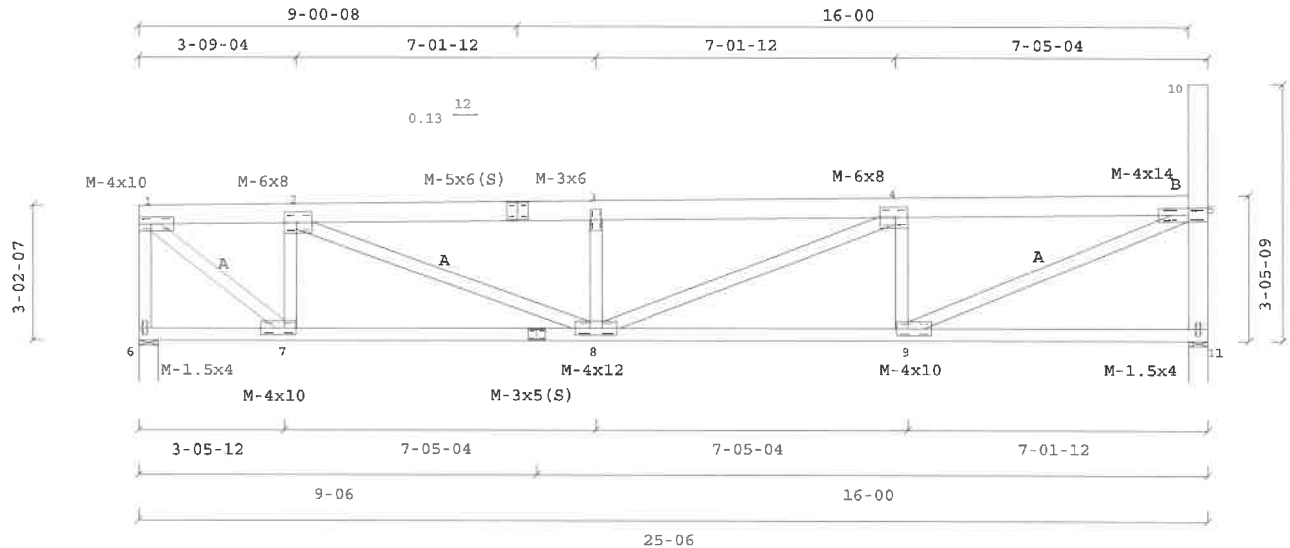
MAX HORIZ. LL DEFL = 0.037" @ 25'- 3.3"
MAX HORIZ. TL DEFL = 0.070" @ 25'- 3.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TC DL=13.2, BC DL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat. 2, Exp. B, MWFRS (Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

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(425) 481-0900



JOB NAME: WESTMAN MILL - A15S

Scale: 0.2284

Truss: A15S

DATE: 5/1/2019
SEQ.: K6059049
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
 2. 2x4 compression web bracing must be installed where shown +.
 3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 6. This design is furnished subject to the limitations set forth by TPJWTCGA in BCSI, copies of which will be furnished upon request.
- MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 7' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF SS
BC: 2x4 DF #1&BTR;
2x6 HF #2 B1
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x4 DF #1&BTR B;
2x6 HF #2 C

TC LATERAL SUPPORT <= 12*OC. UON.
BC LATERAL SUPPORT 120*OC. UON.

TRUSS SPAN 29'- 2.0"
LOAD DURATION INCREASE = 1.15 (Non-Rep)
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)
ADDL: BC CONC LL+DL= 1520.0 LBS @ 3'- 6.5"

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|--------------|---|-------------------|------|-----------------|------|
| 1- 2=(-4029) | 0 | 7- 8=(-28) | 174 | 7- 1=(-3300) | 0 |
| 2- 3=(-4029) | 0 | 8- 9=(0) | 5428 | 1- 8=(0) | 4912 |
| 3- 4=(-6113) | 0 | 9-10=(0) | 5424 | 2- 8=(-579) | 0 |
| 4- 5=(-6112) | 0 | 10-11=(0) | 4290 | 8- 3=(-1891) | 0 |
| 5- 6=(-4135) | 0 | 11-12=(-9) | 134 | 9- 3=(0) | 210 |
| | | | | 3-10=(0) | 926 |
| | | | | 4-10=(-1060) | 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ RREACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. (SPECIES) |
|-------------------|--------------------|----------------------|----------|-------------------------------------|
| 0'- 0.0" | 0/ 3406V | -61/ 72H | 5.50" | 8.41 HF (405) |
| 29'- 2.0" | 0/ 2170V | 0/ 0H | 5.50" | 3.47 DF (625) |

Weight: 165.28 lb

Caution: Note bearing area requirements.

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.257" @ 14'- 7.0" Allowed = 0.942"
MAX DL DEFL = -0.313" @ 14'- 7.0" Allowed = 1.413"
MAX TL DEFL = -0.570" @ 14'- 7.0" Allowed = 1.413"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

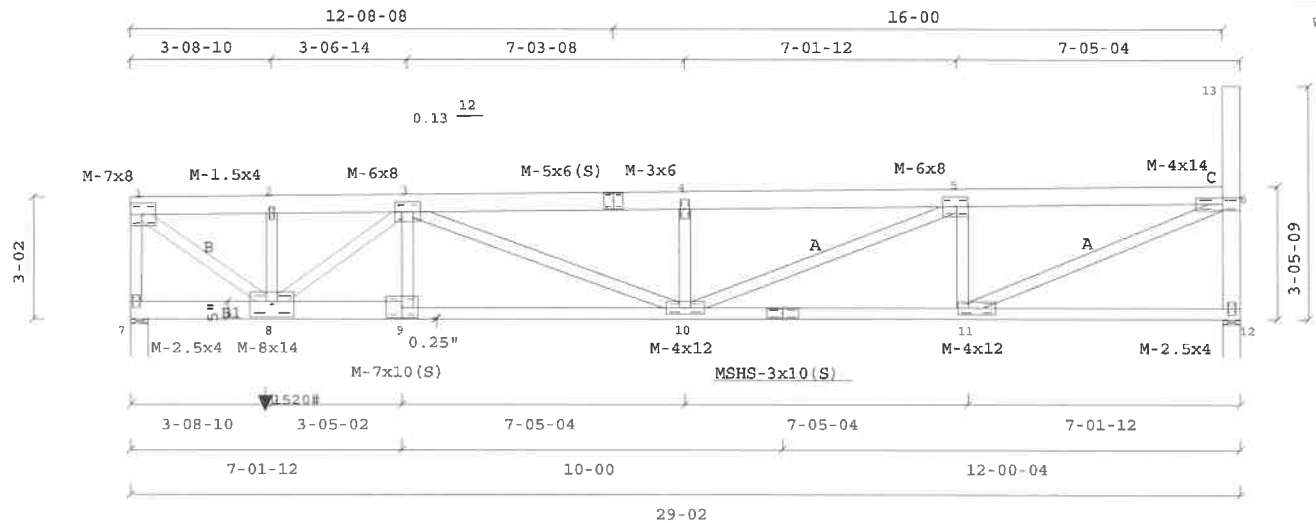
MAX HORIZ. LL DEFL = 0.046" @ 28'- 11.3"
MAX HORIZ. TL DEFL = 0.109" @ 28'- 11.3"

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDD=13.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWPRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

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(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



JOB NAME: WESTMAN MILL - A16S

Scale: 0.2074

Truss: A16S

DATE: 5/1/2019
SEQ.: K6059050
TRANS ID: LINK

WARNINGS:

- Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
- 2x4 compression web bracing must be installed where shown +.
- Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
- No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
- CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
- This design is furnished subject to the limitations set forth by TPJWTCAL in BCSI, copies of which will be furnished upon request.

MiTek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

- This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
- Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TO) and/or drywall(BG).
- 2x Impact bridging or lateral bracing required where shown ++
- Installation of truss is the responsibility of the respective contractor.
- Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
- Design assumes full bearing at all supports shown. Shim or wedge if necessary.
- Design assumes adequate drainage is provided.
- Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
- Digits indicate size of plate in inches.
- For basic connector plate design values see ESR-1311, ESR-1988 (MiTek)



May 2, 2019

LUMBER SPECIFICATIONS

TC: 2x6 HF #2
BC: 2x4 HF #2;
2x6 HF SS B1
WEBS: 2x4 HF STD/STUD:
2x6 HF #2 A

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 27'- 2.0"
LOAD DURATION INCREASE = 1.15 (Non-Rep)
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)
ADDL: BC CONC LL+DL= 1695.0 LBS @ 3'- 5.7"

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|----------------|-----------------|-------------------|------------------|-----------------|--|
| 1- 2=(-1916) 0 | 7- 8=(-42) 178 | 7- 1=(-3412) | 0 | 10- 5=(0) 1726 | |
| 2- 3=(-1918) 0 | 8- 9=(0) 4272 | 1- 8=(0) 3750 | 5-11=(-1328) 72 | | |
| 3- 4=(-4976) 0 | 9-10=(0) 4216 | 2- 8=(-394) 16 | 11- 6=(0) 3636 | | |
| 4- 5=(-4976) 0 | 10-11=(0) 3564 | 8- 3=(-3070) 0 | 12- 6=(-1746) 24 | | |
| 5- 6=(-3438) 0 | 11-12=(-22) 112 | 9- 3=(0) 1018 | 6-13=(0) 0 | | |
| | | 3-10=(-126) 968 | | | |
| | | 4-10=(-918) 82 | | | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. (SPECIES) |
|-------------------|--------------------|--------------------|----------|-------------------------------------|
| 0'- 0.0" | 0/ 3175V | -61/ 72H | 5.50" | 7.84 HF (405) |
| 27'- 2.0" | 0/ 1809V | 0/ 0H | 5.50" | 4.47 HF (405) |

Weight: 302.61 lb

VERTICAL DEFLECTION LIMITS: $L_L=L/360$, $T_L=L/240$
 MAX L_L DEFL = -0.121" @ 12'- 7.0" Allowed = 0.875"
 MAX D_L DEFL = -0.125" @ 12'- 7.0" Allowed = 1.312"
 MAX T_L DEFL = -0.245" @ 12'- 7.0" Allowed = 1.312"
 SEASONED LUMBER IN DRY SERVICE CONDITIONS

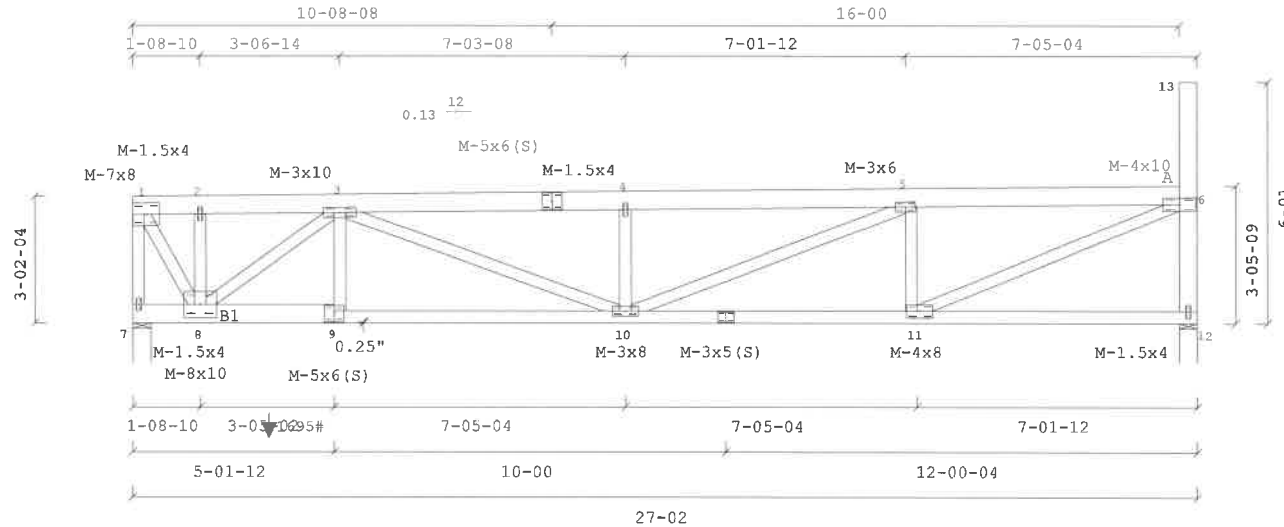
MAX HORIZ. L_L DEFL = 0.024" @ 26'- 11.3"
 MAX HORIZ. T_L DEFL = 0.055" @ 26'- 11.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDL=7.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

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JOB NAME: WESTMAN MILL - A17

Scale: 0.2136

Truss: A17

DATE: 1/24/2019
SEQ.: K5650400
TRANS ID: LINK

WARNINGS:

- Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
- 2x4 compression web bracing must be installed where shown +.
- Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
- No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
- CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
- This design is furnished subject to the limitations set forth by TPJ/WTC in BCSL copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.8.8(1L)-E

GENERAL NOTES. Unless otherwise noted:

- This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
- Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
- 2x Impact bridging or lateral bracing required where shown + +.
- Installation of truss is the responsibility of the respective contractor.
- Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
- Design assumes full bearing at all supports shown. Shim or wedge if necessary.
- Design assumes adequate drainage is provided.
- Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
- Digits indicate size of plate in inches.
- For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 24'- 2.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL (30.0)+DL (12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER FORCES | 4WR/DHF/Cq=1.00 |
|--------------|-------------------|-----------------|
| 1- 2=(-2197) | 0 | 6- 7=(-45) 78 |
| 2- 3=(-3228) | 0 | 7- 8=(0) 2306 |
| 3- 4=(-3227) | 0 | 8- 9=(0) 1864 |
| 4- 5=(-1748) | 13 | 9-10=(-21) 33 |
| | | 6- 1=(-1446) 17 |
| | | 7- 1=(0) 2453 |
| | | 8- 2=(-1102) 75 |
| | | 9- 3=(0) 1156 |
| | | 10- 4=(-1441) 9 |
| | | 11- 5=(0) 84 |
| | | 12- 6=(0) 1611 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. (SPECIES) |
|-------------------|--------------------|---------------------|----------|------------------------------------|
| 0'- 0.0" | 0/ 1485V | -61/ 71H | 5.50" | 3.67 HF (405) |
| 24'- 2.0" | 0/ 1470V | 0/ 0H | 5.50" | 3.63 HF (405) |

Weight: 126.84 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.168" @ 12'- 7.0" Allowed = 0.775"
MAX DL DEFL = -0.100" @ 12'- 7.0" Allowed = 1.163"
MAX TL DEFL = -0.268" @ 12'- 7.0" Allowed = 1.163"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

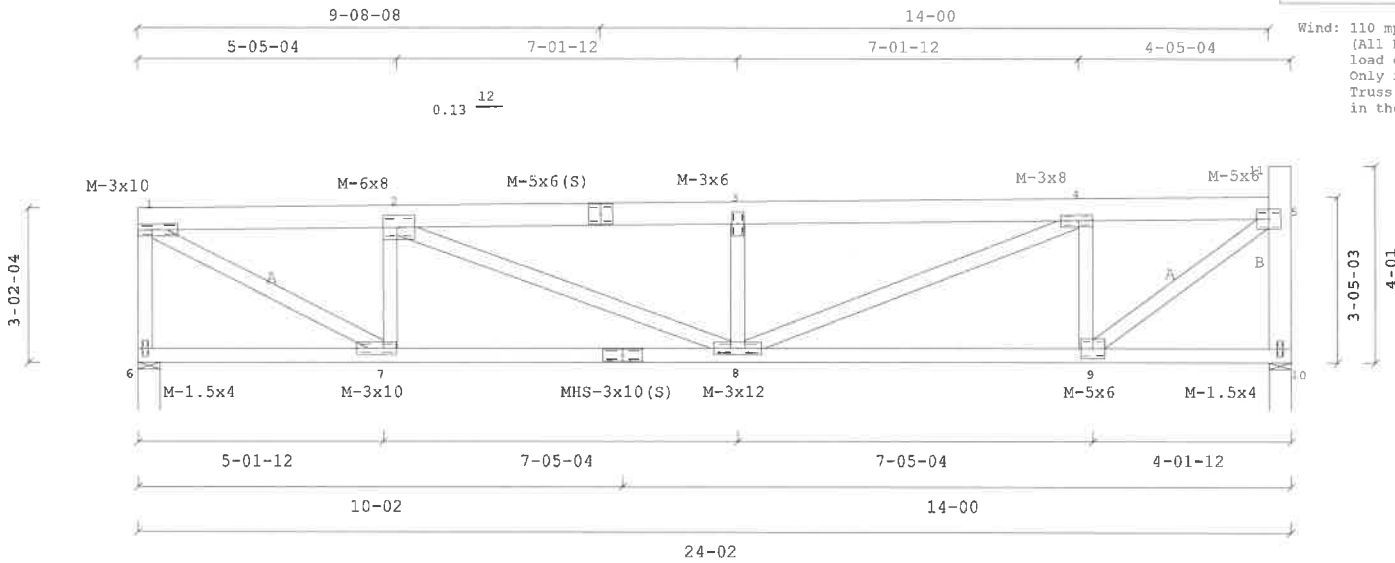
MAX HORIZ. LL DEFL = 0.032" @ 23'- 11.3"
MAX HORIZ. TL DEFL = 0.053" @ 23'- 11.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TC DL=7.2, BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS (Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

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JOB NAME: WESTMAN MILL - A18

Scale: 0.2599

Truss: A18

DATE: 1/25/2019
SEQ.: K5650401
TRANS ID: LINK

- WARNINGS:**
1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
 2. 2x4 compression web bracing must be installed where shown +.
 3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 5. CompuTruss has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 6. This design is furnished subject to the limitations set forth by TPI/WTC in BCS, copies of which will be furnished upon request.
- MITek USA, Inc./CompuTruss Software 7.6.8(1L)-E
- GENERAL NOTES, unless otherwise noted:**
1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
 2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
 3. 2x Impact bridging or lateral bracing required where shown + +
 4. Installation of truss is the responsibility of the respective contractor.
 5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
 6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
 7. Design assumes adequate drainage is provided.
 8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
 9. Digits indicate size of plate in inches.
 10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS

TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 25'-10.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER | FORCES | 4WR/DHF/Cq=1.00 |
|----------------|----------------|-----------------|------------------|
| 1- 2=(-1875) 0 | 6- 7=(-44) 83 | 6- 1=(-1571) 5 | 4- 9=(-1046) 77 |
| 2- 3=(-3575) 0 | 7- 8=(0) 2008 | 1- 7=(0) 2305 | 9- 5=(0) 2942 |
| 3- 4=(-3574) 0 | 8- 9=(0) 2892 | 7- 2=(-1322) 67 | 11- 5=(-1462) 29 |
| 4- 5=(-2794) 2 | 9-11=(-23) 103 | 2- 8=(0) 1817 | 5-10=(0) 0 |
| | | 3- 8=(-897) 81 | |
| | | 8- 4=(0) 943 | |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. (SPECIES) |
|-------------------|--------------------|---------------------|----------|------------------------------------|
| 0'- 0.0" | 0/ 1592V | -62/ 72H | 5.50" | 3.93 HF (405) |
| 25'- 10.0" | 0/ 1524V | 0/ 0H | 5.50" | 3.76 HF (405) |

Weight: 136.17 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.207" @ 11'- 3.0" Allowed = 0.831"
MAX DL DEFL = -0.126" @ 11'- 3.0" Allowed = 1.246"
MAX TL DEFL = -0.332" @ 11'- 3.0" Allowed = 1.246"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

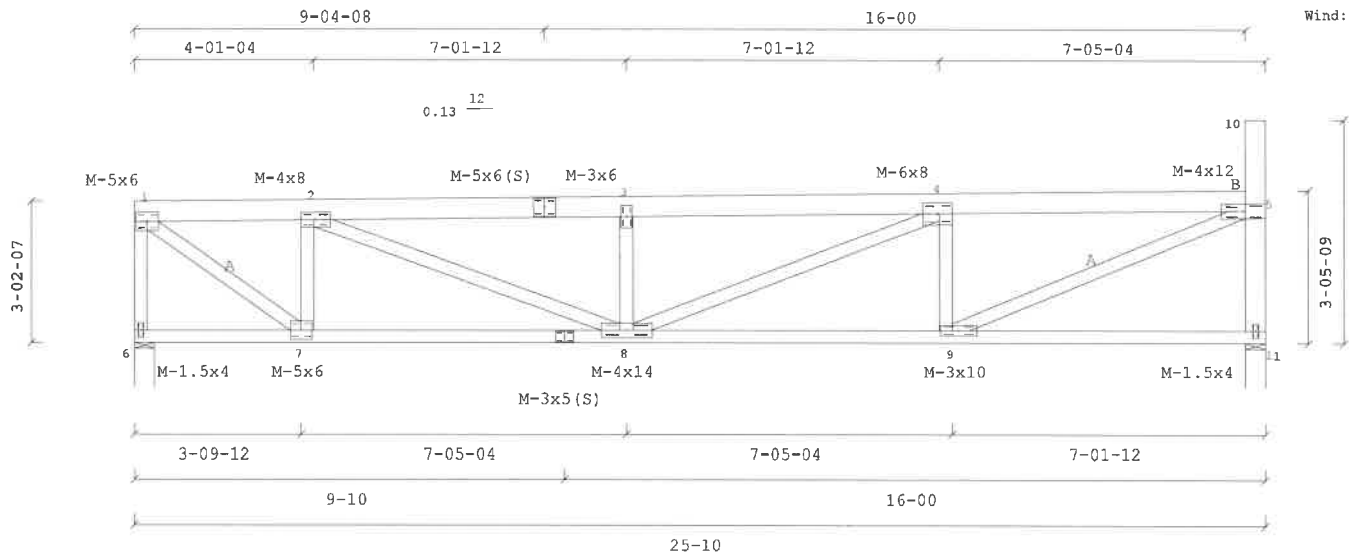
MAX HORIZ. LL DEFL = 0.038" @ 25'- 7.3"
MAX HORIZ. TL DEFL = 0.063" @ 25'- 7.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=7.2, BCDF=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

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(425) 481-0900



JOB NAME: WESTMAN MILL - A19

Scale: 0.2386

Truss: A19

DATE: 1/24/2019
SEQ.: K5650402
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTC in BCST, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.8.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD;
2x4 HF #2 A;
2x6 HF #2 B

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

TRUSS SPAN 22'- 10.0"
LOAD DURATION INCREASE = 1.15
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | MAX MEMBER FORCES | 4WR/DHF/Cq=1.00 |
|--------------|-------------------|------------------|
| 1- 2=(-1658) | 0 | 6- 7=(-45) 76 |
| 2- 3=(-2931) | 0 | 7- 8=(0) 1773 |
| 3- 4=(-2930) | 0 | 8- 9=(0) 1758 |
| 4- 5=(-1649) | 14 | 9-10=(-22) 32 |
| | | 6- 1=(-1408) 7 |
| | | 1- 7=(0) 2037 |
| | | 7- 2=(-1155) 68 |
| | | 2- 8=(0) 1384 |
| | | 3- 8=(-911) 85 |
| | | 8- 4=(0) 1407 |
| | | 4- 9=(-1153) 58 |
| | | 9- 5=(0) 2071 |
| | | 10- 5=(-1369) 10 |
| | | 5-11=(0) 0 |

| BEARING LOCATIONS | MAX VERT REACTIONS | MAX HORIZ REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. | (SPECIES) |
|-------------------|--------------------|---------------------|----------|--------------------------|-----------|
| 0'- 0.0" | 0/ 1430V | -61/ 71H | 5.50" | 3.53 | HF (405) |
| 22'- 10.0" | 0/ 1397V | 0/ 0H | 5.50" | 3.45 | HF (405) |

Weight: 120.49 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.141" @ 11'- 3.0" Allowed = 0.731"
MAX DL DEFL = -0.083" @ 11'- 3.0" Allowed = 1.096"
MAX TL DEFL = -0.224" @ 11'- 3.0" Allowed = 1.096"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

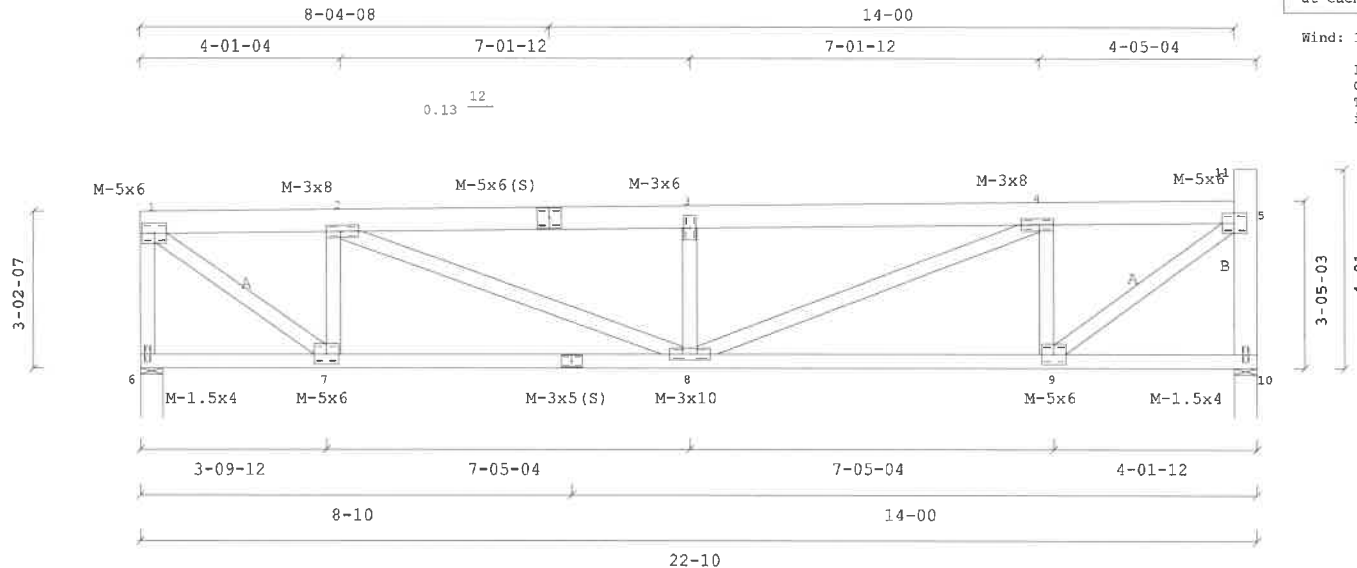
MAX HORIZ. LL DEFL = 0.027" @ 22'- 7.3"
MAX HORIZ. TL DEFL = 0.044" @ 22'- 7.3"

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.

COND. 2: Design checked for 50LBS sprinkler load located at any point along top chord except end panels and overhangs. Additional 250LBS vertical load applied non-concurrently at each sprinkler location.

Wind: 110 mph, h=61.7ft, TCDF=7.2, BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, Only right end vertical is exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
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(425) 481-0900



JOB NAME: WESTMAN MILL - A20

Scale: 0.2665

Truss: A20

DATE: 1/24/2019
SEQ.: K5650403
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJW/TCA in BCST, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES. unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 7' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITak)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

Plateline to peak (L): 6-03-10
Plateline to peak (R): 3-02-11

TRUSS SPAN 5'- 5.0"
LOAD DURATION INCREASE = 1.15 (Non-Rep)
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(22.0) ON TOP CHORD = 52.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 62.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

IBC 2015 MAX MEMBER FORCES 4WR/DHF/Cq=1.00
1-2=(0) 17 3-4=(0) 14 1-3=(-274) 0 2-4=(-273) 0
3-2=(-33) 0

BEARING
LOCATIONS
0'- 0.0"
5'- 5.0"

MAX VERT REACTIONS
0/ 346V
0/ 325V

MAX HORZ REACTIONS
0/ 1H
0/ 0H

BRG SIZE
3.50"

REQUIRED BRG AREA
SQ.IN. (SPECIES)
0.85 HF (405)
0.80 HF (405)

Weight: 29.04 lb

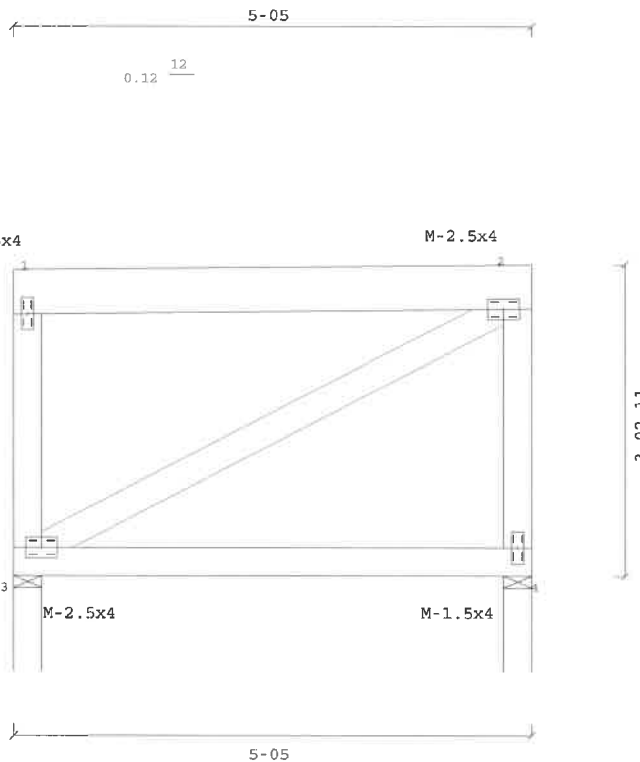
VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX TL DEFL = -0.001" @ 5'- 1.5" Allowed = 0.242"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.000" @ 5'- 3.2"
MAX HORIZ. TL DEFL = 0.000" @ 5'- 3.2"

Wind: 110 mph, h=61.6ft, TC DL=13.2, BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS (Dir),
load duration factor=1.6,
End vertical(s) not exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

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concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - A21S

Scale: 0.5215

Truss: A21S

DATE: 5/1/2019
SEQ.: K6059051
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCGA in BCSI, copies of which will be furnished upon request.

MiTek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MiTek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x6 HF #2
BC: 2x4 HF #2
WEBS: 2x4 HF STD/STUD

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

Plateline to peak (L): 4-08-06
Plateline to peak (R): 3-02-11

TRUSS SPAN 3'- 5.0"
LOAD DURATION INCREASE = 1.15 (Non-Rep)
SPACED 24.0" O.C.

LOADING
LL(30.0)+DL(12.0) ON TOP CHORD = 42.0 PSF
DL ON BOTTOM CHORD = 10.0 PSF
TOTAL LOAD = 52.0 PSF

LL = 30 PSF Ground Snow (Pg)

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

IBC 2015 MAX MEMBER FORCES 4WR/DHF/Cq=1.00
1-2=(0) 8 3-4=(0) 7 1-3=(-137) 18 2-4=(-135) 18
3-2=(-20) 1

| BEARING LOCATIONS | MAX VERT REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. | BRG AREA (SPECIES) |
|-------------------|--------------------|----------|--------------------------|--------------------|
| 0'- 0.0" | 0/ 187V | 3.50" | 0.46 | HF (405) |
| 3'- 5.0" | 0/ 168V | 3.50" | 0.42 | HF (405) |

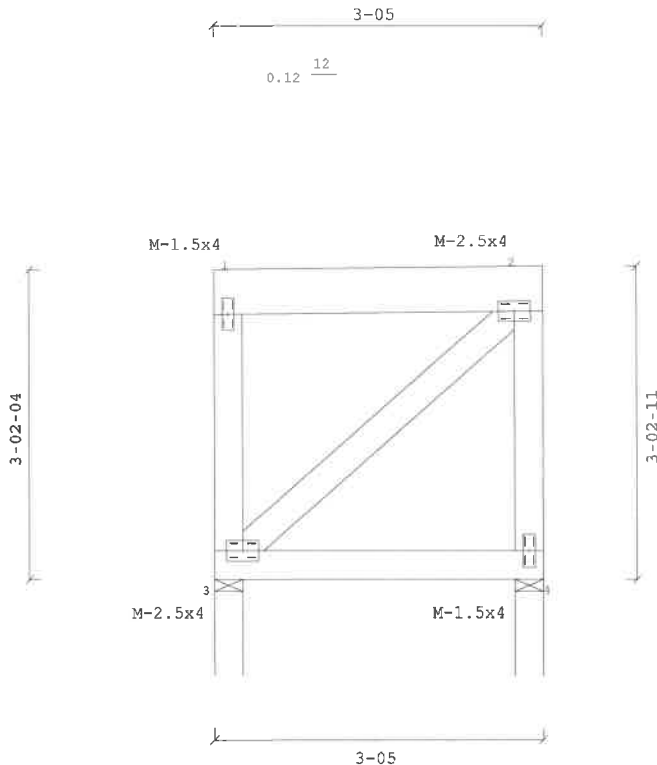
Weight: 21.11 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX TC PANEL LL DEFL = -0.006" @ 1'- 7.6" Allowed = 0.174"
MAX BC PANEL TL DEFL = -0.012" @ 1'- 9.4" Allowed = 0.174"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.000" @ 3'- 3.2"
MAX HORIZ. TL DEFL = 0.000" @ 3'- 3.2"

Wind: 110 mph, h=61.6ft, TCCL=7.2,BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
End vertical(s) not exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900



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structural engineer.

JOB NAME: WESTMAN MILL - A22

Scale: 0.5243

Truss: A22

DATE: 1/24/2019
SEQ.: K5650405
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPI/WTCA in BCS, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES. unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown ++
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1989 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x4 HF #2
BC: 2x6 HF #2
WEBS: 2x4 HF STD/STUD

3-10-08 GIRDER SUPPORTING 23-07-08 FROM 0-11-04 TO 2-11-04
3-10-08 GIRDER SUPPORTING 5-05-00
LOAD DURATION INCREASE = 1.15 (Non-Rep)

IBC 2015 MAX MEMBER FORCES 4WR/DHF/Cq=1.00
1-2=(-341) 0 3-4=(0) 35 3-1=(-692) 0 2-5=(-692) 0
4-5=(0) 35 1-4=(0) 611
4-2=(0) 611

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

LOADING
LL = 30 PSF Ground Snow (Pg)
TC UNIF LL(60.0)+DL(24.0)= 84.0 PLF 0'- 0.0" TO 3'- 10.5" V
BC UNIF LL(0.0)+DL(20.0)= 20.0 PLF 0'- 0.0" TO 0'- 11.2" V
BC UNIF LL(324.4)+DL(257.9)= 582.2 PLF 0'- 11.2" TO 2'- 11.2" V
BC UNIF LL(0.0)+DL(20.0)= 20.0 PLF 2'- 11.2" TO 3'- 10.5" V
BC UNIF LL(51.3)+DL(37.6)= 88.8 PLF 0'- 0.0" TO 3'- 10.5" V

BEARING MAX VERT BRG REQUIRED BRG AREA
LOCATIONS REACTIONS SIZE SQ. IN. (SPECIES)
0'- 0.0" 0/ 936V 3.50" 2.31 HF (405)
3'- 10.5" 0/ 936V 3.50" 2.31 HF (405)

Plateline to peak: 3-02-10

Single member as shown.
Hangers attached to the bottom chord
will have 1.5" max. nail penetration.

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 25.78 lb

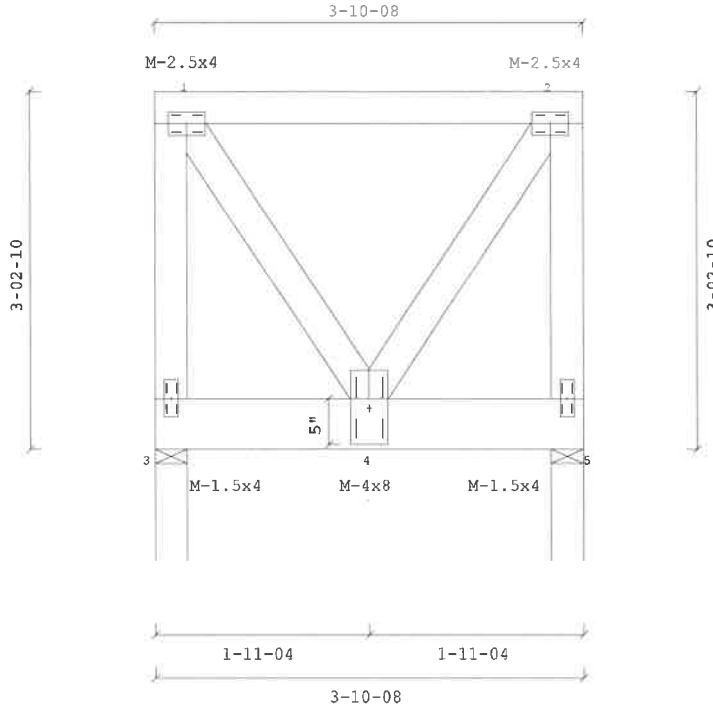
VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.005" @ 1'- 11.2" Allowed = 0.110"
MAX DL DEFL = -0.004" @ 1'- 11.2" Allowed = 0.165"
MAX TL DEFL = -0.009" @ 1'- 11.2" Allowed = 0.165"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.000" @ 3'- 8.8"
MAX HORIZ. TL DEFL = 0.000" @ 3'- 8.8"

Wind: 110 mph, h=21.6ft, TCDL=7.2,BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
End vertical(s) not exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

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the supplier designed components is
the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - X1

Scale: 0.6026

Truss: X1

DATE: 1/24/2019
SEQ.: K5650420
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TP/W/TCA in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x4 HF #2
BC: 2x6 HF #2
WEBS: 2x4 HF STD/STUD;
2x8 HF #2 A

8-11-00 GIRDER SUPPORTING 22-07-08 FROM 0-00-00 TO 4-00-00
8-11-00 GIRDER SUPPORTING 25-07-08 FROM 6-00-00 TO 8-11-00
LOAD DURATION INCREASE = 1.15 (Non-Rep)

IBC 2015 MAX MEMBER FORCES 4WR/DHF/Cq=1.00
1-2=(0) 8 4-5=(0) 36 1-4=(-184) 0 2-6=(-54) 0
2-3=(0) 8 5-6=(0) 36 4-2=(-54) 0 6-3=(-184) 0
5-2=(-496) 0

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

LOADING
LL = 30 PSF Ground Snow (Pg)
TC UNIF LL(60.0)+DL(44.0)= 104.0 PLF 0'- 0.0" TO 8'- 11.0" V
BC UNIF LL(309.4)+DL(350.0)= 659.4 PLF 0'- 0.0" TO 4'- 0.0" V
BC UNIF LL(0.0)+DL(20.0)= 20.0 PLF 4'- 0.0" TO 8'- 11.0" V
BC UNIF LL(354.4)+DL(378.0)= 732.4 PLF 6'- 0.0" TO 8'- 11.0" V

| BEARING LOCATIONS | MAX VERT REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ. IN. | (SPECIES) |
|-------------------|--------------------|----------|---------------------------|-----------|
| 0'- 0.0" | 0/ 1490V | 3.50" | 3.68 | HF (405) |
| 4'- 5.5" | 0/ 2791V | 7.00" | 6.89 | HF (405) |
| 8'- 11.0" | 0/ 1518V | 3.50" | 3.75 | HF (405) |

Plateline to peak: 3-02-06



(2) complete trusses required.
Attach 2 ply with 3"x.131 DIA GUN nails staggered:
9" oc in 1 row(s) throughout 2x4 top chords,
5" oc in 2 row(s) throughout 2x6 bottom chords,
9" oc in 1 row(s) throughout 2x4 webs,
9" oc in 2 row(s) throughout 2x8 webs.

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

PROVIDE FULL BEARING: Truss+4, 5, 6
Weight: 104.82 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX BC PANEL LL DEFL = -0.015" @ 2'- 2.7" Allowed = 0.180"
MAX BC PANEL TL DEFL = -0.031" @ 2'- 2.7" Allowed = 0.239"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.000" @ 8'- 7.5"
MAX HORIZ. TL DEFL = 0.000" @ 8'- 7.5"

Wind: 110 mph, h=61.6ft, TCDF=13.2,BCDL=6.0, ASCE 7-10, (All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir), load duration factor=1.6, End vertical(s) not exposed to wind, Truss designed for wind loads in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for conformance of design criteria and concept. Structural performance of the supplier designed components is the responsibility of the components structural engineer.



JOB NAME: WESTMAN MILL - X2S

Scale: 0.5587

Truss: X2S

DATE: 5/1/2019
SEQ.: K6059052
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TPJ/WTCA in BCSI, copies of which will be furnished upon request.

MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown +.
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1588 (MITek)



May 2, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x4 HF #2
BC: 2x6 HF #2
WEBS: 2x4 HF STD/STUD

5-10-08 GIRDER SUPPORTING 23-07-08 FROM 0-11-04 TO 4-11-04
5-10-08 GIRDER SUPPORTING 5-05-00
LOAD DURATION INCREASE = 1.15 (Non-Rep)

IBC 2015 MAX MEMBER FORCES 4WR/DHF/Cq=1.00
1-2=(-1000) 0 3-4=(0) 64 3-1=(-1254) 0 2-5=(-1254) 0
4-5=(0) 64 1-4=(0) 1374
4-2=(0) 1374

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

LOADING
LL = 30 PSF Ground Snow (Pg)
TC UNIF LL(60.0)+DL(24.0)= 84.0 PLF 0'- 0.0" TO 5'- 10.5" V
BC UNIF LL(0.0)+DL(20.0)= 20.0 PLF 0'- 0.0" TO 0'- 11.2" V
BC UNIF LL(324.4)+DL(257.9)= 582.2 PLF 0'- 11.2" TO 4'- 11.2" V
BC UNIF LL(0.0)+DL(20.0)= 20.0 PLF 4'- 11.2" TO 5'- 10.5" V
BC UNIF LL(51.3)+DL(37.6)= 88.8 PLF 0'- 0.0" TO 5'- 10.5" V

BEARING MAX VERT BRG REQUIRED BRG AREA
LOCATIONS REACTIONS SIZE SQ.IN. (SPECIES)
0'- 0.0" 0/ 1691V 3.50" 4.18 HF (405)
5'- 10.5" 0/ 1691V 3.50" 4.18 HF (405)

Plateline to peak: 3-02-10

Single member as shown.
Hangers attached to the bottom chord
will have 1.5" max. nail penetration.

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

Weight: 34.77 lb

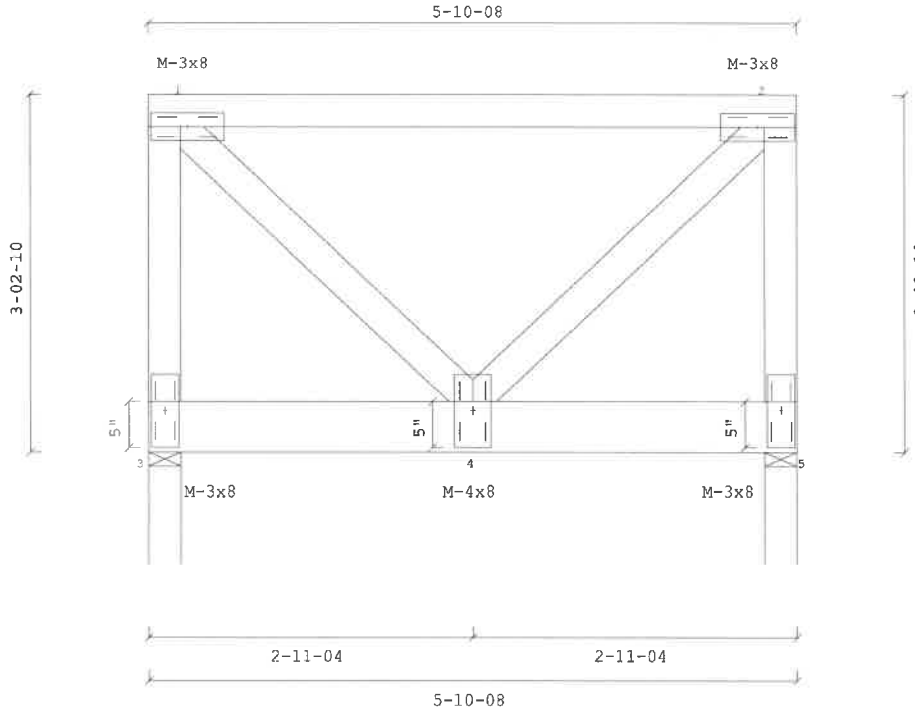
VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.014" @ 2'- 11.2" Allowed = 0.176"
MAX DL DEFL = -0.011" @ 2'- 11.2" Allowed = 0.265"
MAX TL DEFL = -0.025" @ 2'- 11.2" Allowed = 0.265"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.000" @ 5'- 8.8"
MAX HORIZ. TL DEFL = 0.000" @ 5'- 8.8"

Wind: 110 mph, h=21.6ft, TC DL=7.2, BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
End vertical(s) not exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
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(425) 481-0900

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the responsibility of the components
structural engineer.



JOB NAME: WESTMAN MILL - X3

Scale: 0.6016

Truss: X3

DATE: 1/24/2019
SEQ.: K5650422
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
2. 2x4 compression web bracing must be installed where shown +.
3. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer.
4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
6. This design is furnished subject to the limitations set forth by TP/W/TCA in BCSL copies of which will be furnished upon request.

MiTek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES, unless otherwise noted:

1. This design is based only upon the parameters shown and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer.
2. Design assumes the top and bottom chords to be laterally braced at 2' o.c. and at 10' o.c. respectively unless braced throughout their length by continuous sheathing such as plywood sheathing(TC) and/or drywall(BC).
3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
6. Design assumes full bearing at all supports shown. Shim or wedge if necessary.
7. Design assumes adequate drainage is provided.
8. Plates shall be located on both faces of truss, and placed so their center lines coincide with joint center lines.
9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MiTek)



January 25, 2019

This design prepared from computer input by
RTS - DENNIS

LUMBER SPECIFICATIONS
TC: 2x4 HF #2
BC: 2x6 HF #2
WEBS: 2x4 HF STD/STUD

9-09-00 GIRDER SUPPORTING 25-10-00
LOAD DURATION INCREASE = 1.15 (Non-Rep)

TC LATERAL SUPPORT <= 12"OC. UON.
BC LATERAL SUPPORT 120"OC. UON.

LOADING
LL = 30 PSF Ground Snow (Pg)
TC UNIF LL{ 60.0}+DL{ 24.0}= 84.0 PLF 0'- 0.0" TO 9'- 9.0" V
BC UNIF LL{ 357.5}+DL{ 282.2}= 639.7 PLF 0'- 0.0" TO 9'- 9.0" V

BOTTOM CHORD CHECKED FOR 10PSF LIVE LOAD. TOP
AND BOTTOM CHORD LIVE LOADS ACT NON-CONCURRENTLY.

| IBC 2015 | | MAX MEMBER FORCES | | 4WR/DHF/Cq=1.00 | |
|-------------|------|-------------------|------|-----------------|--------------------|
| 1-2=(| 0) 6 | 5-6=(0) | 2418 | 1-5=(-122) | 6 7-3=(|
| 2-3=(-2632) | 0 | 6-7=(0) | 2552 | 5-2=(-3372) | 0 3-8=(-3478) |
| 3-4=(| 0) 6 | 7-8=(0) | 2552 | 2-6=(| 0) 2072 8-4=(-130) |
| | | | | 6-3=(| 0) 112 |

| BEARING LOCATIONS | MAX VERT REACTIONS | BRG SIZE | REQUIRED BRG AREA SQ.IN. | (SPECIES) |
|-------------------|--------------------|----------|--------------------------|-----------|
| 0'- 0.0" | 0/ 3528V | 3.50" | 8.71 | HF (405) |
| 9'- 9.0" | 0/ 3528V | 3.50" | 8.71 | HF (405) |

Weight: 116.12 lb

VERTICAL DEFLECTION LIMITS: LL=L/360, TL=L/240
MAX LL DEFL = -0.021" @ 6'- 4.8" Allowed = 0.306"
MAX DL DEFL = -0.016" @ 6'- 4.8" Allowed = 0.458"
MAX TL DEFL = -0.037" @ 6'- 4.8" Allowed = 0.458"
SEASONED LUMBER IN DRY SERVICE CONDITIONS

MAX HORIZ. LL DEFL = 0.007" @ 9'- 5.5"
MAX HORIZ. TL DEFL = 0.013" @ 9'- 5.5"

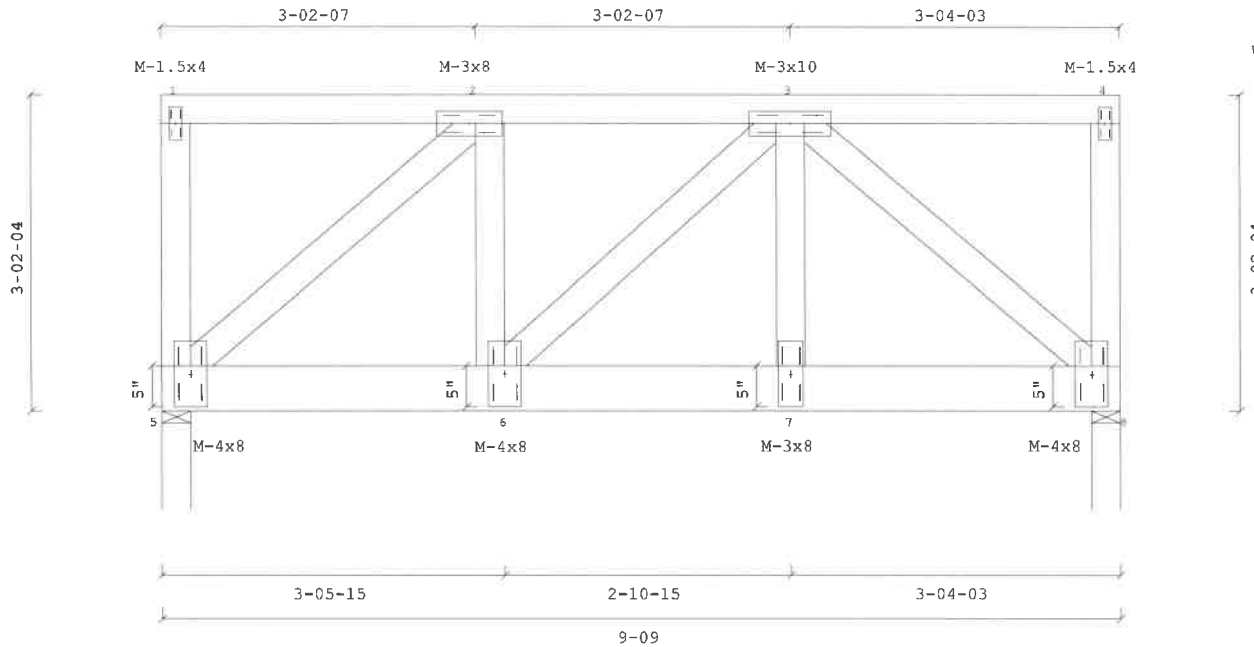
Wind: 110 mph, h=21.6ft, TCDF=7.2,BCDL=6.0, ASCE 7-10,
(All Heights), Enclosed, Cat.2, Exp.B, MWFRS(Dir),
load duration factor=1.6,
End vertical(s) not exposed to wind,
Truss designed for wind loads
in the plane of the truss only.

ROOF TRUSS SUPPLY, INC.
P.O. BOX 532
WOODINVILLE, WA 98072
(425) 481-0900

Checking by PCS is only for
conformance of design criteria and
concept. Structural performance of
the supplier designed components is
the responsibility of the components
structural engineer.

Plateline to peak: 3-02-04

(2) complete trusses required.
Attach 2 ply with 3"x.131 DIA GUN
nails staggered:
9" oc in 1 row(s) throughout 2x4 top chords,
6" oc in 2 row(s) throughout 2x6 bottom chords,
9" oc in 1 row(s) throughout 2x4 webs.



JOB NAME: WESTMAN MILL - X4

Scale: 0.5358

Truss: X4

DATE: 1/24/2019
SEQ.: K5650423
TRANS ID: LINK

WARNINGS:

1. Builder and erection contractor should be advised of all General Notes and Warnings before construction commences.
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 4. No load should be applied to any component until after all bracing and fasteners are complete and at no time should any loads greater than design loads be applied to any component.
 5. CompuTrus has no control over and assumes no responsibility for the fabrication, handling, shipment and installation of components.
 6. This design is furnished subject to the limitations set forth by TP/W/TGA in BCST, copies of which will be furnished upon request.
- MITek USA, Inc./CompuTrus Software 7.6.8(1L)-E

GENERAL NOTES. unless otherwise noted:

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3. 2x Impact bridging or lateral bracing required where shown + +
4. Installation of truss is the responsibility of the respective contractor.
5. Design assumes trusses are to be used in a non-corrosive environment, and are for "dry condition" of use.
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7. Design assumes adequate drainage is provided.
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9. Digits indicate size of plate in inches.
10. For basic connector plate design values see ESR-1311, ESR-1988 (MITek)



January 25, 2019



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architecture studios

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360.915.8775 | tasolympia.com

**TOWNZEN & ASSOCIATES
PLAN APPROVAL**

The plans submitted for review are approved in accordance with local state applicable standards. This approval does not relieve the applicant of the responsibility of compliance with the applicable codes.

Approved as submitted. 09/25/2019

**ROBISON
ENGINEERING, INC**

19401 40TH AVE W., SUITE 302
LYNNWOOD, WA 98036
206-364-3543 TEL.
CONTACT: JON ROBISON



Olympia
Building Plans Examiner
Community Planning & Development Department
601 4th Ave East
Olympia, WA 98501
(360) 753-6248
robison@ci.olympia.wa.us

**EAST BAY LOT A
WESTMAN MILL**
510 STATE AVE OLYMPIA, WA, 98501

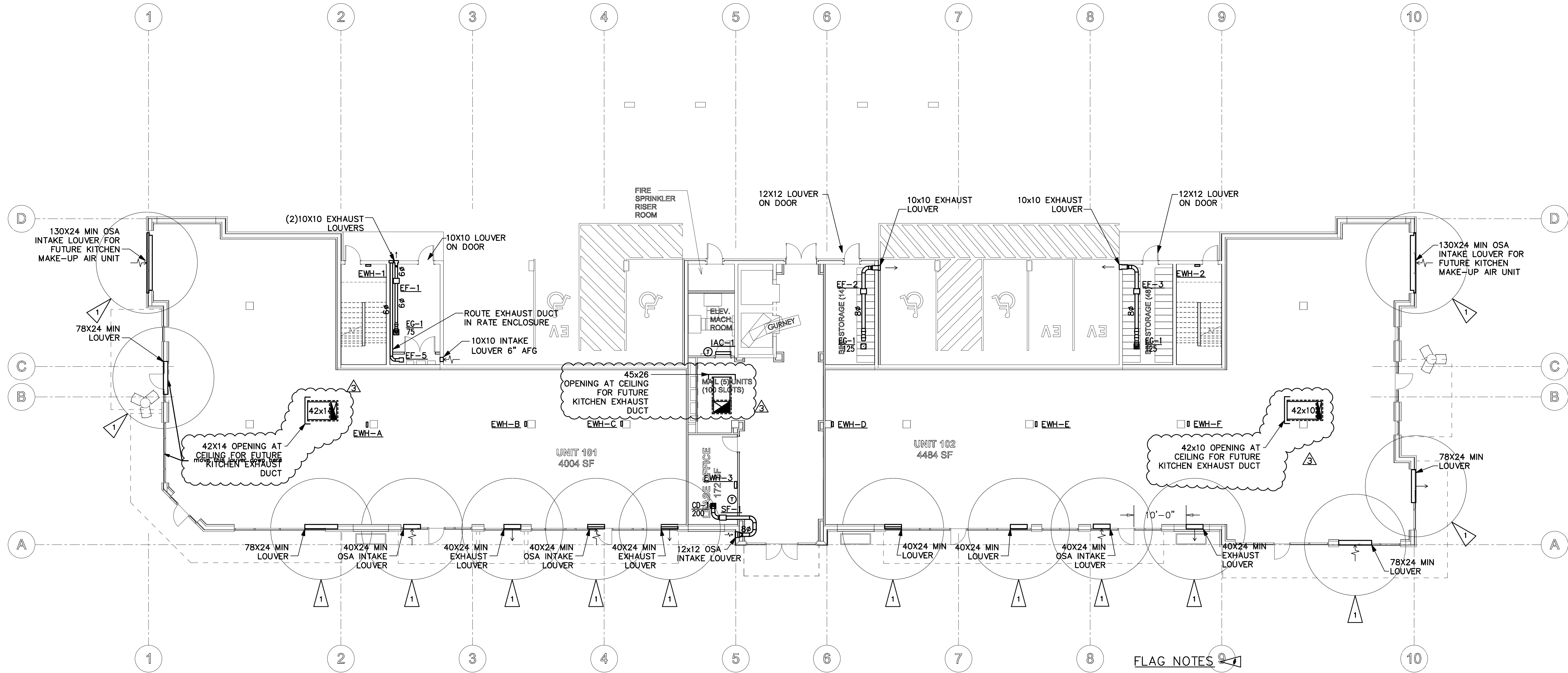
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |

MIXED USED
PLANS - LEVEL 1

M201

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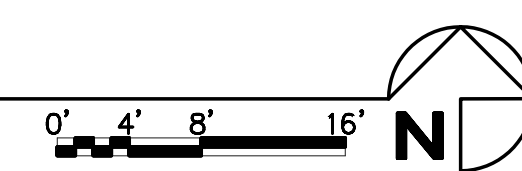


FLAG NOTES

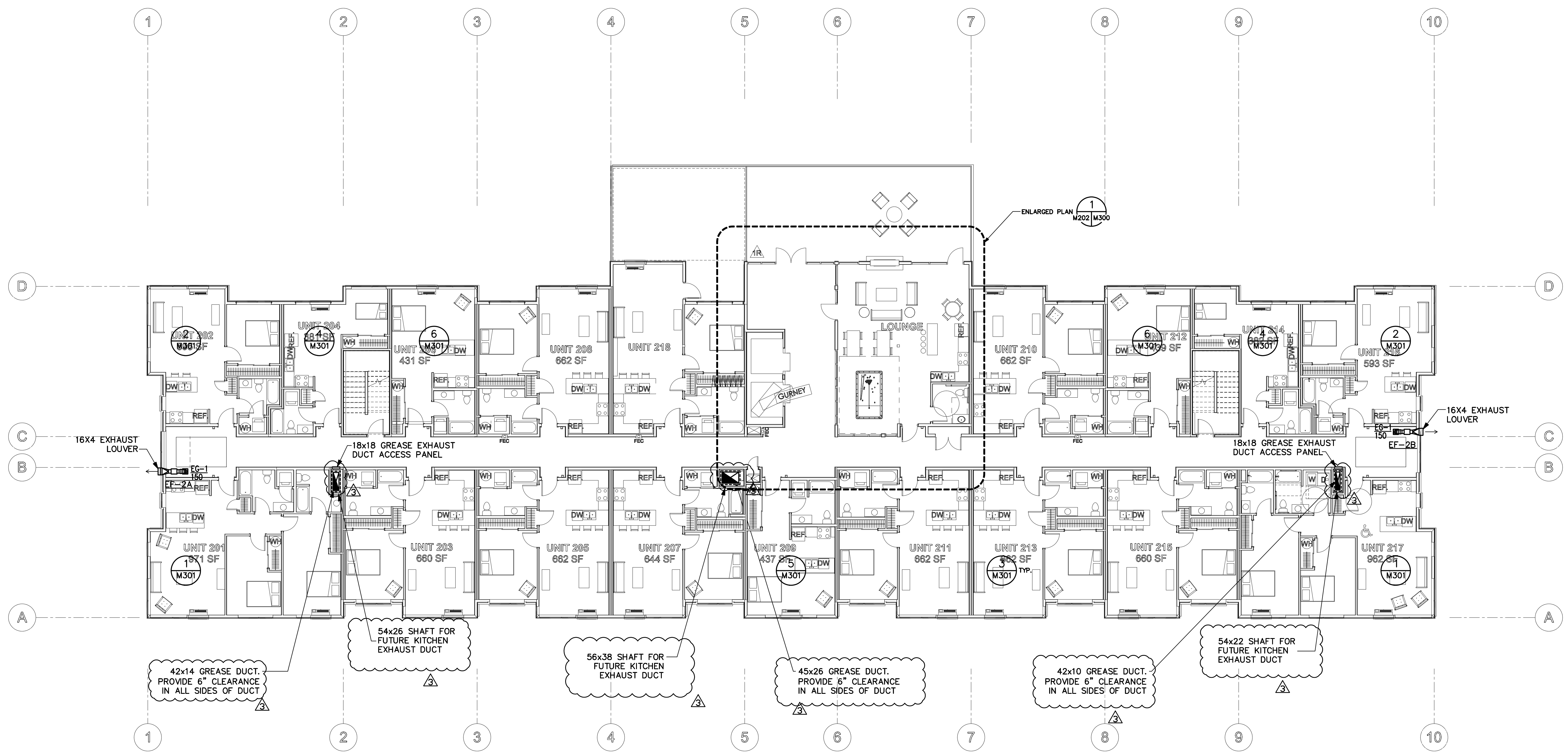
- 1. 10' CLEARANCE BETWEEN LOUVERS

FIRST FLOOR PLAN

SCALE: 3/32" = 1'-0"



| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |



SECOND FLOOR PLAN
SCALE: 3/32" = 1'-0"



**TOWNZEN & ASSOCIATES
PLAN APPROVAL**

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Approved as submitted. 09/25/2019 *Joe Lanza*

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CONTACT: JON ROBISON

PROFESSIONAL ENGINEER
MARK D. ROBISON
STATE OF WASHINGTON
NO. 10070
06/05/18

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Community Planning & Development Department
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(360) 753-8148
robison@ci.olympia.wa.us

**EAST BAY LOT A
WESTMAN MILL**
510 STATE AVE OLYMPIA, WA, 98501

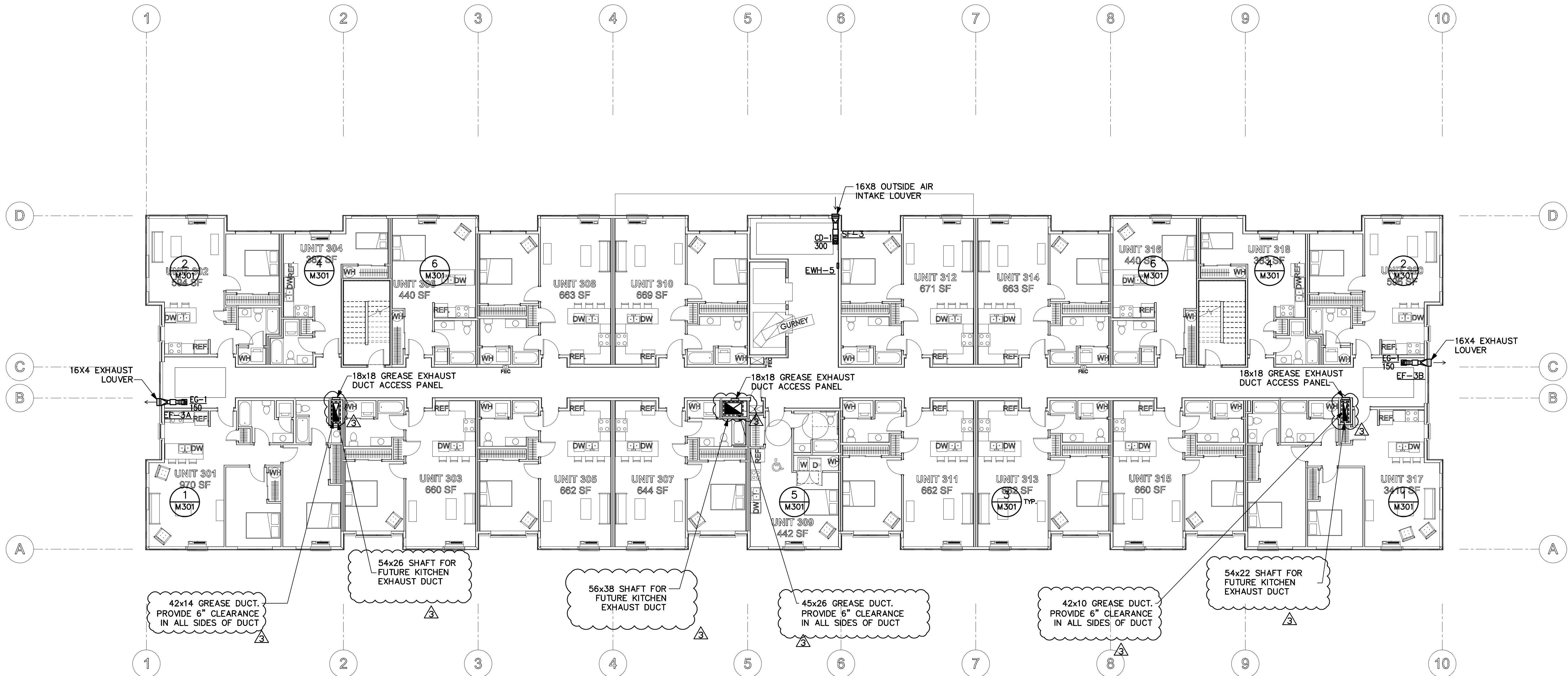
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |

**MIXED USED
PLANS - LEVEL 3**

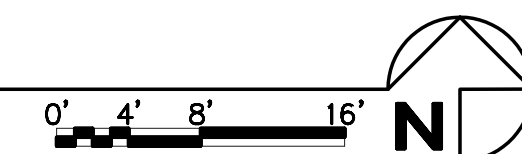
M203

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THIRD FLOOR PLAN

SCALE: 3/32" = 1'-0"





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PLAN APPROVAL**

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Approved as submitted. 09/25/2019

[Signature]

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**EAST BAY LOT A
WESTMAN MILL**

510 STATE AVE OLYMPIA, WA. 98501

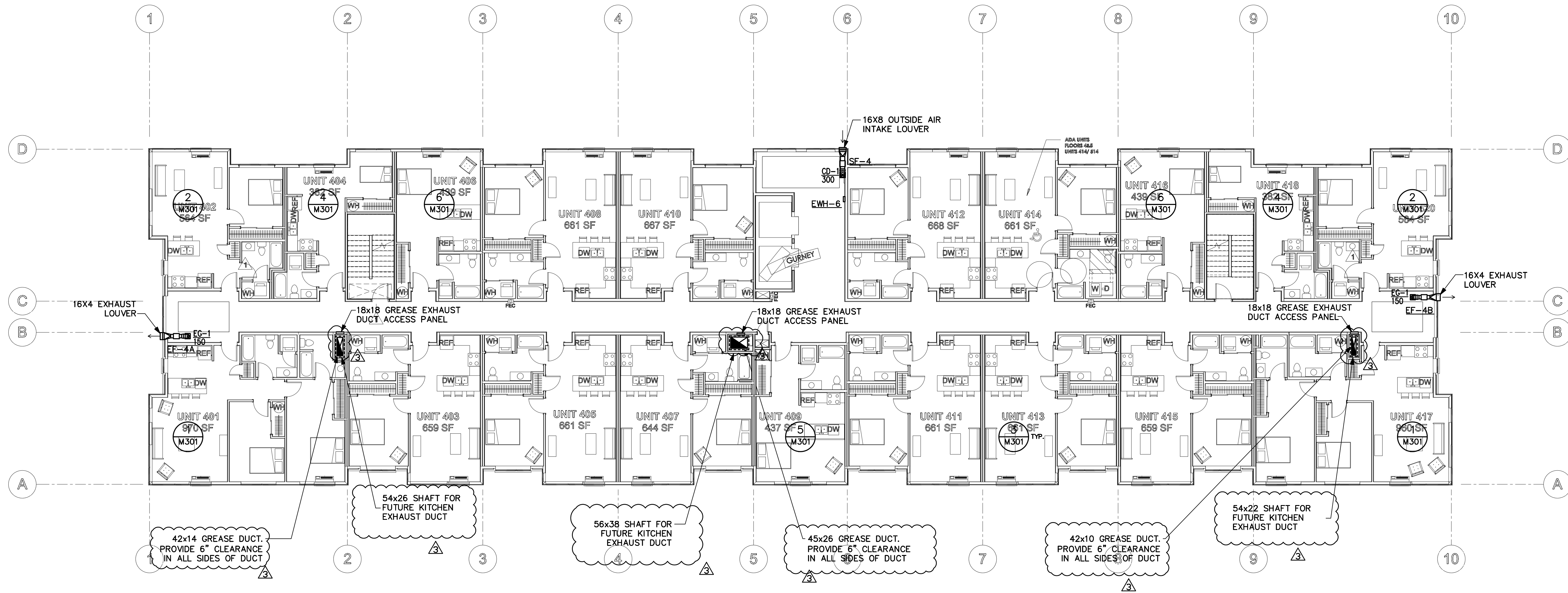
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| △ | 01/30/19 | PERMIT COMMENT |
| △ | 01/30/19 | OTHER CHANGES |
| △ | 06/12/19 | GREASE WASTE |
| △ | 09/03/19 | ASI-3 |

MIXED USED
PLANS - LEVEL 4

M204

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FOURTH FLOOR PLAN
SCALE: 3/32" = 1'-0"



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**TOWNZEN & ASSOCIATES
PLAN APPROVAL**

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Approved as submitted. 09/25/2019

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Community Planning & Development Department
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360.753.4248
robison@ci.olympia.wa.us

**EAST BAY LOT A
WESTMAN MILL**

510 STATE AVE OLYMPIA, WA, 98501

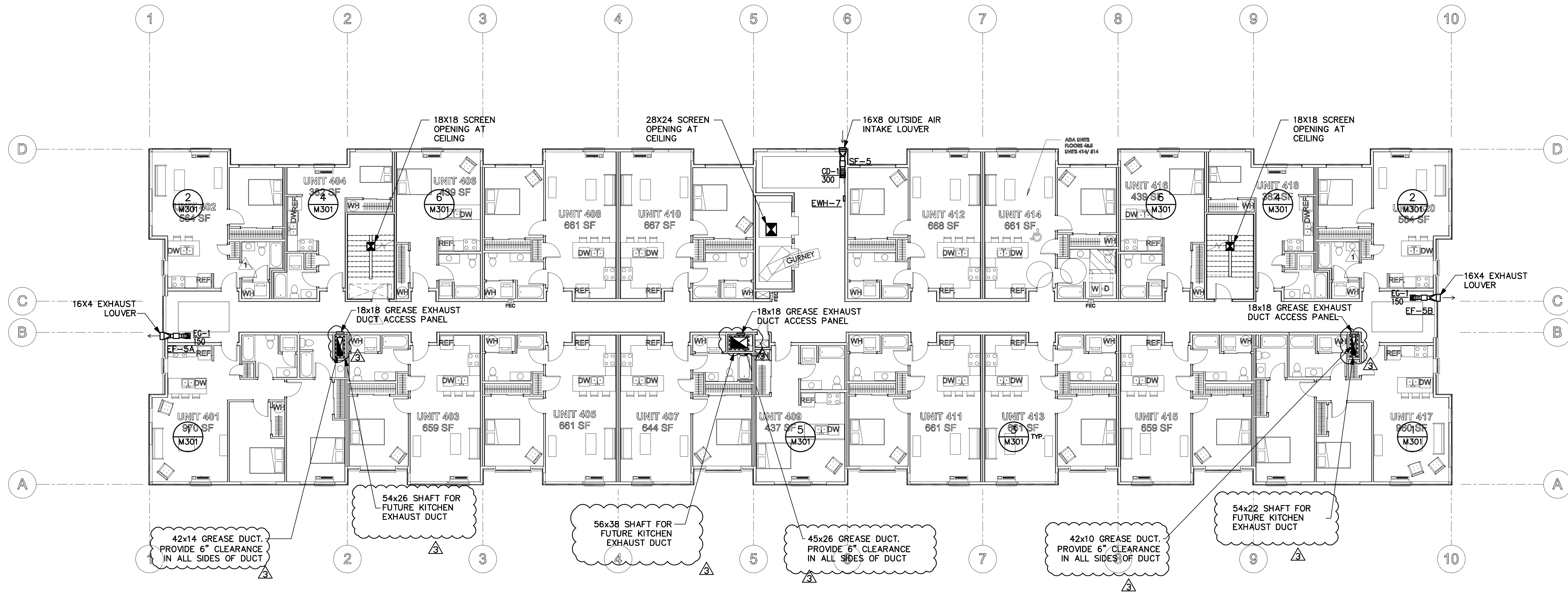
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |

MIXED USED
PLANS - LEVEL 5

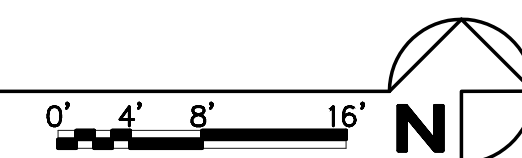
M205

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FIFTH FLOOR PLAN

SCALE: 3/32" = 1'-0"





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**TOWNZEN & ASSOCIATES
PLAN APPROVAL**
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Approved as submitted. 09/25/2019 *[Signature]*

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**EAST BAY LOT A
WESTMAN MILL**
510 STATE AVE OLYMPIA, WA, 98501

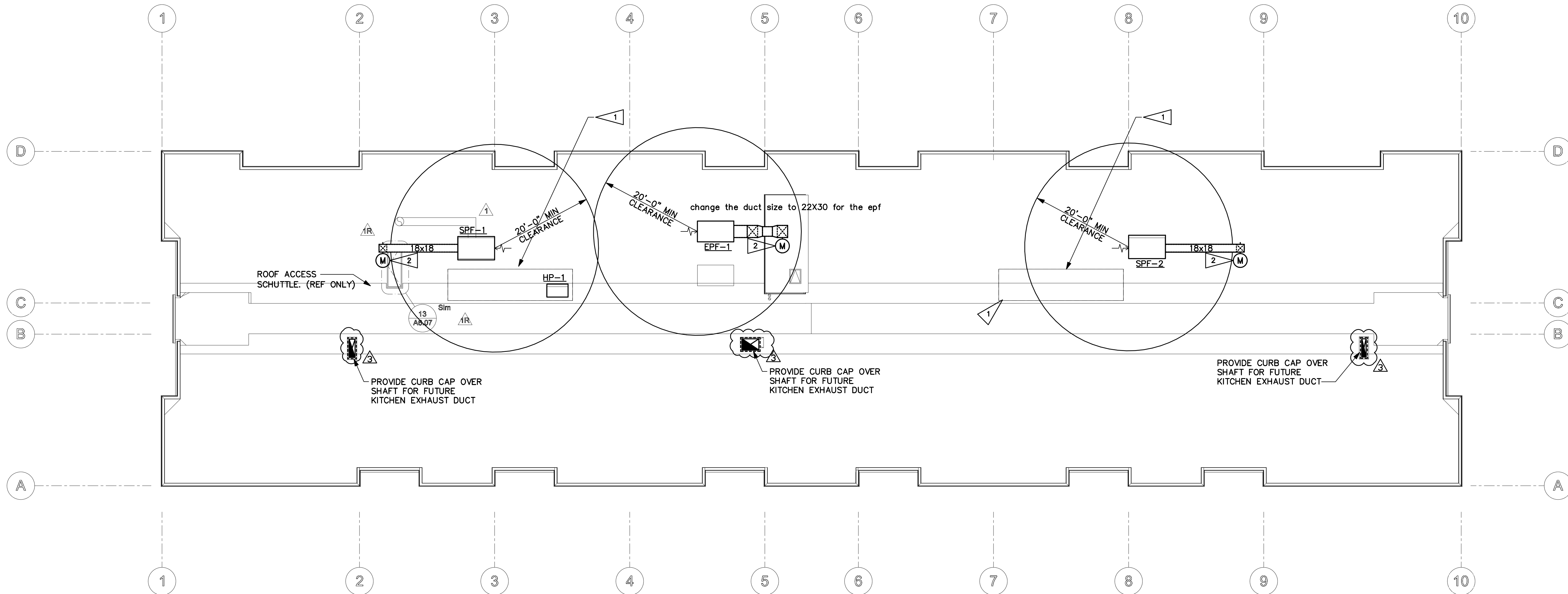
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |

MIXED USED
PLANS - ROOF

M206

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GENERAL NOTES

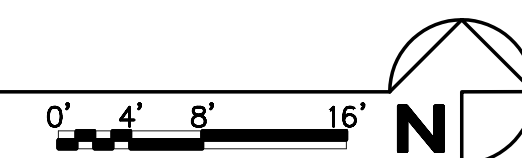
- 1. CONDENSATE DRAIN TO TERMINATE AT APPROVED RECEPTOR WITH INDIRECT CONNECTION.

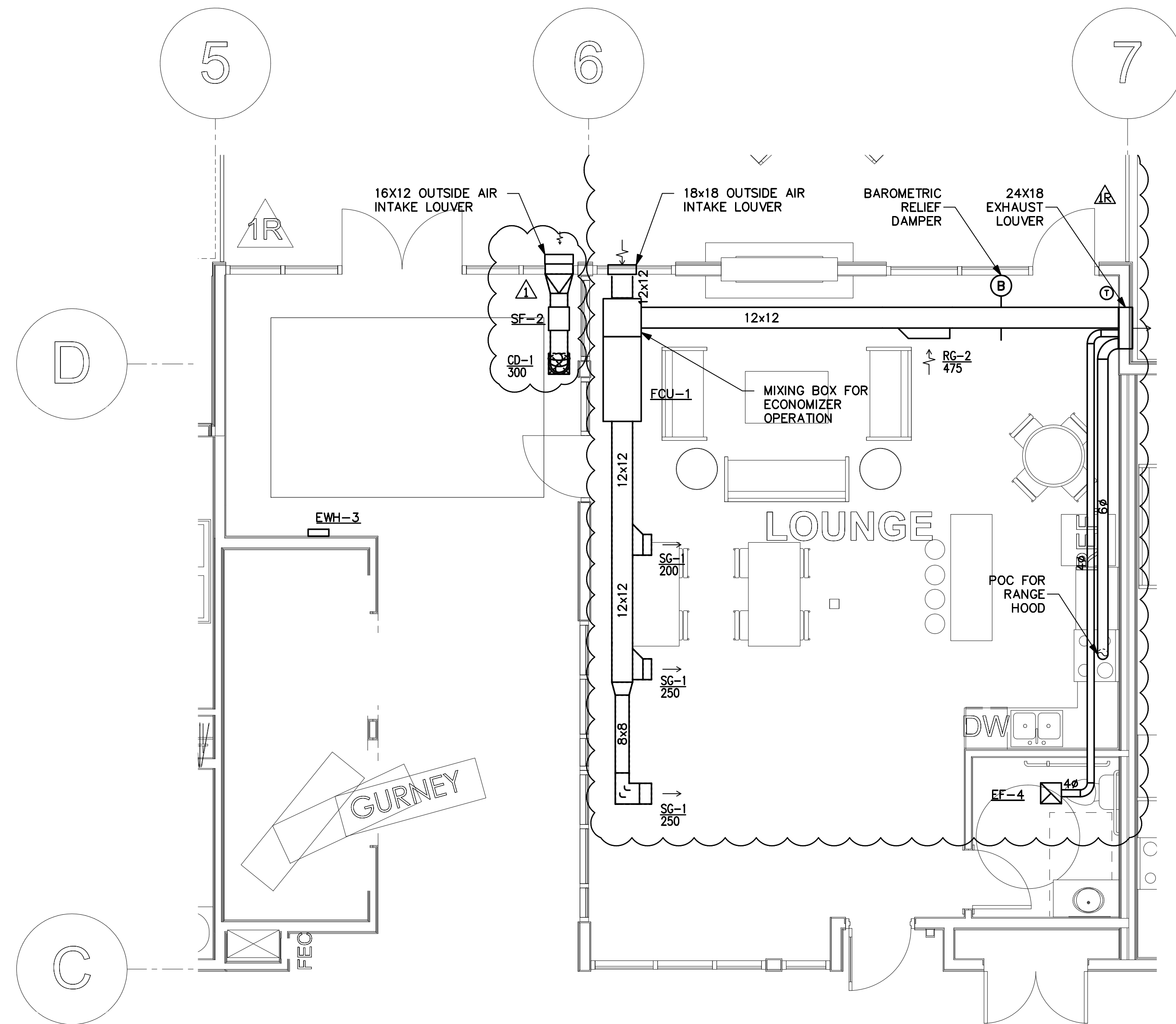
FLAG NOTES

- 1. ROOF PAD FOR FUTURE OUTDOOR HEAT PUMP
- 2. MOTORIZED DAMPER INTERLOCK WITH SMOKE CONTROL SYSTEM.

ROOF PLAN

SCALE: 3/32" = 1'-0"





L2 LOUNGE
ENLARGED PLAN
SCALE: 1/4" = 1'

1
M300

GENERAL NOTES

1. MOUNT REMOTE THERMOSTAT AT 48" AFF.
2. PROVIDE ACCESS PANELS FOR ALL ENERGY RECOVERY VENTILATOR, FAN COIL UNITS, HEAT RECOVERY UNITS, AND FIRE SMOKE DAMPERS.
3. CONDENSATE DRAIN TO TERMINATE AT APPROVED RECEPTOR WITH INDIRECT CONNECTION. REFER TO PLUMBING PLANS FOR CONDENSATE PIPE ROUTING AND ADDITIONAL INFORMATION.



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**TOWNZEN & ASSOCIATES
PLAN APPROVAL**

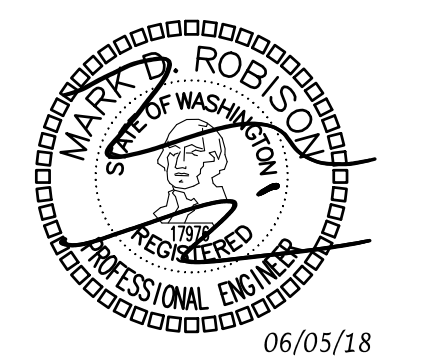
The plans submitted for review are approved in accordance with local state applicable standards. This approval does not relieve the applicant of the responsibility of compliance with the applicable codes.

Approved as submitted: 09/25/2019

[Signature]

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**EAST BAY LOT A
WESTMAN MILL**
510 STATE AVE OLYMPIA, WA, 98501

Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1R | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |

MIXED USED
PLANS -
ENLARGED

M300

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WESTMAN MILL**
510 STATE AVE OLYMPIA, WA, 98501

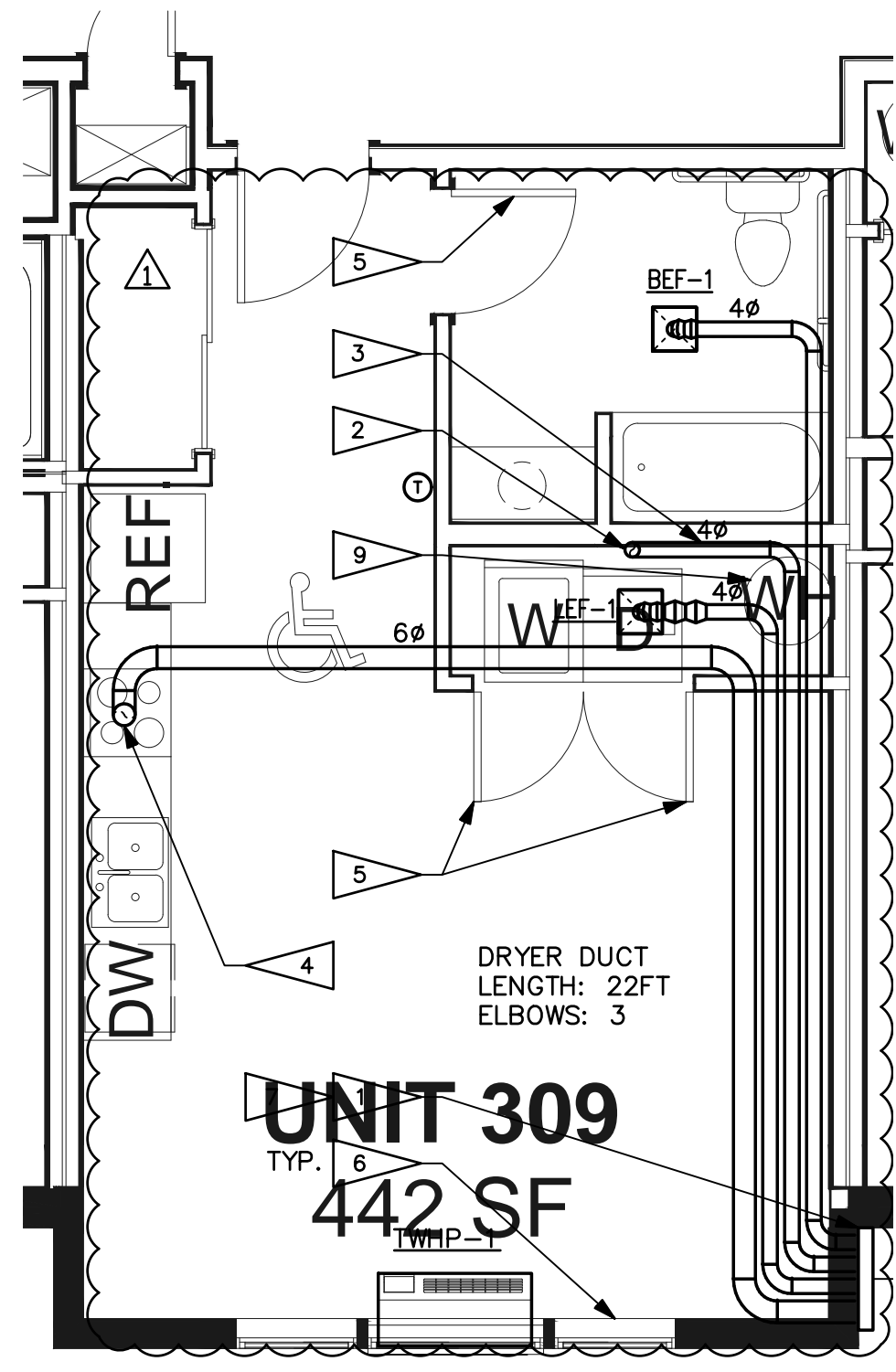
Project No: 1514
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| ▲ | 01/30/19 | PERMIT COMMENT |
| ▲ | 01/30/19 | OTHER CHANGES |
| ▲ | 06/12/19 | GREASE WASTE |
| ▲ | 09/03/19 | ASI-3 |

MIXED USED
PLANS -
ENLARGED

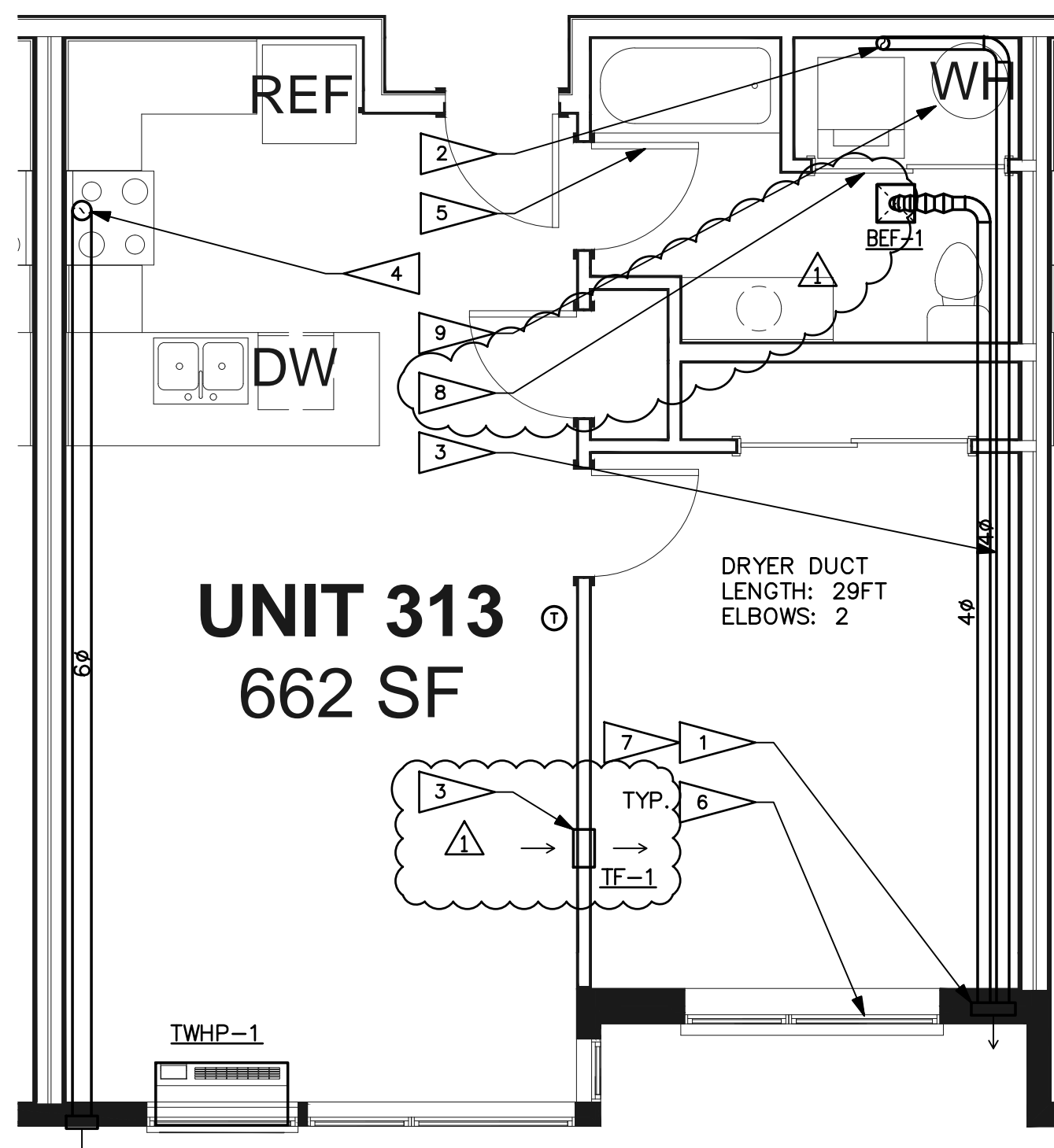
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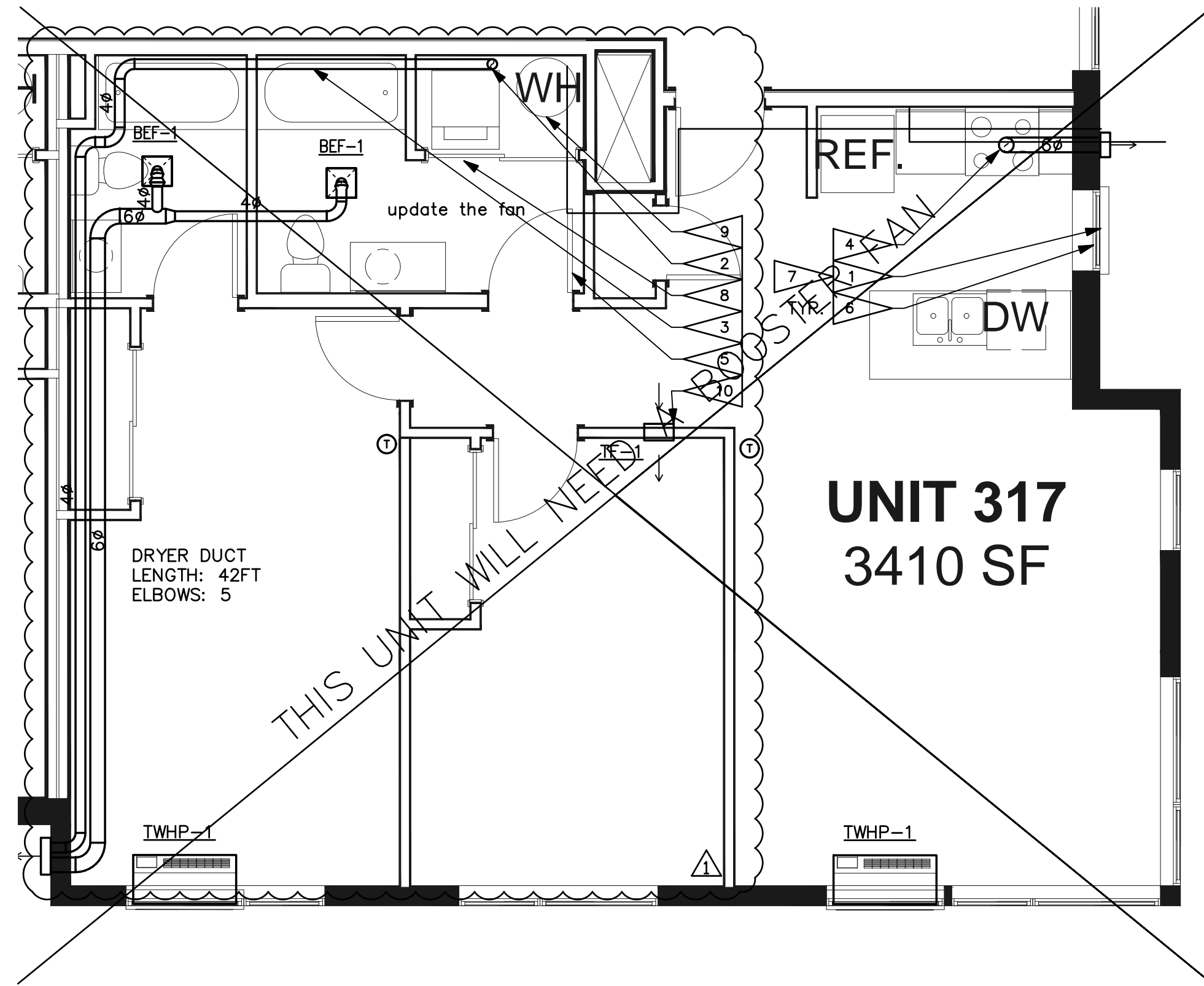
0-BD/1-BA
ENLARGED PLAN
SCALE: 1/4" = 1'

5
M301



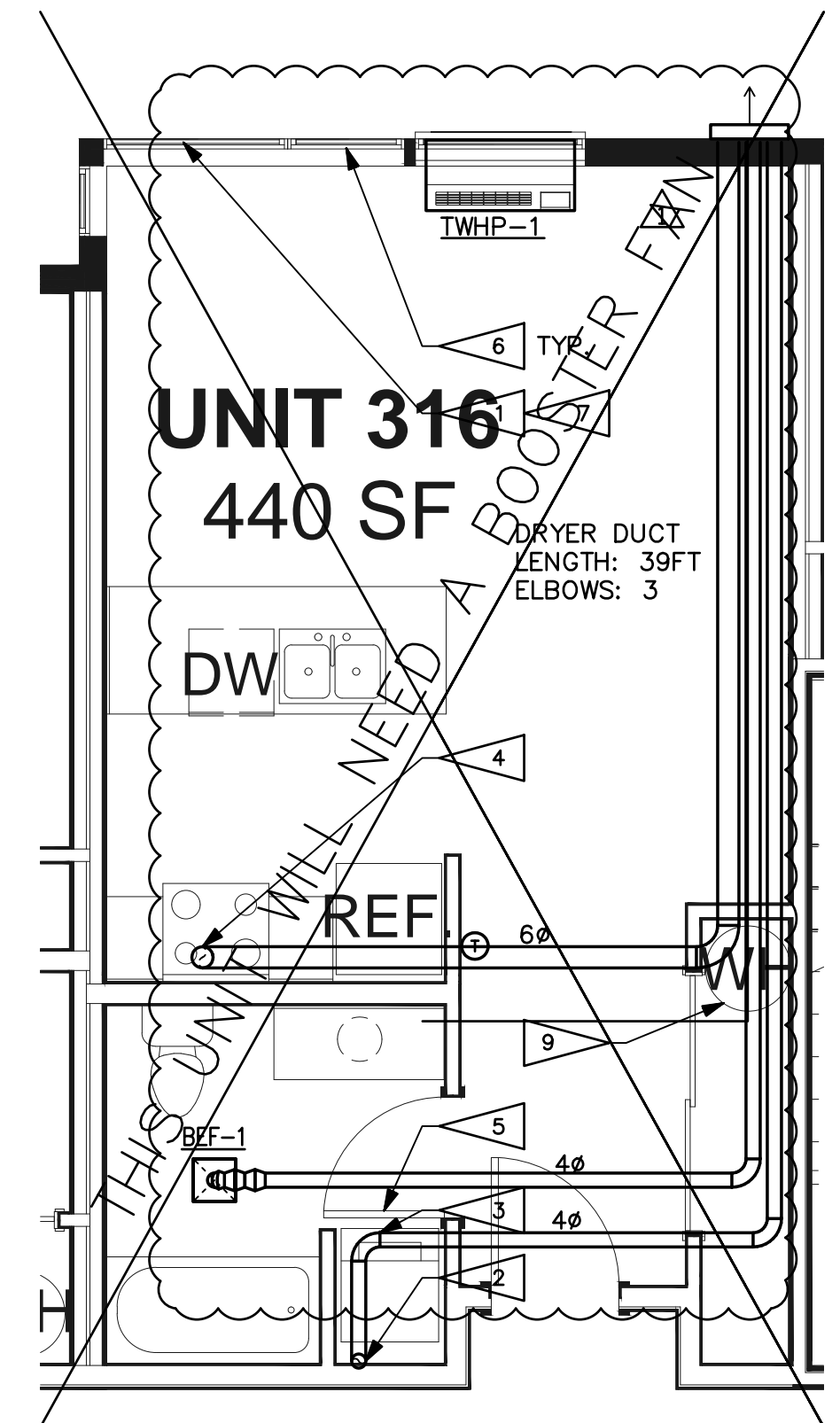
1-BD/1-BA
ENLARGED PLAN
SCALE: 1/4" = 1'

3
M301



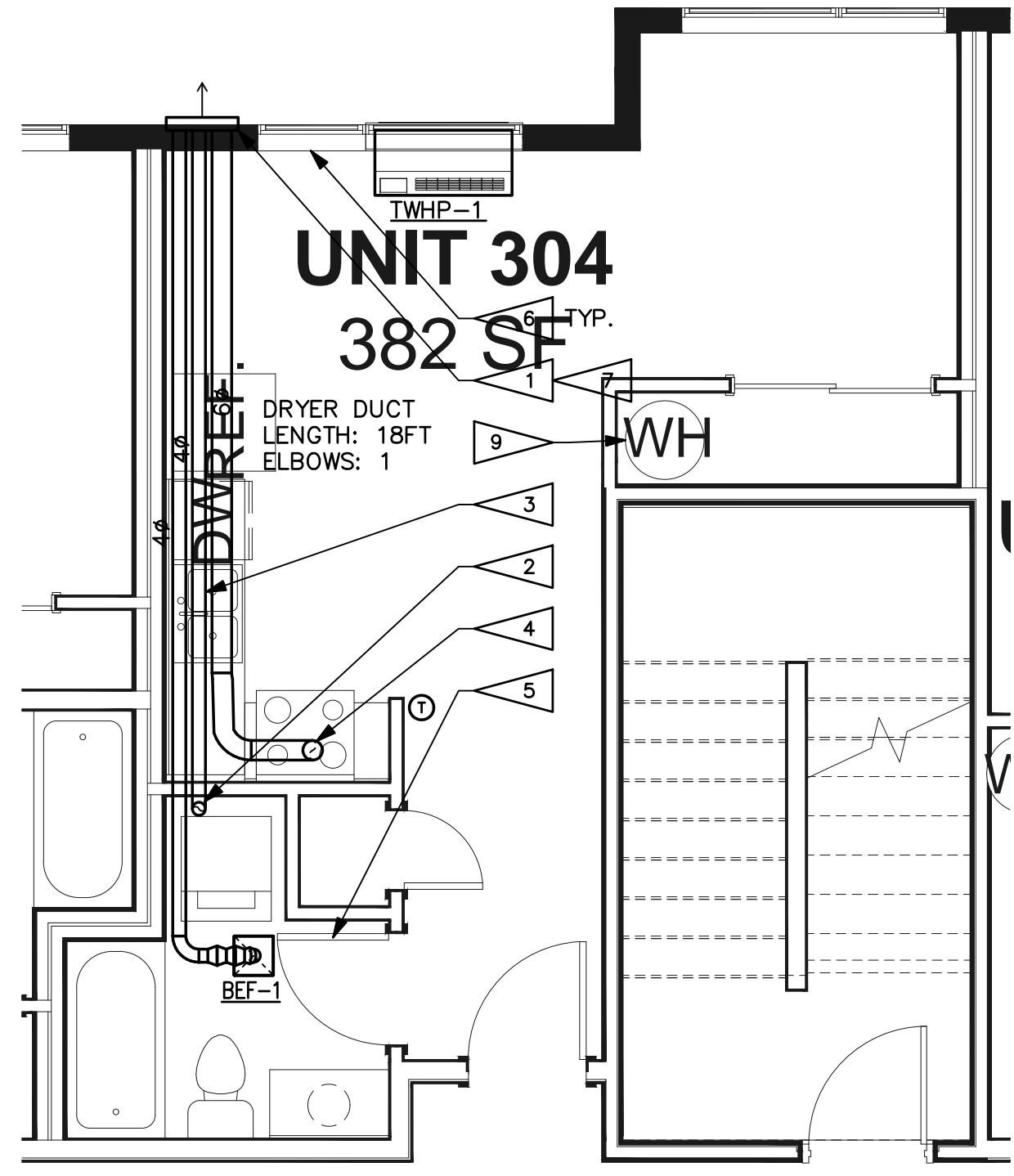
2-BD/2-BA
ENLARGED PLAN
SCALE: 1/4" = 1'

1
M301



0-BD/1-BA
ENLARGED PLAN
SCALE: 1/4" = 1'

6
M301



0-BD/1-BA
ENLARGED PLAN
SCALE: 1/4" = 1'

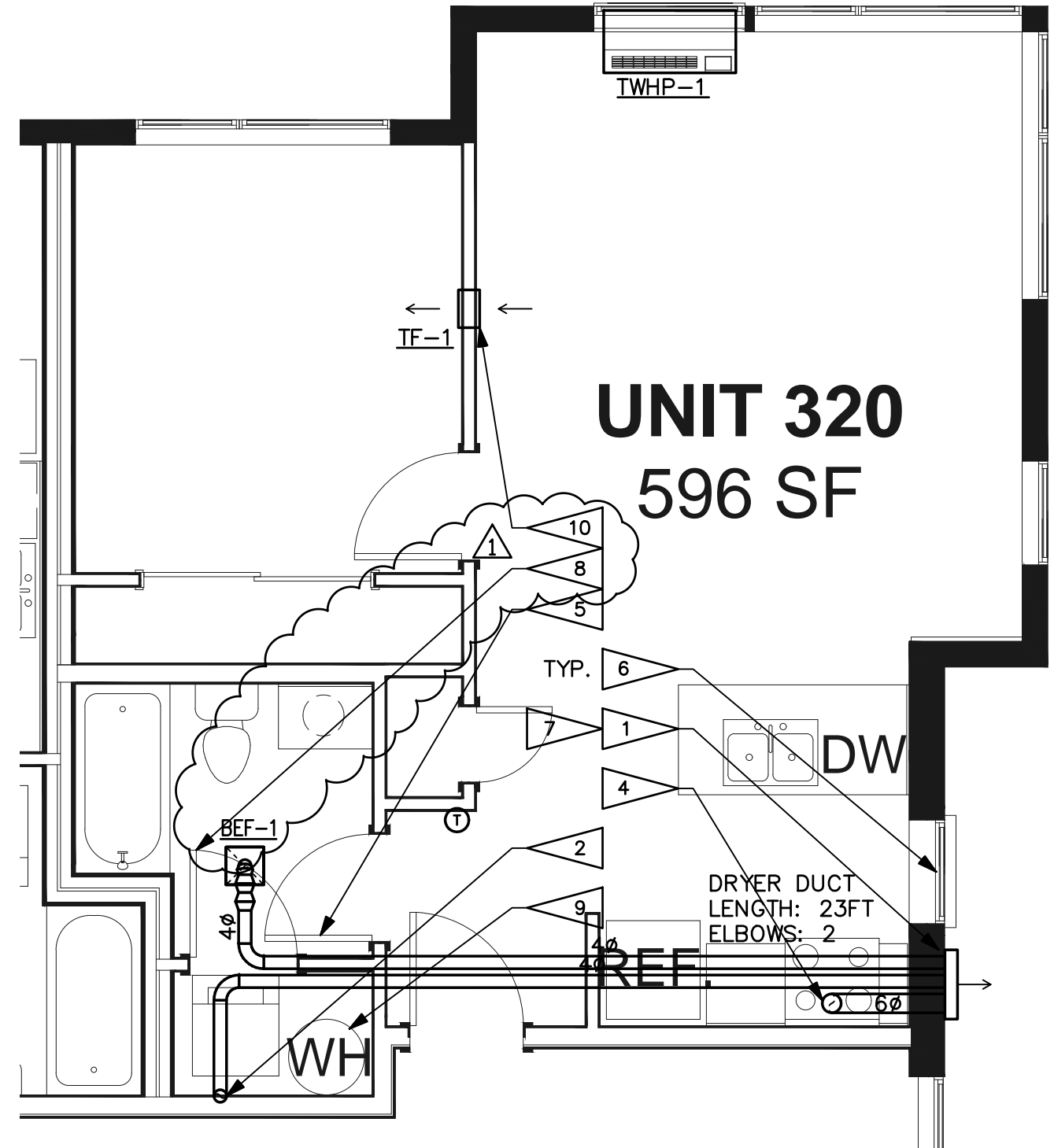
4
M301

GENERAL NOTES

- MOUNT REMOTE THERMOSTAT AT 48" AFF.
- OUTDOOR AIR INLET COMPLY WITH SECTION 403.8.6.1 AND LOCATED COMPLYING WITH SECTION 403.8.3 OF 2015 IMC WITH WASHINGTON STATE AMENDMENTS. VERIFY THAT THE UNIT OUTSIDE AREA INTAKE WILL MEET WHOLE HOUSE VENTILATION REQUIREMENT ON SHEET M001.
- CONDENSATE DRAIN TO TERMINATE AT APPROVED RECEPTOR WITH INDIRECT CONNECTION. REFER TO PLUMBING PLANS FOR CONDENSATE PIPE ROUTING AND ADDITIONAL INFORMATION..

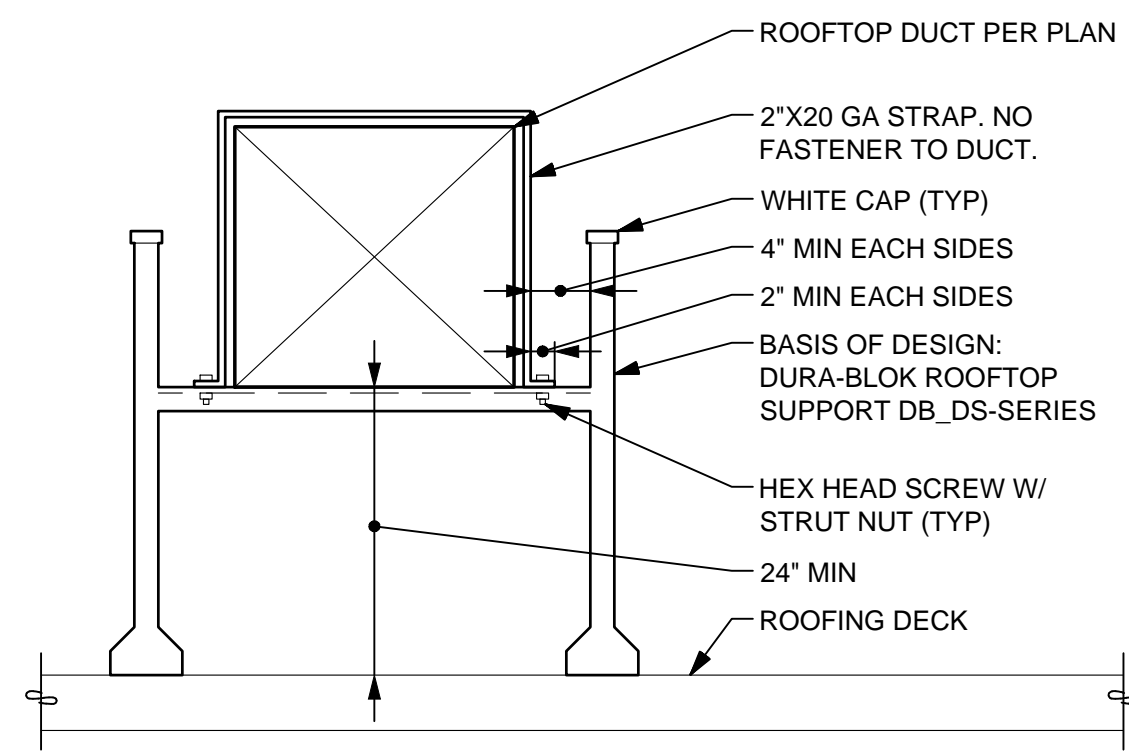
FLAG NOTES

- LOCATE EXHAUST OUTLETS MINIMUM 3- FEET FROM OPERABLE WINDOWS AND DOORS.
- POC TO DRYER.
- REFER TO MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR THE MAXIMUM LENGTH OF THE DRYER VENT. PROVIDE DRYER BOOSTER FAN IF NECESSARY. PROVIDE DRYER BOX
- POC RANGE HOOD
- UNDERCUT ALL BATHROOM AND LAUNDRY DOORS BY 1/2" FOR TRANSFER OF MAKE-UP AIR FOR THE EXHAUST SYSTEM.
- WINDOWS WILL BE EQUIPPED WITH OUTDOOR AIR INLETS.
- WALL CAP WITH SCREENED OPENING.
- 10X10 LOUVER AT LOWER DOOR FOR DRYER MAKEUP AIR.
- ELECTRIC WATER HEATER. VENTING NOT REQUIRED.
- TRANSFER FAN: COORDINATE WITH ELECTRICIAN TO PROVIDE SWITCH IN BEDROOM WITH CIRCUIT TO FAN.



1-BD/1-BA
ENLARGED PLAN
SCALE: 1/4" = 1'

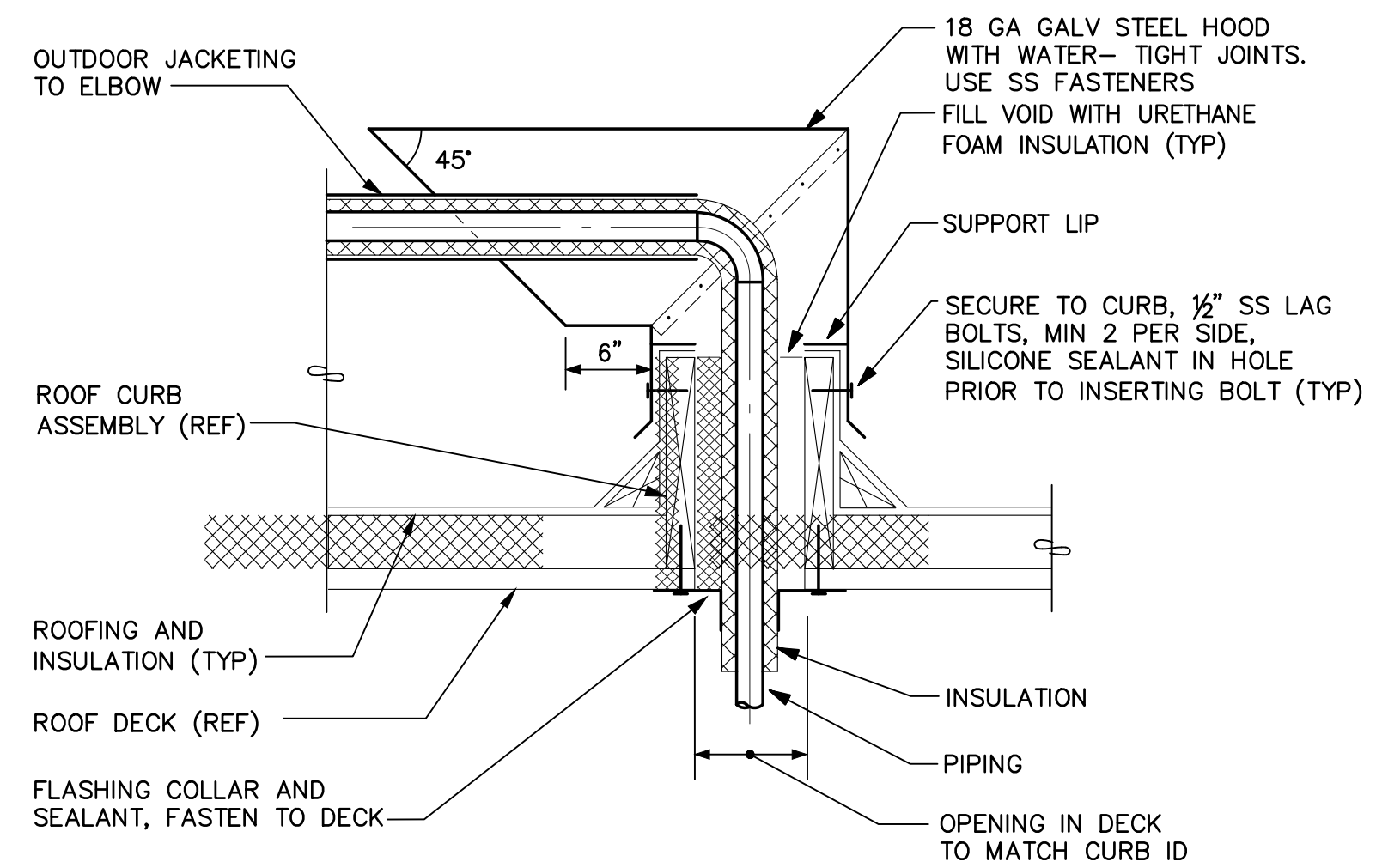
2
M301



**ROOFTOP DUCT SUPPORT
DETAIL**

SCALE: NONE

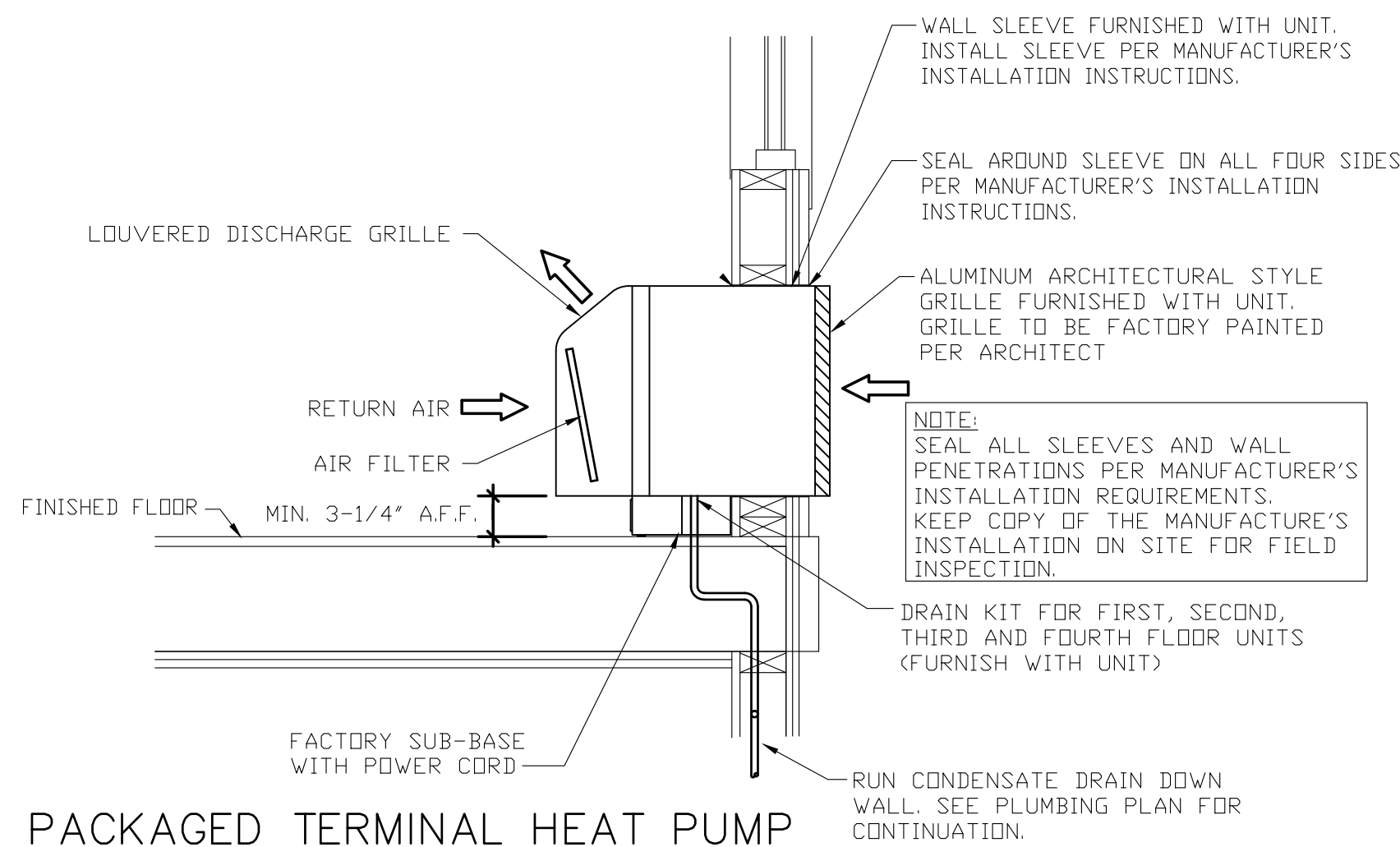
6
M400



**REFRIGERANT PIPE PENETRATION THRU ROOF
DETAIL**

SCALE: NONE

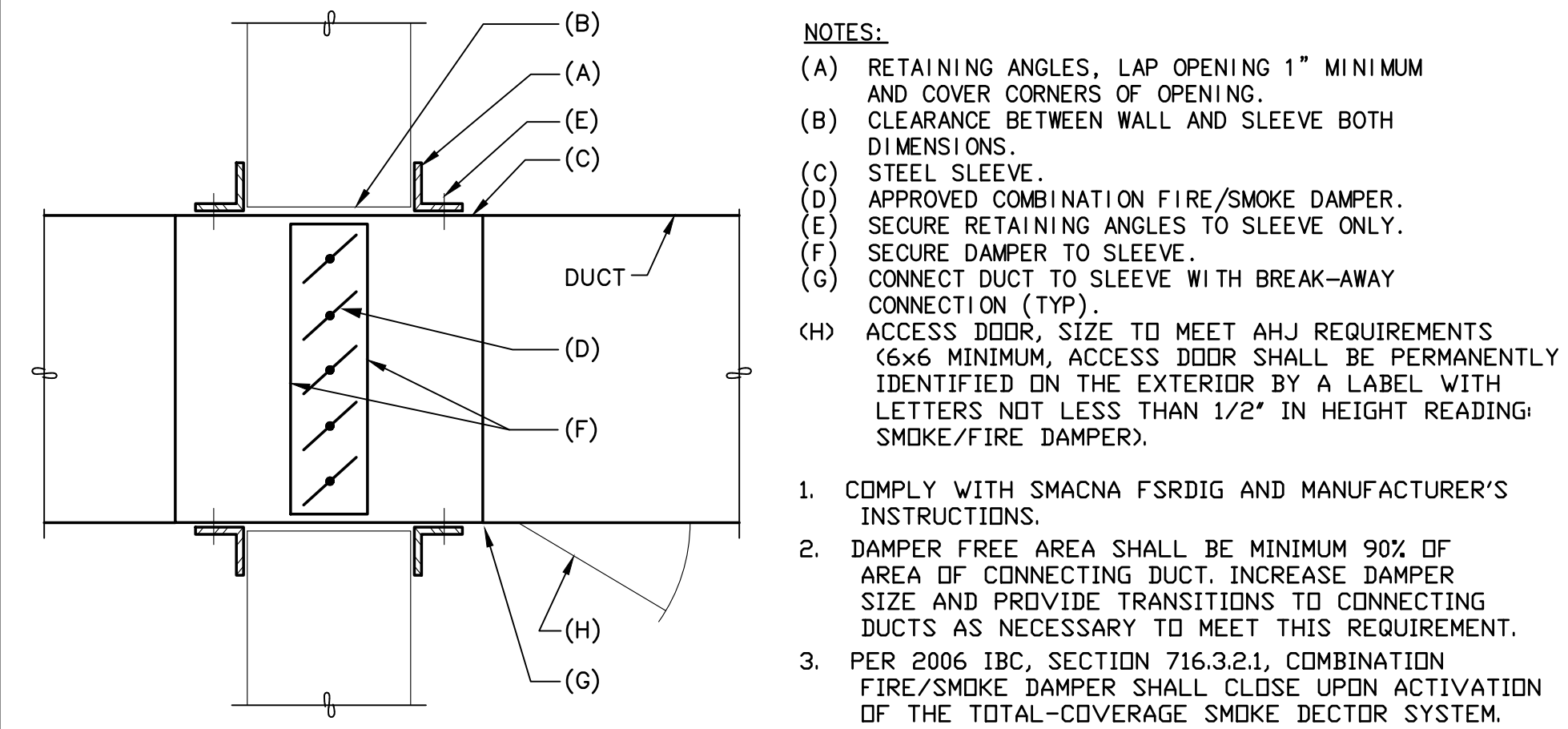
3
M400



**PACKAGED TERMINAL HEAT PUMP
DETAIL**

SCALE: NONE

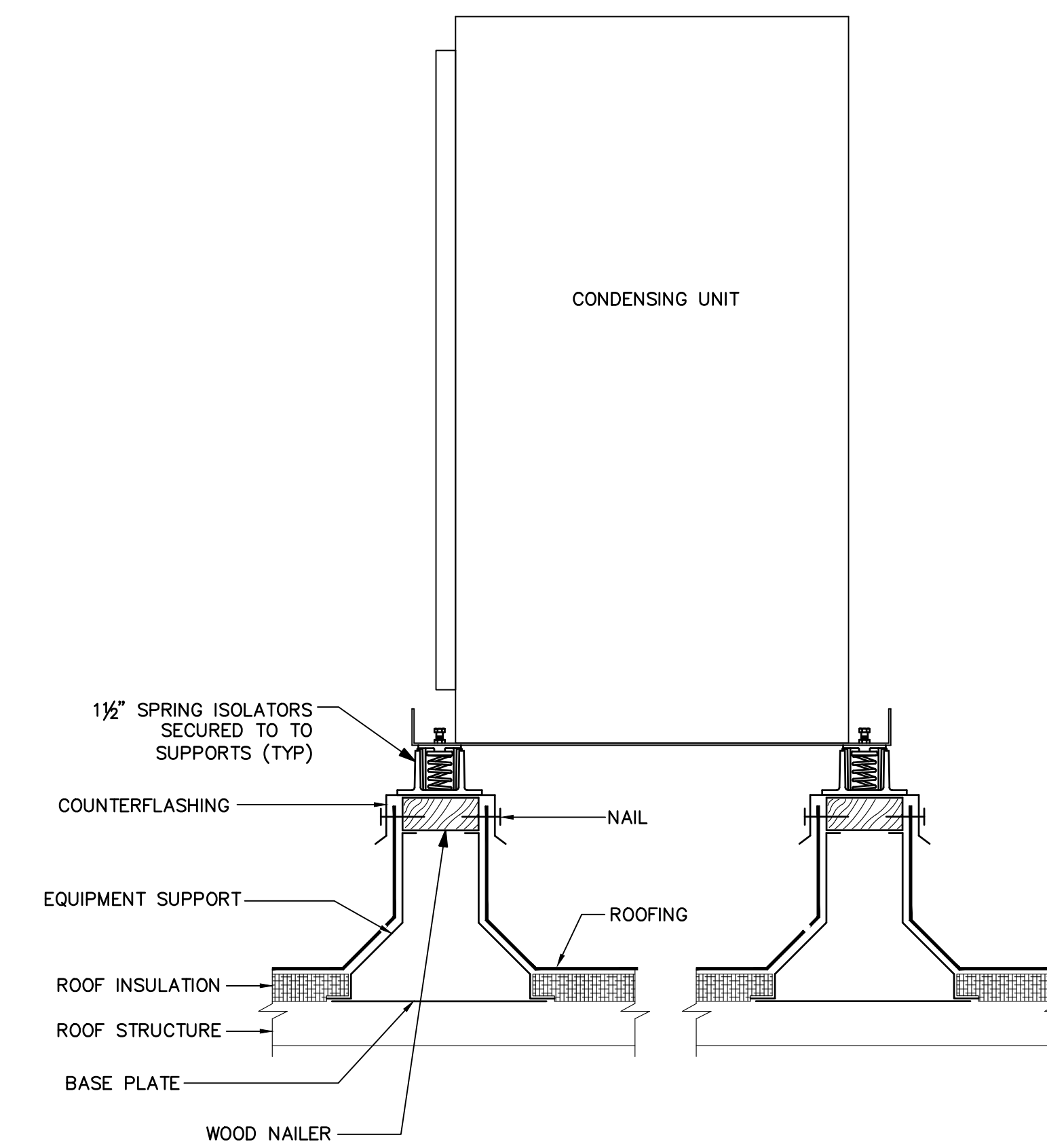
7
M400



**COMBINATION FIRE/SMOKE DAMPER INSTALLATION
DETAIL**

SCALE: NONE

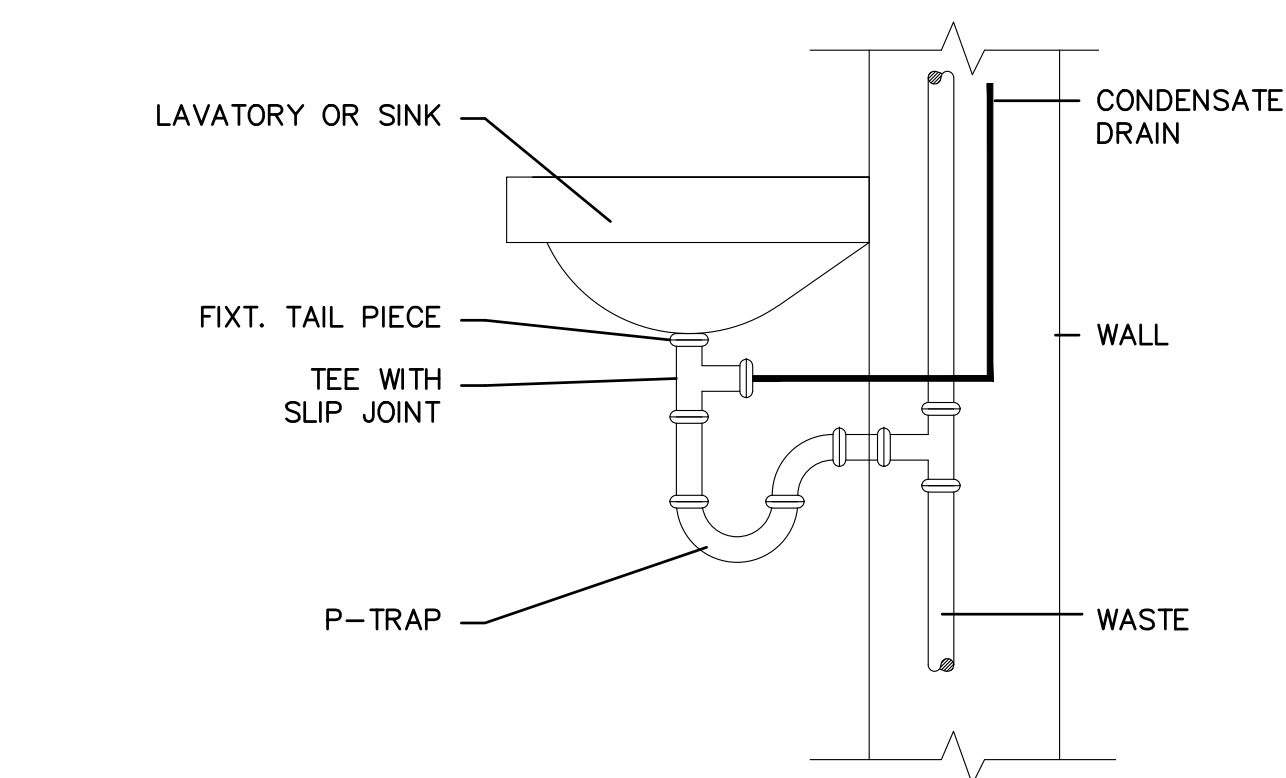
4
M400



**ROOFTOP CONDENSING UNIT MOUNTING DETAIL
DETAIL**

SCALE: NONE

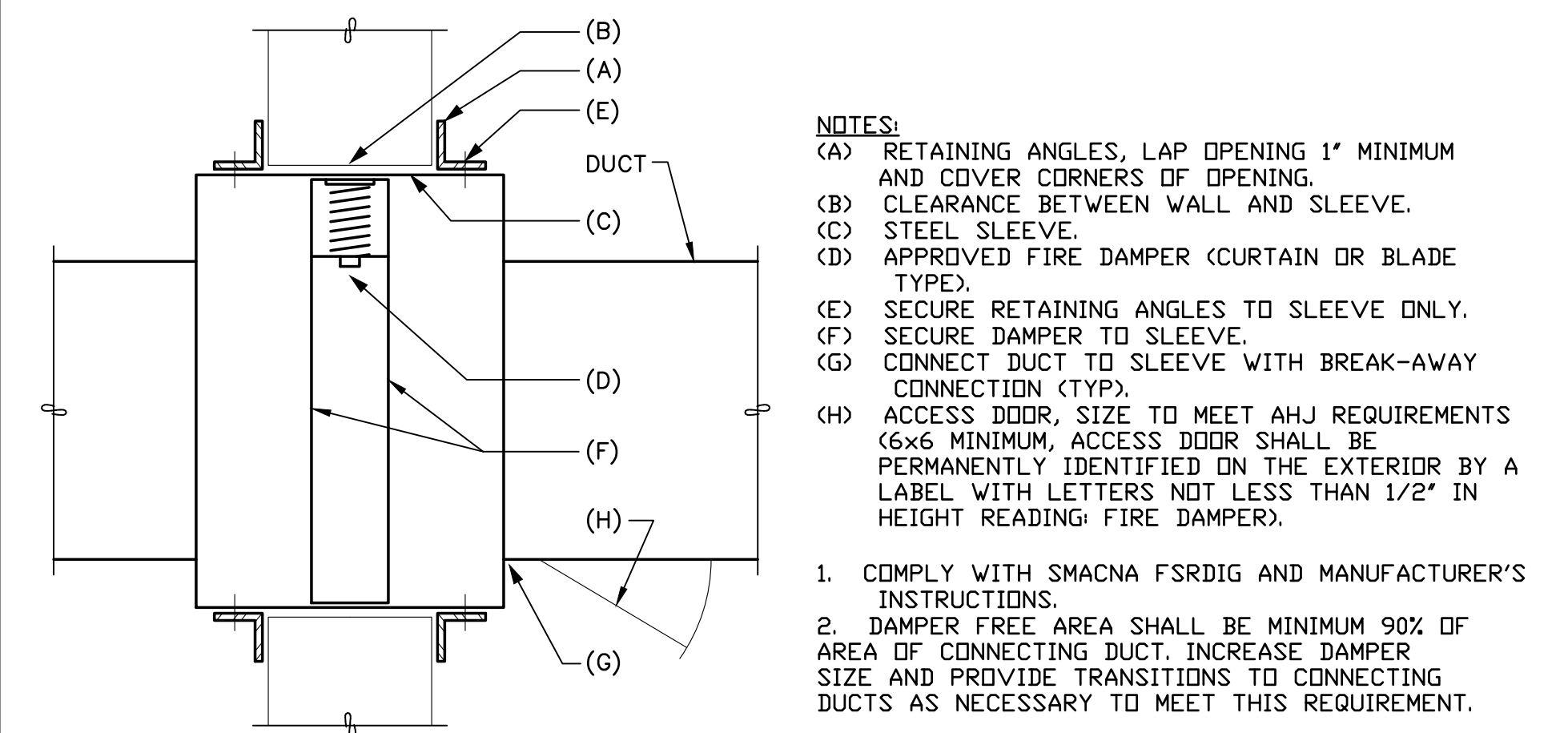
1
M400



**CONDENSATE TERMINATION
DETAIL**

SCALE: NONE

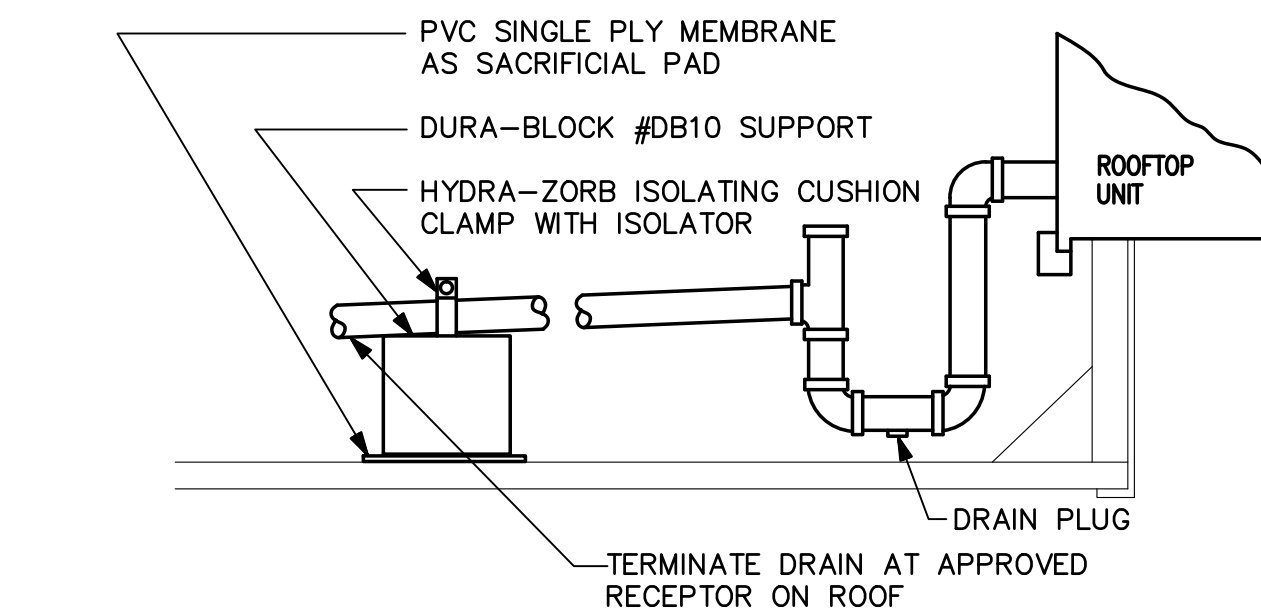
8
M400



**FIRE DAMPER INSTALLATION
DETAIL**

SCALE: NONE

5
M400



**RTU CONDENSATE DRAIN
DETAIL**

SCALE: NONE

2
M400

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |

System No. W-L-7214

| | |
|--|---|
| ANSI/UL 1479 (ASTM E814) | CANULC S115 |
| F Ratings — 1 and 2 Hr (See Items 1 and 3) | F Ratings — 1 and 2 Hr (See Items 1 and 3) |
| T Rating — 0 Hr | FT Rating — 0 Hr |
| | FH Ratings — 1 and 2 Hr (See Items 1 and 3) |
| | FTH Rating — 0 Hr |

CLASSIFIED
C US
Classified by Underwriters Laboratories, Inc. to UL 1479 and CANULC-S115

W-L-7214

SECTION A-A

- Wall Assembly — The 1 or 2 hr fire rated wallboard/stud wall assembly shall be constructed of the materials and in the manner specified in the individual U300, U400 or V400 Series Wall and Partition Designs in the UL Fire Resistance Directory and shall include the following construction features.
 - Studs — Wall framing may consist of either wood studs or steel channel studs. Wood studs to consist of nom 2 by 4 in. (51 by 102 mm) lumber spaced 16 in. (406 mm) OC. Steel studs to be min 3-1/2 in. (89 mm) wide and spaced 24 in. (610 mm) OC.
 - Gypsum Board — For 1 hr assembly, one layer of min 5/8 in. (16 mm) thick wallboard as required in the individual Wall and Partition Design. For 2 hr assembly, two layers of min 5/8 in. (16 mm) thick wallboard as required in the individual Wall and Partition Design. Max diam of opening is 14-1/2 in. (368 mm) for wood stud walls and 21-3/4 in. (552 mm) for steel stud walls. The hourly F and FH Ratings of the firestop system are equal to the hourly fire rating of the wall assembly in which it is installed.
- Through Penetrant — Galv steel duct to be installed concentrically or eccentrically within the firestop system. The annular space between the duct and periphery of opening shall be 0 in. (point contact) and max 1-1/2 in. (38 mm). Duct to be rigidly supported on both sides of wall assembly.
 - Spiral Wound HVAC Duct — Nom 20 in. (508 mm) diam (or smaller) No. 24 MSG (or heavier) galv steel spiral wound duct.
 - Sheet Metal Duct — Nom 12 in. (305 mm) diam (or smaller) No. 28 MSG (or heavier) galv sheet steel duct.
- Fill, Void or Cavity Material — Sealant — Min 5/8 in. and 1-1/4 in. (16 and 32 mm) thickness of fill material applied within annulus, flush with both surfaces of wall assembly for 1 or 2 hr rated walls, respectively. At the point contact location between duct and wallboard, a min 1/2 in. (13 mm) diam bead of sealant shall be applied at the wallboard/duct interface on both surfaces of wall assembly.

HILTI CONSTRUCTION CHEMICALS, DIV OF HILTI INC — CFS-S SIL GG Sealant
*Bearing the UL Classification Mark

Hilti Firestop Systems

Reproduced by HILTI, Inc. Courtesy of Underwriters Laboratories, Inc. January 14, 2014

**FIRESTOP SYSTEM
DETAIL**

SCALE: NONE

5
M401

MECHANICAL UNIT SUPPORT REQUIREMENTS:

- MECHANICAL UNITS TO BE SUPPORTED FROM STRUCTURE AS SHOWN AT ALL FOUR CORNERS.
- PROVIDE RUBBER GROMMETS AT CORNER SUPPORTS AT UNIT.
- PROVIDE MASON #30N COMBINATION DOUBLE DEFLECTION AND SPRING ISOLATORS SECURED TO STRUT SECTIONS. SUSPEND THREADED ROD FROM ISOLATOR.
- PROVIDE FLEX CONNECTION BETWEEN UNIT AND DUCTWORK.
- SUPPORT RODS AND ANCHORS TO BE PLACED SUCH THAT THE SUPPORTS TO NOT CONTACT THE MECHANICAL UNIT.
- PROVIDE SECTION OF RUBBER HOSE TO ISOLATE THE COPPER CONDENSATE DRAIN. SECURE THE HOSE TO PIPE WITH HOSE CLAMPS.
- PROVIDE FLEXIBLE HOSES FOR CONDENSER WATER CONNECTIONS (WHERE APPLICABLE).

**HORIZONTAL EQUIPMENT MOUNTING
DETAIL**

SCALE: NONE

1
M401

NOTE:

- PROVIDE ACCESS DOORS AT CHANGES IN DIRECTION OF DUCTWORK FOR INSPECTION AND CLEANING
- IN LIEU OF THE CLEANOUT DETAIL, A PRE-MANUFACTURED, UL LISTED CLEANOUT ACCESS DOOR MAY BE USED.

**DUCT PENETRATION THROUGH ROOF
DETAIL**

SCALE: NONE

3
M401

2
M401

PROVIDE DUCT SUPPORTS AND RESTRAINTS IN ACCORDANCE WITH IMC 603.10

**DIFFUSER CONNECTION & DUCT SUPPORT
DETAIL**

SCALE: NONE

4
M401



**TOWNZEN & ASSOCIATES
PLAN APPROVAL**

The plans submitted for review are approved in accordance with local state applicable standards. This approval does not relieve the applicant of the responsibility of compliance with the applicable codes.

Approved as submitted. 09/25/2019

**ROBISON
ENGINEERING, INC**

19401 40TH AVE W., SUITE 302
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CONTACT: JON ROBISON



Olympia
Building Plans Examiner
Community Planning & Development Department
501 4th Ave East
Olympia, WA 98501
(360) 753-6146
robison@ci.olympia.wa.us

**EAST BAY LOT A
WESTMAN MILL**

510 STATE AVE OLYMPIA, WA, 98501

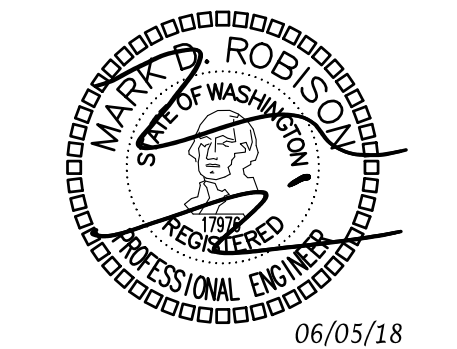
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| ▲ | 01/30/19 | PERMIT COMMENT |
| ▲ | 01/30/19 | OTHER CHANGES |
| ▲ | 06/12/19 | GREASE WASTE |
| ▲ | 09/03/19 | ASI-3 |

MECHANICAL
DETAILS &
DIAGRAMS

M401

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EAST BAY LOT A
WESTMAN MILL
 510 STATE AVE OLYMPIA, WA, 98501

Project No: 1514
 PERMIT SET
 5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |
| 4 | 09/03/19 | ASI-3 |

DETAILS

M402

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NOTES:

1. EXPOSED DUCTWORK SHALL BE 18 GAGE STAINLESS STEEL.
2. CLEANOUTS SHALL BE INSTALLED AT CHANGES IN DIRECTION, AT 12' INTERVALS OF HORIZONTAL DUCTWORK, AT THE BASE OF VERTICAL DUCTS, AND AT EVERY FLOOR OF VERTICAL RISERS.
3. FIELD APPLIED OR FACTORY BUILT GREASE DUCT ENCLOSURE SHALL BE INSTALLED PER THE LATEST MANUFACTURER'S INSTALLATION INSTRUCTIONS AND IN COMPLIANCE WITH TERMS OF ITS LISTING.
4. ALL END CUTS OR CUTS IN THE FOIL JACKET OF FIELD APPLIED GREASE DUCT ENCLOSURE SHALL BE SEALED PER THE MANUFACTURER'S RECOMMENDATIONS.
5. ALL INSTALLATIONS SHALL BE COMPLETELY ACCESSIBLE FOR VISUAL INSPECTION.
6. AT TIME OF INSPECTION, THE FIELD APPLIED OR FACTORY BUILT GREASE DUCT ENCLOSURE INSTALLATION INSTRUCTIONS SHALL BE MADE AVAILABLE AT THE JOB SITE.
7. LISTED GREASE HOOD ASSEMBLIES SHALL BE INSTALLED IN ACCORDANCE WITH THE TERMS OF THEIR LISTING AND THE MANUFACTURER'S INSTALLATION INSTRUCTIONS.
8. FIRE DEPARTMENT APPROVAL SHALL BE REQUIRED ON FIRE PROTECTION SYSTEM FOR GREASE HOODS AND DUCTS AS REQUIRED BY SECTION 513 OF THE MECHANICAL CODE AND AS REQUIRED BY THE FIRE CODE.
9. ALL FIRE-EXTINGUISHING SYSTEMS SHALL BE INTERCONNECTED TO THE FUEL OR CURRENT SUPPLY SO THAT THE FUEL IS AUTOMATICALLY SHUT OFF TO ALL EQUIPMENT UNDER THE HOOD WHEN THE SYSTEM IS ACTIVATED.
10. OWNER OF ESTABLISHMENT SHALL BE RESPONSIBLE FOR CLEANLINESS, MAINTENANCE, AND INSPECTION OF KITCHEN EXHAUST SYSTEM, FIRE PROTECTION, AND COOKING EQUIPMENT.
11. ALL SEAMS, JOINTS, AND PENETRATIONS OF THE HOOD ENCLOSURE THAT DIRECT AND CAPTURE GREASE-LADEN VAPORS AND EXHAUST GASES SHALL HAVE A LIQUID TIGHT CONTINUOUS EXTERNAL WELD TO THE LOWER OUTERMOST PERIMETER OF THE HOOD.
12. PERFORM LIGHT BULB TEST PER MECH CODE AND SEAL DUCT.

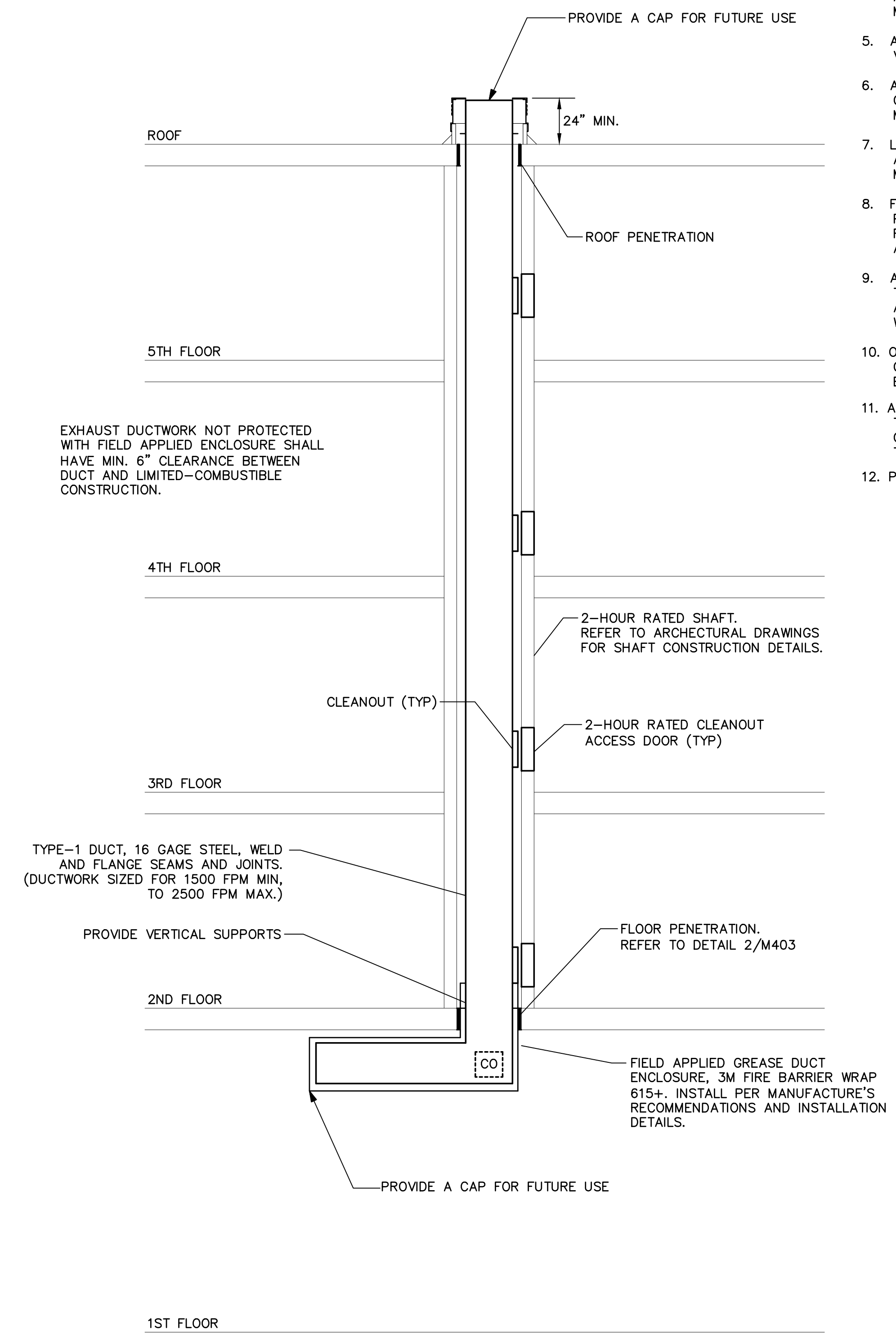


DIAGRAM – TYPE I EXHAUST DUCT SYSTEM
 SCALE: NONE



THOMAS architecture studios

525 COLUMBIA ST. | OLYMPIA, WA 98501
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TOWNZEN & ASSOCIATES PLAN APPROVAL. The plans submitted for review are approved in accordance with local state applicable standards. This approval does not relieve the applicant of the responsibility of compliance with the applicable codes.

ROBISON ENGINEERING, INC. 19401 40TH AVE W., SUITE 302 LYNNWOOD, WA 98036 206-364-3543 TX. CONTACT: JON ROBISON



Olympia Building Plans Examiner Community Planning & Development Department 601 4th Ave East Olympia, WA 98501 (360) 73-6248 rbuilders@cityofolympia.wa.us

EAST BAY LOT A WESTMAN MILL 510 STATE AVE OLYMPIA, WA, 98501

Project No: 1514 PERMIT SET 5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| ▲ | 01/30/19 | PERMIT COMMENT |
| ▲ | 01/30/19 | OTHER CHANGES |
| ▲ | 06/12/19 | GREASE WASTE |
| ▲ | 09/03/19 | ASI-3 |

ENERGY COMPLIANCE FORMS

M601

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2015 Washington State Energy Code Compliance Forms for Commercial, R2 and R3 over 3 stories and all R1

Economizer Exceptions, cont. MECH-ECONO. Project Title: East Bay Lot A Westman Mill. Date: 5/11/2018. Section 10: Economizer Exceptions - New Construction and Additions, Continued. Section 11: Economizer Exceptions - Mechanical System Alterations or Replacement. Section 12: Economizer Exceptions - Simple Systems.

2015 Washington State Energy Code Compliance Forms for Commercial, R2 and R3 over 3 stories and all R1 - Page 9 of 18

Mechanical Permit Plans Checklist MECH-CHK. Project Title: East Bay Lot A Westman Mill. Date: 5/11/2018. Table with columns: Application, Code Section, Code Provision, Information Required - Must be in permit documents, Location in Documents, Building Department Notes.

2015 Washington State Energy Code Compliance Forms for Commercial, R2 and R3 over 3 stories and all R1

Mechanical Fan System Power Allowance MECH-FANSYS-SUM. Project Title: East Bay Lot A Westman Mill. Date: 5/11/2018. HVAC Air Distribution System Schedule table with columns: System or Primary Supply, Speed Control, Description, System Total Nameplate HP, Fan Power Calculation Required.

2015 Washington State Energy Code Compliance Forms for Commercial, R2 and R3 over 3 stories and all R1

Economizer Exceptions MECH-ECONO. Project Title: East Bay Lot A Westman Mill. Date: 5/11/2018. Section 10: Economizer Exceptions - New Construction and Additions. Section 11: Economizer Exceptions - Mechanical System Alterations or Replacement. Section 12: Economizer Exceptions - Simple Systems.



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11/21/2017 Westman Mill - Mixed

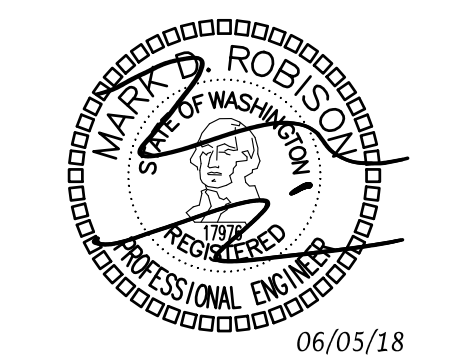
Table: Load Total Summary - System (Includes Ventilation and Plenum Loads). Columns include Location, Area, CFM, Peak, Cooling (Total, Sensible, Latent), Heating (Total, Sensible, Latent), and other metrics.

11/21/2017 Westman Mill - Mixed

Table: Cooling Load Details - Room (Break % of Total). Columns include Location, Peak, Roof, Wall, Glass, Horizontal Partitions, Lighting, Equipment, and Infiltration.

Approval stamp: TOWNZEN & ASSOCIATES PLAN APPROVAL. Includes date 09/25/2019 and signature.

ROBISON ENGINEERING, INC. 19401 40TH AVE W., SUITE 302 LYNWOOD, WA 98036



Olympia Building Plans Examiner Community Planning & Development Department

file:///F:/751-001%20Westman%20Mills%20Apartments/Dwg/dm_hvac-loads1.html

11/12

11/21/2017 Westman Mill - Mixed

Table: Load Total Summary - Room (Excludes Ventilation and Plenum Loads). Columns include Location, Area, CFM, Peak, Cooling, Heating, and other metrics.

file:///F:/751-001%20Westman%20Mills%20Apartments/Dwg/dm_hvac-loads1.html

9/12

11/21/2017 Westman Mill - Mixed

Table: Heating Load Details - System and Room (Break % of System Total). Columns include Location, Roof, Wall, Glass, Slab, Horizontal Partitions, Ventilation, and Infiltration.

file:///F:/751-001%20Westman%20Mills%20Apartments/Dwg/dm_hvac-loads1.html

12/12

file:///F:/751-001%20Westman%20Mills%20Apartments/Dwg/dm_hvac-loads1.html

10/12

EAST BAY LOT A WESTMAN MILL 510 STATE AVE OLYMPIA, WA, 98501

Project No: 1514 PERMIT SET 5/16/18

Table: Revision Log with columns Rev#, Date, and Description. Includes entries for 01/30/19 PERMIT COMMENT, 01/30/19 OTHER CHANGES, 06/12/19 GREASE WASTE, and 09/03/19 ASI-3.

LOAD CALCULATIONS

M611

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TABLES

WASHINGTON STATE-COMMERCIAL ENERGY CODE EFFICIENT HEATED WATER SUPPLY PIPING (1)(2)(3)(8)

| NOMINAL PIPE SIZE (IN) | PIPE LENGTH METHOD (RECOMMENDED) | | PIPE VOLUME METHOD | | | |
|------------------------|---|----------------|-----------------------------------|---|----------------|--|
| | MAXIMUM ALLOWABLE PIPING LENGTH (4)(7) (FT) | | PIPE VOLUME (7) (FLUID OZ / FEET) | MAXIMUM ALLOWABLE PIPING LENGTH (5)(6) (FT) | | |
| | PUBLIC LAVATORY FAUCET | OTHER FIXTURES | | PUBLIC LAVATORY FAUCET | OTHER FIXTURES | |
| 3/8 | 8 | 33 | 0.79 | 0.37 | 83 | |
| 1/2 | 2 | 43 | 1.5 | 0.33 | 43 | |
| 5/8 | 1 | 32 | 2 | 0.25 | 32 | |
| 3/4 | 0.5 | 21 | 3 | 0.17 | 21 | |
| 7/8 | 0.5 | 16 | 4 | 0.13 | 16 | |
| 1 | 0.5 | 13 | 5 | 0.10 | 13 | |
| 1-1/4 | 0.5 | 8 | 8 | 0.06 | 8 | |
| 1-1/2 | 0.5 | 6 | 11 | 0.05 | 6 | |
| 2 OR LARGER | 0.5 | 4 | 18 | 0.03 | 4 | |

NOTES:

- CONTRACTOR MAY USE METHOD 1 OR 2 TO DETERMINE MAXIMUM ALLOWABLE PIPING LENGTH FROM SOURCE OF HEATED WATER.
- PER 2015 WSEC SECTION C404.3 WATER HEATER, CIRCULATING WATER SYSTEM & HEAT TRACE TEMPERATURE MAINTENANCE SHALL BE CONSIDERED SOURCE OF HEATED WATER.
- THIS TABLE IS BASED ON MINIMUM CODE REQUIREMENT. CONTRACTOR SHALL FOLLOW OWNERSHIP/DEVELOPER REQUIREMENT AND/OR BRAND STANDARD REGARDING MAXIMUM WAITING TIME FOR HOT WATER DELIVERY [OR ALLOWABLE NON-CIRCULATING HOT WATER PIPING LENGTH] AS LONG AS IT IS STRICTER THAN CODE MINIMUM. CONTACT ENGINEERING AS NECESSARY.
- PIPE LENGTH METHOD ONLY: WHERE THE PIPING CONTAINS MORE THAN ONE SIZE OF PIPE, THE LARGEST SIZE OF PIPE SHALL BE USED FOR DETERMINING THE MAXIMUM ALLOWABLE LENGTH OF PIPING.
- PIPE VOLUME METHOD ONLY: PER WSEC SECTION C404.3.2 THE VOLUME FROM HEATED WATER TO THE TERMINATION OF FIXTURE SUPPLY PIPE SHALL NOT EXCEED 2 FLUID OUNCES FOR PUBLIC LAVATORIES AND 0.5 GALLON (64 FLUID OUNCES) FOR OTHER FIXTURES.
- PIPE VOLUME METHOD ONLY: PER C404.3.2.1 WATER VOLUME SHALL BE THE SUM OF INTERNAL VOLUMES OF PIPE, VALVES, METERS AND MANIFOLD BETWEEN THE NEAREST SOURCE OF HEATED WATER AND TERMINATION OF THE FIXTURE SUPPLY PIPE. PROVIDED CALCULATION DOES NOT INCLUDE VALVES, METERS, MANIFOLDS.
- PER WSEC TABLE C404.3.1
- REFER TO MANUFACTURER RECOMMENDATIONS AND PLUMBING FIXTURE SCHEDULE IN COMPLIANCE WITH 2015 UPC SECTION A106 AND TABLES 610.3 & A2.1 FOR MINIMUM BRANCH PIPE SIZES.

REQUIRED STANDARDS FOR PLUMBING FIXTURES AND FIXTURE FITTINGS

| | |
|--|--|
| WATER CLOSETS – FLUSHOMETER VALVE TYPE | ASME A 112.19.2/CSA B45.1 1.28 GAL (4.8 L) |
| WATER CLOSETS – TANK TYPE | U.S. EPA WATERSENSE TANK-TYPE HIGH-EFFICIENCY TOLIET SPECIFICATION |
| PUBLIC LAVATORY FAUCETS: MAXIMUM FLOW RATE – 0.5 GPM (1.9 L/MIN) | ASME A 112.18.1/CSA B125.1 |
| SHOWERHEADS: MAXIMUM FLOW RATE – 2.5 GPM (9.5 L/MIN) | ASME A 112.18.1/CSA B125.1 |
| THERMOSTATIC MIXING VALVE | ASSE 1070/CSA B125.3 (1) |

- NOTE: (1) APPLICABLE TO THE NON-RESIDENTIAL PORTIONS OF THE BUILDING
(2) REFER TO 2015 UPC SECTION 421.2.

2015 WASHINGTON STATE PIPE INSULATION SCHEDULE (1)(2)

| SERVICE | OPTION 1 | | OPTION 2 | | VAPOR RETARDER REQUIRED | NOTES |
|---|---|---|----------|---|-------------------------|--------------------------|
| | MATERIAL | THICKNESS | MATERIAL | THICKNESS | | |
| DOMESTIC COLD WATER, IRRIGATION WATER, CONDENSATE DRAINS, STORM DRAIN (IN CONDITIONED SPACE) | MINERAL-FIBER WITH JACKET | ALL SIZES: 1/2" | PVC/NBR | ALL SIZES: 3/8" | YES | (9)(10) |
| DOMESTIC COLD WATER, IRRIGATION WATER, CONDENSATE DRAINS, WASTE (OUTSIDE THE CONDITIONED SPACE) | MINERAL-FIBER WITH JACKET | (R-3) 1/2" PIPE: 1/2" ALL OTHER SIZES: 1" | PVC/NBR | (R-3) 1/2" PIPE: 1/2" ALL OTHER SIZES: 3/4" | YES | (5)(6)(8) |
| ROOF DRAIN BODIES | MINERAL-FIBER OR CELLULAR GLASS WITH JACKET | 1" | PVC/NBR | 1" | YES | (9) |
| DOMESTIC HOT WATER AND RECIRCULATED HOT WATER (RESIDENTIAL) | MINERAL-FIBER WITH JACKET | (R-3) 1/2" PIPE: 1/2" ALL OTHER SIZES: 1" | PVC/NBR | (R-3) 1/2" PIPE: 1/2" ALL OTHER SIZES: 3/4" | NO | (3)(8) |
| DOMESTIC HOT WATER AND RECIRCULATED HOT WATER (NONRESIDENTIAL) | MINERAL-FIBER WITH JACKET | 1/2"-1 1/4" PIPE: 1" 1 1/2"-4" PIPE: 1.5" | PVC/NBR | 1/2"-1 1/4" PIPE: 1" 1 1/2"-4" PIPE: 1.5" | NO | (4)(7) |
| EXPOSED SANITARY DRAINS AND DOMESTIC WATER SUPPLIES AND STOPS FOR ADA FIXTURES. | TRUEBRO LAV-GUARD | N/A | N/A | N/A | NO | P-TRAP AND SUPPLY COVERS |

NOTES:

- FOR APPLICABLE CODES REFER TO PLUMBING COVER SHEET.
- PIPING INSULATION EXPOSED TO WEATHER SHALL BE PROTECTED FROM DAMAGE. CONTRACTOR SHALL PROVIDE SHIELDING FROM SOLAR RADIATION THAT CAN CAUSE DEGRADATION OF THE MATERIAL. ADHESIVE TAPE SHALL NOT BE PERMITTED.
- PER 2015 WSEC SECTION R403.5.3 (RESIDENTIAL-PERSPECTIVE) INSULATION FOR HOT WATER PIPE SHALL HAVE A MINIMUM R-VALE OF R-3.
- PER 2015 WSEC SECTION C404.6 (NON-RESIDENTIAL):
 - PIPING FROM WATER HEATER TO THE TERMINATION OF HEATED WATER SUPPLY PIPE SHALL BE INSULATED IN ACCORDANCE WITH TABLE C403.2.9.
 - ON BOTH THE INLET AND OUTLET PIPING OF A STORAGE HOT WATER HEATER, THE FIRST 8 FEET OF PIPING OR PIPING FROM WATER HEATER TO HEAT TRAP SHALL BE INSULATED.
 - HEAT TRACED PIPING SHALL BE INSULATED IN THE SAME MANNER AS NOT HEAT TRACED PIPING OR PER HEAT TRACE MANUFACTURER SPEC.
 - TUBULAR PIPING INSULATION SHALL NOT BE REQUIRED FOR THE FOLLOWING:
 - TUBING FROM FROM THE CONNECTION AT THE TERMINATION OF THE FIXTURE PIPING TO A PLUMBING FIXTURE.
 - VALVES, PUMPS, STRAINERS AND THREADED UNIONS IN PIPING THAT IS 1 INCH OR LESS.
 - PIPING FROM USER CONTROLLED SHOWER AND BATH MIXING VALVES TO THE WATER OUTLET.
 - COLD WATER PIPING OF A DEMAND RETICULATION WATER SYSTEM
 - TUBING FROM A HOT DRINKING WATER UNIT TO THE WATER OUTLET.
 - PIPING AT LOCATION WHERE A VERTICAL SUPPORT OF PIPING IS INSTALLED.
 - PIPING SURROUNDED BY BUILDING INSULATION WITH R-VALUE OF NOT LESS THAT R-3.
- PER 2015 WSPC, SECTION 312.6 NO WATER, SOIL OR WASTE PIPE SHALL BE INSTALLED OR PERMITTED OUTSIDE OF THE BUILDING, IN ATTIC OR IN AN EXTERIOR WALL UNLESS ADEQUATE PROVISION IS MADE TO PROTECT SUCH PIPE FROM FREEZING. ALL HOT AND COLD WATER PIPES OUTSIDE THE CONDITIONED SPACE SHALL BE INSULATED TO MINIMUM R-3.
- HEAT TRACING SHALL BE PROVIDED FOR COLD WATER AND IRRIGATION WATER IN UNCONDITIONED SPACES. REFER TO HEAT TRACING SCHEDULE FOR DETAIL. CONTACT ENGINEERING IF NECESSARY. PER 2015 WSEC SECTION C403.2.4.6 FREEZE PROTECTION SYSTEMS, SUCH AS HEAT TRACING OF OUTDOOR PIPING SHALL INCLUDE AUTOMATIC CONTROLS CONFIGURED TO SHUT OFF THE SYSTEM WHEN OUTDOOR AIR TEMPERATURES ARE ABOVE 40°F.
- PER 2015 WSEC TABLE C403.2.9 (FOR NON-RESIDENTIAL) INSULATION FOR HOT WATER AND HOT WATER RETICULATION SHALL HAVE CONDUCTIVITY OF 0.21-0.28 (BTU.IN/H.FT².F) AT OPERATING TEMPERATURE.
- INSULATION R-VALUE SHALL MEET MINIMUM REQUIREMENT. THICKNESS IS BASED ON GRAINGER SAMPLE DATA FOR K-FLEX(PVC/NBR) AND OWENS CORNING(FIBER GLASS).
- REQUIRED BY ENGINEERING BASED ON BEST PRACTICE.
- INSULATION IS NOT REQUIRED ON PLASTIC PIPING.

PIPING SUPPORTS

ALL SUSPENDED WATER SUPPLY PIPE SHALL BE SUPPORTED AS FOLLOWS:

| MATERIAL | MAX. HORIZONTAL SPACING | MAX. VERTICAL SPACING |
|------------------------|-------------------------|-----------------------|
| COPPER PIPE | 12 FT. | 10 FT. |
| COPPER TUBING ≤ 1 1/4" | 6 FT. | 10 FT. |
| COPPER TUBING > 1 1/2" | 10 FT. | 10 FT. |
| CPVC ≤ 1" | 3 FT. | 10 FT. |
| CPVC ≥ 1 1/4" | 4 FT. | 10 FT. |

ALL SUSPENDED SANITARY AND VENT PIPE SHALL BE SUPPORTED AS FOLLOWS:

| MATERIAL | MAX. HORIZ. SPACING | MAX. VERT. SPACING |
|-----------------------------------|---------------------|--------------------|
| ABS | 4 FT. | 10 FT. |
| PVC (TYPE DWV) | 4 FT. | 10 FT. |
| CAST-IRON (<10 FT. PIPE SECTIONS) | 5 FT. | 15 FT. |
| CAST-IRON (10 FT. PIPE SECTIONS) | 10 FT. | 15 FT. |

MAXIMUM FIXTURE FLOW RATES (1)

| FIXTURE | FLOW RATE |
|------------------------------------|---------------|
| SHOWERHEADS | 2.5 GPM |
| LAVATORIES – RESIDENTIAL | 2.5 GPM |
| LAVATORIES – PUBLIC (NON-METERING) | 0.5 GPM |
| LAVATORIES – PUBLIC (2) | 0.5 GPM |
| KITCHEN FAUCETS | 2.5 GPM |
| WATER CLOSETS | 1.6 GAL/FLUSH |

- NOTES: (1) FLOW RATES PER 2015 WAC 51-56-0400
(2) PER 2015 WAC 51-56 SECTION 407.4 LAVATORY FAUCET INTENDED FOR USE BY THE GENERAL PUBLIC SHALL BE EQUIPPED WITH SELF CLOSING METERING VALVE.

**TOWNZEN & ASSOCIATES
PLAN APPROVAL**

The plans submitted for review are approved in accordance with local state applicable standards. This approval does not relieve the applicant of the responsibility of compliance with the applicable codes.

Approved as submitted. 09/25/2019 *Joe Lanna*

ROBISON ENGINEERING, INC.

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(360) 753-8268
robison@cityofolympia.wa.us

EAST BAY LOT A
WESTMAN MILL
510 STATE AVE OLYMPIA, WA. 98501

Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |

TABLES

P004

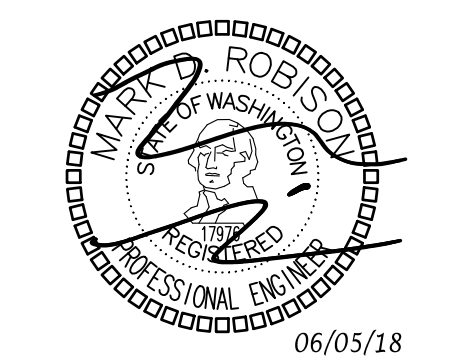


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TOWNZEN & ASSOCIATES
PLAN APPROVAL
The plans submitted for review are approved in accordance with local state applicable standards. This approval does not relieve the applicant of the responsibility of compliance with the applicable codes.
Approved as submitted. 09/25/2019

ROBISON
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19401 40TH AVE W, SUITE 302
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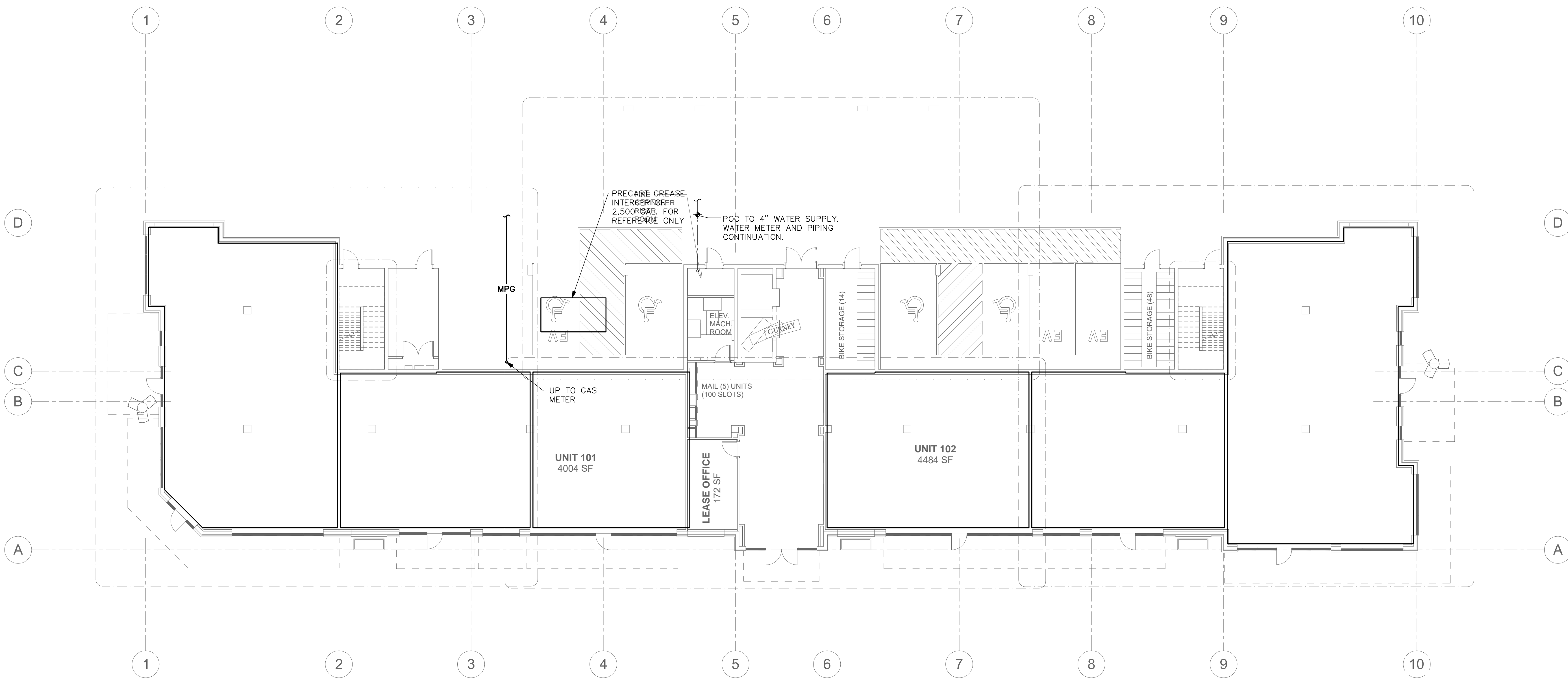
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UNDERSLAB
PLUMBING PLAN-
SUPPLY

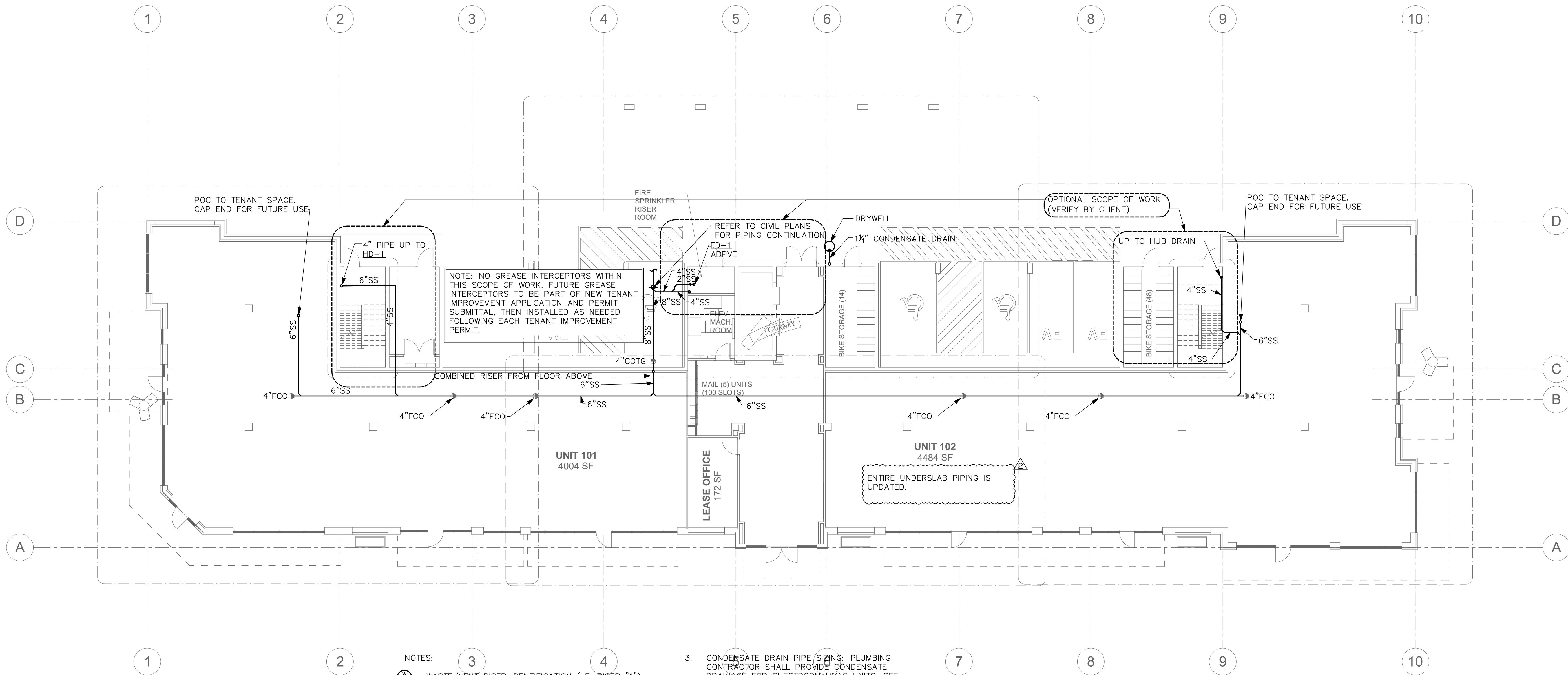
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| SUPPLY PIPE SIZING SCHEDULE | | | | | | | | Copper Type: Type L | | | |
|-----------------------------|-----------|----------|---------------|-----------|----------|---------------|-----------|---------------------|---------------|--|--|
| FLUSH TANK CW | | | | HOT WATER | | | | FLUSH VALVE CW | | | |
| PIPE SIZE | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS | FIXTURE UNITS | | |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- | | |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- | | |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- | | |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- | | |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 | | |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 | | |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 | | |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 | 564.0 | | |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 | | |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 5250.0 | | |

UNDERSLAB SUPPLY PLAN
SCALE: 3/32" = 1'-0"
0' 4' 8' 16'



NOTES:
⊕ = WASTE/VENT RISER IDENTIFICATION (I.E. RISER "1"). REFER TO P400-P401 FOR RISER DETAILS AND SIZING.

1. STORM DRAIN SIZING: STORM DRAINAGE PIPING SIZED PER 2015 UPC TABLE 1101.7, FOR 1.0" PER HOUR RAINFALL RATE, AT 1/8" PER FOOT SLOPE:

| PIPE SIZE | VERTICAL | HORIZONTAL |
|-----------|------------|------------|
| 3" | 8,800 SF | 3,288 SF |
| 4" | 18,400 SF | 7,520 SF |
| 6" | 54,000 SF | 21,400 SF |
| 8" | 116,000 SF | 46,000 SF |
| 10" | 116,000 SF | 82,800 SF |

2. WASTE & VENT SIZING: WASTE & VENT PIPING SIZED PER 2015 UPC TABLE 703.2. PIPING SHALL BE SLOPED AT 1/4" PER FOOT:

| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1½" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 216 DFU | 256 DFU |
| 6" | 1,380 DFU | 720 DFU | 1,380 DFU |
| 8" | 3,600 DFU | 2,640 DFU | 3,600 DFU |

3. CONDENSATE DRAIN PIPE SIZING: PLUMBING CONTRACTOR SHALL PROVIDE CONDENSATE DRAINAGE FOR GUESTROOM HVAC UNITS. SEE MECHANICAL DRAWINGS FOR EXACT QUANTITY OF UNITS. SEE P601 FOR CONDENSATE RISER DIAGRAMS. CONDENSATE DRAIN PIPING SHALL BE SLOPED AT 1/8" PER FOOT. CONDENSATE DRAIN PIPING SIZED PER 2015 UPC TABLE 814.1:

| PIPE SIZE | (TONS OF COOLING) |
|-----------|-------------------|
| ¾" | 20 |
| 1" | 40 |
| 1¼" | 90 |
| 1½" | 125 |
| 2" | 250 |

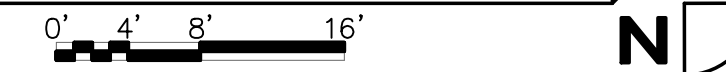
4. PROVIDE TRAP PRIMERS FOR ALL FLOOR DRAINS, HUB DRAINS SEE DETAIL 8 ON SHEET P600.

5. PROVIDE FLOOR DRAINS AT ALL ROLL-IN SHOWER BATHROOMS.

6. REFER TO ARCH PLANS LOCATION AND REQUIREMENT ON ADA TYPE A AND TYPE B UNITS.

UNDERSLAB PLUMBING WASTE PLAN

SCALE: 3/32" = 1'-0"



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| 3 | 06/12/19 | GREASE WASTE |

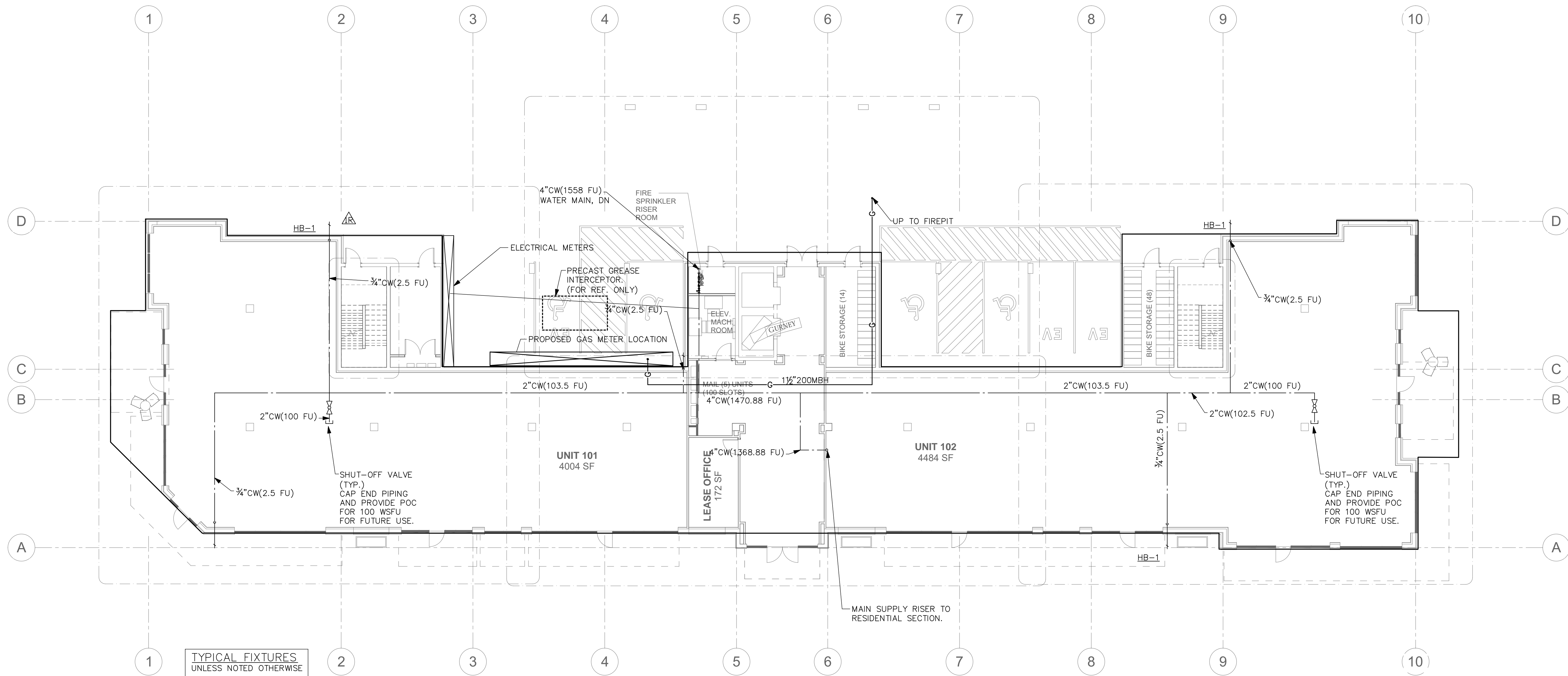
UNDERSLAB PLUMBING PLAN - WASTE & VENT

P200W

**TOWNZEN & ASSOCIATES
 PLAN APPROVAL**

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Approved as submitted. 09/25/2019



TYPICAL FIXTURES
 UNLESS NOTED OTHERWISE

- WH-1
- KS-1
- KR-1
- LAV-1
- WM-1
- BT-1
- SH-1
- WC-1
- HB-1
- FD-1
- HD-1
- DW-1
- RF-1
- MS-1

| SUPPLY PIPE SIZING SCHEDULE | | | | | | | | Copper Type: Type L | | |
|-----------------------------|---------------|----------|---------------|-----------|-----------|---------------|-----------|---------------------|---------------|--|
| PIPE SIZE | FLUSH TANK CW | | | | HOT WATER | | | FLUSH VALVE CW | | |
| | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS | |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- | |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- | |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- | |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- | |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 | |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 | |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 | |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 390.0 | 150.0 | 7.4 | 564.0 | |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 | |
| 6" | 650.0 | 8.0 | 6250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 5250.0 | |

FIRST FLOOR SUPPLY PLAN

SCALE: 3/32" = 1'-0"



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**EAST BAY LOT A
 WESTMAN MILL**
 510 STATE AVE OLYMPIA, WA, 98501

Project No: 1514
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FIRST FLOOR
 PLUMBING PLAN-
 SUPPLY

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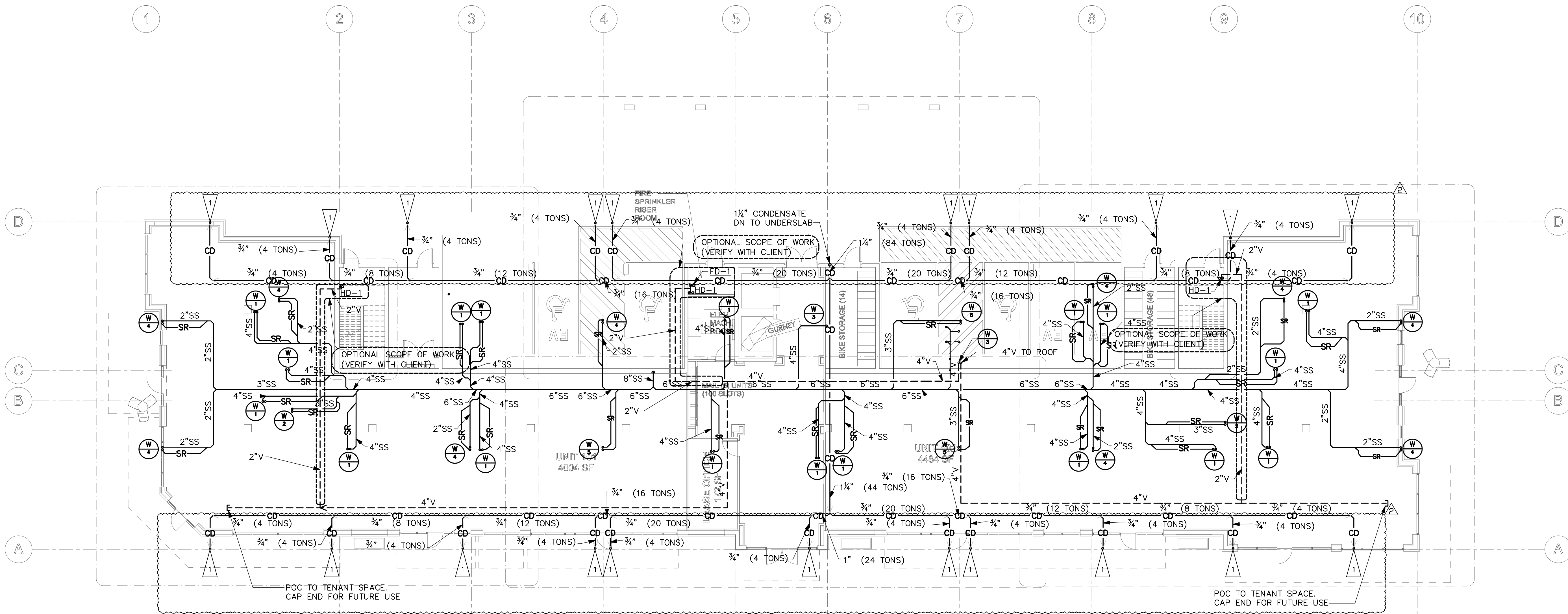
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FIRST FLOOR
PLUMBING PLAN-
WASTE & VENT

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**TYPICAL FIXTURES
UNLESS NOTED OTHERWISE**

| | |
|--|-------|
| | WH-1 |
| | KS-1 |
| | KR-1 |
| | LAV-1 |
| | WM-1 |
| | BT-1 |
| | SH-1 |
| | WC-1 |
| | HB-1 |
| | FD-1 |
| | HD-1 |
| | DW-1 |
| | RF-1 |
| | MS-1 |

NOTES:

⊕ = WASTE/VENT RISER IDENTIFICATION (I.E. RISER "1"). REFER TO P400-P401 FOR RISER DETAILS AND SIZING.

1. STORM DRAIN SIZING: STORM DRAINAGE PIPING SIZED PER 2015 UPC TABLE 1101.7, FOR 1.0" PER HOUR RAINFALL RATE, AT 1/8" PER FOOT SLOPE:

| PIPE SIZE | VERTICAL | HORIZONTAL |
|-----------|------------|------------|
| 3" | 8,800 SF | 3,288 SF |
| 4" | 18,400 SF | 7,520 SF |
| 6" | 54,000 SF | 21,400 SF |
| 8" | 116,000 SF | 46,000 SF |
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| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1 1/2" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 216 DFU | 256 DFU |
| 6" | 1,380 DFU | 720 DFU | 1,380 DFU |
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| 3/4" | 20 |
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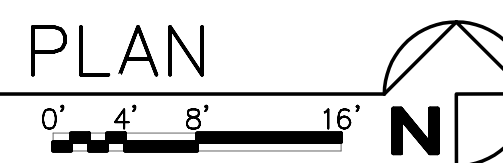
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- PROVIDE FLOOR DRAINS AT ALL ROLL-IN SHOWER BATHROOMS.
- REFER TO ARCH PLANS LOCATION AND REQUIREMENT ON ADA TYPE A AND TYPE B UNITS.

FLAG NOTES

- PROVIDE 3/4" CONDENSATE DRAIN CONNECTION FROM 1 TON PTHP TO 3/4" RISER.

FIRST FLOOR WASTE PLAN

SCALE: 3/32" = 1'-0"





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TOWNZEN & ASSOCIATES
PLAN APPROVAL
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 Approved as submitted. 09/25/2019 *[Signature]*

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06/05/18

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**EAST BAY LOT A
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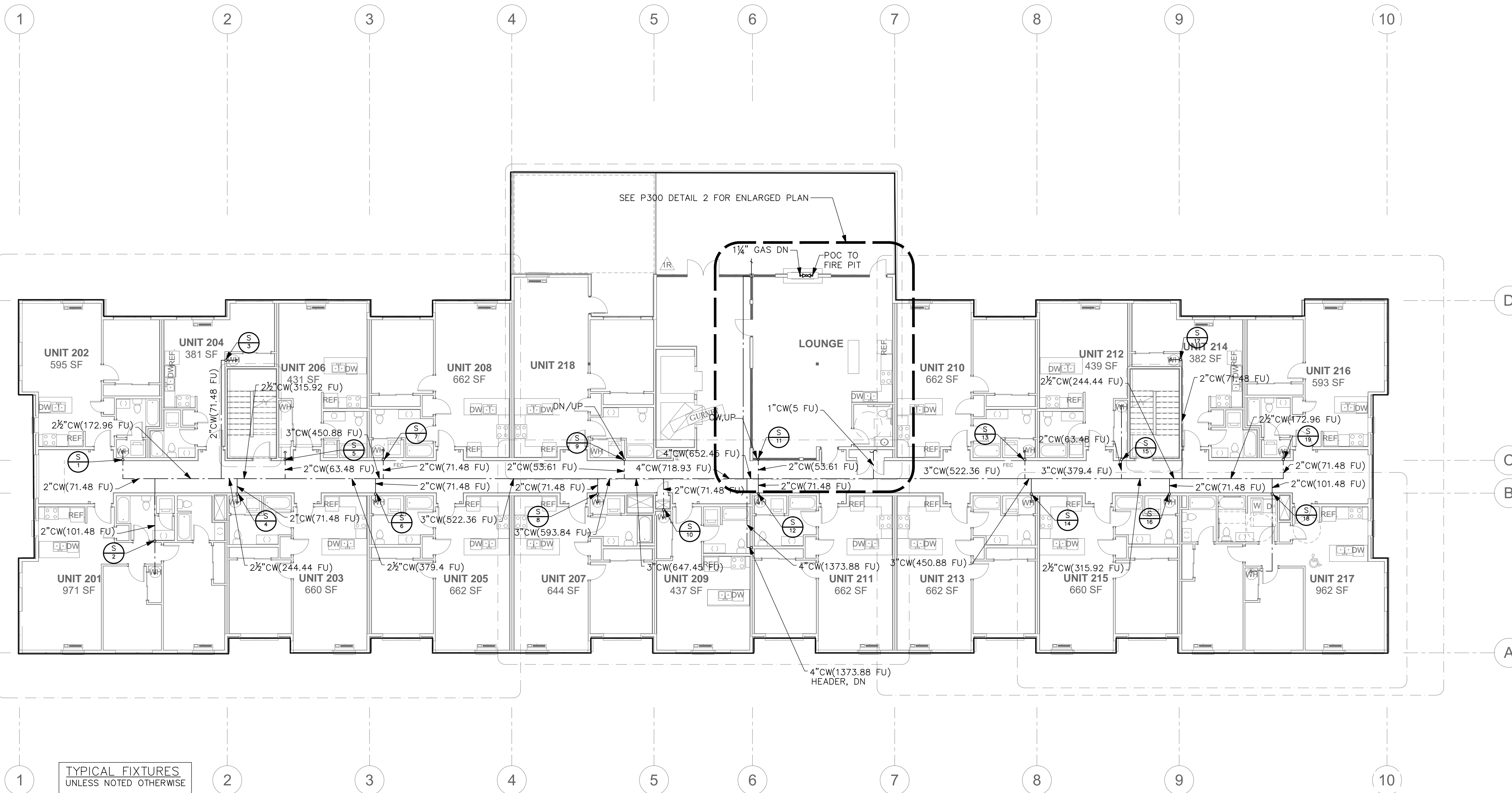
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**SECOND FLOOR
PLUMBING PLAN-
SUPPLY**

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TYPICAL FIXTURES
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- WH-1
- KS-1
- KR-1
- LAV-1
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- BT-1
- SH-1
- WC-1
- HB-1
- FD-1
- HD-1
- DW-1
- RF-1
- MS-1

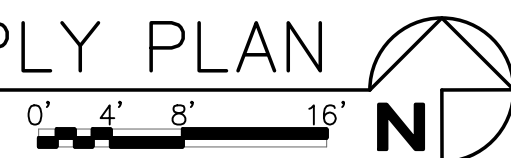
NOTES:

⊙ = SUPPLY RISER IDENTIFICATION (I.E. RISER "1"). REFER TO P501 FOR RISER DETAILS AND SIZING.

| SUPPLY PIPE SIZING SCHEDULE | | | | | | Copper Type: Type L | | | |
|-----------------------------|-----------|----------|---------------|-----------|----------|---------------------|-----------|----------|---------------|
| FLUSH TANK CW | | | HOT WATER | | | FLUSH VALVE CW | | | |
| PIPE SIZE | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS | FIXTURE UNITS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 | 564.0 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 |
| 6" | 650.0 | 8.0 | 6250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 6250.0 |

SECOND FLOOR SUPPLY PLAN

SCALE: 3/32" = 1'-0"





T H O M A S
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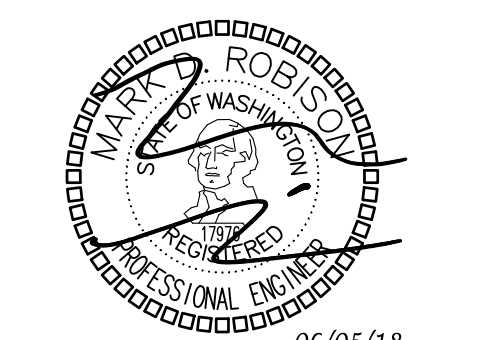
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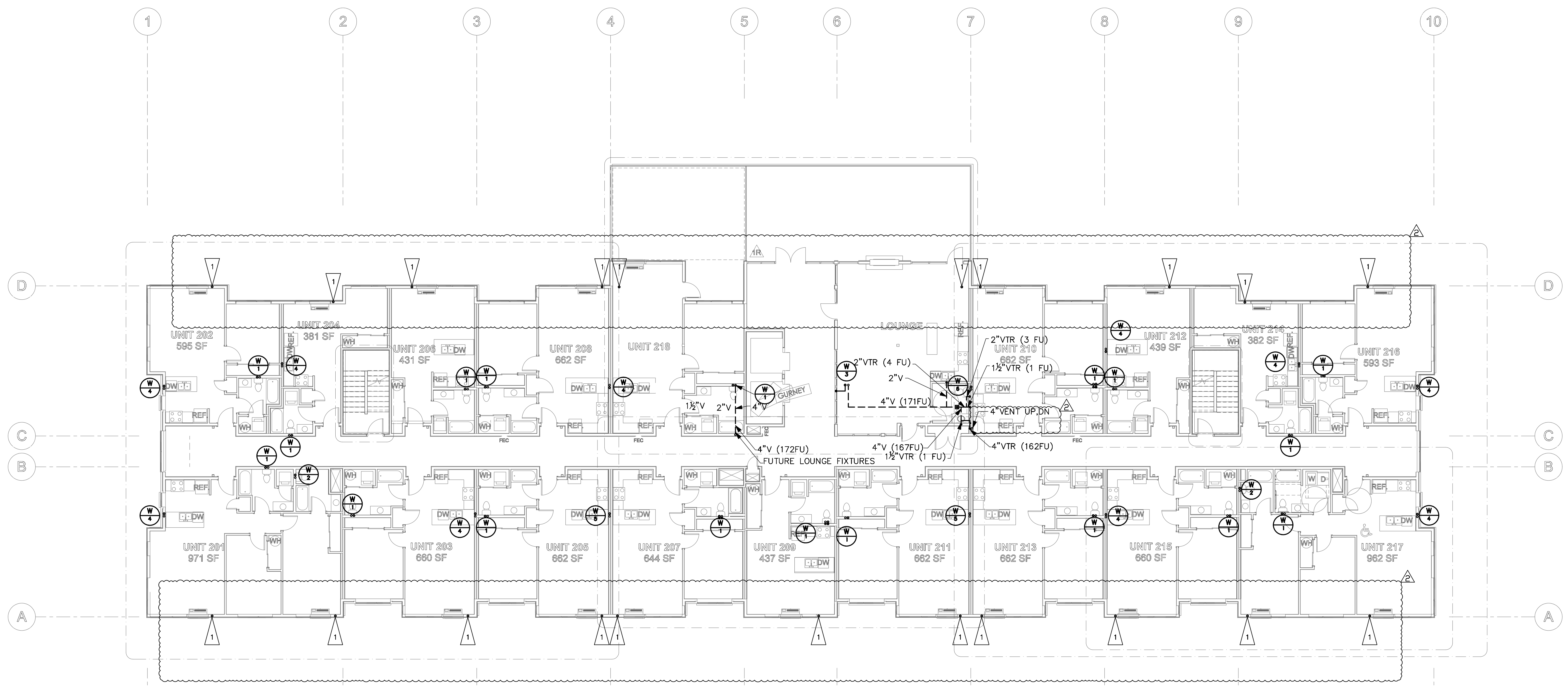
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SECOND FLOOR PLUMBING PLAN- WASTE & VENT

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TYPICAL FIXTURES
UNLESS NOTED OTHERWISE

| |
|-------|
| WH-1 |
| KS-1 |
| KR-1 |
| LAV-1 |
| WM-1 |
| BT-1 |
| SH-1 |
| WC-1 |
| HB-1 |
| FD-1 |
| HD-1 |
| DW-1 |
| RF-1 |
| MS-1 |

NOTES:

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| 4" | 18,400 SF | 7,520 SF |
| 6" | 54,000 SF | 21,400 SF |
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| 10" | 116,000 SF | 82,800 SF |

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| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1 1/2" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 216 DFU | 256 DFU |
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- | PIPE SIZE | (TONS OF COOLING) |
|-----------|-------------------|
| 3/4" | 20 |
| 1" | 40 |
| 1 1/4" | 90 |
| 1 1/2" | 125 |
| 2" | 250 |
4. PROVIDE TRAP PRIMERS FOR ALL FLOOR DRAINS, HUB DRAINS SEE DETAIL 8 ON SHEET P600.
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- FLAG NOTES**
1. PROVIDE 3/4" CONDENSATE DRAIN CONNECTION FROM 1 TON PTHP TO 3/4" RISER.

SECOND FLOOR WASTE PLAN

SCALE: 3/32" = 1'-0"



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PLAN APPROVAL

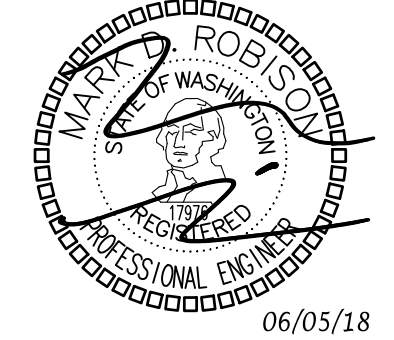
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Joe Lemo

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robison@olympia.wa.us

EAST BAY LOT A
WESTMAN MILL
510 STATE AVE OLYMPIA, WA., 98501

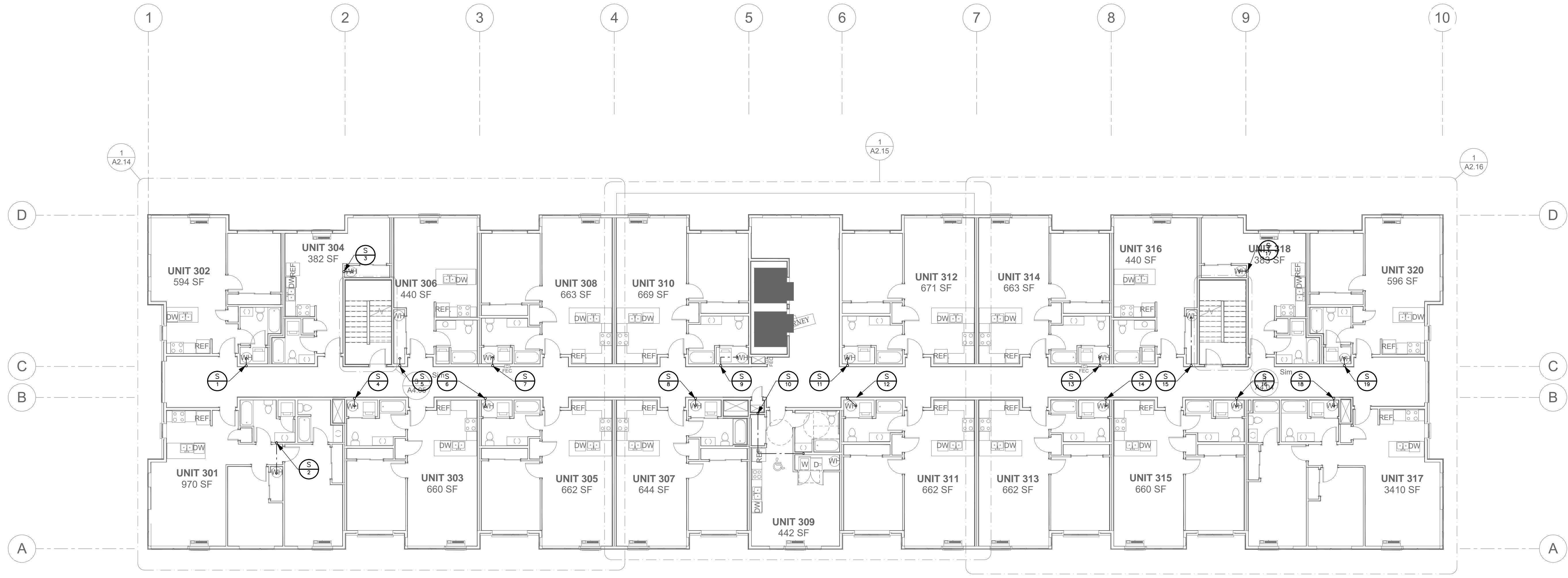
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |

THIRD FLOOR
PLUMBING PLAN-
SUPPLY

P203S

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TYPICAL FIXTURES
UNLESS NOTED OTHERWISE

- WH-1
- KS-1
- KR-1
- LAV-1
- WM-1
- BT-1
- SH-1
- WC-1
- HB-1
- FD-1
- HD-1
- DW-1
- RF-1
- MS-1

NOTES:

⊕ = SUPPLY RISER IDENTIFICATION (I.E. RISER "1").
REFER TO P501 FOR RISER DETAILS AND SIZING.

| SUPPLY PIPE SIZING SCHEDULE | | | | | | Copper Type: Type L | | | |
|-----------------------------|-----------|----------|---------------|-----------|----------|---------------------|-----------|----------|---------------|
| FLUSH TANK CW | | | HOT WATER | | | FLUSH VALVE CW | | | |
| PIPE SIZE | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS | FIXTURE UNITS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 380.0 | 150.0 | 7.4 | 564.0 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 5250.0 |

THIRD FLOOR SUPPLY PLAN

SCALE: 3/32" = 1'-0"





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**TOWNZEN & ASSOCIATES
PLAN APPROVAL**

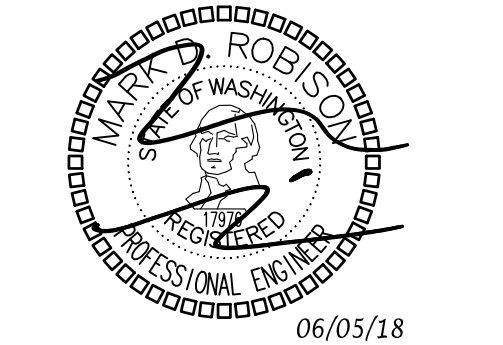
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Approved as submitted. 09/25/2019 *Jon Lango*

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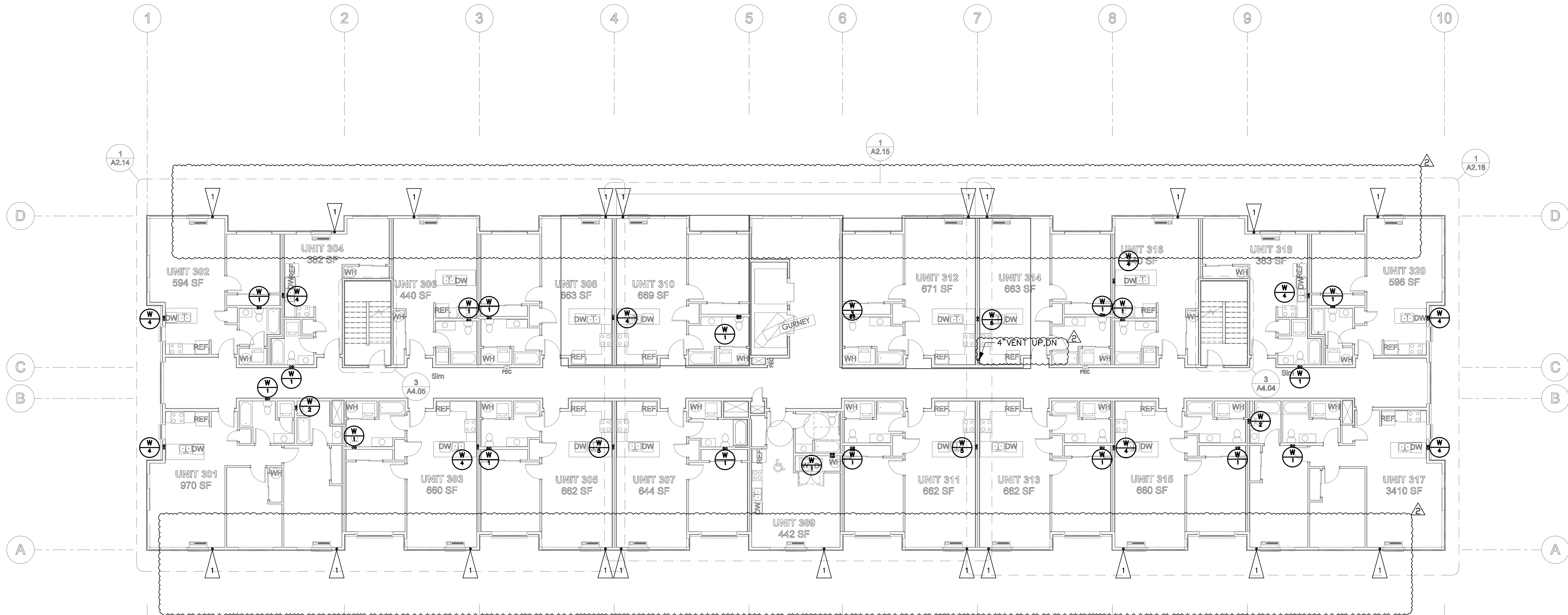
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**THIRD FLOOR
PLUMBING PLAN-
WASTE & VENT**

P203W

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- HD-1
- DW-1
- RF-1
- MS-1

NOTES:

1. STORM DRAIN SIZING: STORM DRAINAGE PIPING SIZED PER 2015 UPC TABLE 1101.7, FOR 1.0" PER HOUR RAINFALL RATE, AT 1/8" PER FOOT SLOPE:

| PIPE SIZE | VERTICAL | HORIZONTAL |
|-----------|------------|------------|
| 3" | 8,800 SF | 3,288 SF |
| 4" | 18,400 SF | 7,520 SF |
| 6" | 54,000 SF | 21,400 SF |
| 8" | 116,000 SF | 46,000 SF |
| 10" | 116,000 SF | 82,800 SF |

2. WASTE & VENT SIZING: WASTE & VENT PIPING SIZED PER 2015 UPC TABLE 703.2. PIPING SHALL BE SLOPED AT 1/4" PER FOOT:

| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1 1/2" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 216 DFU | 256 DFU |
| 6" | 1,380 DFU | 720 DFU | 1,380 DFU |
| 8" | 3,600 DFU | 2,640 DFU | 3,600 DFU |

3. CONDENSATE DRAIN PIPE SIZING: PLUMBING CONTRACTOR SHALL PROVIDE CONDENSATE DRAINAGE FOR GUESTROOM HVAC UNITS. SEE MECHANICAL DRAWINGS FOR EXACT QUANTITY OF UNITS. SEE P601 FOR CONDENSATE RISER DIAGRAMS. CONDENSATE DRAIN PIPING SHALL BE SLOPED AT 1/8" PER FOOT. CONDENSATE DRAIN PIPING SIZED PER 2015 UPC TABLE 814.1:

| PIPE SIZE | (TONS OF COOLING) |
|-----------|-------------------|
| 3/4" | 20 |
| 1" | 40 |
| 1 1/4" | 90 |
| 1 1/2" | 125 |
| 2" | 250 |

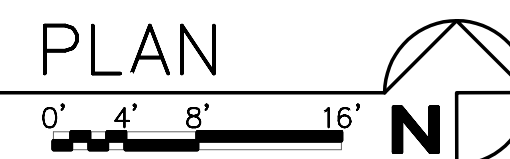
- 4. PROVIDE TRAP PRIMERS FOR ALL FLOOR DRAINS, HUB DRAINS SEE DETAIL 8 ON SHEET P600.
- 5. PROVIDE FLOOR DRAINS AT ALL ROLL-IN SHOWER BATHROOMS.
- 6. REFER TO ARCH PLANS LOCATION AND REQUIREMENT ON ADA TYPE A AND TYPE B UNITS.

FLAG NOTES

- 1. PROVIDE 3/4" CONDENSATE DRAIN CONNECTION FROM 1 TON PTHP TO 3/4" RISER.

THIRD FLOOR WASTE PLAN

SCALE: 3/32" = 1'-0"





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FOURTH FLOOR
PLUMBING PLAN-
SUPPLY

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TYPICAL FIXTURES
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- LAV-1
- WM-1
- BT-1
- SH-1
- WC-1
- HB-1
- FD-1
- HD-1
- DW-1
- RF-1
- MS-1

NOTES:

⊕ = SUPPLY RISER IDENTIFICATION (I.E. RISER "1"). REFER TO P501 FOR RISER DETAILS AND SIZING.

| PIPE SIZE | FLUSH TANK CW | | | HOT WATER | | | FLUSH VALVE CW | | |
|-----------|---------------|----------|---------------|-----------|----------|---------------|----------------|----------|---------------|
| | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 | 564.0 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 5250.0 |

FOURTH FLOOR SUPPLY PLAN

SCALE: 3/32" = 1'-0"





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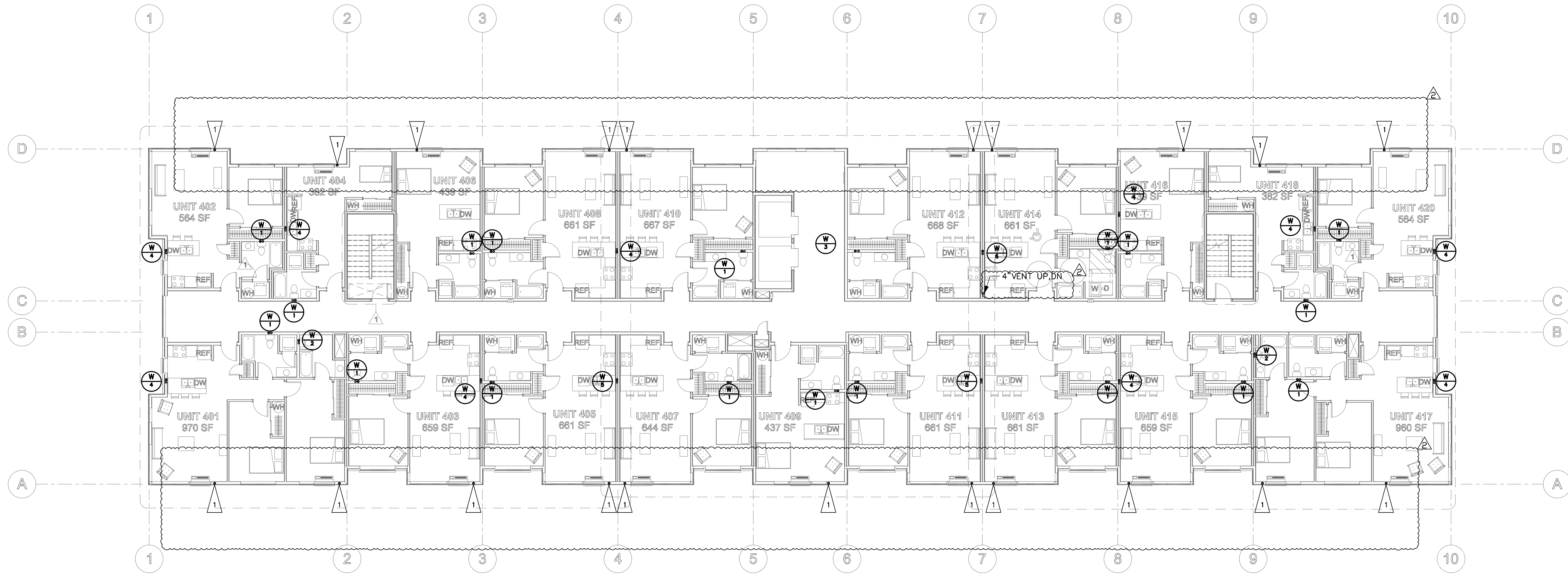
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**FOURTH FLOOR
PLUMBING PLAN-
WASTE & VENT**

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**TYPICAL FIXTURES
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- WM-1
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- WC-1
- HB-1
- FD-1
- HD-1
- DW-1
- RF-1
- MS-1

NOTES:

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1. STORM DRAIN SIZING: STORM DRAINAGE PIPING SIZED PER 2015 UPC TABLE 1101.7, FOR 1.0" PER HOUR RAINFALL RATE, AT 1/8" PER FOOT SLOPE:

| PIPE SIZE | VERTICAL | HORIZONTAL |
|-----------|------------|------------|
| 3" | 8,800 SF | 3,288 SF |
| 4" | 18,400 SF | 7,520 SF |
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| 10" | 116,000 SF | 82,800 SF |

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| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1 1/2" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 216 DFU | 256 DFU |
| 6" | 1,380 DFU | 720 DFU | 1,380 DFU |
| 8" | 3,600 DFU | 2,640 DFU | 3,600 DFU |

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| PIPE SIZE | (TONS OF COOLING) |
|-----------|-------------------|
| 3/4" | 20 |
| 1" | 40 |
| 1 1/4" | 90 |
| 1 1/2" | 125 |
| 2" | 250 |

- PROVIDE TRAP PRIMERS FOR ALL FLOOR DRAINS, HUB DRAINS SEE DETAIL 8 ON SHEET P600.
- PROVIDE FLOOR DRAINS AT ALL ROLL-IN SHOWER BATHROOMS.
- REFER TO ARCH PLANS LOCATION AND REQUIREMENT ON ADA TYPE A AND TYPE B UNITS.

FLAG NOTES

1. PROVIDE 3/4" CONDENSATE DRAIN CONNECTION FROM 1 TON PTHP TO 3/4" RISER.

FOURTH FLOOR WASTE PLAN

SCALE: 3/32" = 1'-0"





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TOWNZEN & ASSOCIATES
PLAN APPROVAL

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FIFTH FLOOR
PLUMBING PLAN-
SUPPLY

P205S

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TYPICAL FIXTURES
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- WH-1
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- LAV-1
- WM-1
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- SH-1
- WC-1
- HB-1
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- DW-1
- RF-1
- MS-1

NOTES:

⊕ = SUPPLY RISER IDENTIFICATION (I.E. RISER "1"). REFER TO P501 FOR RISER DETAILS AND SIZING.

| SUPPLY PIPE SIZING SCHEDULE | | | | | | | | |
|-----------------------------|---------------|----------|---------------|-----------|----------|---------------|---------------------|----------|
| PIPE SIZE | FLUSH TANK CW | | | HOT WATER | | | Copper Type: Type L | |
| | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS | FIXTURE UNITS | FLOW, GPM | VEL. FPS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 |

FIFTH FLOOR SUPPLY PLAN

SCALE: 3/32" = 1'-0"





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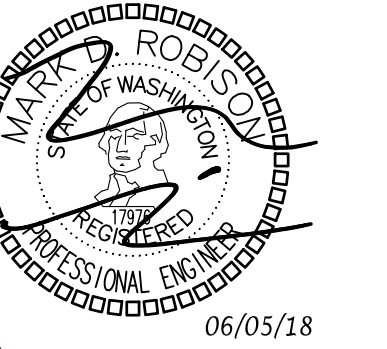
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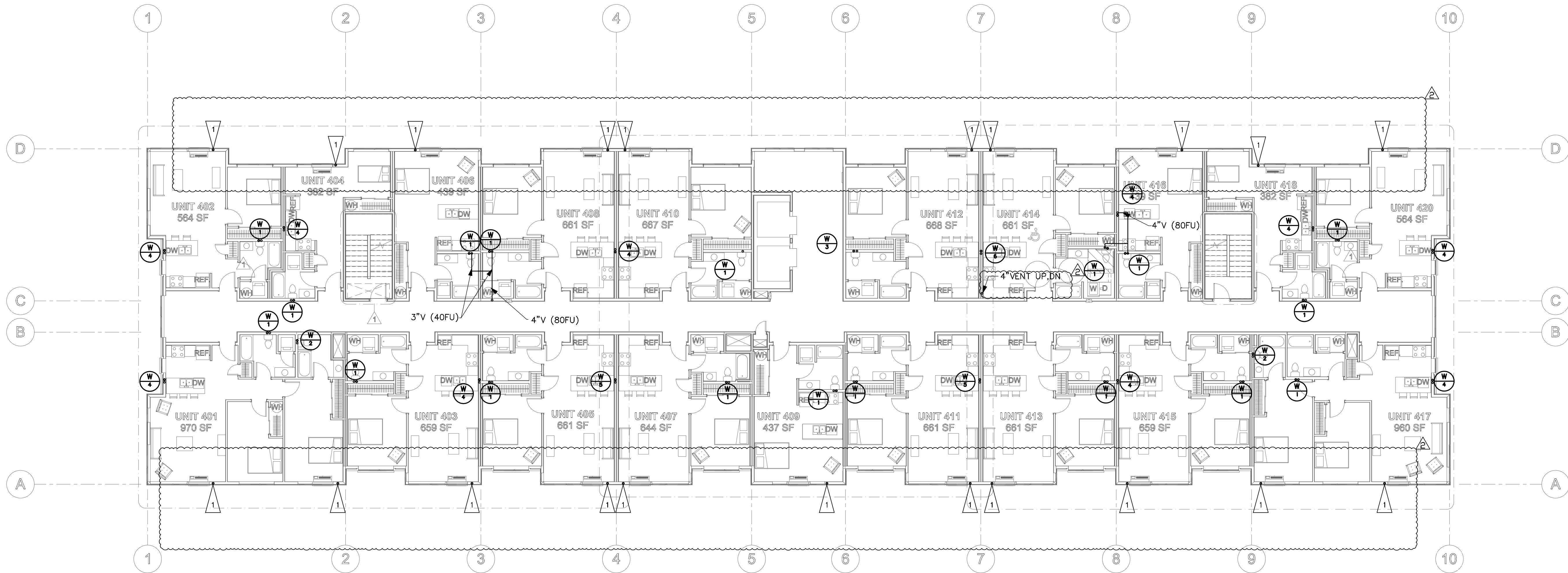
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FIFTH FLOOR
PLUMBING PLAN-
WASTE & VENT

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FIFTH FLOOR WASTE PLAN

SCALE: 3/32" = 1'-0"





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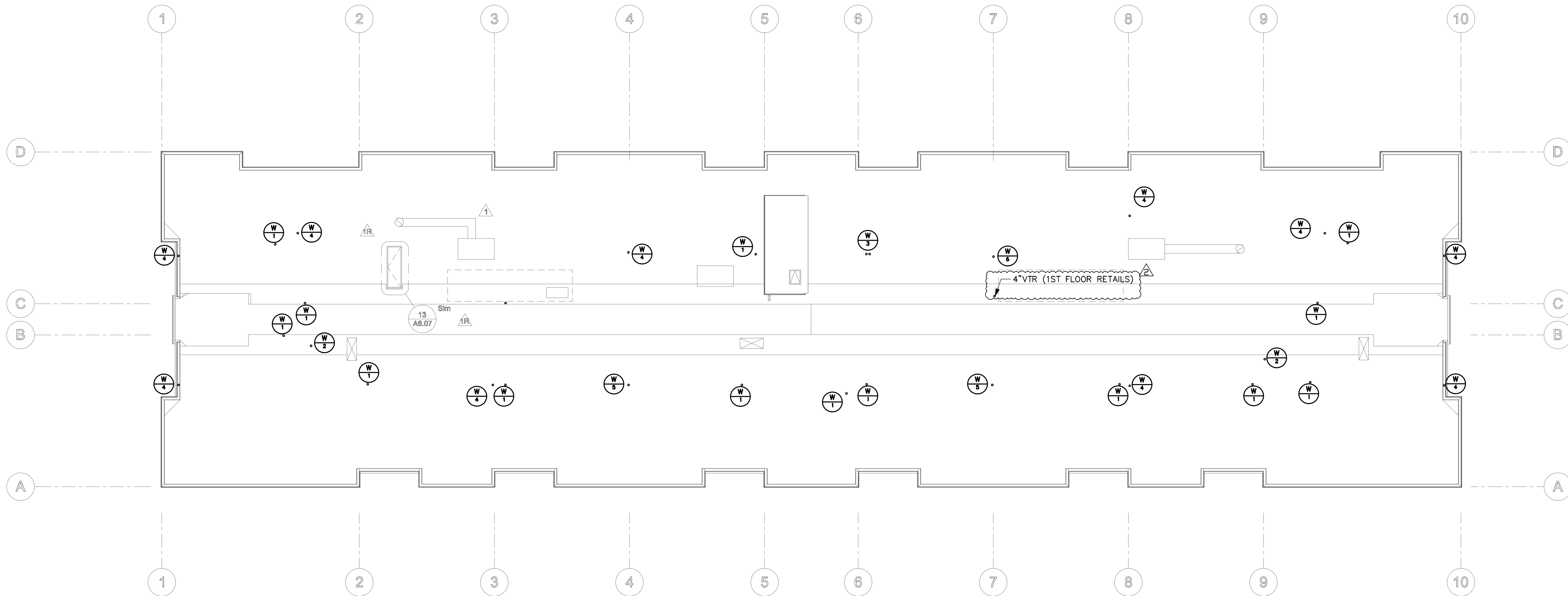
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**ATTIC & ROOF
PLAN- WASTE &
VENT**

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NOTES:

⊕ = WASTE/VENT RISER IDENTIFICATION (I.E. RISER "1"). REFER TO P400-P401 FOR RISER DETAILS AND SIZING.

1. STORM DRAIN SIZING: STORM DRAINAGE PIPING SIZED PER 2015 UPC TABLE 1101.7, FOR 1.0" PER HOUR RAINFALL RATE, AT 1/8" PER FOOT SLOPE:

| PIPE SIZE | VERTICAL | HORIZONTAL |
|-----------|------------|------------|
| 3" | 8,800 SF | 3,288 SF |
| 4" | 18,400 SF | 7,520 SF |
| 6" | 54,000 SF | 21,400 SF |
| 8" | 116,000 SF | 46,000 SF |
| 10" | 116,000 SF | 82,800 SF |

2. WASTE & VENT SIZING: WASTE & VENT PIPING SIZED PER 2015 UPC TABLE 703.2. PIPING SHALL BE SLOPED AT 1/4" PER FOOT:

| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1 1/2" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 216 DFU | 256 DFU |
| 6" | 1,380 DFU | 720 DFU | 1,380 DFU |
| 8" | 3,600 DFU | 2,640 DFU | 3,600 DFU |

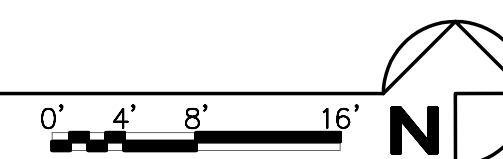
3. CONDENSATE DRAIN PIPE SIZING: PLUMBING CONTRACTOR SHALL PROVIDE CONDENSATE DRAINAGE FOR GUESTROOM HVAC UNITS. SEE MECHANICAL DRAWINGS FOR EXACT QUANTITY OF UNITS. SEE P601 FOR CONDENSATE RISER DIAGRAMS. CONDENSATE DRAIN PIPING SHALL BE SLOPED AT 1/8" PER FOOT. CONDENSATE DRAIN PIPING SIZED PER 2015 UPC TABLE 814.1:

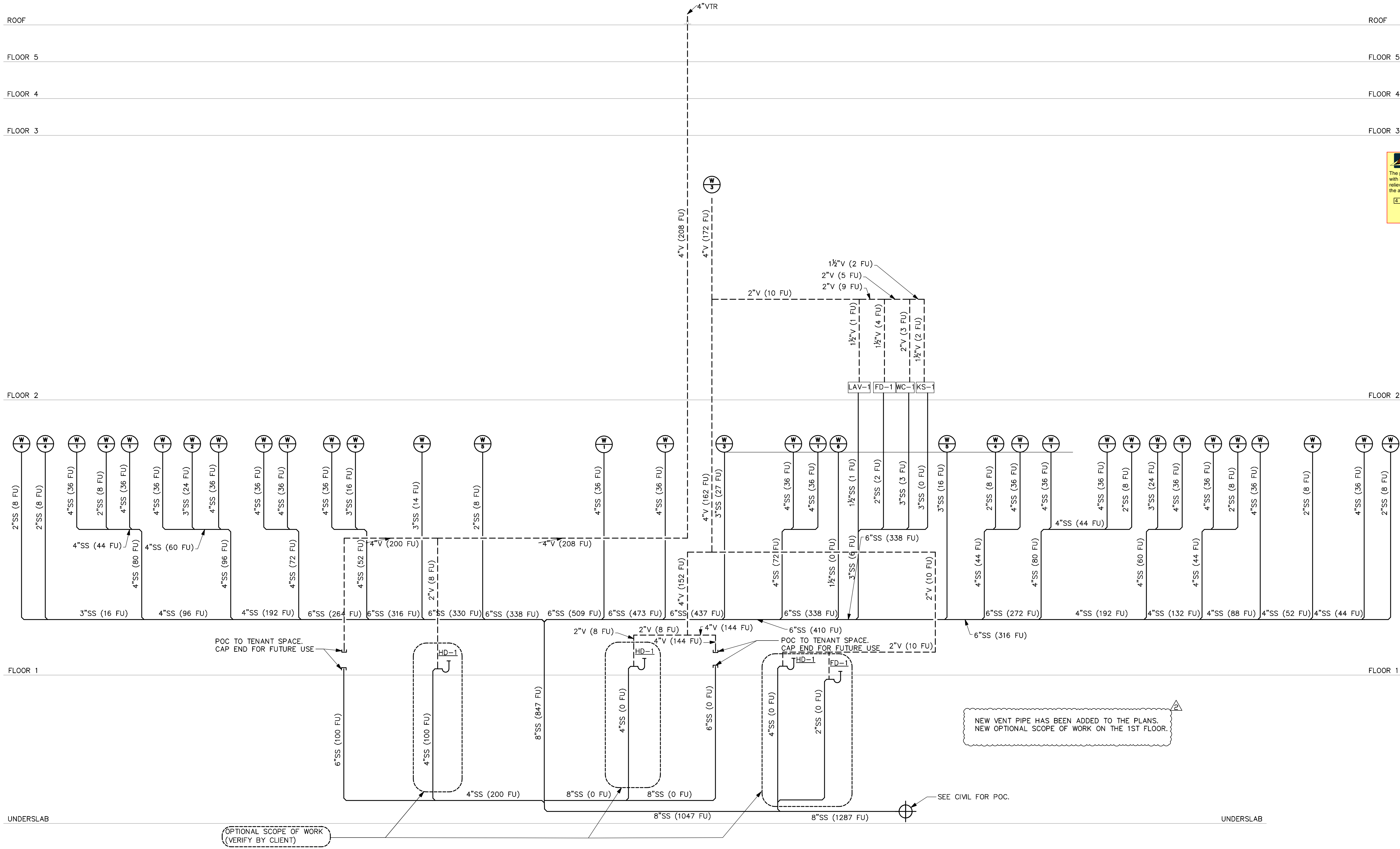
| PIPE SIZE | (TONS OF COOLING) |
|-----------|-------------------|
| 3/4" | 20 |
| 1" | 40 |
| 1 1/4" | 90 |
| 1 1/2" | 125 |
| 2" | 250 |

- 4. PROVIDE TRAP PRIMERS FOR ALL FLOOR DRAINS, HUB DRAINS SEE DETAIL 8 ON SHEET P600.
- 5. PROVIDE FLOOR DRAINS AT ALL ROLL-IN SHOWER BATHROOMS.
- 6. REFER TO ARCH PLANS LOCATION AND REQUIREMENT ON ADA TYPE A AND TYPE B UNITS.

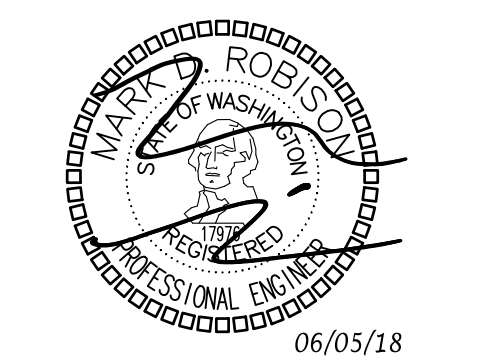
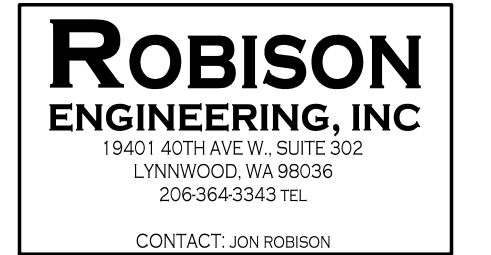
ATTIC & ROOF PLAN

SCALE: 3/32" = 1'-0"





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**EAST BAY LOT A
WESTMAN MILL**
510 STATE AVE OLYMPIA, WA, 98501

Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |

**WASTE & VENT
FLOW DIAGRAM**

P400

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**W
1**
GENERAL NOTES

RISER IDENTIFICATION. SEE RISER DIAGRAM ON P401. FOR EXAMPLE SEE RISER W/1 ON SHEET P4001.

FLAG NOTES: #

1. OFFSET PIPING AS REQUIRED. SEE FLOOR PLANS (P2.X) FOR DETAIL.

NOTE: WASTE & VENT SIZING: WASTE & VENT PIPING SIZED PER 2016 UPC TABLE 703.2. PIPING SHALL BE SLOPED AT 1/4" PER

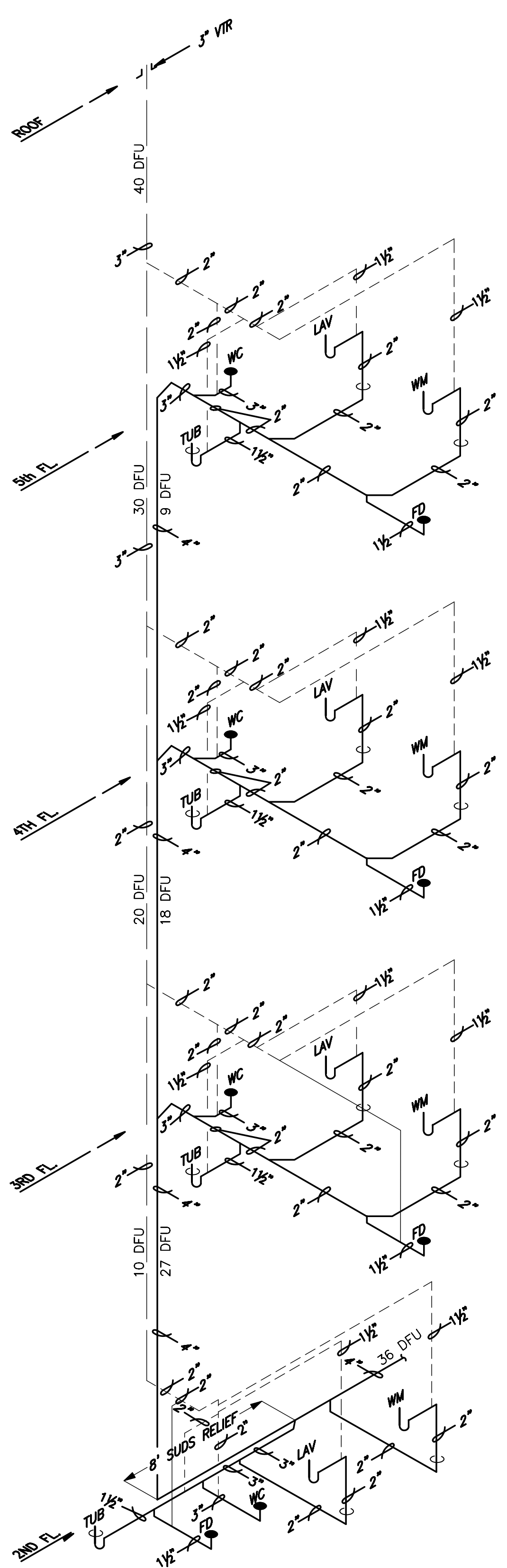
| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1 1/2" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 2 1/2" | 32 DFU | 16 DFU | 48 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 216 DFU | 256 DFU |
| 6" | 1,380 DFU | 720 DFU | 1,380 DFU |
| 8" | 3,600 DFU | 2,640 DFU | 3,600 DFU |

TUB = BATHTUB (2 FU)
LAV = LAVATORY (1 FU)
WC = WATER CLOSET (3 FU)
KS = SINK WITH DISHWASHER (2 FU)
SH = SHOWER (2FU)
WM = WASHING MACHINE (3 FU)
FD = FLOOR DRAIN (1 FU)
PWW = PUBLIC WASHING MACHINE (6 FU)

**WASTE & VENT
FLOW DIAGRAMS**

SCALE: NONE

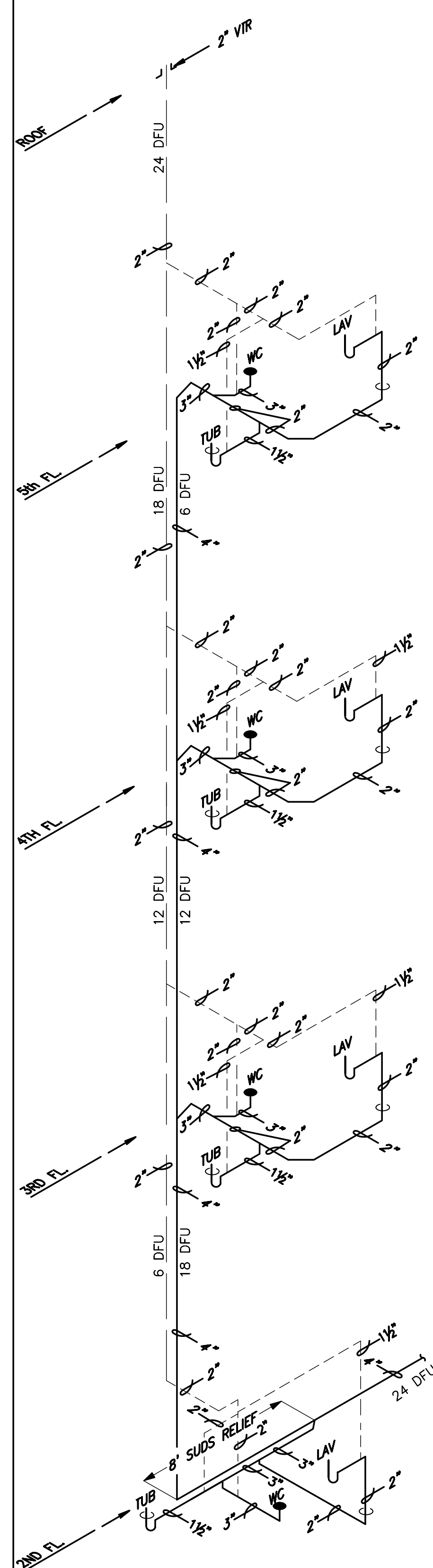
NOTE: SEE FIRE STOP SYSTEM ON SHEET P603 FOR PIPING PENETRATING THE 3HR FLOOR (2ND FLOOR SLAB)



WASTE RISER

SCALE: NONE

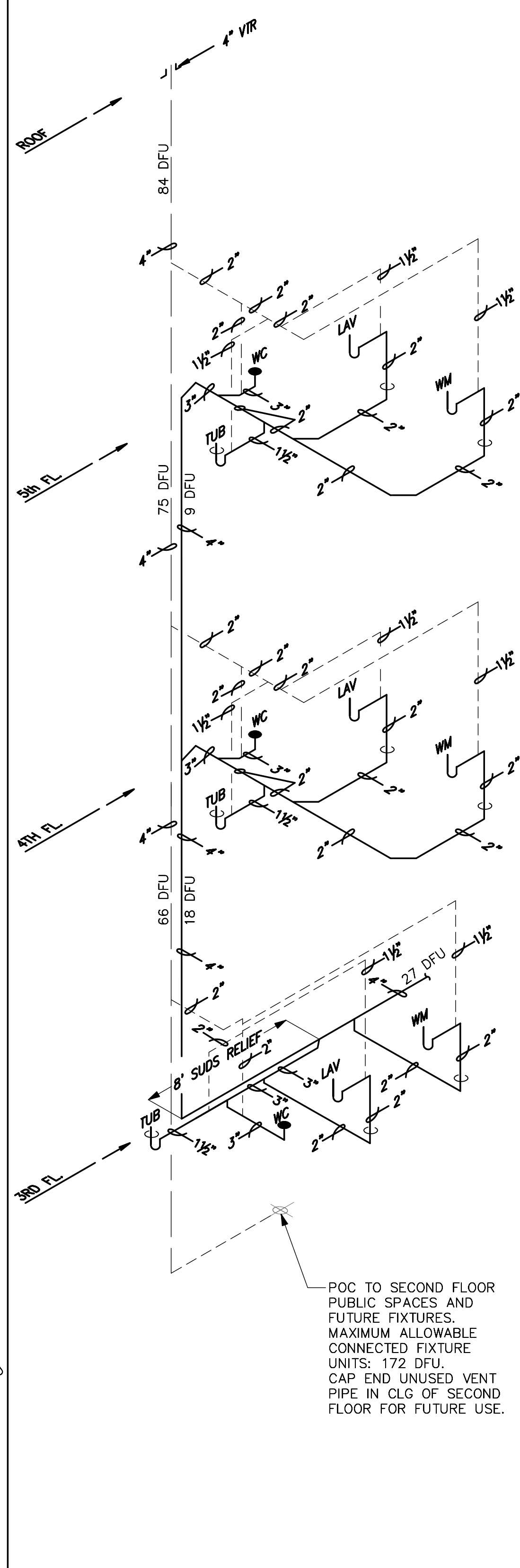
W
1



WASTE RISER

SCALE: NONE

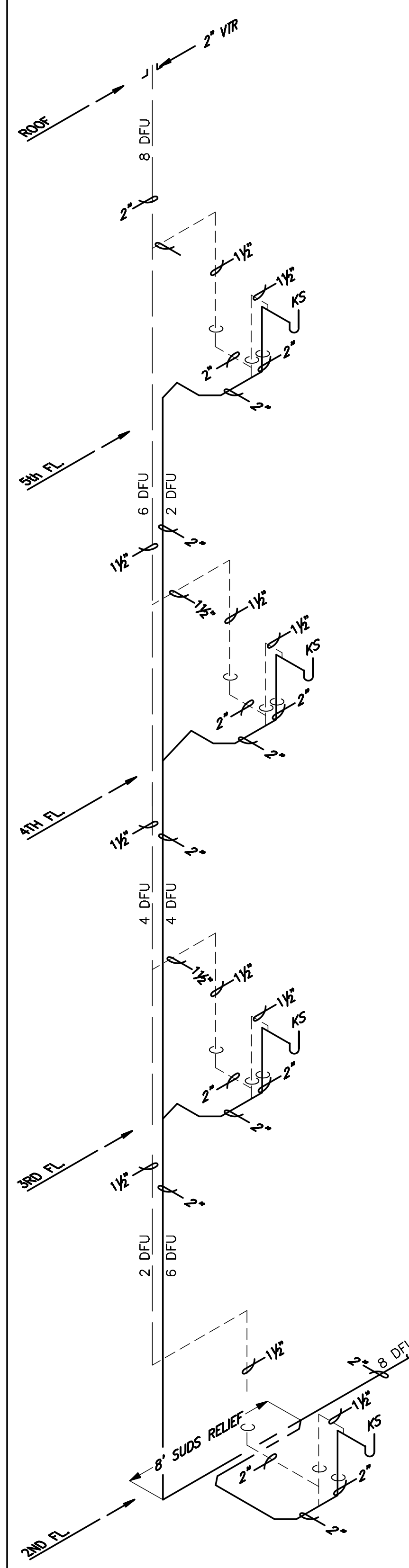
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2



WASTE RISER

SCALE: NONE

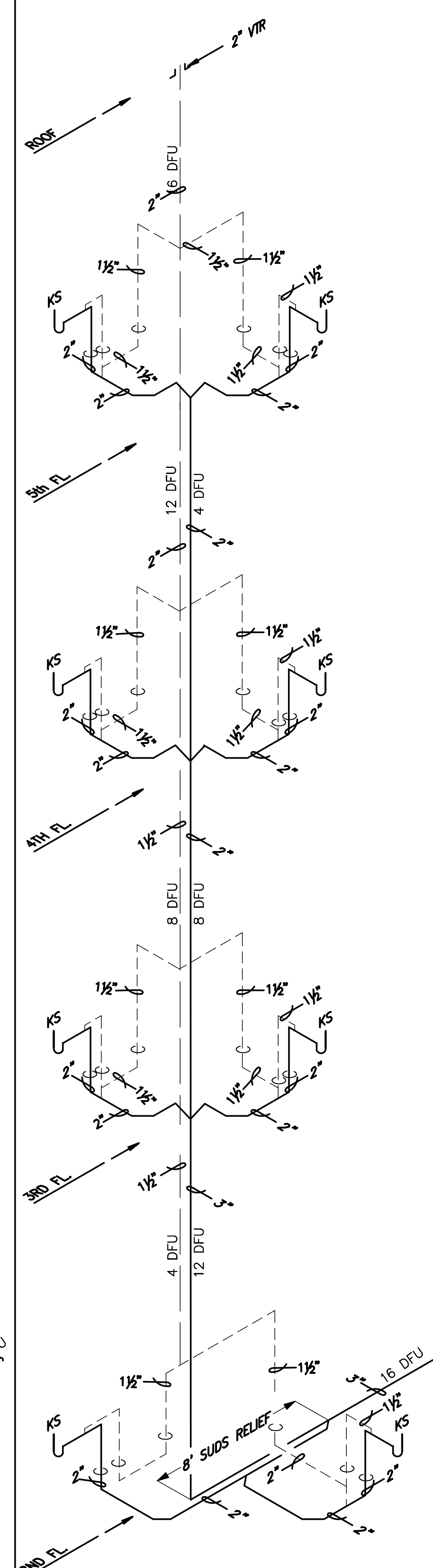
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3



WASTE RISER

SCALE: NONE

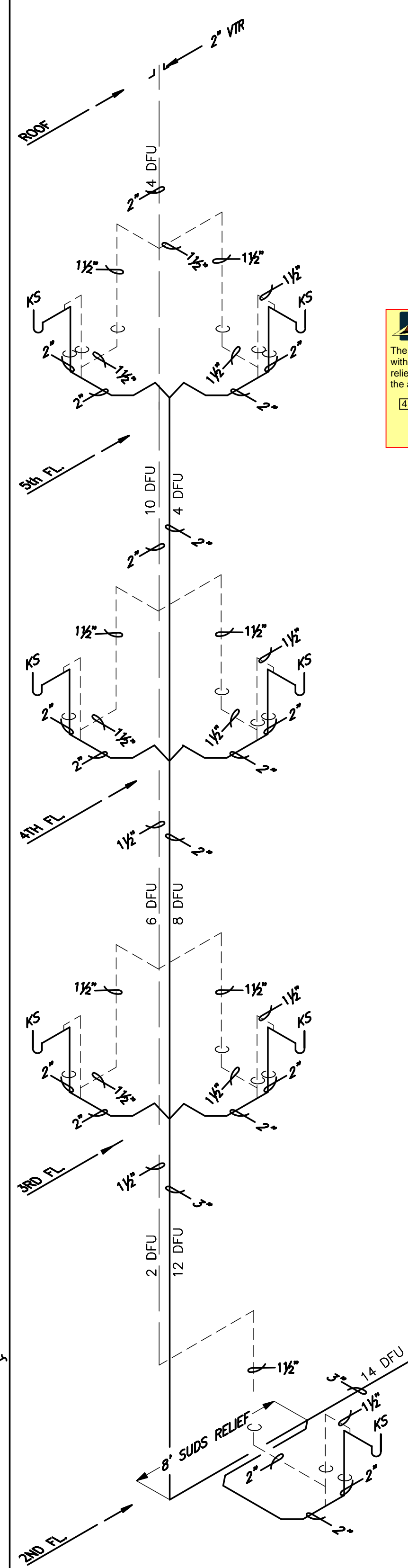
W
4



WASTE RISER

SCALE: NONE

W
5



WASTE RISER

SCALE: NONE

W
6

POC TO SECOND FLOOR PUBLIC SPACES AND FUTURE FIXTURES. MAXIMUM ALLOWABLE CONNECTED FIXTURE UNITS: 172 DFU. CAP END UNUSED VENT PIPE IN CLG OF SECOND FLOOR FOR FUTURE USE.

NOTE

C/O ARE NOT SHOWN ON THE PLANS. CONTRACTOR SHALL PROVIDE CLEANOUTS AS

FLAG NOTES: #

- REFER TO FLOOR PLANS (P2.X) FOR CONTINUATION.
- OFFSET PIPING AS REQUIRED. SEE FLOOR PLANS (P2.X) FOR CONTINUATION.

NOTE:

| PIPE SIZE | VERT. | HORIZ. | VENT |
|-----------|-----------|-----------|-----------|
| 1 1/2" | 2 DFU | 1 DFU | 8 DFU |
| 2" | 16 DFU | 8 DFU | 24 DFU |
| 2 1/2" | 32 DFU | 14 DFU | 48 DFU |
| 3" | 48 DFU | 35 DFU | 84 DFU |
| 4" | 256 DFU | 172 DFU | 256 DFU |
| 6" | 1,380 DFU | 576 DFU | 1,380 DFU |
| 8" | 3,600 DFU | 2,112 DFU | 3,600 DFU |

- TUB = BATHTUB (2 FU)
- LAV = LAVATORY (1 FU)
- WC = WATER CLOSET (3 FU)
- KS = SINK WITH DISHWASHER (2 FU)
- SH = SHOWER (2 FU)
- WM = WASHER BOX (3 FU)
- FD = FLOOR DRAIN (1 FU)
- PWM = PUBLIC WASHING MACHINE (6 FU)

WASTE & VENT RISER DIAGRAMS

SCALE: NONE

NOTE: SEE FIRE STOP SYSTEM ON SHEET P603 FOR PIPING PENETRATING THE 3HR FLOOR (2ND FLOOR SLAB)



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TOWNZEN & ASSOCIATES
PLAN APPROVAL
The plans submitted for review are approved in accordance with local state applicable standards. This approval does not relieve the applicant of the responsibility of compliance with the applicable codes.
Approved as submitted. 09/25/2019

ROBISON ENGINEERING, INC.
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CONTACT: JON ROBISON



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Community Planning & Development Department
601 4th Ave East
Olympia, WA 98501
360.753.4148
rbadders@ci.olympia.wa.us

**EAST BAY LOT A
WESTMAN MILL**
510 STATE AVE OLYMPIA, WA. 98501

Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |

WASTE & VENT RISER DIAGRAM

P401

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ROOF

FLOOR 5

FLOOR 4

FLOOR 3

FLOOR 2

FLOOR 1

ROOF

FLOOR 5

FLOOR 4

FLOOR 3

FLOOR 2

FLOOR 1



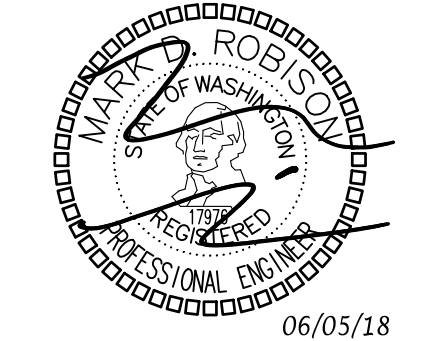
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EAST BAY LOT A
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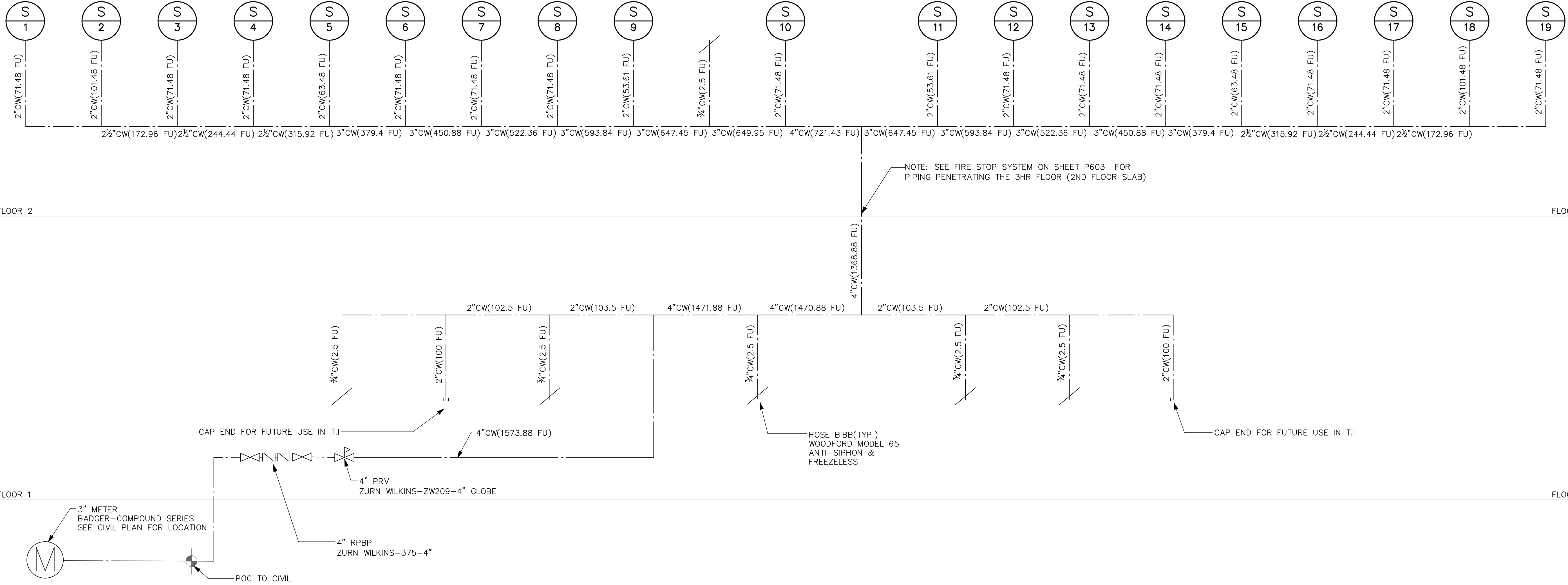
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SUPPLY
DISTRIBUTION
DIAGRAM

P500

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| SUPPLY PIPE SIZING SCHEDULE | | | | | | | | Copper Type: Type L | |
|-----------------------------|---------------|----------|---------------|-----------|----------|---------------|----------------|---------------------|---------------|
| PIPE SIZE | FLUSH TANK CW | | | HOT WATER | | | FLUSH VALVE CW | | |
| | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 | 564.0 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 5250.0 |

GENERAL NOTES

S/1 RISER IDENTIFICATION. SEE RISER DIAGRAM ON P501. FOR EXAMPLE SEE RISER S/1 ON SHEET P501.

- TUB = BATHTUB
- LAV = LAVATORY
- WC = WATER CLOSET
- KS = SINK WITH DISHWASHER
- SH = SHOWER
- WM = WASHING MACHINE
- FD = FLOOR DRAIN
- WH = WATER HEATER

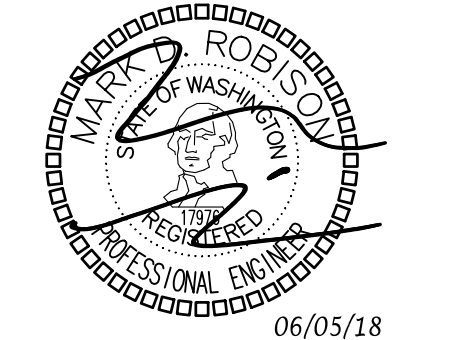
SUPPLY
DISTRIBUTION DIAGRAM

SCALE: NONE



THOMAS
architecture studios

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**EAST BAY LOT A
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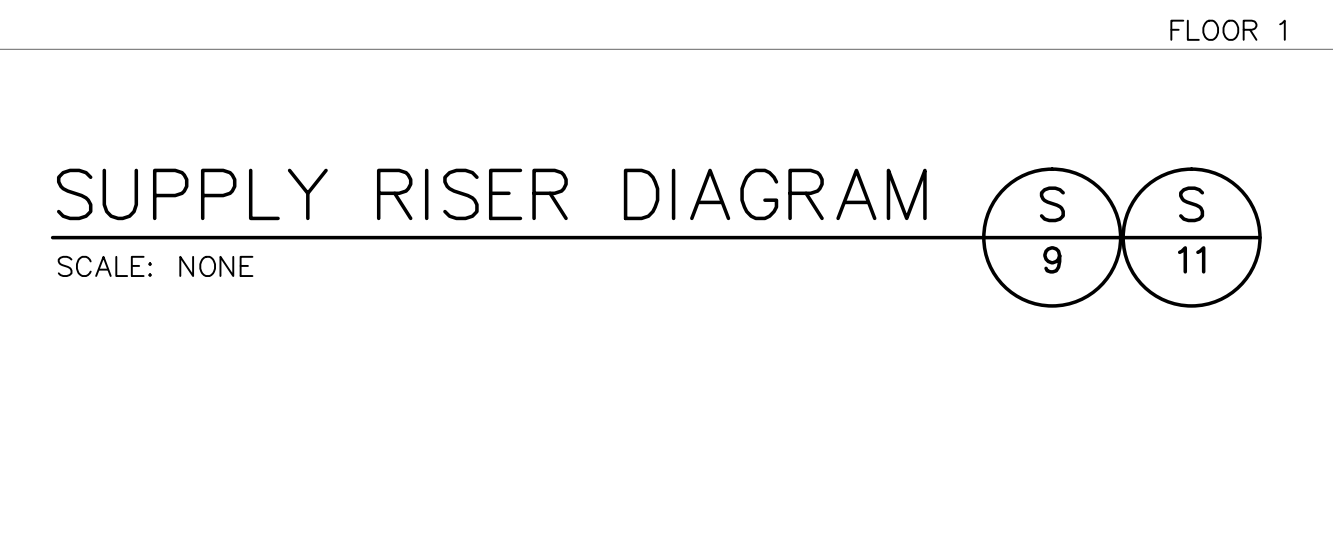
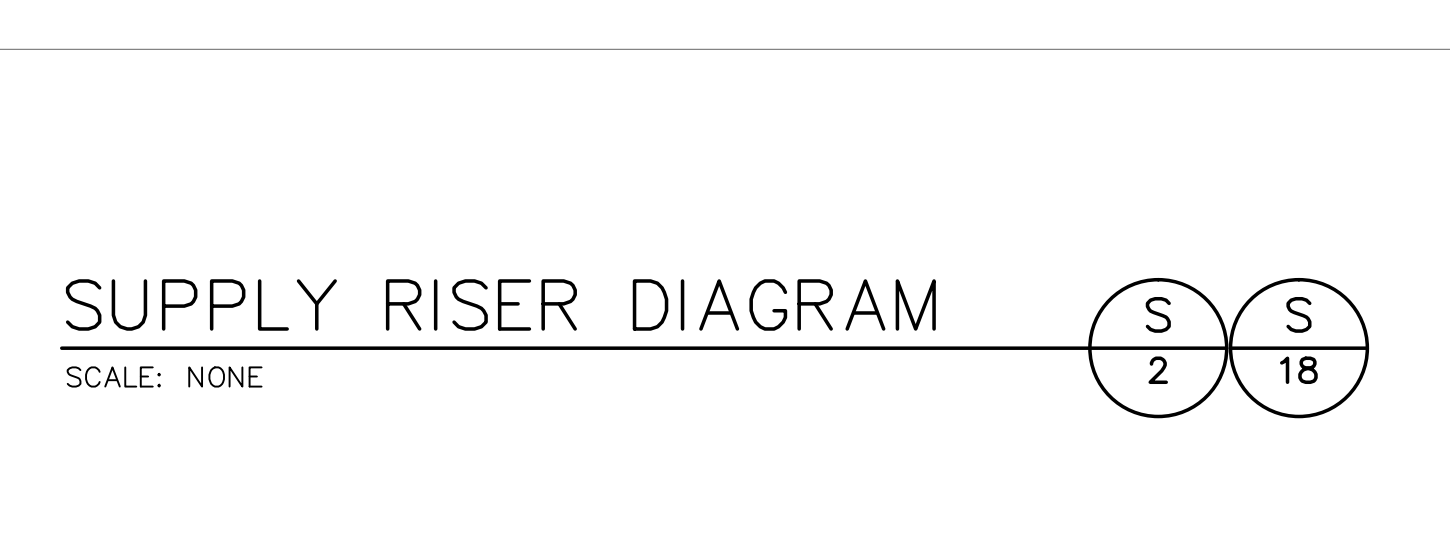
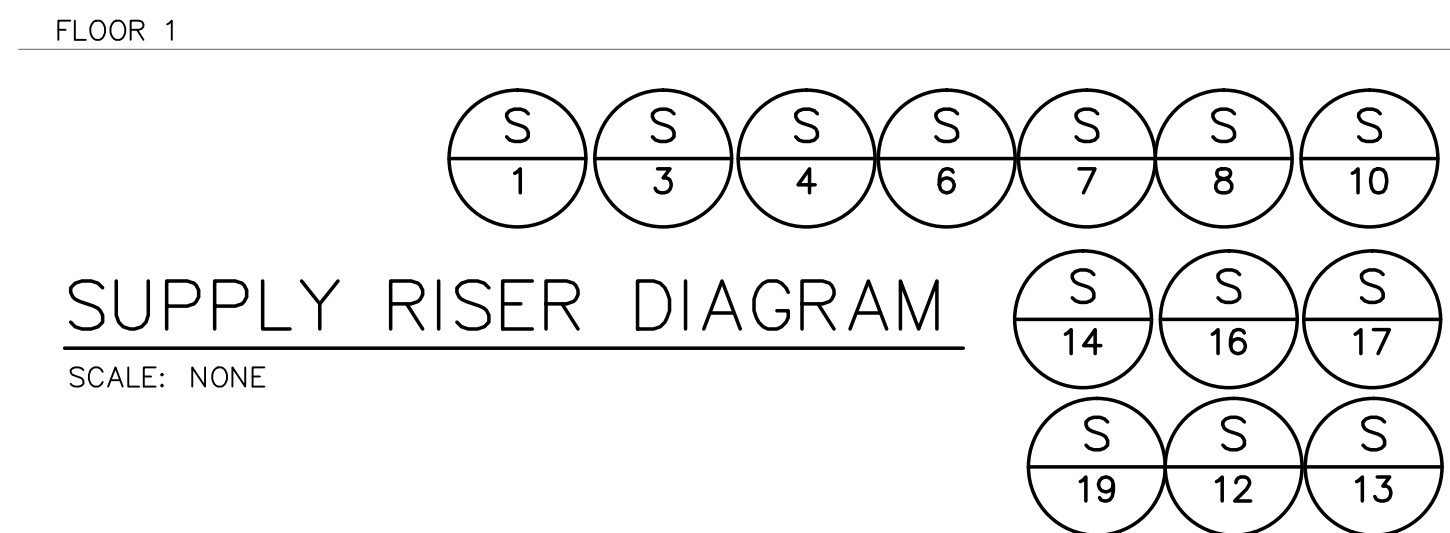
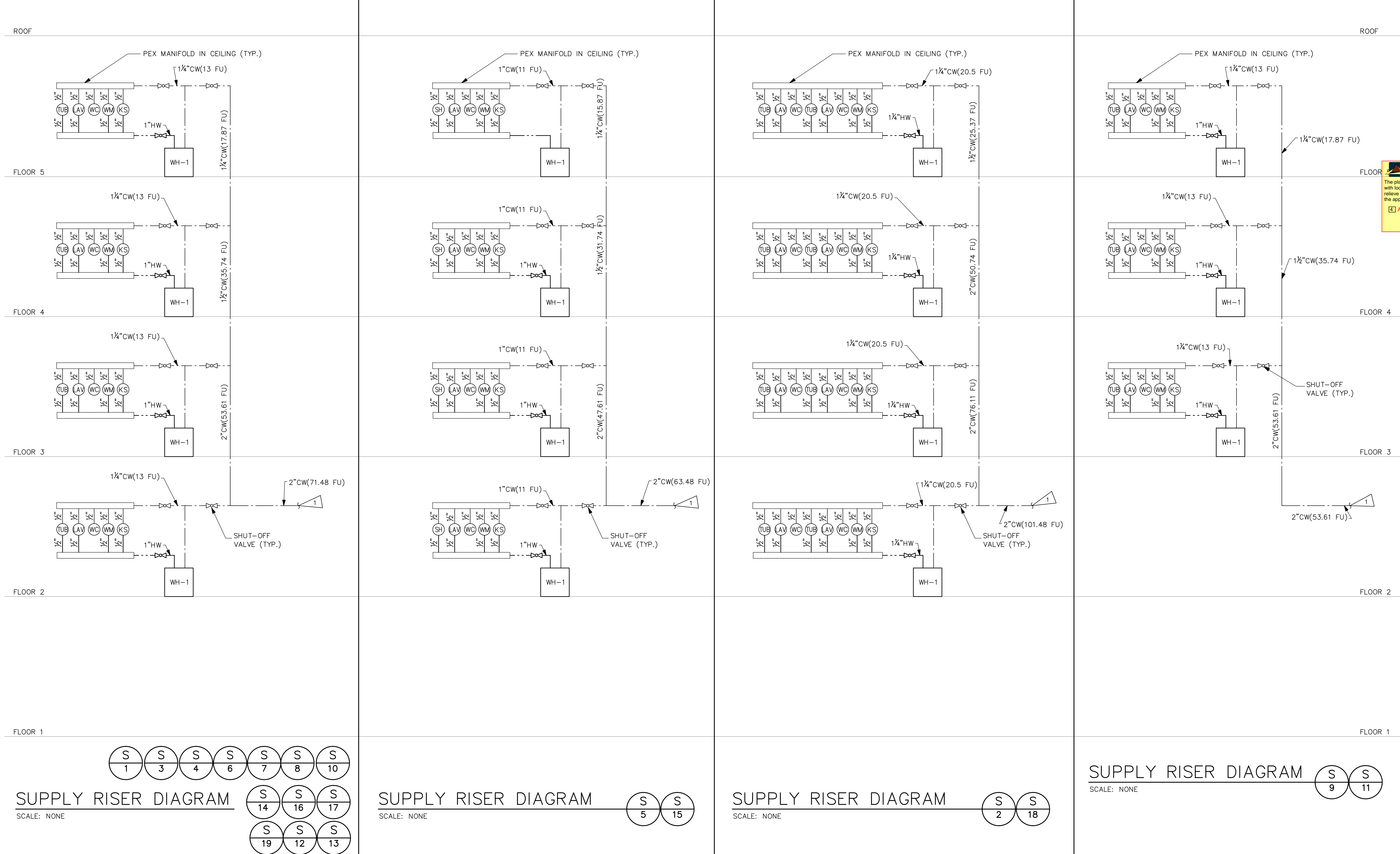
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |

SUPPLY RISER DIAGRAMS

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| SUPPLY PIPE SIZING SCHEDULE | | | | | | Copper Type: Type L | | |
|-----------------------------|---------------|----------|---------------|-----------|----------|---------------------|----------------|----------|
| PIPE SIZE | FLUSH TANK CW | | | HOT WATER | | | FLUSH VALVE CW | |
| | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 |

FLAG NOTES: #

1. REFER TO FLOOR PLANS (P202S) FOR CONTINUATION.

TUB = BATHTUB
LAV = LAVATORY
WC = WATER CLOSET
KS = SINK WITH DISHWASHER
SH = SHOWER
WM = WASHING MACHINE
FD = FLOOR DRAIN
WH = WATER HEATER

SUPPLY RISER DIAGRAMS
SCALE: NONE

| SUPPLY PIPE SIZING SCHEDULE | | | | | | | | | |
|-----------------------------|---------------|----------|---------------|-----------|----------|---------------|---------------------|----------|---------------|
| PIPE SIZE | FLUSH TANK CW | | | HOT WATER | | | Copper Type: Type L | | |
| | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 | 564.0 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 5250.0 |

3/4" CONDENSATE DRAIN TO PTHP

NOTE:

- 1- PROVIDE 1/4 TURN ANGEL STOP VALVE FOR ALL PLUMBING FIXTURES.
- 2- WATER HEATER: PROVIDE THE FOLLOWING
 - A. ISOLATION VALVE ON HOT AND COLD.
 - B. CHECK VALVE ON COLD WATER INLET.
 - C. ASSE 1017/1070 MIXING VALVE (SET @ 110)
 - D. T&P VALVE

TERMINATE DISH WASHER DRAINAGE HOSE TO KITCHEN SINK TAIL PIECE VIA AIR GAP
BRANCH OFF FROM HOT WATER PIPE AFTER 1/4 TURN ANGEL STOP VALVE FOR DISH WASHER WATER SUPPLY.

REF.

DW

KS-1

1/2" CW (1.125 CWFU)
1/2" HW (1.125 HWFU)

WC-1

1/2" CW (2.5FU)
3/4" CW (3.625FU)
1/2" HW (1.125FU)

LAV-1

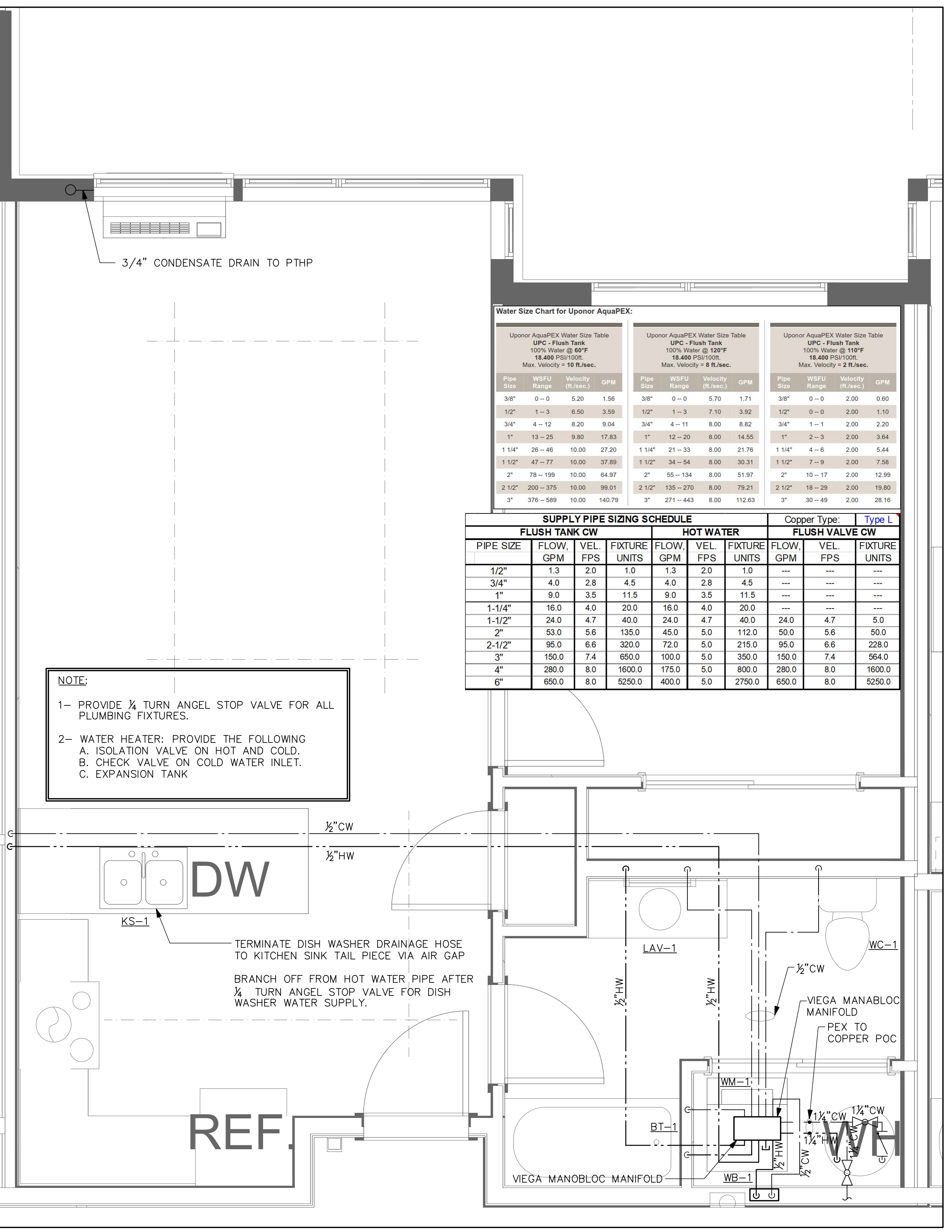
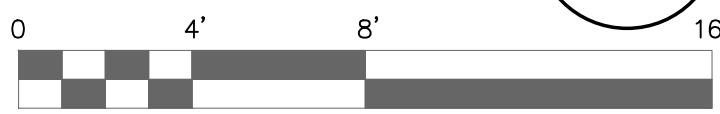
WH-2, MOUNT UNDER THE COUNTER

ENLARGED PLAN

LOUNGE

SCALE: 1/2" = 1'-0"

2
P300



NOTE:

- 1- PROVIDE 1/4 TURN ANGEL STOP VALVE FOR ALL PLUMBING FIXTURES.
- 2- WATER HEATER: PROVIDE THE FOLLOWING
 - A. ISOLATION VALVE ON HOT AND COLD.
 - B. CHECK VALVE ON COLD WATER INLET.
 - C. EXPANSION TANK

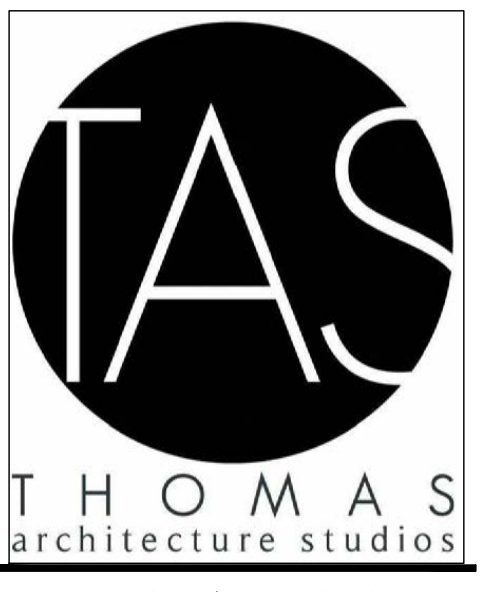
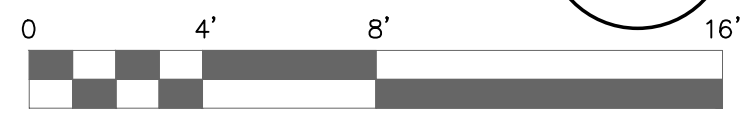
| SUPPLY PIPE SIZING SCHEDULE | | | | | | | | | |
|-----------------------------|---------------|----------|---------------|-----------|----------|---------------|---------------------|----------|---------------|
| PIPE SIZE | FLUSH TANK CW | | | HOT WATER | | | Copper Type: Type L | | |
| | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS | FLOW, GPM | VEL, FPS | FIXTURE UNITS |
| 1/2" | 1.3 | 2.0 | 1.0 | 1.3 | 2.0 | 1.0 | --- | --- | --- |
| 3/4" | 4.0 | 2.8 | 4.5 | 4.0 | 2.8 | 4.5 | --- | --- | --- |
| 1" | 9.0 | 3.5 | 11.5 | 9.0 | 3.5 | 11.5 | --- | --- | --- |
| 1-1/4" | 16.0 | 4.0 | 20.0 | 16.0 | 4.0 | 20.0 | --- | --- | --- |
| 1-1/2" | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 40.0 | 24.0 | 4.7 | 5.0 |
| 2" | 53.0 | 5.6 | 135.0 | 45.0 | 5.0 | 112.0 | 50.0 | 5.6 | 50.0 |
| 2-1/2" | 95.0 | 6.6 | 320.0 | 72.0 | 5.0 | 215.0 | 95.0 | 6.6 | 228.0 |
| 3" | 150.0 | 7.4 | 650.0 | 100.0 | 5.0 | 350.0 | 150.0 | 7.4 | 564.0 |
| 4" | 280.0 | 8.0 | 1600.0 | 175.0 | 5.0 | 800.0 | 280.0 | 8.0 | 1600.0 |
| 6" | 650.0 | 8.0 | 5250.0 | 400.0 | 5.0 | 2750.0 | 650.0 | 8.0 | 5250.0 |

ENLARGED PLAN

TYPICAL APT

SCALE: 1/2" = 1'-0"

1
P300



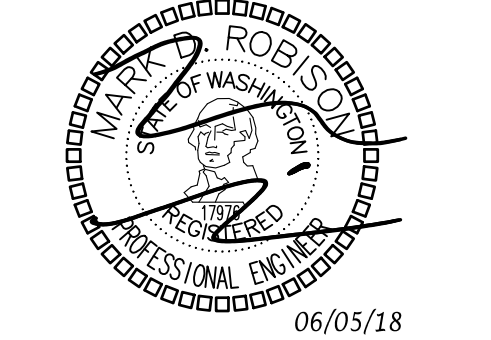
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TOWNZEN & ASSOCIATES
PLAN APPROVAL

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Approved as submitted. 09/25/2019 *Joe Lanza*

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EAST BAY LOT A
WESTMAN MILL
510 STATE AVE OLYMPIA, WA, 98501

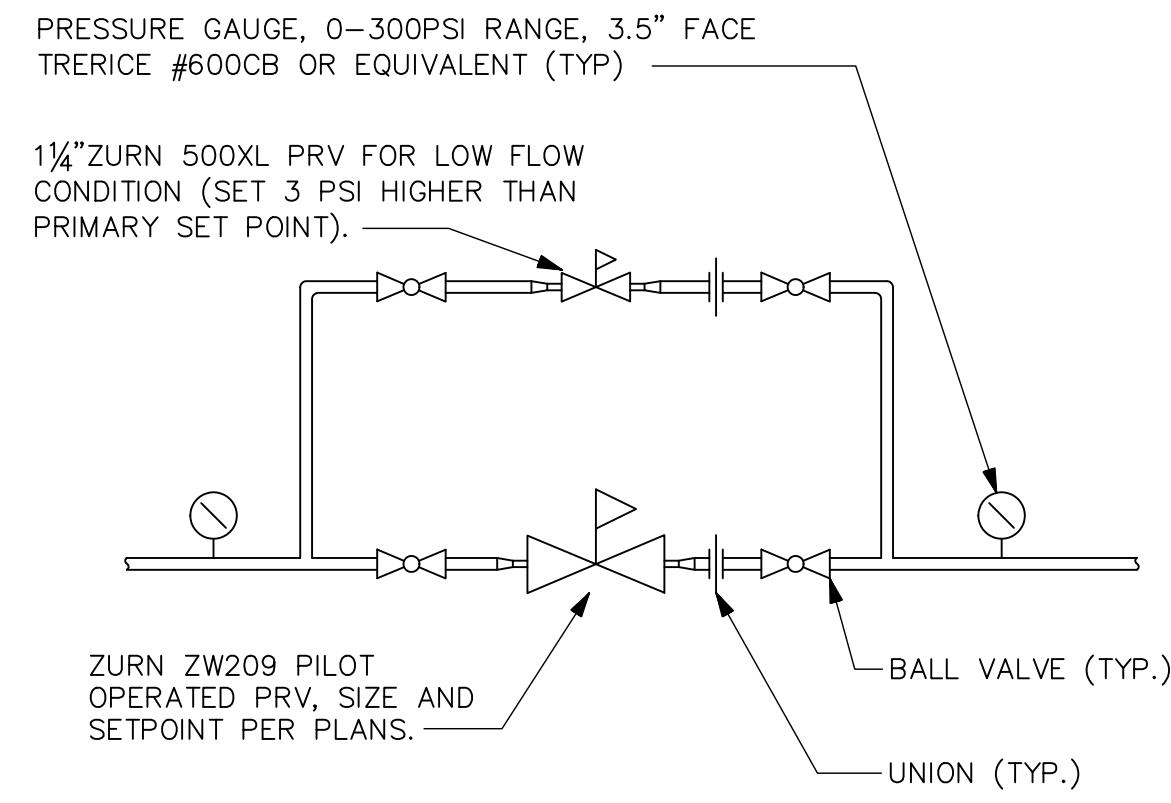
Project No: 1514
PERMIT SET
5/16/18

| Rev# | Date | Description |
|------|----------|----------------|
| 1 | 01/30/19 | PERMIT COMMENT |
| 2 | 01/30/19 | OTHER CHANGES |
| 3 | 06/12/19 | GREASE WASTE |

ENLARGED
PLUMBING PLAN

P300

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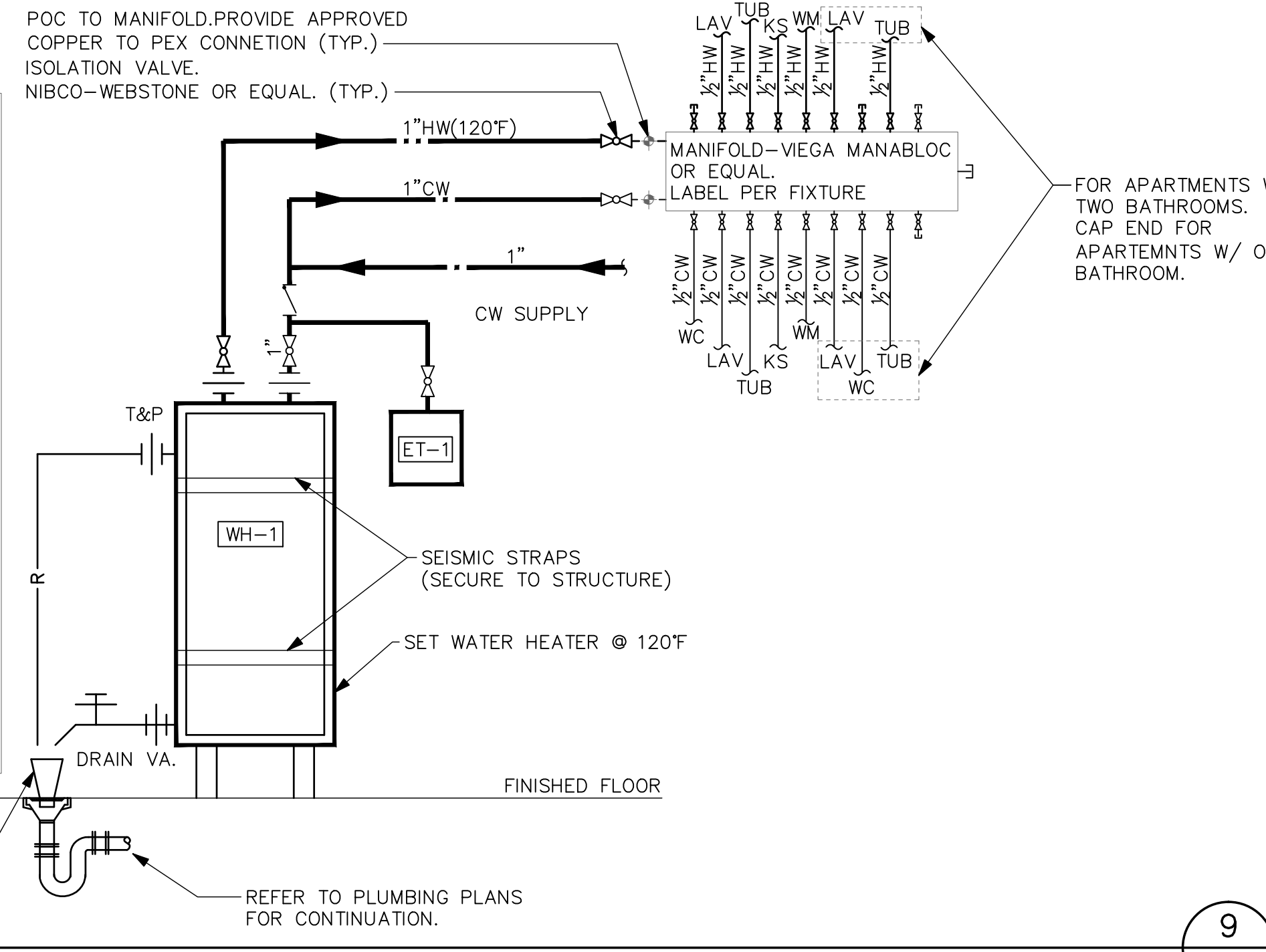
PRV STATION PIPING
DIAGRAM
 SCALE: NONE

10
P600

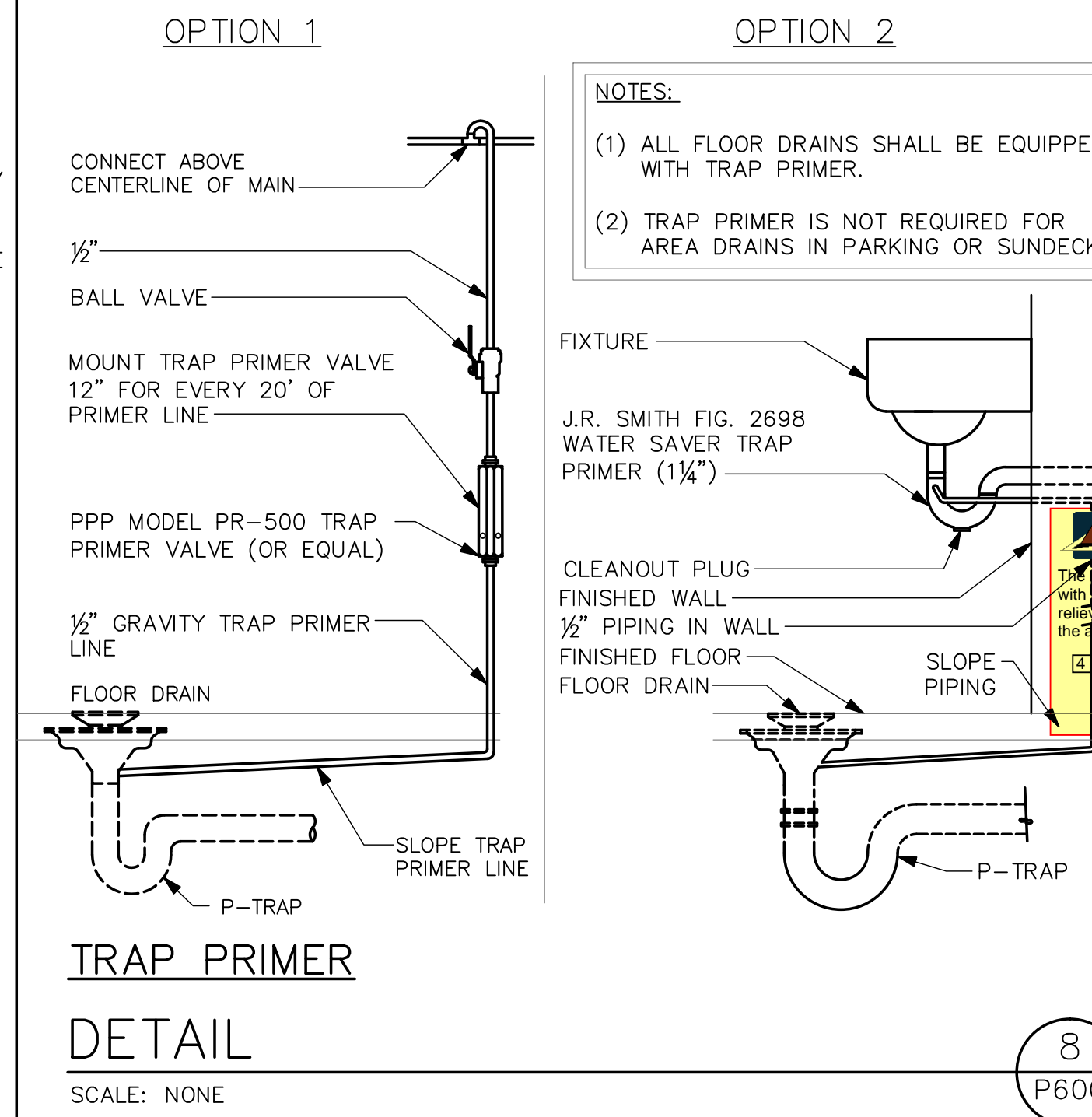
- NOTES:**
- THE SEISMIC STRAPS SHALL BE AT POINTS WITHIN THE UPPER 1/3 AND LOWER 1/3 OF THE WATER HEATER. AT THE LOWER POINT A MINIMUM OF 4" SHALL BE MAINTAINED ABOVE THE CONTROLS WITH THE STRAP.
 - INSTALL MANIFOLD ON THE WALL IN THE WATER HEATER CLOSET.
 - PER 2015 UPC SECTION 608.5, DISCHARGE PIPING SERVING A TEMPERATURE RELIEF VALVE, PRESSURE RELIEF VALVE OR COMBINATION OF BOTH SHALL HAVE NO VALVES, OBSTRUCTION, OR MEANS OF ISOLATION AND BE PROVIDED WITH THE FOLLOWING:
 - EQUAL TO THE SIZE OF THE VALVE OUTLET AND SHALL DISCHARGE FULL SIZE TO THE FLOOD LEVEL OF THE AREA RECEIVING THE DISCHARGE AND POINTING DOWN.
 - DISCHARGE PIPE SHALL DISCHARGE INDEPENDENTLY BY GRAVITY THROUGH AND AIR GAP INTO THE DRAINAGE SYSTEM OR OUTSIDE OF THE BUILDING WITH THE END PIPE NOT EXCEEDING 2 FEET AND NOT LESS THAN 6 INCHES ABOVE THE GROUND AND POINTING DOWNWARDS.
 - NO PART OF DISCHARGE SHALL BE TRAPPED OR SUBJECT TO FREEZING. THE TERMINATION OF THE END PIPE SHALL NOT BE THREADED.
 - DISCHARGE FROM A RELIEF VALVE INTO A WATER HEATER PAN SHALL BE PROHIBITED.

WATER HEATER PIPING
DETAIL
 SCALE: NONE

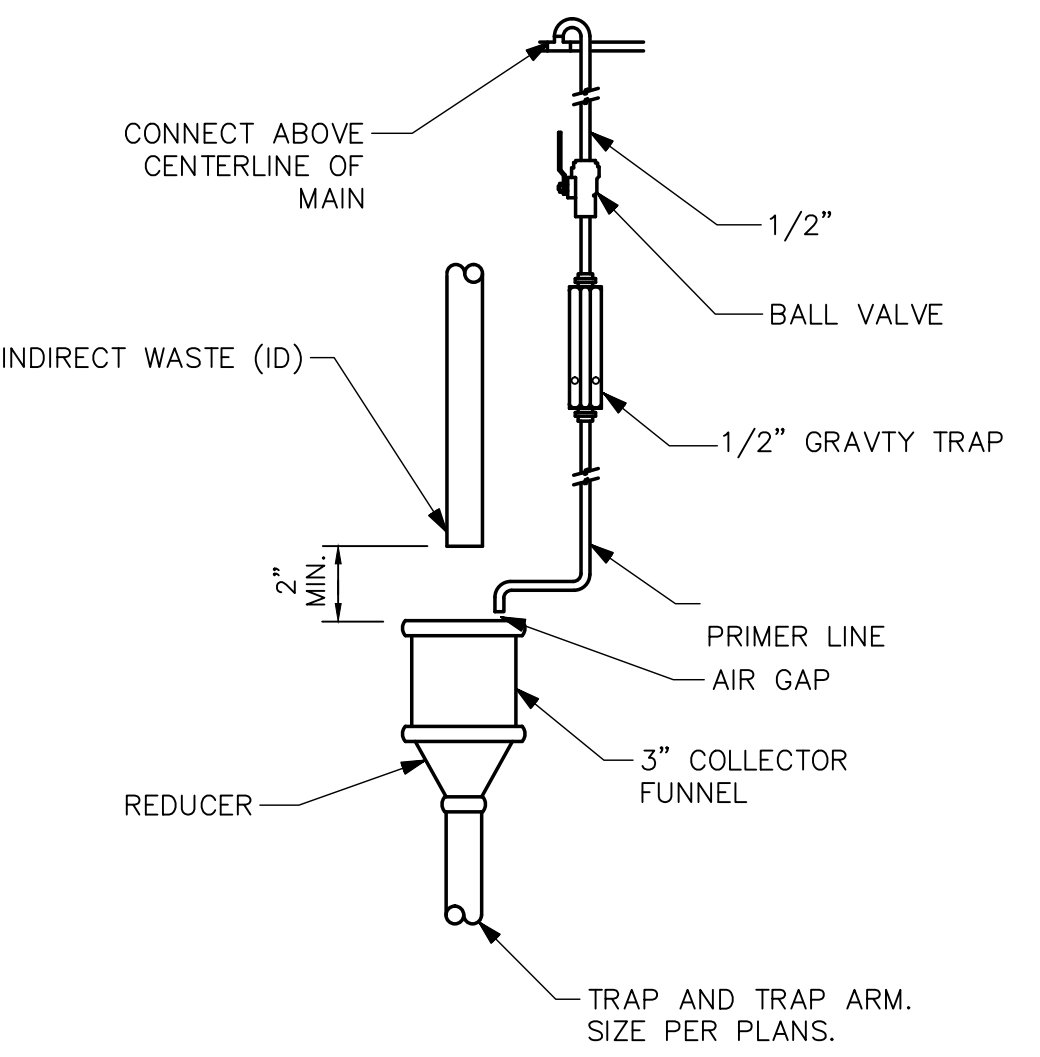
FLOOR DRAIN W/ FUNNEL. PROVIDE TRAP PRIMER
 REFER TO PLUMBING PLANS FOR CONTINUATION.



9
P0.0

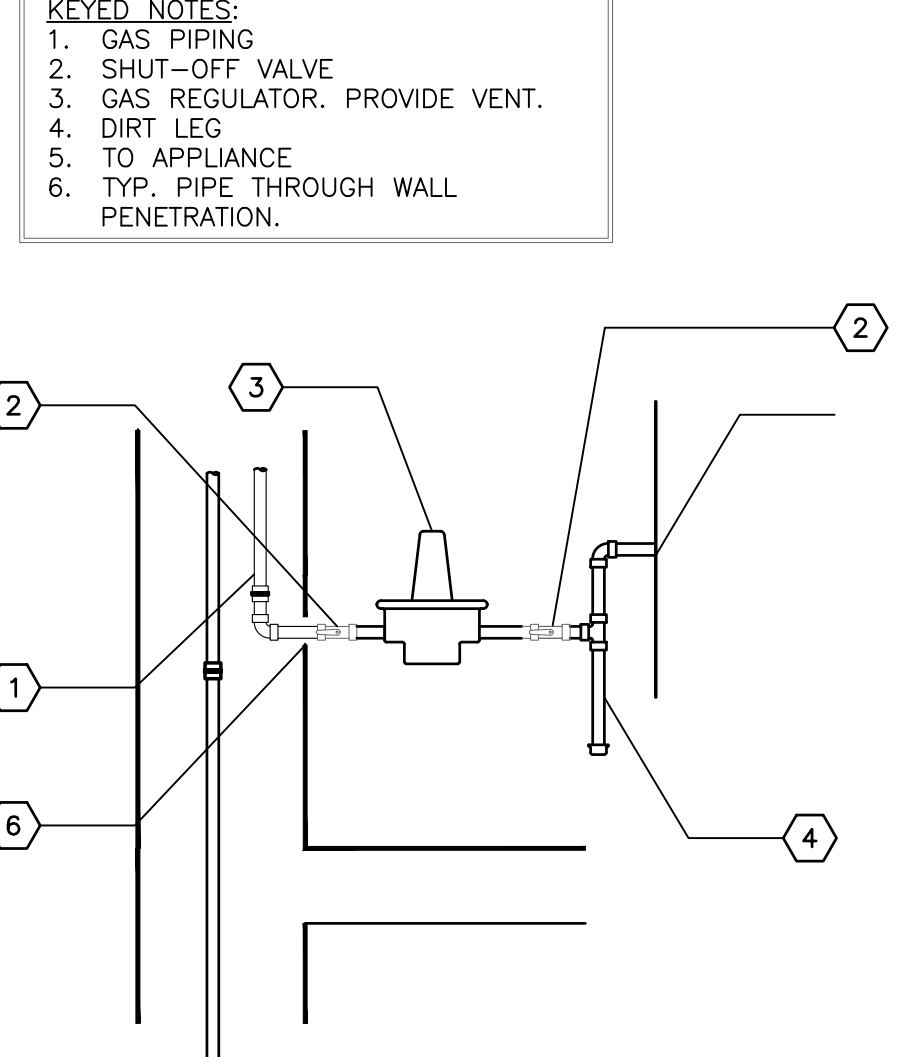


8
P600



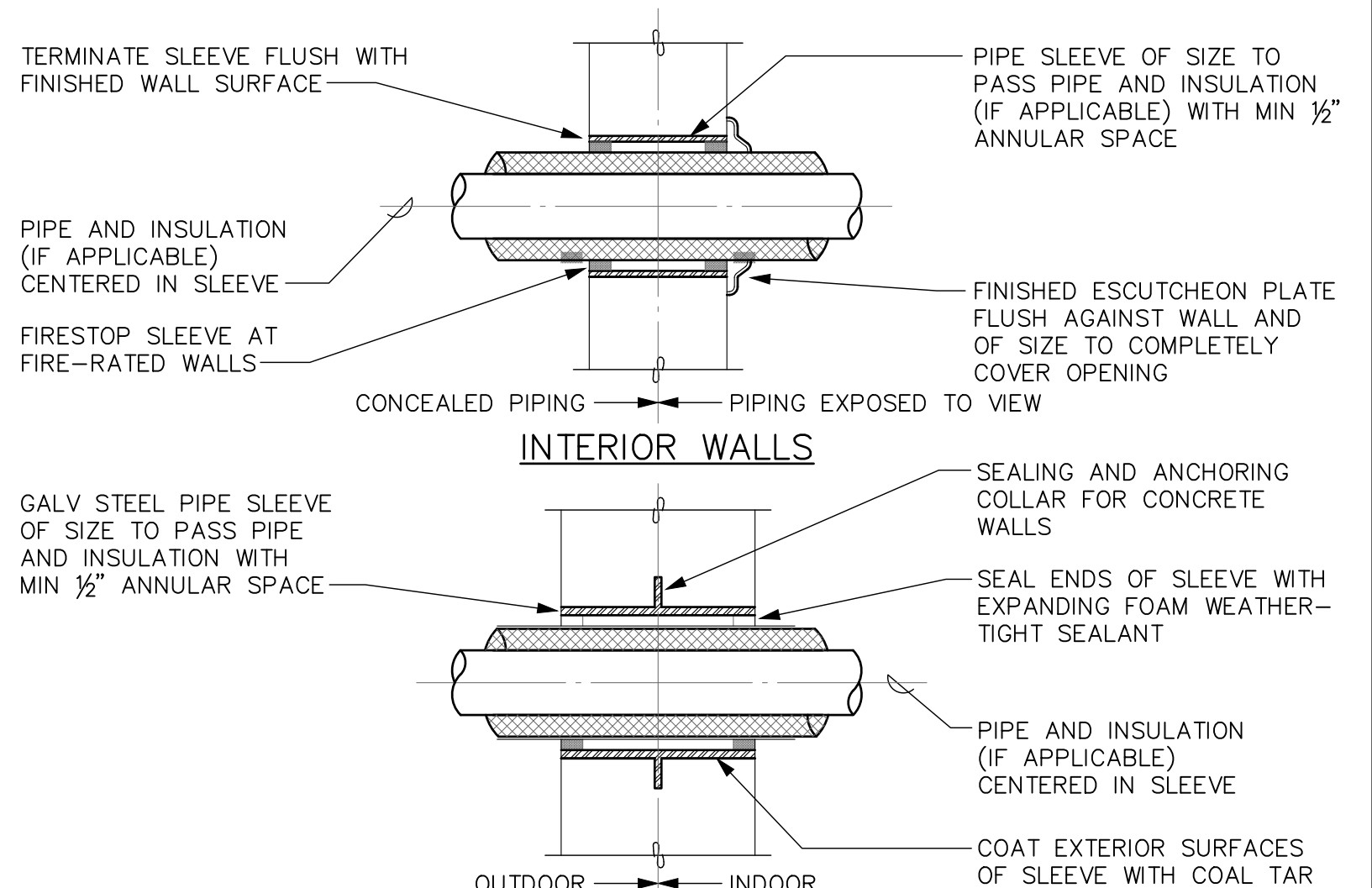
HUB DRAIN
DETAIL
 SCALE: NONE

4
P600



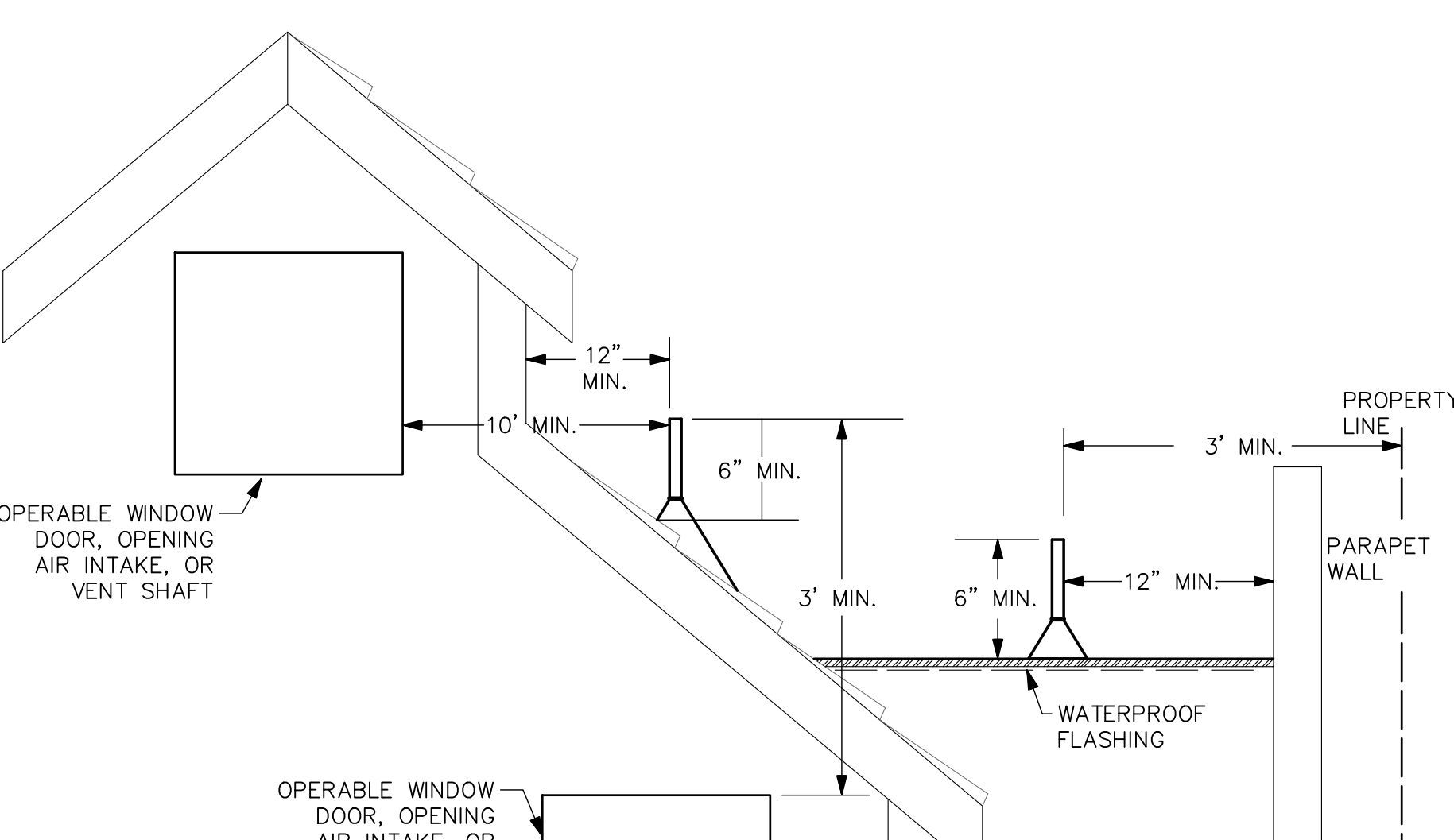
GAS REGULATOR ASSEMBLY
DETAIL
 SCALE: NONE

5
P600



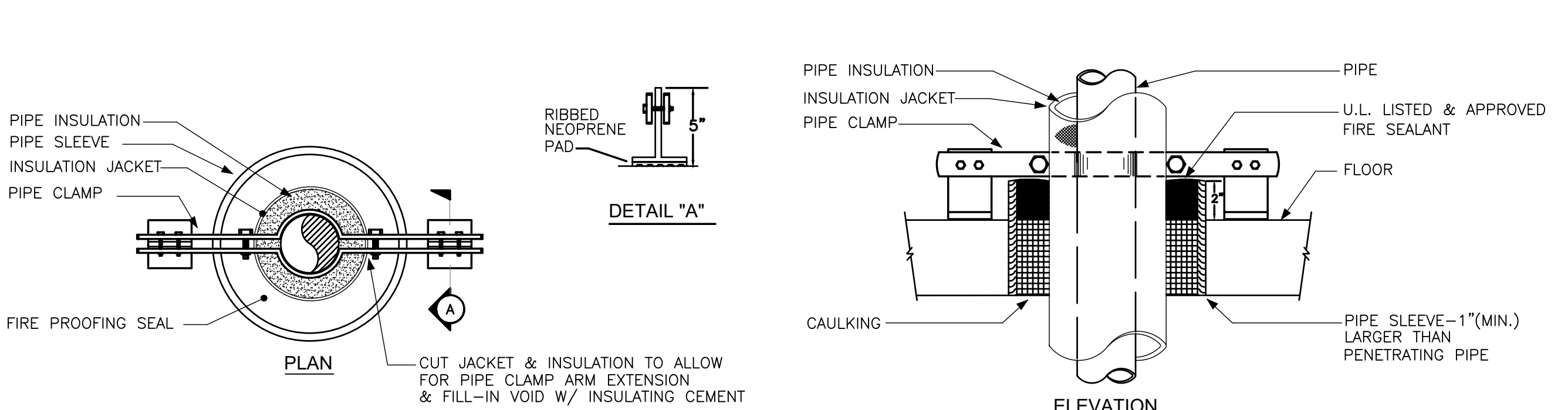
PIPE SLEEVES THROUGH WALLS
DETAIL
 SCALE: NONE

6
P600



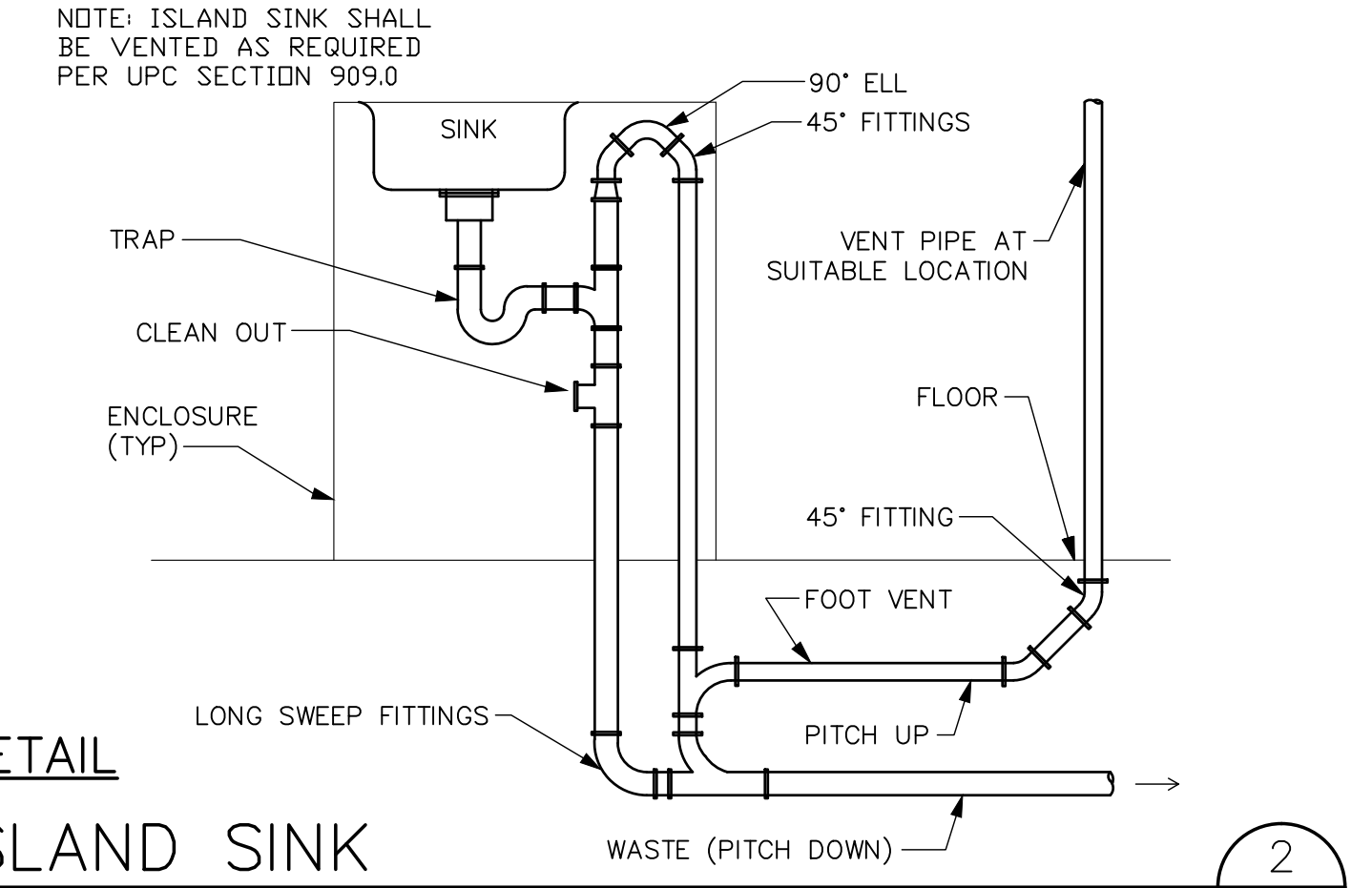
VENT TERMINATION
DETAIL
 SCALE: NONE

7
P600



RISER PIPE SUPPORT
DETAIL
 SCALE: NONE

1
P600



ISLAND SINK
DETAIL
 SCALE: NONE

2
P600



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TOWNZEN & ASSOCIATES
 PLAN APPROVAL
 Plans submitted for review are approved in accordance with local state applicable standards. This approval does not release the applicant of the responsibility of compliance with the applicable codes.
 Approved as submitted. 09/25/2019

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DETAILS & DIAGRAMS

P600

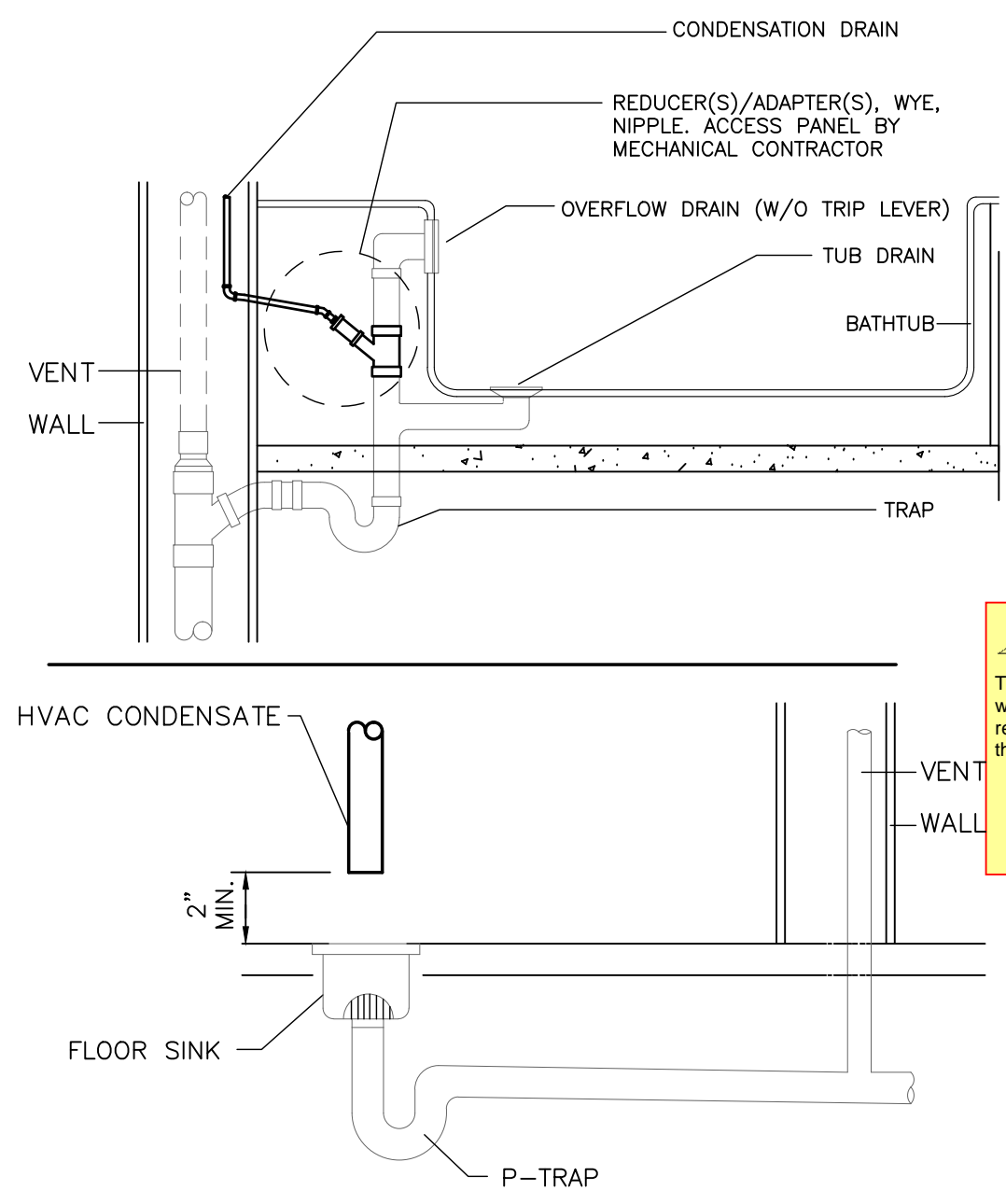
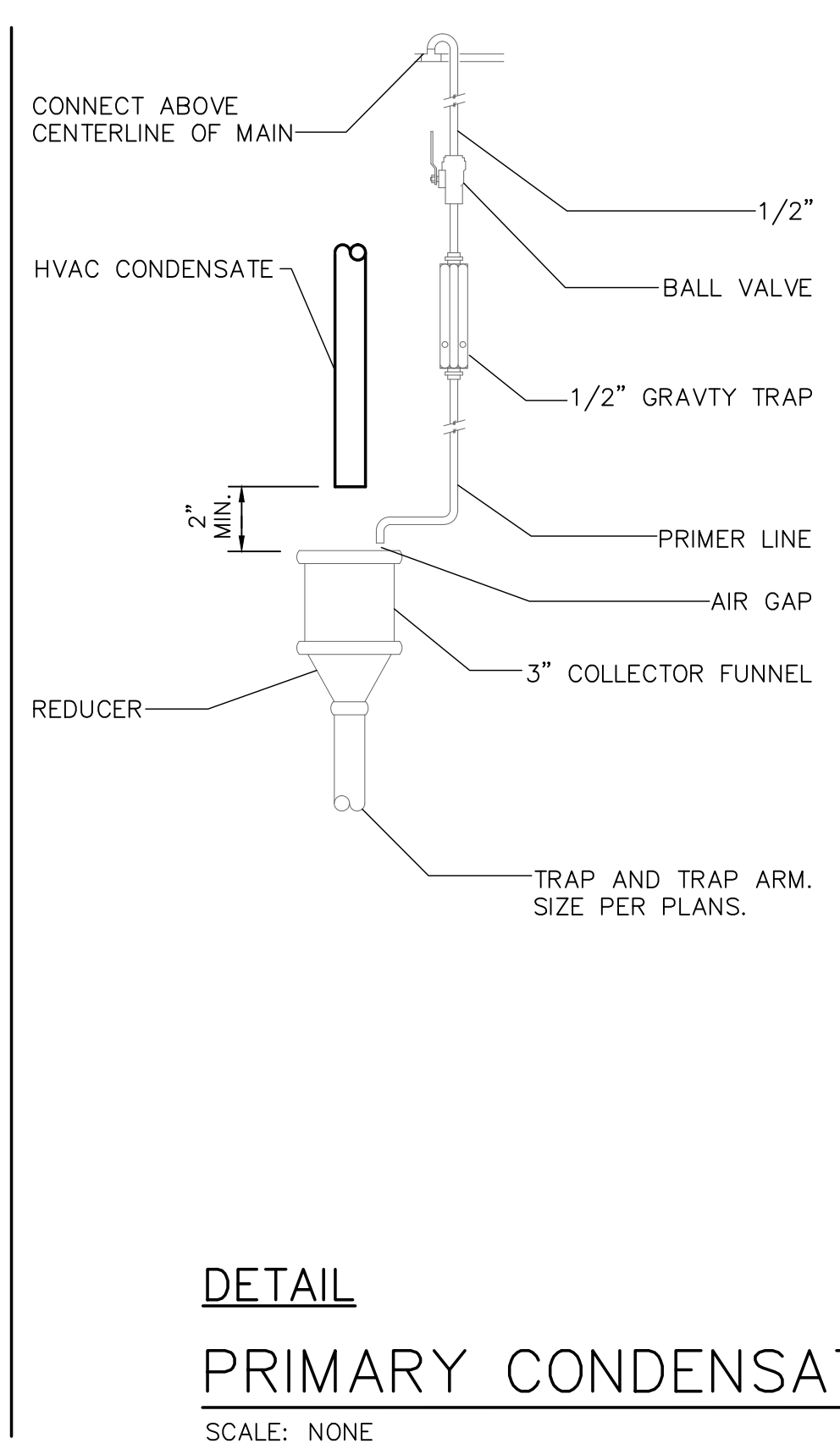
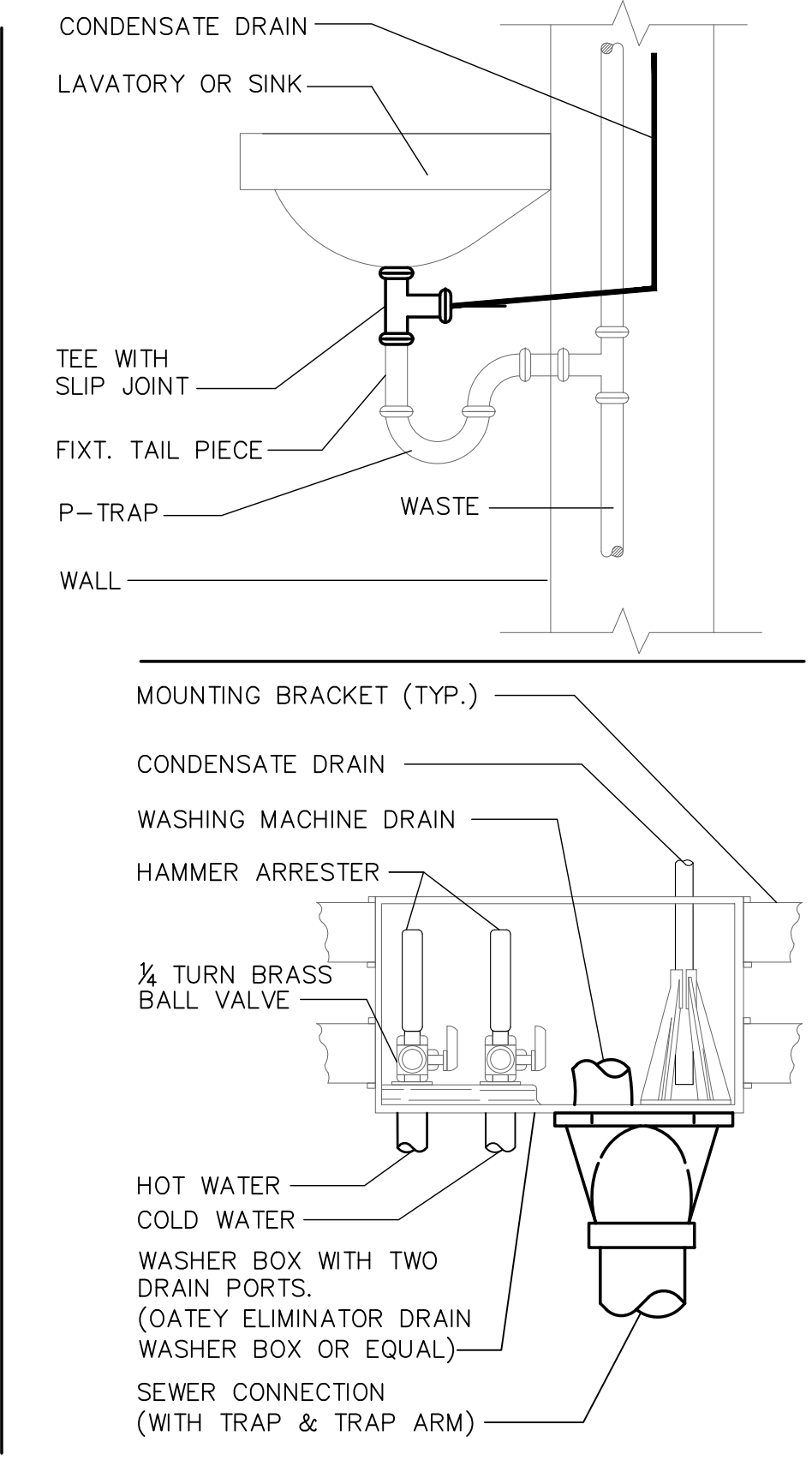
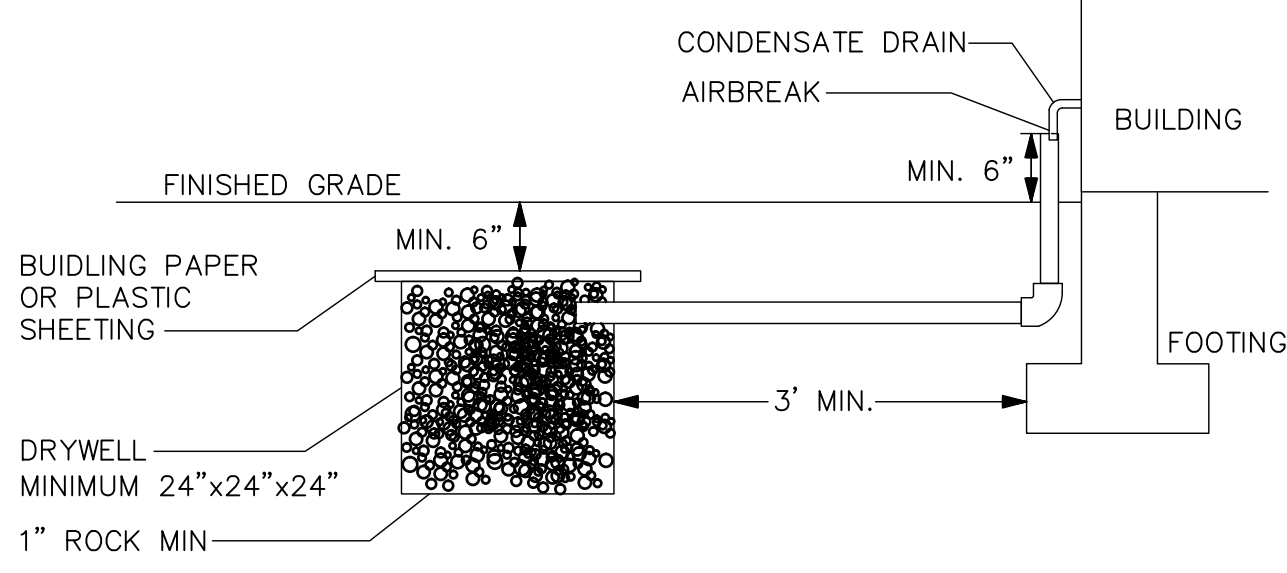
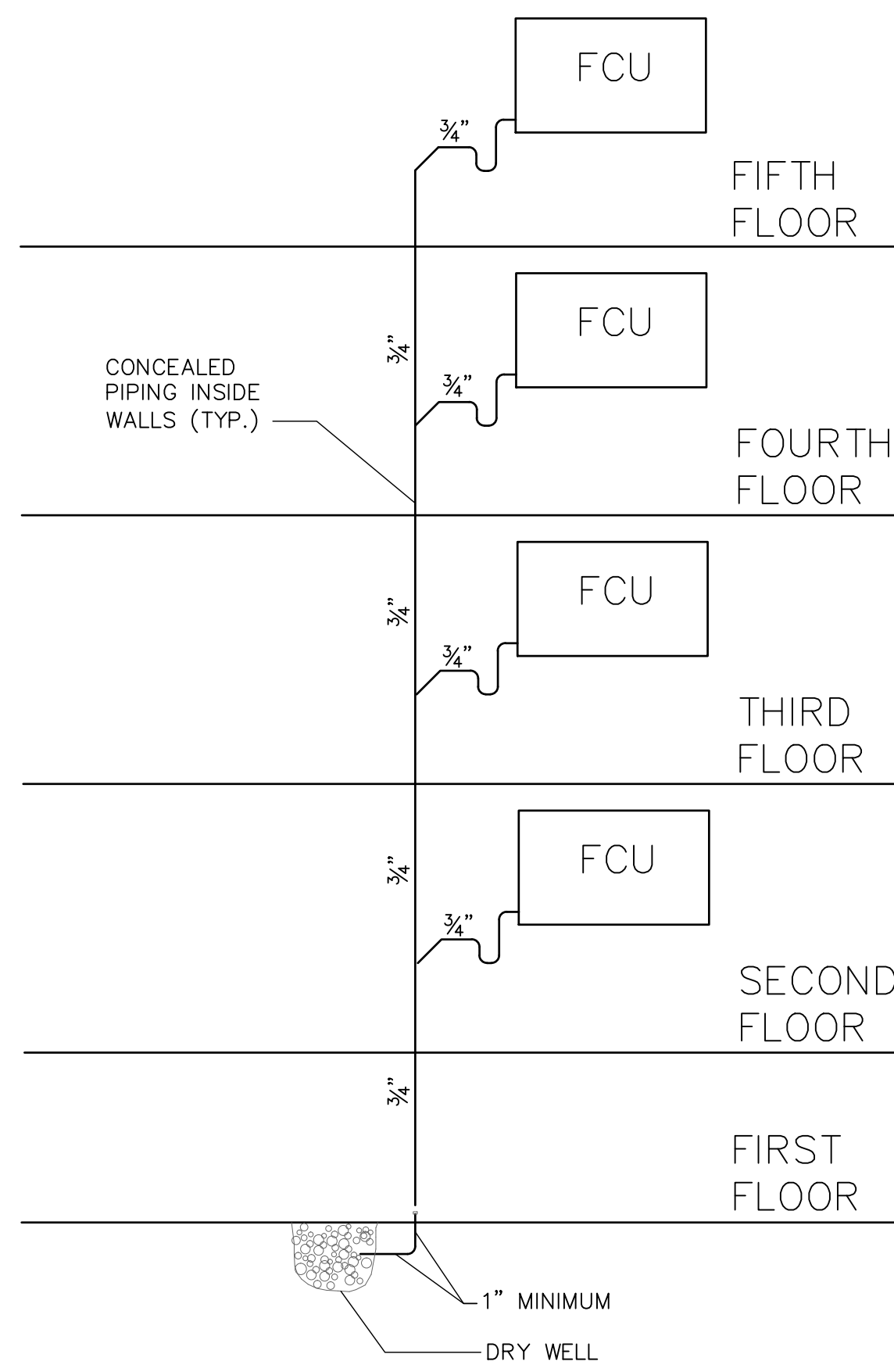
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NOTE:

- PER 2016 CPC SECTION 814.6 (AND 807.2 UPC) WHERE CONDENSATE WASTE FROM AIR CONDITIONING COILS DISCHARGE BY DIRECT CONNECTION TO A LAVATORY TAILPIECE OR TO AN APPROVED ACCESSIBLE INLET ON A BATHTUB OVERFLOW, THE CONNECTION SHALL BE LOCATED IN THE AREA CONTROLLED BY THE SAME PERSON CONTROLLING THE AIR CONDITIONING SPACE.
- ROUTING OF CONDENSATION DRAIN IS NOT SHOWN ON THE PLANS. CONTRACTOR SHALL FIELD TERMINATE CONDENSATION DRAIN. REFER TO 2015 UPS TABLE 814.1 FOR PIPING SIZING. PRIOR TO CONSTRUCTION, CONTRACTOR SHALL CONFIRM ROUTING OF CONDENSATION DRAIN WITH MECHANICAL CONTRACTOR & PLUMBING INSPECTOR.

CONDENSATE PIPE SIZING
PER 2015 UPC TABLE 814.1

| PIPE SIZE | CAPACITY (TONS) |
|-----------|-----------------|
| 3/4" | 20 |
| 1" | 40 |
| 1 1/4" | 90 |
| 1 1/2" | 125 |
| 2" | 250 |

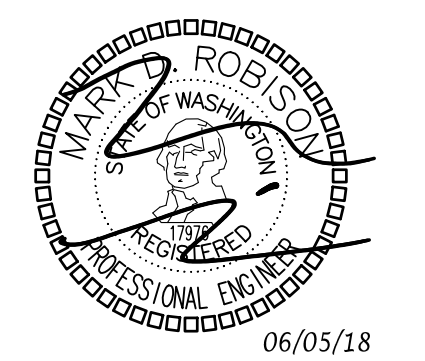


DETAIL
PRIMARY CONDENSATE TERMINATION OPTIONS
SCALE: NONE

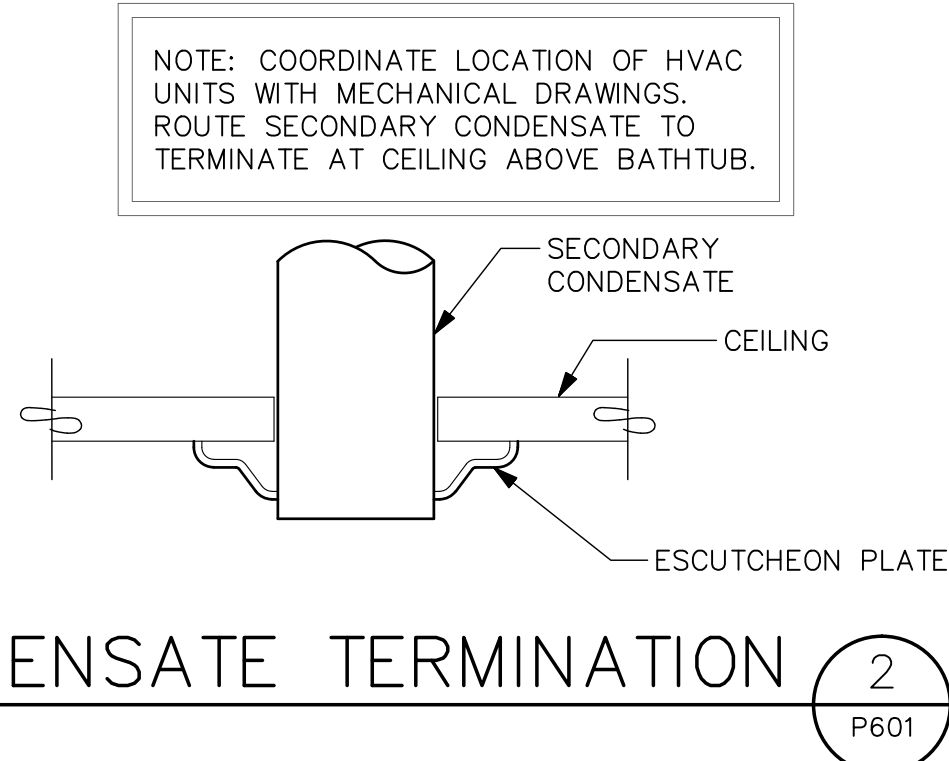
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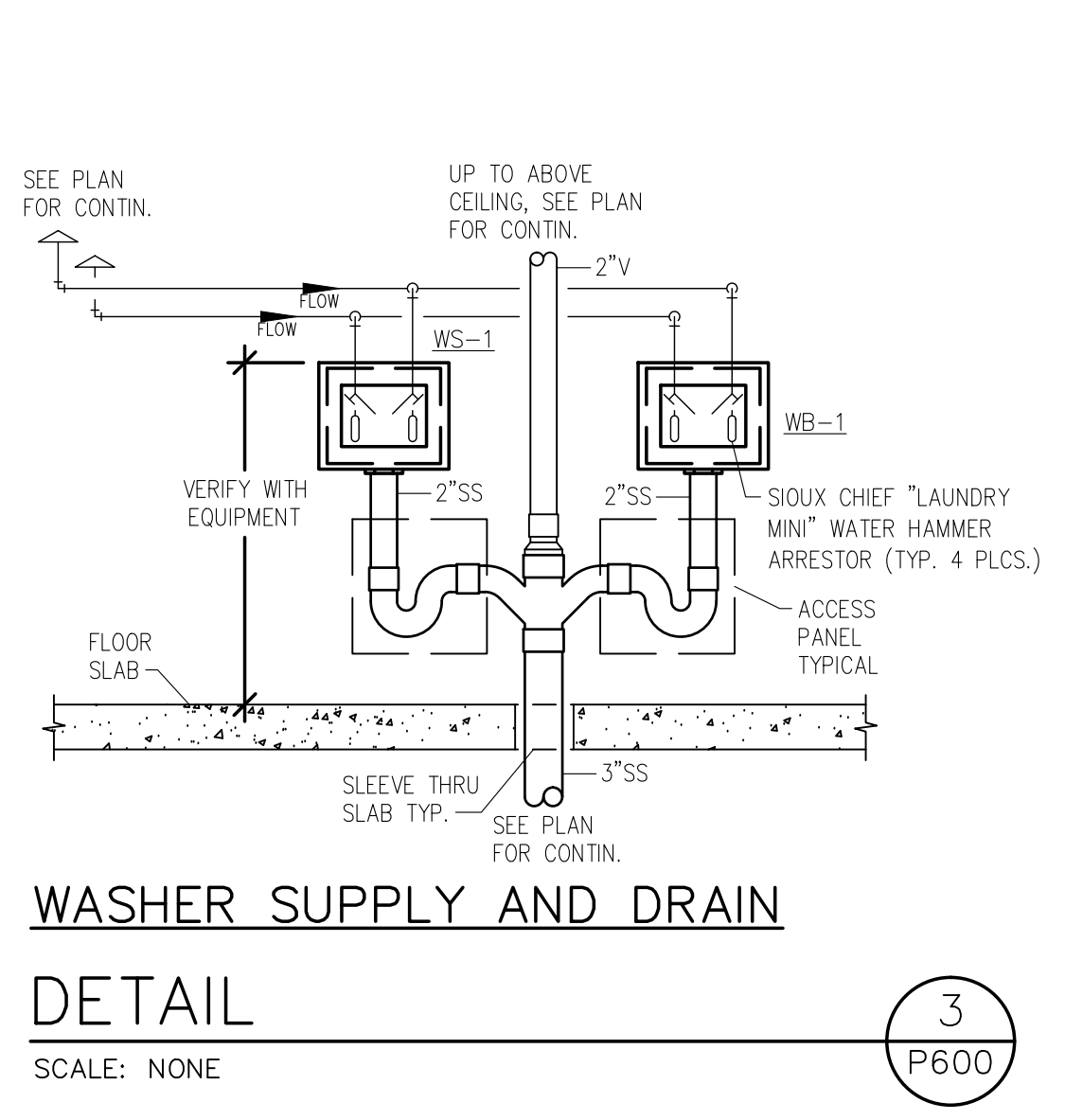
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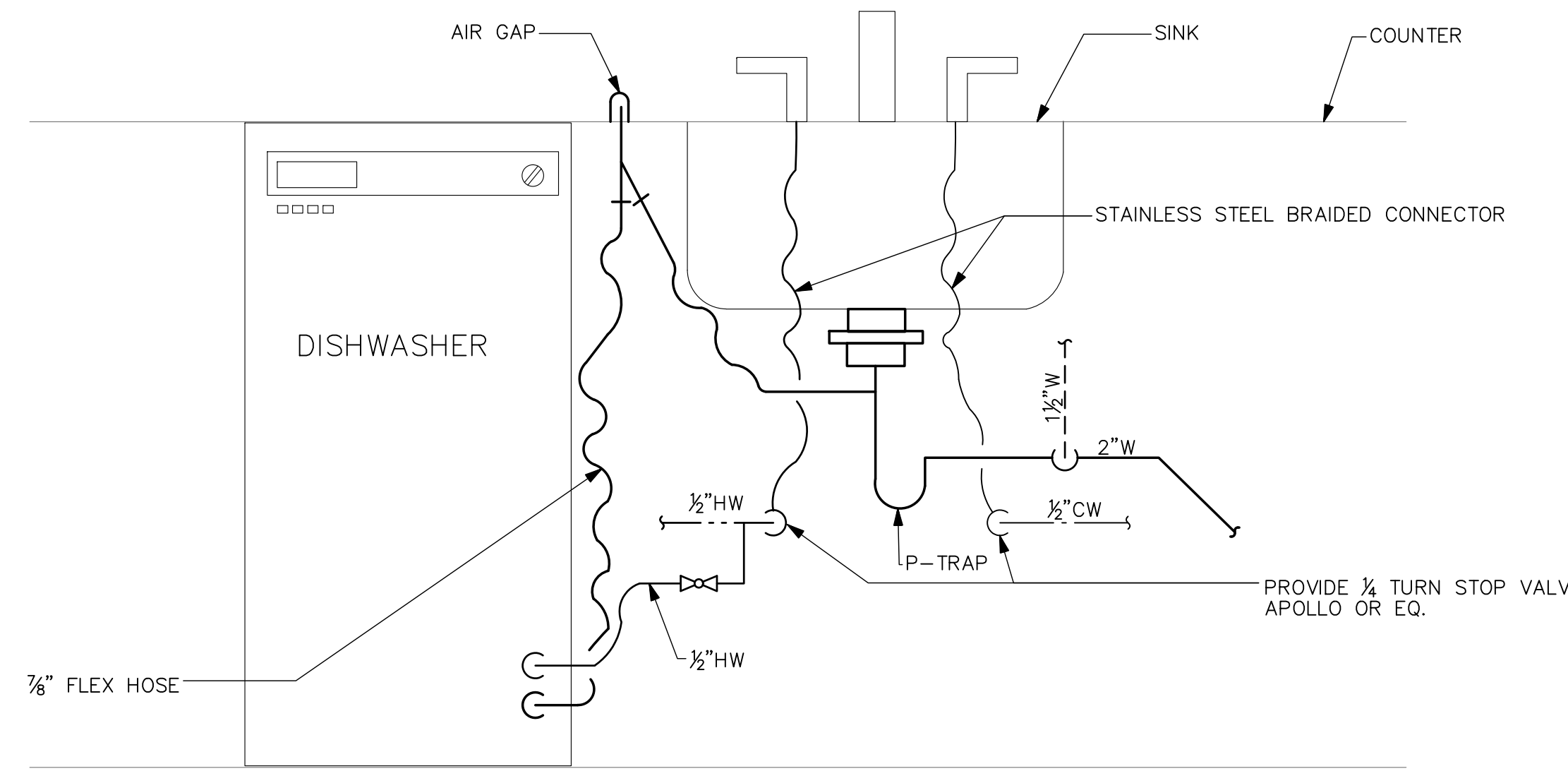
- NOTES**
- PROVIDE SECONDARY DRAINS AS REQUIRED PER SECTION 307.2.3 OF THE 2015 IMC.
 - FOR CONCEALED FAN COIL UNITS TERMINATE SECONDARY DRAIN ON CEILING ABOVE LAUNDRY TRENCH, OR MECHANICAL ROOM.
 - SECONDARY CONDENSATE DRAIN IS NOT REQUIRED FOR PTACS AND FAN COIL UNITS WITH FLOAT SWITCH THAT SHUTS DOWN THE EQUIPMENT BEFORE THE PAN OVERFLOWS.
 - PROVIDE CONDENSATION PUMP AS NECESSARY.



DETAIL
SECONDARY CONDENSATE TERMINATION
SCALE: NONE



WASHER SUPPLY AND DRAIN
DETAIL
SCALE: NONE



DETAIL
RESIDENTIAL DISHWASHER CONNECTION
SCALE: NONE

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P601

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