

BNSF Railway Company & Husky Oil Operations Limited

Engineering Design Report

BNSF Railway Black Tank Property

November 2021

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Signature Page

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Acronyms and Abbreviations

°C degrees Celsius
°F degrees Fahrenheit
amsl above mean sea level

AO Agreed Order

ARAR applicable or relevant and appropriate requirements

bgs below ground surface
BNSF BNSF Railway
BS biosparging

BSI biosparging injection

BV bioventing

CAP Cleanup Action Plan
CD Consent Decree

CMP Compliance Monitoring Plan

CO₂ carbon dioxide

cPAH carcinogenic polycyclic aromatic hydrocarbon

cSt centistoke

CUL cleanup level DO dissolved oxygen

DPWP Deep Contamination Cleanup Action Design Parameters Work Plan

Ecology Washington State Department of Ecology

EDR Engineering Design Report

ERM ERM-West, Inc. FS Feasibility Study

gal HC/acre-yr gallons of hydrocarbons per acre per year

HASP Health and Safety Plan
Husky Oil Operations Limited

IAWP Interim Action Work Plan for Shallow Soil Removal ITRC Interstate Technology and Regulatory Council

LNAPL light non-aqueous phase liquid MDPE medium-density polyethylene mg/kg milligrams per kilogram mg/L milligrams per liter MTCA Model Toxics Control Act

MW monitoring well

NAVD 88 North America Vertical Datum of 1988

NOI Notice of Intent **NPS** nominal pipe size NSC North Spokane Corridor **NSZD** natural source zone depletion **OMP** Operations and Maintenance Plan P&ID piping and instrumentation diagram PLC programmable logic controller PLP potentially liable person psi pound per square inch

psig pounds per square inch-gauge

PVC polyvinyl chloride

RCW Revised Code of Washington
RI Remedial Investigation
ROI radius of influence
RTF restoration timeframe

scfm standard cubic feet per minute

SD standard deviation

SEE steam-enhanced extraction

SF6 sulfur hexafluoride SGC silica gel cleanup

Site BNSF Railway Black Tank Property SOP standard operating procedure

SOW Scope of Work

SPT Steam Propagation Test

SSA surface soil area

SVRP Spokane-Valley Rathdrum-Prairie

TPH-D/HO total petroleum hydrocarbon-diesel and heavy oil

VFD variable frequency drive

WAC Washington Administrative Code

WSDOT Washington State Department of Transportation

1. INTRODUCTION

This Engineering Design Report (EDR) was prepared by ERM-West, Inc. (ERM) on behalf of BNSF Railway (BNSF) and Husky Oil Operations Limited (Husky) for the BNSF Railway Black Tank Property (Site) (Facility Site #98615712, Cleanup Site #3243), generally located at 3202 East Wellesley Avenue in Spokane, Spokane County, Washington (Figure 1-1). This EDR is required as part of the cleanup process under the Model Toxics Control Act (MTCA), Ch. 70.105D Revised Code of Washington (RCW), implemented by the Washington State Department of Ecology (Ecology). The EDR was prepared in accordance with the requirements of Washington Administrative Code (WAC) 173-340-400.

Ecology selected the remedy and documented it in the final *Cleanup Action Plan* (CAP; Ecology 2018a). Ecology based its selection on information presented in the *Remedial Investigation / Feasibility Study* (RI/FS; ERM 2013) and other relevant documents in the administrative record. Ecology named BNSF and Marathon Oil Company as the potentially liable persons (PLPs), with Husky performing Marathon Oil Company's obligations for this project. Site investigation activities were completed under Agreed Order No. 9188 and the CAP is being implemented under Consent Decree (CD) No. 19203114-32 (Ecology 2019). The selected remedy includes, but is not limited to the following activities:

- Excavation of shallow impacted soil (less than 15 feet below ground surface [bgs]) and transportation of the soil to an appropriate off-Site disposal facility. A substantial portion of this work was completed as an Interim Action under Amendment No. 1 to Agreed Order No. 9188. The remainder of the excavation will be completed by WSDOT during the NSC project and track realignment with confirmation sampling and documentation performed by the PLPs.
- Installation and operation of a bioventing/biosparging (BV/BS) system to address mobile¹ light non-aqueous phase liquid (LNAPL) petroleum hydrocarbons. Optimization and expansion of the BV/BS system will occur, if needed, to meet performance criteria established in the CAP. If the BV/BS system does not meet the performance criteria specified in the CAP, a contingent remedy (steam-enhanced extraction [SEE]) may be initiated if SEE is determined to be technically feasible based on steam propagation testing, pilot testing, and other criteria as specified in the CAP.

The remedy described in the CAP (Exhibit B of the CD) is required to be completed in accordance with the scope of work (SOW) and schedule (Exhibit C) included in the CD. The work plan for pre-design field investigations was completed in August 2020 and the pre-design field investigation was completed in November 2020 (CD SOW Task B1). This EDR and its companion documents, the Compliance Monitoring Plan (CMP), Health and Safety Plan (HASP) and Operations and Maintenance Plan (OMP), fulfill the requirements of CD SOW Tasks B3, B5, and B6. Following Ecology's approval of the EDR, the detailed plans and specifications needed to construct and operate the BV/BS system will be prepared, fulfilling the requirements of CD SOW Task B4. The EDR, CMP, HASP, and OMP are supplements to the construction plans and specifications. All personnel involved with the execution of the remedy will review the EDR, CMP, HASP, and OMP with the construction plans and specifications.

¹ Per ITRC (2019), measurable LNAPL in a well is evidence of mobile LNAPL. Concentrations exceeding residual saturation can, but do not always, indicate mobile LNAPL. Per Section 4.4.2.2 of the CAP, "Ecology's residual saturation-based CUL for TPH-D/HO in subsurface soils (i.e., below 15 feet bgs) is 5,630 mg/kg. It is, however, understood that residual saturation varies at the Site. Therefore, the TPH-D/HO CUL for subsurface soil is used to identify areas requiring cleanup action, but compliance with the cleanup standards will be based on the absence of LNAPL in monitoring wells (and not on TPH-D/HO concentrations in subsurface soils)." Therefore, the area of mobile LNAPL at the Site requiring cleanup action includes intermediate soils exceeding the TPH-D/HO CUL of 5,630 and the area of measurable LNAPL in wells.

1.1 Purpose of the EDR

Per WAC 173-340-400(4)(a), the purpose of the EDR is to provide sufficient information for the development and review of the construction plans and specifications for the remedy. The specific information requirements of WAC 173-340-400(4) for the EDR are listed in Table 1-1 with the corresponding section of this EDR where the required information is provided.

2. SITE BACKGROUND

This section describes the historical, physical, and environmental conditions that are relevant to the design of the remedy. The information is from the RI/FS (ERM 2017), subsequent groundwater monitoring reports, the *Interim Action Completion Report* (ERM 2019a), and publicly available information from the Washington State Department of Transportation (WSDOT).

2.1 Site Description

The Black Tank Property is situated within an industrial and transportation corridor in the Hillyard neighborhood of northeast Spokane. Its specific location is in the northwest quarter of Section 3, Township 25 North, Range 43 East of the Willamette Meridian, along a main north-south BNSF rail line (Figure 1-1). Currently, operations include active rail traffic and WSDOT's construction of the North Spokane Corridor (NSC) freeway project. All structures associated with historic operations have been removed.

Based on the information currently known, the Site is generally congruent with the property shown on Figure 1-1. It consists of approximately 18 acres, and is generally bounded by Market Street to the west, Wellesley Avenue to the north, the former Aluminum Recycling Corporation site to the east, and the Sem Materials cleanup site (currently Western States Asphalt Inc.), and undeveloped land to the south (Figure 2-1). BNSF and WSDOT own the property. BNSF's property is zoned Light Industrial, and WSDOT's property is zoned Center and Corridor Core; this zoning allows for many types of uses including commercial, office, residential, and parks.

2.2 Site History

BNSF and its predecessors have owned most of the property since at least 1910. BNSF provided WSDOT with an easement through a portion of the property for the NSC freeway project. BNSF and its predecessors historically leased portions of the property and infrastructure to other operators, including Blackline Asphalt Sales, Husky, Intermountain Asphalt Company, and Koch Materials.

Based on historical documents, the area was developed as early as 1913. Historical operations included railroad transport, the Black Tank fueling and maintenance system, the Red Tank fueling and maintenance system, the Chemical Solution Pipeline, and an asphalt storage and transfer system (the Liquid Asphalt Pipeline). The Black Tank was a 50-foot-diameter, 420,000-gallon, aboveground storage tank used to store Bunker C oil for the purpose of refueling locomotives from circa 1913 until at least 1956. The Black Tank was subsequently leased to other operators, who stored oil for asphalt and other petroleum products until its removal in 2006. The Red Tank was a 420,000-gallon, aboveground storage tank used to store diesel in support of locomotive refueling until its removal sometime between 1990 and 1997. At some point or points in time, operations resulted in infrastructure leaks and/or fuel releases.

2.2.1 Previous Investigations

Characterization activities began in 2006, and culminated with completion of the RI in 2017. The final RI/FS report describes the investigation activities and results obtained through March 2016 (ERM 2017). It documents the nature and extent of hazardous substances in soil and groundwater based on the data available at that time. Post-RI/FS groundwater monitoring continued on a quarterly basis from 2016 through 2018 and semi-annually from 2019 through 2020.

The RI characterized the physical, chemical, and biological conditions needed to select a remedy. It defined the nature, magnitude, and extent of impacts that pose a potential risk to human health and/or the environment by identifying the contaminants of concern (COCs) and the media and areas having

concentrations of COCs that exceed MTCA cleanup levels (CULs). It determined petroleum hydrocarbons and constituents thereof as the only COCs. Specifically, petroleum hydrocarbons (total, diesel, heavy oil, and free product), naphthalene, and carcinogenic polycyclic aromatic hydrocarbons (cPAHs) are the COCs that exceed MTCA CULs in the soil and groundwater. Analytical testing demonstrates that the petroleum hydrocarbons are a mix of Bunker C, asphaltic oils, and diesel that is weathered and viscous.

Soil and/or groundwater with LNAPL and/or total petroleum hydrocarbon-diesel and heavy oil (TPH-D/HO) concentrations exceeding the applicable CULs define the areas of impact requiring cleanup. Media with CUL exceedances include surface soil (≤ 15 feet bgs), intermediate soil (greater than 15 feet to top of the smear zone at 156 feet bgs), LNAPL in smear zone soil (156 to 185 feet bgs or 1,854 to 1,883 feet above mean sea level [amsl]), and groundwater. Section 2.4 details the nature and extent of impacts, and Figure 2-1 illustrates the areas of impact requiring cleanup.

2.2.2 Previous Remediation

Through previous or ongoing remediation work, all accessible primary sources of COCs have been removed. Accessible refers to not being beneath or within the setback areas of the active rail lines. In 2006, the Black Tank was decommissioned and removed along with 10,270 tons of petroleum-impacted soil from the area around and beneath the tank (Figure 2-2). Additional structures and appurtenances were also removed including the Red Tank pipeline, a water pipeline, and portions of the Liquid Asphalt Pipeline and Black Tank pipeline encountered in the Black Tank excavation. The impacted soil and other waste materials were transported to a permitted landfill for disposal.

In late 2018, the first phase of a surface soil interim action was completed. The work was performed in accordance with Amendment No. 1 to AO No. 9188 (Ecology 2018b) and the final *Interim Action Work Plan for Shallow Soil Removal* (IAWP; ERM 2018). Section 7.1 of the CAP and Exhibit C of the CD also describe the work requirements. The *Interim Action Cleanup Report* (ERM 2019a) describes the first phase interim action work and confirmation sampling results. This work included removal of the remaining accessible underground piping, concrete infrastructure, and petroleum-impacted surface soil exceeding CULs. In addition to soils, infrastructure removed included 402 linear feet of asbestos-containing piping, 1,680 linear feet of non-asbestos-containing piping, 510 tons of concrete debris (including the Black Tank sump), and 660 gallons of oily liquid from within the piping and sump (Figure 2-2). Approximately 7,994 tons of petroleum-impacted surface soil were removed from SSA-1, SSA-4, SSA-5, and the eastern half of SSA-2 (Figure 2-2). Confirmation sampling results demonstrated that the CULs were met down to 15 feet bgs in all areas except the western sidewall of SSA-2, which will be addressed during the second phase of the interim action. Excavated soil and other waste materials were transported to Waste Management's Graham Road landfill for disposal.

Manual LNAPL removal began with LNAPL skimming tests in the first quarter of 2016 and continued quarterly during monitoring events from the fourth quarter of 2016 through 2018, and then semi-annually through 2020. Fourteen LNAPL removal events occurred between the first quarter 2016 and third quarter 2020, and each event included 8 to 15 monitoring wells. Removal rates have ranged from 1.1 to 8 gallons per event, and the total volume of LNAPL removed is approximately 30 gallons. The waste LNAPL is transported to an appropriately permitted treatment facility, where it is treated and recycled or disposed.

2.3 Physical Site Characteristics

2.3.1 Topography and Climate

The Hillyard area is generally level with an approximate elevation of 2,035 feet NAVD 88;² however, the rail lines running north-south through the Site are located in an excavated area approximately 100 feet wide and 5 to 10 feet below the surrounding grade. The region is semi-arid, receiving around 18 inches of precipitation annually with warm and dry summers. Most of the precipitation occurs in late fall through early spring; winter precipitation is usually in the form of snow. The annual mean temperature is about 50 degrees Fahrenheit (°F).

2.3.2 Regional Geology

The regional geology is primarily basalt flows of the Columbia Plateau overlain by Quaternary glacial flood deposits. The flood deposits are composed of thickly bedded, poorly sorted boulders, cobbles, gravel, sand, and silt, and are approximately 300 feet thick in the immediate vicinity. Depth to bedrock is believed to be 500 to 600 feet bgs. The coarse nature of the glacial flood deposits results in very high permeability. Intermittent layers of sand, silty sand, and silt/clay are present within the coarse deposits. The RI encountered fine-grained deposits at depths of 1,855 to 1,879 feet NAVD 88.

2.3.3 Site Geology

Glacial flood deposits composed of thickly bedded, poorly sorted boulders, cobbles, gravel, sand, and silt with intermittent layers of silty sand and silt/clay typify the substrate to a maximum drilled depth of 197 feet bgs (approximately 1,838 feet NAVD 88). As summarized in the RI/FS report (ERM 2017), the substrate consists of two grain-size-based facies:

- Sand and gravel facies—poorly sorted; gravel; coarse-, medium-, and fine-grained sand; and interbedded zones of these materials, with generally less than 10 percent silt and clay; and
- Silt and silty sand facies—lenses and beds of silt, silty sand, sand with silt, and interbedded zones of these materials with generally 20 to 40 percent silt and clay, but may have as much as 85 percent silt and clay.

The substrate predominantly consists of the sand and gravel facies, and contains more gravel at shallow depths. Discontinuous lenses of the silt and silty sand facies are scattered throughout the sand and gravel facies, and a relatively thin layer of the silt and silty sand facies is reported near the groundwater table, between the depths of 156 and 180 feet bgs (i.e., 1,855 to 1,879 feet NAVD 88) on the geologic logs of most of the monitoring wells. The consistency in occurrence of the silt and silty sand facies at the same horizon suggests that it may be a laterally continuous layer. Alternatively, it may be a series of discontinuous lenses of silt and silty sand facies at the same horizon.

2.3.4 Regional Hydrogeology

The primary underlying aquifer is the Spokane-Valley Rathdrum-Prairie (SVRP) aquifer. The aquifer flows from northern Idaho to the west and southwest down the Spokane Valley and, in some areas, at rates of over 100 feet per day. Once reaching the greater downtown Spokane area, groundwater flow in the aquifer turns north, where it flows through the Hillyard Trough area, which extends north from the Spokane River area to the Little Spokane River. In the Hillyard Trough, the average transmissivity is 3.9 million square feet per day, which translates to a horizontal hydraulic conductivity of 12,000 feet/day and velocities ranging from 41 to 47 feet/day (Drost and Seitz 1977).

² North America Vertical Datum of 1988

2.3.5 Site Hydrogeology

The SVRP aquifer is generally unconfined, and the groundwater table occurs at depths ranging from approximately 158 to 179 feet bgs as measured in monitoring wells. Groundwater levels fluctuate from 4 to 7 feet annually. Groundwater flow is to the north-northwest with a horizontal hydraulic gradient of approximately 0.0013 to 0.0014 (Figure 2-3). Assuming the hydraulic conductivity of the sand and gravel facies is 12,000 feet/day, the porosity ranges from 25 to 35 percent, and the flow gradient is 0.0014, groundwater flow velocities in the sand and gravel facies range from 48 to 67 feet/day. However, hydraulic conductivity testing of samples of the silt and silty sand facies near the groundwater table indicate much lower groundwater velocities in that portion of the aquifer. With hydraulic conductivities ranging from 0.006 to 38.5 feet/day, porosities ranging from 25 to 50 percent and a flow gradient of 0.0014, groundwater flow velocities in the silt and silty sand facies range from 0.000017 to 0.2 foot/day.

2.4 Site Impacts

In most areas of the Site, the impacts were limited to surface soils and did not migrate to groundwater. However, in the area of the Black Tank, Black Tank Sump, and the Chemical Solution Pipelines and Dispensers, the COCs migrated in narrow vertical columns through the surface soil and the intermediate soil to the groundwater table where it formed LNAPL in the smear zone soil and an intermittent dissolved-phase plume. Figure 2-4 is a north-south cross-sectional view showing the vertical and horizontal extent of impacts. Figure 2-5 shows a plan view of current soil and groundwater LNAPL and TPH-D/HO impacts. The following subsections describe the media (i.e., surface soil, intermediate soil, LNAPL, and groundwater) having COCs exceeding CULs.

2.4.1 Surface Soil

Surface soil (0 to 15 feet bgs) samples defined five areas where COCs (TPH-D/HO, cPAHs, and naphthalenes) exceeded CULs. These areas are defined as SSA-1 through SSA-5 (Figure 2-2). SSA-1, SSA-4, and SSA-5 and a portion of SSA-2 were excavated as part of the first phase of the surface soil interim action (see Section 2.2.2 and Figure 2-2). The western part of SSA-2 and all of SSA-3 remain as they are beneath or within the setback areas of active rail lines (Figure 2-2) and will be excavated once rail realignment is complete.

2.4.2 Intermediate Soil and Mobile LNAPL

Intermediate soils are defined as vadose zone soils that extend from below the surface soils (i.e., greater than 15 feet bgs) to the top of the smear zone (approximately 156 feet bgs; 1,883 feet amsl). Analytical data from intermediate soil samples demonstrates that concentrations of COCs (i.e., TPH-D/HO) exceed the residual-saturation-based CUL (5,630 mg/kg)¹ applicable to intermediate soil. Figure 2-1 shows a plan view of intermediate soils exceeding the CULs; it encompasses the former area of the Black Tank, Black Tank Sump, and Chemical Solution Pipelines operations. Figure 2-4 illustrates the vertical extent of the intermediate soils exceeding the CULs.

2.4.3 Smear Zone Soil and Mobile LNAPL

The smear zone soil extends from 156 to 185 feet bgs (i.e.,1,883 to 1,854 feet amsl) and straddles the high and low groundwater elevations (Figure 2-4). Analytical data from the smear zone soil samples demonstrate TPH-D/HO impact above CULs. Measurable LNAPL is present in some groundwater monitoring wells having screens that straddle the smear zone.

The Interstate Technology and Regulatory Council defines mobile LNAPL as LNAPL that will accumulate in a well; therefore, the basis of the areal extent of mobile LNAPL at the Site is the presence or absence

of measurable LNAPL in monitoring wells. As illustrated on Figure 2-1, there are two areas of mobile LNAPL; the main mobile LNAPL area covers approximately 5.8 acres and the small mobile LNAPL area covers approximately 0.2 acre. The lateral extent of the mobile LNAPL areas (based on the presence/absence of LNAPL in monitoring wells) has not increased since thickness gauging began in 2014.

The main mobile LNAPL area is divided into three subareas to reflect the relative time needed to remediate each area. The mobile LNAPL area encompassing the historically thickest accumulations of LNAPL is expected to take the longest time to achieve remediation goals, and is identified as the high restoration timeframe (RTF) area. The mobile LNAPL area having greater than 1 foot of measureable LNAPL, but excluding the high RTF area, is the medium RTF area, and the area having less than 1 foot of measureable LNAPL is the low RTF area (Figure 2-5). Estimates of the volume of mobile LNAPL in each RTF area based on the size of each area, the gauged LNAPL thicknesses measured in monitoring wells in March 2016, and estimates of the porosity and percent mobile LNAPL saturation were presented in the RI/FS report (ERM 2017) and are summarized in Table 2-1.

Since completion of the RI, additional LNAPL thickness gauging data has been collected and a more rigorous evaluation of the data has been performed as follows:

- LNAPL thicknesses were gauged in monitoring wells quarterly from 2016 through 2018 and semiannually from 2019 through 2020, and seasonal and long-term trends in gauged LNAPL thicknesses were evaluated.
- Three additional water table monitoring wells (MW-22R, MW-23R, and MW-31) were constructed in the mobile LNAPL area and LNAPL thicknesses were gauged in those wells in September 2020.
- Hydrostratigraphic and gauged LNAPL thickness data from monitoring wells were evaluated to assess whether the LNAPL at each monitoring well is under unconfined, confined, and/or perched conditions, and whether those hydrostratigraphic conditions exaggerate the gauged LNAPL thickness in the well relative to the actual thickness of mobile LNAPL in the formation.
- The estimated volume of mobile LNAPL in each RTF area was updated based on the updated size of each area and the gauged LNAPL thicknesses measured in monitoring wells in September 2019.

Post-RI LNAPL gauging data is presented in Table 2-2. Figures 2-6 and 2-7 show changes in gauged LNAPL thicknesses over time and seasonally for monitoring wells having LNAPL thicknesses greater than, and less than, 1 foot of LNAPL, respectively. Appendix A presents an evaluation of the hydrostratigraphic data from the monitoring wells with measurable LNAPL.

Since March 2016, when the improved LNAPL thickness gauging procedure and LNAPL recovery from the wells was first instituted, gauged LNAPL thickness has generally decreased at monitoring wells having LNAPL thicknesses greater than 1 foot (Figure 2-6). As of September 2019—the last monitoring event that included all of the monitoring wells—four of the wells historically having LNAPL thicknesses greater than 1 foot (MW-03, MW-04, MW-17, and MW-20) had lower gauged LNAPL thicknesses than in March 2016. Only MW-07 had a higher gauged LNAPL thickness in September 2019 than in March 2016. As of September 2020, MW-03, MW-04, MW-07, and MW-17 have LNAPL thicknesses less than 1 foot (Figure 2-6). MW-20 was decommissioned following the September 2019 monitoring event. MW-20 and MW-03 demonstrated the largest decreases in gauged LNAPL thickness between March 2016 and September 2019. The gauged LNAPL thickness at MW-20 decreased from 9.34 feet to 3.36 feet from March 2016 to September 2019, and the gauged LNAPL thickness at MW-03 decreased from 8.55 feet to 0.11 foot from June 2016 to September 2020 (Figure 2-6).

Figure 2-7 shows gauged LNAPL thickness at monitoring wells having LNAPL thicknesses less than 1 foot have been generally stable since March 2016. Notable exceptions are:

- MW-19 decreased from 1.27 feet to 0.02 foot from March 2016 to September 2019. The well was decommissioned following the September 2019 monitoring event, and
- MW-01 increased from 0.03 foot to 1.2 feet from March 2016 to September 2020.

Overall, data indicate a decrease in gauged LNAPL thickness since March 2016. This decrease is attributable to the revised LNAPL gauging method yielding more accurate and verified LNAPL thickness data, natural source zone depletion (NSZD), and LNAPL recovery.

Post-RI data was used to update the lateral extent of the RTF areas. The updates include:

- Extending the medium RTF area to the southeast to include MW-01 because recent gauging of that well consistently showed LNAPL thicknesses greater than 1 foot, and
- Identifying a potential future contracting of the northwestern extent of the low RTF area to exclude new monitoring well MW-22R since there was no measurable LNAPL at that well. However, the well has only been gauged once and additional gauging is needed to verify the presence or absence of mobile LNAPL at that location, before making any adjustment to the low RTF area.

The updated RTF areas are shown on Figure 2-5. The only update considered for designing the remedial action is expanding the medium RTF area to include MW-01.

The hydrostratigraphic evaluation (Appendix A) demonstrates that the LNAPL is generally under unconfined conditions.³ Therefore, the gauged thicknesses measured in monitoring wells or determined from the volume of LNAPL removed from the wells is generally representative of the formation thickness of mobile LNAPL. However, five of the monitoring wells (MW-2, MW-3, MW-7, MW-18, and MW-20) occasionally exhibited perched LNAPL conditions. Research by Kirkman et al. (2013) and Reyenga and Hawthorne (2015) demonstrated that perched LNAPL conditions can result in exaggerated gauged LNAPL thicknesses. Appendix A describes the LNAPL conditions identified at each of the monitoring wells and the data used to identify those conditions.

MW-2, MW-3, MW-7, and MW-18 exhibit perched LNAPL conditions when the groundwater table drops below the top of a confining layer situated near the bottom of their normal groundwater fluctuation intervals. These low groundwater conditions only occur in the late summer and fall of dry years; thus, the perched conditions are an occasional seasonal condition and not the norm (Appendix A). The wells exhibit unconfined conditions during most seasons, and the gauged LNAPL thickness data from those seasons are not exaggerated. However, the occasional spike in gauged LNAPL thickness observed at these wells is likely the result of seasonal perched LNAPL conditions.

MW-20 has three silt confining layers, one of which is 2.5 feet thick, within its normal groundwater fluctuation interval (Appendix A; Figure A-11). Based on staining and fluorescence data from the MW-20 soil core samples, LNAPL is present above, between, and within these potential confining layers (Appendix A). The top of the LNAPL layer is consistently above these potential confining layers. However, the LNAPL/groundwater interface was below the top of the 2.5-foot-thick potential confining layer from March 2016 through March 2018 and above it from June 2018 through September 2019. These data indicate MW-20 had perched LNAPL conditions and exaggerated gauged LNAPL thicknesses from March 2016 through March 2018 and unconfined LNAPL conditions with no exaggerated gauged LNAPL thicknesses from June 2018 through September 2019 (Appendix A: Table A-1 and Figure A-11).

³ The site data show that the aquifer is generally unconfined with respect to both groundwater and LNAPL. However, it exhibits perched LNAPL conditions in localized areas during certain groundwater level conditions. In the same aquifer materials, groundwater may migrate vertically with little or no restriction, displaying unconfined conditions; whereas, the presence of residual LNAPL in the pore space of the aquifer materials and the higher viscosity and capillary limitations of LNAPL may restrict its vertical migration, resulting in perched LNAPL conditions.

Although occasional perched LNAPL conditions at MW-2, MW-3, MW-7, and MW-18 exaggerate some of the gauged LNAPL thicknesses at those wells, most of the gauged LNAPL data from those wells are not exaggerated. Correcting the exaggerated data results in relatively modest changes in the reported LNAPL thicknesses for those wells, and the changes do not affect the footprint of the LNAPL RTF areas. Similarly, the perched LNAPL conditions at MW-20 from March 2016 through March 2018 exaggerate the gauged LNAPL thicknesses from that period, but the unconfined LNAPL conditions from June 2018 through September 2019 yield gauged LNAPL data for MW-20 that are not exaggerated. The actual formation thickness of LNAPL at MW-20 can be determined from the June 2018 through September 2019 well data (1.87 to 2.46 feet). Correcting the exaggerated LNAPL thicknesses reported at MW-20 does not affect the footprint of the LNAPL RTF areas, but it does reduce the estimated volume of mobile LNAPL in that general area.

Updated baseline LNAPL viscosity and transmissivity were collected during the pre-design investigation. Those data and an updated estimate of the volume of mobile LNAPL are described in Section 4.

2.4.4 TPH-D/HO Groundwater Plume

TPH-D/HO concentrations exceeding the CULs are occasionally detected in groundwater samples from monitoring wells immediately cross-gradient (MW-15 and MW-24) and/or downgradient (MW-11, MW-16, and MW-22) of the mobile LNAPL area (Figures 2-1 and 2-5). The TPH-D/HO in groundwater is a combination of petroleum hydrocarbons released from the LNAPL and petroleum metabolites generated from biodegradation of the LNAPL. Analyses with and without silica gel cleanup (SGC) demonstrates that the TPH-D/HO concentrations in groundwater include petroleum metabolites from LNAPL biodegradation. It appears that groundwater TPH-D/HO concentrations mostly appear in the spring and early summer, possibly the result of groundwater rise and liberation of metabolites generated from biodegradation of LNAPL in the upper portion of the smear zone. This typically results in the TPH-D/HO plume expanding in downgradient (and occasionally cross-gradient) directions during the spring and summer, and generally contracting in the fall and winter. Petroleum metabolites are highly degradable (Zemo et al. 2016); therefore, it is unlikely that they will extend north past Wellesley Avenue at concentrations exceeding the CUL.

2.5 Natural Source Zone Depletion

To estimate baseline NSZD rates carbon dioxide (CO₂) soil flux monitoring (carbon traps and carbon flux chambers), metabolic gas monitoring, and groundwater NSZD parameter data were collected during the RI. The NSZD data and evaluation are presented in the RI/FS report (ERM 2017). The evaluation concludes that the NSZD rates for: (1) the vadose zone soil ranges from 229 to 1,681 gallons of hydrocarbons per acre per year (gal HC/acre-yr) with an average of 791 gal HC/acre-yr, and (2) the saturated zone ranges from 6.4 to 73 gal HC/acre-yr with an average of 43 gal HC/acre-yr. No post-RI NSZD data have been collected; therefore, the baseline NSZD rates have not been updated.

2.6 North Spokane Corridor Project

The NSC project involves construction of the remaining 5 miles of a 10.5-mile-long freeway (U.S. Route 395) running north-south along the eastern border of Spokane. WSDOT's alignment for the freeway places part of it over the Site's western portion. WSDOT, BNSF, and Ecology coordinated plans to allow freeway construction and remediation to proceed with minimal interference. The part of the NSC project that affects the timing of the remedy construction includes:

Rerouting existing active rail lines to areas east and west of the planned freeway corridor;

- Rerouting the access road that currently accesses the Site and Western States Asphalt, Inc. from the north (Wellesley Ave) to access the east via E. Rich Avenue and N. Ferrall Street;
- Constructing a new interchange for Wellesley Avenue immediately north of the Site;
- Constructing the freeway through the Site's west-central portion and a bike and pedestrian path west
 of the rail realignment;
- Constructing sound barriers or fences along the eastern and western boundaries of the NSC corridor;
 and
- Restricting access to the NSC corridor during, and following completion of, the project.

WSDOT's final alignment for the NSC covers slightly more than the Site's western half; however, it avoids most of the area having the highest mass of COCs (i.e., the high RTF LNAPL and intermediate soil areas). Figure 2-8 shows the currently proposed NSC alignment relative to the impacted areas.

WSDOT initiated construction of the on-Site portion of the NSC project in early 2020. Their current forecast for completion is 2024.

The NSC project constrains the remedy as follows:

- Phase 2 of the Shallow Soil Interim Action cannot be performed until rail realignment is completed, which will allow access to the remaining impacted soil and infrastructure.
- Existing monitoring wells located in or near NSC construction elements (freeway, rail lines, etc.) were
 decommissioned and some replacement wells were constructed. Additional replacement wells are
 proposed in the CMP for construction in the accessible areas.
- Accessibility for installation, operation, maintenance, and monitoring of active remediation systems is limited because of access restrictions in the NSC corridor.
- Installation of active remediation systems cannot be initiated until after WSDOT completes the on-Site portion of the NSC project.

Phase 2 of the Shallow Soil Interim Action involves removing and disposing of the remaining impacted surface soil and associated underground piping and concrete infrastructure at SSA-2 and SSA-3. As illustrated on Figure 2-2, the western part of SSA-2 and all of SSA-3 were not excavated during Phase I of the Shallow Soil Interim Action because they are beneath or within the setback areas of active rail lines. Removal of this soil and infrastructure will be performed after rail realignment is complete, and the soil and infrastructure beneath the former rail lines is accessible. Based on the NSC project schedule for rail relocation, the area for Phase 2 of the Surface Soil Interim Action will be accessible in mid to late 2021.

In June 2020, WSDOT and BNSF determined that 14 existing monitoring wells required decommissioning to accommodate construction of the NSC project. Wells were decommissioned in September 2019 (ERM 2019b). Table 2-3 summarizes the decommissioned wells and Figure 2-8 shows their locations.

Ecology, WSDOT, and the PLPs previously agreed that no wells or other remediation infrastructure would be installed if that infrastructure would potentially require removal or relocation to accommodate WSDOT access and work for the NSC. Accordingly, the fences depicted in the drawings of the NSC project are located on the edge of their permanent easement for the NSC project. Placement of remediation system infrastructure outside the fence is acceptable without WSDOT involvement.

2.7 Active BNSF Rail Lines

The NSC includes active BNSF rail lines on the east and west sides of the freeway. The presence of these active rail lines poses safety hazards that restrict work on or near the rail lines. The requirements for work on BNSF property are presented in BNSF's General Safety Requirements (Appendix G of the HASP). All remediation system infrastructure should be located more than 25 feet from the nearest rail.

3. CLEANUP ACTION PLAN

3.1 Goals of the Remedy

The selected remedy will protect human health and the environment from indicator hazardous substances (referred to previously in this document as COCs) exceeding CULs at the Site. As part of a cleanup action plan cleanup levels are set for indicator hazardous substances rather than COCs. The remedy will be conducted in accordance with MTCA regulations (WAC 173-340).

Ecology has established an overall RTF of 20 years. However, it is generally agreed that cleanup standards should be attained as quickly as practicable. The initiation of the RTF starts following construction, start-up, and an initial period of shakedown for the selected remedy. The 20-year RTF is intended to be the measure by which Ecology will evaluate performance of the selected remedy alternatives.

3.2 Cleanup Standards

Per MTCA, the components of cleanup standards are CULs, points of compliance, and all applicable or relevant and appropriate requirements (ARARs) based on local, state, and federal laws and associated regulations.

3.2.1 Cleanup Levels

A CUL is the concentration at which a substance does not threaten human health or the environment. The process for establishing CULs for the impacted media is presented in the CAP (Ecology 2018a). Tables 3-1 and 3-2 present the final CULs for groundwater and soil, respectively.

Ecology's residual saturation-based CUL for TPH-D/HO in subsurface soils (i.e., below 15 feet bgs) is 5,630 milligrams per kilogram. It is, however, understood that residual saturation varies. Therefore, the TPH-D/HO CUL for subsurface soil is used to identify areas requiring remedial action, but compliance with the cleanup standards will be based on the absence of LNAPL in monitoring wells (and not on TPH-D/HO concentrations in subsurface soils).

A mobile LNAPL thickness of 1 foot or greater has been established as a screening level for where active cleanup technologies will be used for mobile LNAPL. This screening level is not the LNAPL CUL. The screening level is set higher than the CUL and is used to focus more aggressive cleanup technologies (and/or a contingent cleanup technology) on areas having the highest accumulations of mobile LNAPL.

3.2.2 Points of Compliance

Points of compliance are the locations where CULs must be met. Standard points of compliance are applicable. The standard point of compliance for groundwater CULs will be all groundwater beneath the Site from the top of the saturated zone to the lowest depth that could be affected by the Site. The standard point of compliance for surface soil (based on protection of the direct contact pathway) is all soil from ground surface to a depth of 15 feet. The standard point of compliance for soil CULs based on protection of groundwater is the entire soil column beneath the Site.

3.2.3 Applicable or Relevant and Appropriate Requirements

MTCA requires that all remedies comply with local, state, and federal ARARs. Table 3-3 presents the final Site ARARs. The CAP provides detail regarding compliance with the ARARs and an exemption from procedural requirements for remedies conducted under a consent decree.

3.3 Who Will Own, Operate, and Maintain the Remedy

As the identified PLPs, BNSF and Husky will own the remedy and will be responsible for operation and maintenance during and following construction. BNSF and Husky will procure contractors to implement the remedy, including operating, maintaining, and monitoring any remediation systems, on their behalf.

3.4 Description of Selected Remedy

Ecology selected the remedy described in the CAP because it will be protective of human health and the environment and is consistent with the preference for permanent solutions stated in RCW 70.105D.030(1)(b). The components of the selected remedy are described in the CAP and summarized in the following subsections.

3.4.1 Excavation of Impacted Surface Soil

The selected remedy for surface soil is excavation of impacted soil exceeding CULs to depths of 15 feet bgs. Excavation will occur in the five impacted surface soil areas (SSA-1 through SSA-5) identified in the RI and shown on Figure 2-2. A substantial portion of this work (i.e., excavation of SSA-1, SSA-4, SSA-5, and the eastern half of SSA-2) was completed as Phase 1 of the Surface Soil Interim Action. That work is described in Section 2.2.2 and illustrated on Figure 2-2. Phase 2 of the Surface Soil Interim Action will involve removing and disposing of the impacted surface soil and associated underground piping and concrete infrastructure that was not addressed during Phase I (i.e., the western part of SSA-2 and all of SSA-3).

3.4.2 Bioventing/Biosparging in High and Medium Restoration Timeframe Areas

The selected remedy for deep impacts in the high and medium RTF areas are BV and BS, which involve the injection of air into subsurface soil and groundwater, respectively. These remedial technologies accelerate the natural biodegradation processes normally achieved by NSZD in the unsaturated and saturated zones. A preliminary conceptual layout of the BV/BS system was provided in the FS report and is included herein as Figure 3-1. The preliminary BV/BS system layout as presented in the FS was designed considering:

- WSDOT's 2016 draft NSC alignment available at the time of the FS
- Gauged LNAPL thickness data from March 2016
- Assumed BV and BS radius of influence (ROI)

Figure 3-1 also highlights the difference between the NSC freeway layout during the conceptual layout of the BV/BS system and the layout in WSDOT's current NSC freeway design drawings.

Monitoring in the medium and high RTF areas of respiration rate, LNAPL viscosity, LNAPL transmissivity, gauged LNAPL thickness, mobile LNAPL mass, and TPH-D/HO concentrations in groundwater will be used to:

- Assess performance of the BV/BS systems relative to achieving CULs in the medium and high RTF area within the 20-year RTF and inform system optimization decisions.
- Determine whether portions of the medium and high RTF areas can transition to the low RTF area and from active remediation to NSZD.
- Assess whether achieving CULs in the high RTF area within the 20-year RTF is at material risk, and implementation of the contingent remedy should be considered.
- Determine if CULs have been achieved and active remediation can be terminated.

Per the CAP, the full range of monitoring parameters described above is needed to assess BV/BS performance. Respiration rates will provide the earliest and most consistent indication of BV/BS performance over the course of the remedy. The next monitoring parameters expected to show changes are LNAPL viscosity and transmissivity, resulting from biodegradation changing the composition and physical character of the LNAPL. Increases in TPH-D/HO concentrations (without SGC) in groundwater are also anticipated as enhanced biodegradation of the LNAPL results in increased petroleum metabolite concentrations. Reductions in gauged LNAPL thickness, mobile LNAPL mass and TPH-D/HO concentrations (with SGC) in groundwater may not occur early in the operation of the BV/BS system, but will follow when sufficient biodegradation has occurred to exhibit measurable changes in mobile LNAPL. Because the monitoring parameters have differing response times, a holistic evaluation of the monitoring data, based on the weight of the evidence, is needed to assess BV/BS performance with gauged LNAPL thickness being the current regulatory performance metric.

If monitoring indicates that the 20-year RTF is at risk, optimization of the BV/BS systems will be performed. Optimization techniques could include changing and/or increasing the air flow to existing wells, cycling, combining air injection with extraction in a push-pull configuration, increasing the density of BV and/or BS wells, bioaugmentation, and/or heated bioventing. Optimization, if needed, will be performed in a sequential manner, and each optimization step will be evaluated over a reasonable period of time before moving to the next optimization step.

The BV/BS systems must be operated until mobile LNAPL is no longer present in the high and medium RTF areas, or until no appreciable increase in the destruction of indicator hazardous substances over that of NSZD is attained, so long as cleanup standards for mobile LNAPL and dissolved-phase impacts will be achieved within the 20-year RTF.

Dissolved-phase groundwater treatment may be required if monitoring indicates the dissolved-phase plume poses a material risk of: (1) significantly increasing off-Site size and/or magnitude, or (2) not achieving the dissolved-phase groundwater cleanup standards within the 20-year RTF.

BS wells (or other applicable remedial technologies) may be constructed in other areas of the Site as needed to address the dissolved-phase groundwater plume.

3.4.3 Natural Source Zone Depletion in Low Restoration Timeframe Area

The selected remedy for deep impacts in the low RTF area is NSZD, which are the natural biodegradation processes normally achieved in the unsaturated and saturated zones. Monitoring of NSZD rates will be achieved for the saturated zone using groundwater NSZD parameter data and for the unsaturated zone using discrete vertical intervals of thermal and/or soil vapor data, and/or CO₂ soil flux data from carbon traps and/or carbon flux chambers. These data along with LNAPL viscosity, LNAPL transmissivity, gauged LNAPL thickness, and mobile LNAPL mass data will be used to assess:

- Progress of the remedy
- Whether achieving the CULs in the low RTF area within the 20-year RTF is at material risk

NSZD monitoring is not meaningful in areas where active remediation (e.g., the influence of the BV/BS systems) results in enhanced biodegradation rates. Those areas, which may include all or parts of the low RTF area, will be included in the BV/BS performance monitoring and not in NSZD monitoring. However, when active remediation no longer impacts (i.e., the BV/BS systems are no longer adding oxygen to the subsurface) an area with measureable LNAPL and/or TPH-D/HO concentrations exceeding CULs in groundwater, NSZD monitoring will be implemented in that area.

3.4.4 Institutional Controls and Environmental Covenant

The selected remedy includes institutional controls that may be necessary to limit or prohibit activities that interfere with the integrity of the remedies or result in exposure to hazardous substances, as described in Section 7.5 of the CAP and Section VI.6 of the CD. Institutional controls are measures undertaken to limit or prohibit activities that may interfere with the integrity of a remedy or result in exposure to hazardous substances. Such measures are required to assure both the continued protection of human health and the environment and the integrity of the remedy whenever hazardous substances remain at concentrations exceeding applicable CULs. Institutional controls can include both engineering and administrative controls.

Institutional controls will include an Environmental Covenant prohibiting excavation of impacted soil or the extraction of impacted groundwater for purposes other than remediation, and restricting future activities and uses of the Site as agreed to by Ecology and BNSF. The Environmental Covenants will be prepared consistent with WAC 173-340-440, RCW 64.70 (Uniform Environmental Covenants Act), and any policies or procedures specified by Ecology. The Environmental Covenant for affected properties will be recorded with the office of the Spokane County Auditor. Institutional Controls and an Environmental Covenant are further discussed in Section 6.4.

3.4.5 Contingent Remedy—Steam Enhanced Extraction

Ecology selected SEE as a contingent remedy for the high RTF area. If performance monitoring conducted following optimization efforts in the high RTF area demonstrate that achieving CULs in that area within the 20-year RTF is at material risk, Ecology will consider implementation of the contingent remedy in that area. The tasks to be implemented and the decision-making process for system optimization and contingent remedy implementation are described in Sections 7.3 and 7.3.3 of the CAP. The decision-making process was designed by the PLPs and Ecology to encourage continuous improvement and adaptive management of the remedy to achieve the cleanup standards within the 20-year RTF. If the contingent remedy must be pursued, then a work plan for pilot testing SEE will be prepared and will include a steam propagation test (SPT). In a letter dated 3 August 2020, Ecology approved combining the SPT with the SEE pilot test, if needed. However, those tests will be performed only if work completed per the CD SOW determines that the contingent remedy is necessary. If the SEE pilot test and SPT are performed and the results demonstrate that SEE is technically feasible based on the criteria established in the CAP, the PLPs will prepare a supplemental EDR for the contingent remedy. Therefore, the contingent remedy is not considered further in this EDR.

4. RESULTS OF PRE-DESIGN INVESTIGATIONS

Pre-design investigations were performed to collect: (1) data needed to inform the design of the BV/BS system, and (2) updated critical baseline data. The investigations included BV/BS ROI testing and updated baseline LNAPL parameter testing. The objectives, scope of work, and procedures for the investigations are presented in the final *Deep Contamination Cleanup Action Design Parameters Work Plan* (DPWP; ERM 2020). The following sections summarize the scope of the investigations and present the results and conclusions drawn from those investigations.

4.1 BV/BS Radius of Influence Tests

BV/BS ROI testing was designed to collect data that informs the ROIs that can be achieved by BV in the vadose zone soil and by BS in the saturated zone soil and groundwater. BV ROI testing was performed by injecting air into a BV injection well and monitoring parameters in BV performance monitoring wells situated at various distances from the injection well. BS ROI testing was performed using similar procedures, but with a BS injection well and BS performance monitoring wells. To avoid having the two tests interfere with each other, they were performed at different times and in different areas of the Site.

4.1.1 Bioventing Test

A BV air injection compressor and associated process piping and gauges were used for the BV ROI testing. The well array consisted of MW-08 for air injection, and new monitoring wells MW-22R and MW-23R and existing monitoring wells MW-05, MW-07, and MW-09 for performance monitoring. As shown on Figure 4-1, the performance monitoring wells are situated at distances from the BV injection well of 32 feet (MW-23R), 68 feet (MW-07), 96 feet (MW-22R), 114 feet (MW-09), and 124 feet (MW-05). A cross-sectional view of the BV ROI testing area (Figure 4-2) shows that the BV injection well and all of the performance monitoring wells had exposed screen above the top of the mobile LNAPL layer. Thus, all of the wells were suitable for use in the BV ROI test. The soil boring, well construction, and well development logs for the new wells (MW-22R and MW-23R) are included in Appendix B. Photographs of the BV test equipment are included in Appendix C.

The following two BV ROI tests were performed:

Pressure versus Flow Rate Step Test

A step test was conducted at the BV injection well by slowly increasing air pressure and monitoring the injection flow rate. Flow rates were increased by adjusting the pressure and flow control valves on the test manifold in steps of 20, 40, 65, 80, and 110 standard cubic feet per minute (scfm), with each step conducted over approximately 90 minutes. Helium was injected at an approximate concentration of 1 percent by volume for the duration of the step test. Wellhead concentrations of helium, oxygen, CO₂, and methane as well as pressure and flow were collected at the performance monitoring wells.

Sustained BV Test

The step test results indicated that 110 scfm was the optimal flow rate for the sustained test (Figure C-1 in Appendix C). A 4-day sustained BV test began immediately following collection of the final step test data. Helium was injected at an approximate concentration of 1 percent by volume for the first 24 hours of the sustained test. After completion of the 4-day test, the flow was increased to 150 scfm and the system was monitored for a little less than a day. Air injection was then stopped. Wellhead concentrations of helium, oxygen, CO₂, and methane as well as pressure and flow were collected at the performance monitoring wells throughout the sustained BV tests and for 2 days after air injection ceased.

4.1.1.1 Performance Monitoring

Monitoring at the performance monitoring wells occurred approximately every 2 to 3 hours during the step and sustained BV tests. Before each round of monitoring, at least one well volume of air was purged from each performance monitoring well. Performance monitoring data from the BV tests is summarized in Table C-1 in Appendix C. Although monitored, pressure and flow changes were not observed at any of the performance monitoring wells during the BV tests.

Notable results from the monitoring of wellhead concentrations of helium, oxygen, CO₂, and methane during the BV tests are summarized as follows:

- Pre-air injection readings from the performance monitoring wells show detectable concentrations of methane (0.3 to 3.2 percent), low concentrations of oxygen (0 to 16.5 percent) relative to atmospheric levels (20.9 percent), and high concentrations of CO₂ (3.1 to 6.2 percent) relative to atmospheric levels (0.04 percent). These data indicating anaerobic conditions indicative of petroleum biodegradation were present in the LNAPL smear zone throughout the test area prior to initiating the BV test.
- Oxygen concentrations increased from 0.7 to 21.5 percent at MW-23R, from 16.5 to 20.2 percent at MW-07, and from 0.2 to 6.4 percent at MW-22R during the 4-day sustained BV test (Table 4-1 and Figure 4-3). The oxygen concentrations at these performance monitoring wells increased further during the subsequent higher flow rate period. No substantive changes in oxygen concentration were observed at MW-9 and MW-5 during the BV testing (Table 4-1).
- CO₂ concentrations decreased from 3.2 to 0.3 percent at MW-23R and from 3.1 to 1.6 percent at MW-07 during the 4-day sustained BV test (Table C-1 in Appendix C). The CO₂ concentrations at these performance monitoring wells decreased further during the subsequent higher flow rate period. No substantive changes in CO₂ concentration were observed at MW-22R, MW-9, and MW-5 during the BV testing (Table C-1 in Appendix C).
- Helium (the tracer gas injected at MW-8) was observed at MW-23R (32 feet from MW-8) approximately 1 day after starting the sustained BV test, and at MW-07 (68 feet from MW-8) approximately 2 days after starting the sustained BV test (Table C-1 in Appendix C). Although helium injection was terminated before the beginning of the second day of the sustained test, helium concentrations increased through part of the second day and into the third day at MW-23R and MW-07, respectively (Table 4-2, Figure 4-4). No helium was detected at MW-22R, MW-9, or MW-5 during the BV testing (Table C-1 in Appendix C).
- Methane concentrations decreased from 3.2 to 0.3 percent at MW-23R, from 3 to 1.8 percent at MW-22R and from 0.4 to 0.3 percent at MW-7 during the 4-day sustained BV test (Table C-1 in Appendix C). No sustained changes in methane concentration were observed at MW-9 and MW-5 during the BV testing (Table C-1 in Appendix C).

The BV test data demonstrate that 4 days of injecting air at a rate of 110 scfm into the vadose portion of the LNAPL smear zone via BV injection well MW-8 produced:

- Increased oxygen concentrations in the LNAPL smear zone out to at least 96 feet (at MW-22R)
- Decreased CO₂ concentrations in the LNAPL smear zone out to at least 68 feet (at MW-07)
- Detectable helium tracer gas in the LNAPL smear zone out to at least 68 feet (at MW-07)
- Decreased methane concentrations in the LNAPL smear zone out to at least 96 feet (at MW-22R)

The test data trends also indicate that impacts of the air injection had not reached steady state and were still expanding outward when the 4-day sustained test was terminated. Moreover, the increased flow rate

tested after the 4-day sustained test indicated that faster outward expansion of the impacts could be achieved under higher flow rates. Lastly, the data demonstrate that only 4 days of air injection were needed to transition the LNAPL smear zone at MW-23R (32 feet from the injection well) from anaerobic to aerobic degradation.

To determine the maximum ROI for a BV injection well operating at a flow rate of 110 scfm, an ROI versus time model was created from equation 2-27 of the U.S. Army Corp of Engineers *Soil Vapor Extraction and Bioventing Manual* (USACE 2002). The equation uses air flow rate, estimated soil porosity, and vertical air distribution thickness to predict ROI. The model was calibrated to fit the pilot test data by adjusting the vertical flow distribution thickness. The model backup is included in Appendix E, and the calibrated results of the model are illustrated on Figure 4-5.

The pilot test data calibrated model demonstrates that the ROI rate of increase starts declining (i.e., stabilizing to a maximum ROI) at approximately 225 feet and 20 days of operation. To determine the design ROI, a 1.5 to 1.8 safety factor was applied to the maximum ROI. Per the calibrated model, design ROIs of 125 and 150 feet would be achieved at 6 and 8 days of operation, respectively (Figure 4-5).

4.1.2 Biosparging Test

The BV air injection compressor and associated process piping and gauges were used for the BS ROI testing. The well array consisted of new BSI-1 for air injection, and new monitoring well MW-31 and existing monitoring wells MW-03, MW-04, and MW-05 for performance monitoring. As shown on Figure 4-6, the performance monitoring wells are situated at distances from the BS injection well of 11 feet (MW-31), 28 feet (MW-03), 60 feet (MW-04), and 133 feet (MW-05). A cross-sectional view of the BS ROI testing area (Figure 4-7) shows that the top of the BS injection well screen is approximately 22 feet below the groundwater/LNAPL interface and all of the performance monitoring well screens straddle the LNAPL layer. Thus, all of the wells were suitable for use in the BS ROI test. The soil boring, well construction, and well development logs for the new wells (BSI-1 and MW-31) are included in Appendix B.

The BS ROI testing included the following four tests:

Leak Test

To confirm structural integrity of the BS injection well (BSI-1), flow was introduced slowly until the pressure reached 5 pounds per square inch (psi) (i.e., air depressing water table without breaking out into formation). Flow value was then shut to confirm air pressure could be maintained inside the casing. Pressure was maintained for the duration of the test (i.e., approximately 10 minutes), which demonstrated the integrity of BSI-1.

Breakout Pressure Test

Breakout pressure was measured by slowly increasing the injection pressure until air began to flow into the formation. The breakout pressure was 9 psi.

Pressure versus Flow Rate Step Test

A step test was conducted at BSI-1 by slowly increasing air pressure and monitoring the injection flow rate. Flow rate steps of 10, 20, 35, 65, and 75 scfm were used, with each step conducted over approximately 90 minutes. Helium was injected at an approximate concentration of 1 percent by volume for the duration of the step test. Wellhead concentrations of helium, oxygen, CO₂, and methane as well as pressure and flow were collected at the performance monitoring wells.

Groundwater quality parameters (e.g., dissolved oxygen (DO), oxidation-reduction potential, temperature, conductivity, pH, and turbidity) were recorded at performance monitoring wells MW-04

and MW-31 using an Aqua troll multi-meter. A transducer was installed in MW-03 instead of an Aqua troll multi-meter because field observations during the LNAPL gauging at MW-03 indicated that its well casing had a thick coating of high viscosity LNAPL that would prevent the Aqua troll from passing through the LNAPL layer to the groundwater. MW-5 had an insufficient thickness of groundwater for either an Aqua troll or a transducer; therefore, no monitoring of groundwater quality parameters was possible at that well.

Sustained Biosparging Test

The step test results indicated that 25 scfm was the optimal air flow rate for the sustained test (Figure C-2 in Appendix C). A 4-day sustained BS test began immediately following collection of the final step test data. Helium and sulfur hexafluoride (SF_6) were injected for the first 40 and 60 hours, respectively, of the sustained test. Helium was injected at an approximate concentration of 1 percent by volume and was monitored as a tracer gas in the unsaturated zone. SF_6 was injected at 0.5 liter per minute and was monitored as a tracer gas in groundwater. The performance monitoring wells were monitored for the same parameters as for the step test. Groundwater samples were collected for SF_6 analysis from the performance monitoring wells and the BS injection well approximately 18 hours after completion of the sustained BS test. The system was then restarted at an increased flow rate (approximately 75 scfm) for 4 hours to assess whether measurable oxygen changes would occur at MW-03. A second round of groundwater samples for SF_6 analysis was collected from the performance monitoring wells 2 weeks following completion of the sustained BS test.

4.1.2.1 Performance Monitoring

Vapor monitoring at the performance monitoring wells occurred every 2 to 3 hours during the step test and the sustained BS test. Before each round of sampling, at least one well volume of air was purged from each of the performance monitoring wells. Groundwater quality data collection was continuous at MW-04 and MW-31, and pressure monitoring was continuous at MW-03. Groundwater SF₆ samples were collected twice after completion of the sustained test. Performance monitoring data from the BS tests is presented in Tables C-1 and C-2 of Appendix C. Although monitored, flow and pressure changes were not observed at any of the performance monitoring wells during the BS tests.

Notable results from the vapor and groundwater monitoring during the BS tests are summarized as follows:

- Pre-air-injection readings from the performance monitoring wells demonstrate detectable concentrations of methane (0.2 to 0.5 percent), low concentrations of oxygen (0 to 12.4 percent) relative to atmospheric levels (20.9 percent) and high concentrations of CO₂ (7.0 to 16.9 percent) relative to atmospheric levels (0.04 percent). These data represent anaerobic conditions, indicating petroleum biodegradation was occurring in the LNAPL smear zone throughout the test area prior to initiating the BS test.
- SF₆ was detected in groundwater samples collected from MW-31 (i.e., 11 feet from the injection well) and MW-03 (i.e., 28 feet from the injection well). SF₆ was not detected in groundwater samples collected from any of the other performance monitoring wells. All of the SF₆ data are summarized in Table 4-3, and the detected results from MW-31 and MW-03 are illustrated on Figure 4-8.
- Dissolved oxygen concentrations increased from 0.05 milligrams per liter (mg/L) to 10.08 mg/L at MW-31 as the air injection rate at BSI-01 was increased from 0 to 75 scfm during the step test (Figure 4-9). Dissolved oxygen concentrations varied from 0.49 to 10.25 mg/L at MW-31 while the air injection rate at BSI-01 was maintained at 25 scfm for the 4-day sustained BS test (Figure 4-9). The DO concentration at MW-31 increased to 12.01 mg/L when the air injection rate at BSI-01 was increased to 75 scfm following the sustained BS test (Figure 4-9).

- Dissolved oxygen concentration changes indicative of impacts from the BS injection well were not observed in any of the other performance monitoring wells. However, DO monitoring could not be performed in MW-03 (i.e., 28 feet from the injection well) because the Aqua troll would not fit down the well, past the thick coating of high viscosity LNAPL adjacent to the smear zone.
- Other groundwater quality parameter (e.g., oxidation-reduction potential, temperature, conductivity, pH, and turbidity) results showed no changes indicative of impacts from the BS injection well in any of the performance monitoring wells.
- Vapor constituent (i.e., helium, oxygen, CO₂, and/or methane) results showed no changes indicative
 of impacts from the BS injection well in any of the performance monitoring wells.
- Groundwater level results showed no changes indicative of impacts from the BS injection well in any
 of the performance monitoring wells.

The BS test data demonstrate that injecting air at a rate of 25 scfm into BS injection well BV-01 produced:

- Increased DO in the saturated portion of the LNAPL smear zone out to at least 11 feet (at MW-31).
- Detectable SF tracer gas in the saturated portion of the LNAPL smear zone out to at least 28 feet (at MW-03). Dissolved oxygen monitoring was not possible at MW-03; however, the presence of SF₆ at MW-03 is a clear indicator of BS influence.

Thus, a BS ROI can be conservatively estimated at 25 feet. A similar ROI (30 feet) was reported for the sparging system wells in the same aquifer at the North Market Street site (Ecology 2000, 2008). The performance monitoring data do not yield more lines of evidence for this ROI due to the influence of mobile LNAPL and low permeability soils in the saturated portion of the smear zone, on migration of vapors to the vadose zone where monitoring was performed. The increased, but inconsistent DO concentration trends observed at MW-31 during the 4-day sustained BS tests further suggest that air migration in the saturated zone is dynamic.

4.1.3 Updated Baseline LNAPL Parameter Testing

Updated baseline LNAPL parameter data were collected to ensure the design of the BV/BS system is based on data representing current conditions. The testing established baseline estimates of:

- Mobile LNAPL mass: The mass of mobile LNAPL was estimated in the RI/FS report; however, data collected during monitoring events subsequent to the RI/FS report were used to prepare an updated baseline estimate of mobile LNAPL mass. Mobile LNAPL mass is expected to decrease with time as a result of active cleanup and NSZD.
- LNAPL viscosity: LNAPL viscosity is a measure of LNAPL's resistance to flow and is expected to increase with time as a result of active cleanup and NSZD.
- LNAPL transmissivity: LNAPL transmissivity is an indicator of the potential for LNAPL to move through the formation—an indicator of mobility. Transmissivity is expected to decrease with time as a result of active cleanup and NSZD.

The procedures used for obtaining the updated baseline LNAPL data are described in the DPWP (ERM 2020).

4.1.3.1 Mobile LNAPL Mass

The estimate of mobile LNAPL mass was updated using the most current complete set of gauged LNAPL thickness data, which was collected during the September 2019 monitoring event. After that monitoring event, several LNAPL-containing monitoring wells (MW-18, MW-19, MW-20, MW-23, and MW-28) were decommissioned for NSC construction. To account for measurement and seasonal variations, the

standard deviation (SD - normal variation from the average) of LNAPL thickness measurements was calculated and used to estimate the upper 80th percentile (i.e., the mean plus 1 SD) of the updated mobile LNAPL mass estimate. Table 2-2 summarizes the updated mobile LNAPL mass estimate and the upper 80th percentile of the updated mobile LNAPL mass estimate. The backup calculations for these estimates are included in Appendix D.

An isopleth map of the September 2019 gauged LNAPL thickness data (i.e., updated vertical and horizontal extent of the mobile LNAPL) is included as Figure D-1 in Appendix D. The updated mobile LNAPL mass was calculated using the areas of the isopleths, the average LNAPL thickness represented by each isopleth and the formation porosity and percent mobile LNAPL saturation data reported in the RI. The data and the calculations are presented in Table D-1 of Appendix D. Notable updates from the RI estimates are as follows:

- Gauged LNAPL thickness at MW-01 increased to more than 1 foot; therefore, the medium RTF area
 was extended to the southeast to include that well. The result is that the medium RTF area is
 approximately 0.1 acre larger than indicated in the RI estimate.
- Gauged LNAPL thicknesses in all of the high RTF area wells (MW-3, MW-4, and MW-17) and the medium RTF well that previously demonstrated the largest gauged LNAPL thickness (MW-20) decreased substantively since the March 2016 monitoring event. This decrease has been documented in groundwater monitoring events from June 2016 through 2020.
- The mobile LNAPL estimate in the RI didn't include the LNAPL in the small low RTF area at MW-2.

The mean and SD of LNAPL thickness measurements for each LNAPL-containing well were calculated using gauging data collected from 2017 through 2020 (Table D-2). The SDs from all the LNAPL-containing wells was then averaged (Table D-2). This average SD was added the average LNAPL thickness in each RTF area (Table D-1) to account for normal thickness variation. The upper 80th percentile of the updated mobile LNAPL volume estimate was recalculated using the average LNAPL thickness plus 1 SD.

The mobile LNAPL volume estimate based on the September 2019 data is 90,000 gallons and the upper 80th percentile estimate is 130,000 gallons (Table 2-1). While this is a reduction of estimated mobile LNAPL volume of 29,000 to 69,000 gallons, most of the decrease is the result of having more accurate and verified gauged LNAPL thickness data in September 2019 than was available in March 2016. Most of the decrease is the result of lower gauged LNAPL thicknesses in the high and medium RTF areas (Table 2-1). By including the small low RTF area, the mobile LNAPL estimate for the low RTF area increased by a few thousand gallons. The remaining decrease in estimated mobile LNAPL volume can be attributed to NSZD and manual LNAPL removal.

4.1.3.2 LNAPL Viscosity

Updated baseline LNAPL viscosity data was established by collecting samples of LNAPL and testing them for viscosity: (1) in the field using a variation of ASTM International Standard D6910-04, *Standard Test Method for Marsh Funnel Viscosity of Clay Construction Slurries* (ASTM 2004), and (2) in the laboratory using ASTM Method D445-19 (ASTM 2019). The LNAPL samples were collected from monitoring wells situated within the mobile LNAPL plume (i.e., MW-01 through MW-05, MW-07, MW-09, MW-17, MW-22R, MW-23R, and MW-31).

For the field testing procedure, a funnel was selected that worked for all of the LNAPL types encountered at the Site and modified the testing procedure as needed to work efficiently for conditions. An updated standard operating procedure (SOP) for field viscosity testing that specifies the selected funnel and incorporates lessons learned from the pre-design testing is included in the CMP for use during future

performance monitoring. The field viscosity testing method allows for frequent and efficient viscosity data collection that can be periodically calibrated using laboratory viscosity testing.

The results of the field and laboratory viscosity tests are summarized in Table 4-4, and the laboratory test report is included in Appendix D. The field data include the temperature of the LNAPL sample and the time required for a set volume of LNAPL to pass through the field funnel (i.e., field sample test duration). The laboratory viscosity measured at 40 degrees Celsius (°C) for each sample was converted to viscosity at the field sample temperature (i.e., estimated field viscosity) using the Bunker C viscosity vs. temperature graph developed by WIN GD (2018). A Bunker C viscosity vs. temperature graph showing the converted viscosity results is included as Figure D-2 in Appendix D. The estimated field viscosity data are summarized in Table 4-4.

The laboratory viscosity tests for the pre-design investigation samples were performed at 40°C, whereas the laboratory viscosity tests for the RI samples were performed at 21°C (Table 4-4). To facilitate comparison, the RI viscosity data measured at 21°C was converted to viscosity at 40°C using the Bunker C viscosity vs. temperature graph developed by WIN GD (2018). A Bunker C viscosity vs. temperature graph showing the converted viscosity results is included as Figure D-3 in Appendix D. The converted viscosity results are summarized in Table 4-4.

The relationship of field sample test duration to estimated field viscosity is illustrated on Figure D-4 in Appendix D. The high correlation coefficient ($R^2 = 0.96$) for these data illustrated on Figure D-4 in Appendix D suggests that field sample test duration measurements can be reliably used to estimate field viscosity values. This relationship will be referenced in the CMP for use during future performance monitoring.

Evaluation of the LNAPL viscosity data demonstrates the following:

- The RI and pre-design investigation results are similar for the two wells from which LNAPL samples were collected and tested during both events (MW-01 and MW-03). The RI viscosities were converted for comparison and should be considered approximations; nonetheless, the results (412 and 500 centistoke (cSt) for MW-01, and 871 and 1,000 cSt for MW-03) are similar (Table 4-4).
- The viscosity at MW-03 is approximately twice the viscosity at MW-01 during both sampling events.
- The LNAPL sample collected from MW-31 exhibited the highest viscosity because the sample was too viscous to yield a laboratory test value.
- LNAPL viscosities are highest in the vicinity of the former Black Tank sump (i.e., MW-03, MW-04, and MW-31), decreasing a short distance from the core of the source area (i.e., MW-01, MW-02, MW-05, and MW-18), and lowest at the downgradient margin of the mobile LNAPL plume (i.e., MW-09 and MW-07). This observation suggests that the less viscous LNAPLs migrated the farthest from the source area.
- LNAPL at MW-17 is an outlier to the trend described above because it exhibited a low viscosity, but the sample location is situated between wells having LNAPL with the highest viscosities (i.e., MW-03, MW-04, and MW-31) and wells having LNAPL with intermediate viscosities (i.e., MW-01, MW-02, and MW-18).

4.1.3.3 LNAPL Transmissivity

LNAPL skimming tests were performed in monitoring wells MW-01 through MW-05, MW-07, MW-17, and MW-31 to update transmissivity estimates. Tests were not performed in new monitoring wells MW-22R and MW-23R because those wells contained little or no LNAPL. The manual skimming tests were performed over the course of 4 weeks and were concluded when at least one of the following conditions was achieved:

- Three consecutive well recharge rates were within 25 percent of each other and no consistently decreasing trend was observed;
- Insignificant LNAPL recovery occurred between removal events; or
- Four removal events had been performed.

The skimming test field data and calculated discharge rates are presented in Table D-3 of Appendix D.

The new skimming test data were used with equations 16 and 17 of ASTM Standard E2856-13 (ASTM 2013) to calculate updated baseline LNAPL transmissivities (Table 4-5). This is the same approach used to develop the baseline LNAPL transmissivities presented in the RI. The updated LNAPL transmissivity data were compared to transmissivity values provided in the RI and in the Interstate Technology & Regulatory Council (ITRC) LNAPL guidance documents (ITRC 2009a and 2009b). Evaluation of the transmissivity data demonstrates the following:

- Transmissivities are below the low end of the ITRC transmissivity range (i.e., 0.1 square foot per day) indicating hydraulic recovery of LNAPL is not practicable.
- MW-04 and MW-07 have updated transmissivities two orders of magnitude less than those observed during the RI.
- MW-03 has an updated transmissivity one order of magnitude less than that observed during the RI.
- MW-17 has an updated transmissivity consistent with that observed during the RI.

The reduction in transmissivities at MW-03, MW-04, and MW-07 are attributed to improved skimming test methods and NSZD in the high RTF area. The recent skimming tests occurred over 15 to 25 days, whereas the RI skimming tests occurred over 3 to 6 days. Additionally, the methods used for LNAPL removal and volume measurement are more consistent and reliable than those used during the RI. The longer test period and improved test methodology resulted in the updated transmissivity data being more reliable. An updated SOP for LNAPL skimming tests that specifies the selected test equipment and incorporates lessons learned from the pre-design testing is included in the CMP for use during future performance monitoring. Future tests will be conducted over a 15 to 25-day period, consistent with the recent testing, so results are representative and comparable.

5. CONCEPTUAL DESIGN OF BIOVENTING/BIOSPARGING SYSTEM

This section presents the 30 percent design for the BV/BS system, which is based on the design criteria and engineering concepts presented in this report. It is sufficient for the development of the next phase of engineering, which is the preparation of construction plans and specifications for the system.

5.1 Basis of Design

The basis of design for the bioventing and biosparging systems is to inject sufficient air into the high and medium RTF zones such that increased oxygen available to naturally occurring in-situ microorganisms stimulates enhanced biodegradation of mobile LNAPL and TPH-D/HO groundwater impacts to below cleanup standards. The remedial design and operations criteria have been established through consideration of multiple factors that influence the effectiveness and constructability of the remedy. The design was completed using industry standard engineering practices and calculation methods as well as local, state, and federal building codes and regulations.

The effectiveness of treatment, as it pertains to the successful stimulation of in-situ microbes and the resulting biodegradation of LNAPL, was considered when reviewing both the Site-specific hydrogeology data and the data collected during the BV and BS ROI tests. Specifically, design parameters such as the BV/BS air injection rates and well ROI were the result of evaluating field measurements (i.e., soil permeability, groundwater flow gradient, direction, and velocity, etc.) as well as the BV and BS ROI test data.

Initial respiration rates presented in the RI/FS (ERM 2016) indicate that the selected remedy will effectively reduce LNAPL mass through stimulated bioremediation to achieve the CULs within a 20-year RTF. However, baseline in-situ respiration tests (i.e., baseline NSZD testing in the saturated and unsaturated zones) will be completed after system construction, but prior to turning on the BV/BS systems and introducing additional oxygen to the unsaturated and saturated zones. The results of the baseline NSZD testing will be compared to the performance NSZD monitoring rates resulting from operation of the BV/BS systems to assess the change in biodegradation resulting from operation of the systems and the effectiveness of the selected remedy. The intent is to compare the baseline NSZD biodegradation rates to the biodegradation rates in the areas impacted by the bioventing and biosparging systems.

The future use and layout of the NSC components (i.e., freeway and rail infrastructure) that will pass through the Site were also considered in the system design. Specifically, the NSC impacted well placement, well design, and placement of system components such as mechanical equipment and conveyance piping. The design intentionally minimizes the need to place equipment and personnel in the NSC right-of-way for maintenance and performance monitoring events, and serves to protect workers from vehicular and rail traffic. Furthermore, future work such as rail and roadway maintenance has been considered in the system design.

Finally, the BV/BS systems have been designed to be flexible throughout the life cycle of the active cleanup, with equipment size and process controls designed to allow for future optimization, varied injection schemes, potential vapor extraction, and implementation of the contingent remedy, if necessary.

5.2 System Design

This section discusses the design considerations for the BV/BS systems.

5.2.1 Operating Criteria

To achieve the remedial action objectives, the designs of the BV/BS systems are based on the run conditions and ROI results from the field tests with consideration of Site-specific permeability and groundwater velocities. Specifically, the BV system has been designed to achieve an injection well ROI of 125 to 150 feet or greater in the medium RTF area smear zone under flow conditions of 100 scfm at 2 psi injection pressure, and the BS system has been designed to achieve an injection ROI of 25 feet or greater in the high RTF area saturated smear zone under flow conditions of 25 scfm at 9 to 12 psi injection pressure.

5.2.2 Bioventing System Components

The bioventing system will include the following primary components:

- Five BV air injection wells (BV-1 through BV-5 shown on Figure 5-1)
- A single air injection blower (B-200)
- Subsurface conveyance piping
- Valves, gauges, and flow measuring devices

5.2.2.1 Bioventing Air Injection Wells

The BV injection well layout shown on Figure 5-1 provides for air injection throughout the high and medium RTF areas with adequate overlap in BV injection well ROIs, assuming a conservative ROI of 125 feet. The BV injection well layout also provides for air injection throughout a significant portion of the low RTF area, particularly if an ROI of 150 feet or greater is achieved (Figure 5-1). For operational efficiency and worker safety reasons, the design places the wellheads for all of the BV injection wells east of the NSC freeway right-of-way, in an area having sufficient room for system construction, operation, and maintenance. To accomplish the objectives of (1) achieving full coverage of the high and medium RTF areas with the BV injection well ROIs, and (2) having all of the well heads on the east side of the NCS freeway, two of the BV injection wells (i.e., BV-04 and BV-05) will be high-angled wells (i.e., approximately 30 degrees from vertical) so that the well heads can be located east of the NSC freeway and their well screens can be located beneath the NSC freeway (Figure 5-1).

Bioventing injection wells BV-02 through BV-05 will be constructed in accordance with the SOP for BV well construction included in the CMP. BV-01, formerly MW-08, has already been constructed (i.e., it is a monitoring well that was screened at too shallow of an interval) and was used successfully during the pilot test. Injection screen intervals will be 20 feet in length for vertical wells and 25 feet for the high-angle wells, with the bottom of the injection screens targeting the seasonally low groundwater table for effective smear zone air distribution (Figure 5-2 and Table 5-1). BV-04 and BV-05 will be high-angled wells constructed as shown on Figure 5-3. The two vertical bioventing wells in the high RTF area (BV-02 and BV-03) will be constructed using carbon steel casing and stainless steel injection screens for potential future use as SEE injection wells if the contingent remedy requires implementation. The other BV injection wells are not located in the target area for the contingent remedy and thus will not be constructed as potential SEE injection wells.

5.2.2.2 Bioventing Air Injection Blower

A piping and instrumentation diagram (P&ID) for both the bioventing and biosparging injection systems is shown on Figure 5-4. The blower (B-200) minimum specification will be a flow of 600 scfm at a discharge pressure of 4 psig. This blower specification will provide a minimum injection rate of 100 scfm to each of the five proposed bioventing injection wells, for a total normal operating flow of 500 scfm. Additional flow

capacity will be available above the normal flow value to allow for (1) operation of some BV injection wells at flow rates as high as 150 scfm, or (2) the addition of another BV injection well, if monitoring data indicate optimization of the BV system is necessary. The expected well-head pressure necessary to achieve the minimum design flow rate is approximately 2 psi, based on observed conditions during the pilot test. Including friction losses between the blower and the injection wells, which will be minimized during final pipe size selection, the expected blower discharge pressure will be approximately 3 to 4 psi. The blower will be equipped with a variable frequency drive (VFD) to allow for flexibility in operating conditions, and will also be furnished with safety control devices, such as pressure relief valves, where necessary, to protect both the equipment and workers.

Based on the expected injection pressure and flow rates, a regenerative air blower is being considered for this application. However, final equipment selection will be made in conjunction with vendor input after detailed specifications have been prepared.

5.2.2.3 Subsurface Bioventing Conveyance Piping

The blower will discharge into a single main subsurface piping header, which is expected to be constructed of nominal pipe size (NPS) 6-inch polyvinyl chloride (PVC) or medium-density polyethylene (MDPE) piping. Branch lines will be constructed of the same material and will connect individual wells to the main header through an expected NPS 3-inch pipe. This design will allow for the addition of a second main header for extraction if during the life cycle of the BV/BS system it is determined that alternating between bioventing injection and vapor extraction is necessary. All conveyance piping will be installed in an approximately 36-inch-deep excavated trench.

Each bioventing wellhead will be completed in a subsurface traffic-rated well vault. Well-head components in each bioventing well vault will consist of air isolation valves, a flow measuring device such as a pitot tube and differential pressure gauge, a sample port, and static pressure gauges. A plan view of the conceptual layout of the BV and BS wells, piping, and mechanical facilities is included as Figure 5-5.

5.2.3 Biosparging System Components

The biosparging system will include the following primary components:

- Nine biosparging air injection wells (BS-01 through BS-09 shown on Figure 5-6)
- A single air injection compressor (AC-100)
- Subsurface conveyance piping
- Valves, gauges, and flow measuring devices

5.2.3.1 Biosparging Air Injection Wells

The BS injection well layout shown on Figure 5-6 focuses on the high RTF area and the immediately adjacent portions of the medium RTF area because this area has historically shown the highest mass of mobile LNAPL, and groundwater samples from this area exhibit DO readings ranging from 0 to 0.35 mg/L, which are below the level needed to sustain biodegradation of petroleum hydrocarbons in the saturated zone. The BS injection well layout provides for increased DO concentrations throughout the saturated portion of the LNAPL smear zone in the high RTF area and the immediately adjacent portions of the medium RTF area, assuming a BS ROI of 25 feet (Figure 5-6). In addition, the LNAPL and hydrogeologic data from MW-20 indicate that the area warrants further consideration because the well contained a thicker layer of mobile LNAPL than other Site wells and the well log suggests the presence of a localized lens of finer-grained deposits in and around which the LNAPL occurs. The atypical LNAPL and hydrogeologic conditions at the MW-20 location warrant placement of a biosparge well (BS-10) at that

location. BS-10 will be angle-drilled under the freeway from the east side of the Site and the well screen will be set slightly lower and upgradient of the lens of LNAPL at MW-20.

The BS injection wells are arranged in three transects oriented perpendicular to the groundwater flow direction. The wells within each transect are spaced approximately 45 feet apart to achieve some overlap in the 25-foot ROI of the BS injection wells, except for BS-10, which is located in the area of MW-20. The transects are spaced 100 feet apart on the basis that elevated DO concentrations from the BS injection wells are expected to extend at least 100 feet downgradient of the BS injection wells. The rationale is that groundwater flow velocities in the sand and gravel facies are 48 to 67 feet per day and the recovery data from the BS pilot test demonstrate that 1.5 to 2 days were required for the DO concentrations to decrease to pre-test levels (Figure 4-9). Therefore, elevated DO levels should extend downgradient at least 72 to 100 feet beyond the 25-foot ROI of the BS injection wells.

Most of the high RTF area is situated east of the NSC freeway right- of-way; however, a portion of the high RTF area and most of the medium RTF area extends beneath the freeway, and a portion of each extends beneath the Blue Knight line (Figure 5-6). For the same reasons described for the BV well layout, the BS well layout places the wellheads for all of the BS injection wells east of the NSC freeway right-of-way and west of the Blue Knight line. To accomplish this, BS-10 will be angle-drilled (i.e., approximately 35 to 40 degrees from vertical) under the freeway from the east side of the Site, and one of the BS injection wells (i.e., BS-06) will be low-angled wells (i.e., approximately 5 degrees from vertical) to reach under the Blue Knight line (Figure 5-6).

Biosparging injection wells BS-02 through BS-10 will be constructed in accordance with the SOP for BS well construction included in the CMP. BS-01, formerly BSI-01, was already constructed for, and used successfully during, the BS ROI test. Injection well screens will be 5 feet in length with the top of the screens 20 feet below the seasonally low groundwater table for effective saturated-zone air distribution (Figure 5-7, Table 5-1). BS-06will be such low angle well (i.e., approximately 5 degrees from vertical) that its construction will be similar to that shown on Figure 5-7. BS-10 well construction is shown on Figure 5-8.

The CAP states that BS injection wells located in the high RTF area will be constructed of materials for potential future use as SEE injection wells if the contingent remedy requires implementation. However, the BS injection well screens will be set 20 feet below the seasonally low groundwater table to obtain effective saturated-zone air distribution. This screen depth is too deep for effective steam propagation in the saturated zone. Therefore, the BS injection wells located in the high RTF area will not be constructed for potential use as SEE injection wells. Instead, the two BV injection wells located in the high RTF area will be constructed for potential use as SEE injection wells because their well screens will be placed at an appropriate depth for steam propagation near the top of the LNAPL smear zone. Additionally, some of the BV injection wells may be converted to thermal monitoring points if the contingent remedy requires implementation. However, conversion to thermal monitoring points does not require constructing the BS injection wells using different materials.

5.2.3.2 Biosparging Air Compressor

The biosparge air compressor (AC-100) will be sized to provide an injection rate of 25 scfm to each of the 10 wells. The minimum specification for AC-100 will be a flow rate of 250 scfm at a compressor discharge pressure of 15 psi. The expected well head pressure necessary to achieve the design flow rate is 9 to 12 psi. Including friction losses between the compressor and the wells, the expected compressor discharge pressure will be approximately 15 psi. However, a flow rate safety factor of at least 1.2 will be used for selection of the blower allowing for an increased flow rate to some wells if performance monitoring data indicate optimization is necessary. The compressor will be equipped with a VFD to allow for flexibility in

operating conditions and will also be furnished with safety control devices, such as pressure relief valves, where necessary to protect both the equipment and workers.

Based on the expected injection pressure and flow rates, a rotary claw air compressor is being considered for this application. However, final equipment selection will be made in conjunction with vendor input after detailed specifications have been prepared.

5.2.3.3 Subsurface Biosparging Conveyance Piping

The compressor will discharge into an above-grade piping manifold that will include connections for nine individual conveyance pipes, one for each of the nine biosparging wells. Each leg of the piping manifold will include isolation valves, a flow measuring device, pressure gauges, and a motorized ball valve or other similar remote-actuated valve. The remote-actuated valves will be connected to the system programmable logic controller (PLC), where user-defined custom timers will allow for complete and remote customization of the injection network, pulsing operations, and zone cycling.

The nine conveyance pipes are expected to be constructed of NPS 2-inch MDPE pipe, and will be buried in the subsurface trench along with the bioventing piping header. Each conveyance pipe will be routed to an individual biosparging well, which will be installed in a subsurface traffic-rated vault. Well-head components in each biosparging well vault will consist of isolation valves, static pressure gauges, and a sample port. A plan view of the conceptual layout of the BV and BS wells, piping, and mechanical facilities is included as Figure 5-5.

5.2.4 Controls and Telemetry

The BV and BS systems will be furnished with a single PLC and motor control cabinet that will control both systems. The specific components of the system controls and telemetry systems will be designed in conjunction with an equipment vendor and will be provided with the final constructed blower and compressor skid.

In general, the PLC will collect system operational data from numerous pressure and flow transmitters that will be included with each of the injection systems. The PLC will log transmitter data, which will be accessible via secured remote login. Additionally, the VFD run speeds, discharge pressure, and flow setpoints for both the bioventing blower and the biosparging compressor will be adjustable using the remote access provided by the PLC, allowing for changes to system operation without a dedicated Site visit. Furthermore, the PLC will control the remote-actuated valves located on each leg of the biosparging injection manifold to allow for multiple different operational configurations if performance monitoring data indicate optimization of the system is warranted. Finally, the PLC will automatically communicate mechanical alarm conditions and system faults via email callouts to facilitate timely troubleshooting and maintenance response.

The PLC and data will be accessed via cellular modem with an internal secured server and firewall. All data transmissions will be encrypted with industry standard technology and access will be limited to key project operations and data management personnel.

5.2.5 System Security, Protection, and Noise Suppression

All mechanical and electrical equipment will be located inside a lockable and weatherproof enclosure and may be delivered as a constructed skid and enclosure together. Although both the expected pieces of equipment, the regenerating blower and rotary claw compressor, are generally quieter than other types of fans (e.g., rotary lobe blowers), the enclosure will further attenuate the noise generated from the mechanical equipment. The enclosure will be located within a locked fence or other similar visible barrier. The compound will be installed with downward-facing, motion-activated flood lights for night-time visibility.

A security system with remote-access cameras and door alarms may be evaluated and installed at a later date if necessary.

5.2.6 Waste Management

No vapor emissions, or groundwater or LNAPL effluent is expected to be generated through operation of the BV/BS system. This reduces risks that may be presented to workers through handling of potentially impacted media, and eliminates the need for containment structures or hazardous material release control and alarm systems. If future modifications to the system result in generating emissions or effluent streams, then adequate treatment, containment, and/or release control and alarm systems will be considered in the system modifications.

Waste generated during construction of the BV/BS systems (e.g., soil from drilling and excavation, water from well development and equipment decontamination) will be managed in accordance with the operating procedures described in Section 5.3.3 of the CMP.

5.3 System Construction

5.3.1 Construction Testing

Construction testing (e.g., pipe pressure tests, acceptance testing of electrical and process connection) will be used to demonstrate adequate quality control prior to compliance sampling and system start-up. Specific tests will be outlined in the construction plan report.

5.3.2 Compliance Monitoring

Compliance monitoring that will be performed during and after construction is outlined in the CMP.

5.4 Health and Safety

Procedures proposed to assure health and safety during system construction and operation are detailed in the HASP, Appendix A of the CMP.

5.5 Permitting and State Environmental Policy Act Requirements

Permitting required for this project includes:

- A building permit for the building that will house the system equipment
- An electrical permit for the electrical drop
- Notice of Intent to Construct and Decommission an Environmental Investigation Well (Start Card) for each new well
- A grading permit for trench excavation for subsurface piping and grading for Site preparation

The Washington-licensed drilling contractor that constructs the new wells will prepare Start Cards prior to construction of each new well. The PLP's contractor for construction of the BV/BS system will obtain permits for the building, electrical drop, and grading prior to construction.

A construction stormwater permit will not be needed for this project because (1) the area of land disturbance is below the 1-acre permitting threshold and (2) there is an extremely low potential for stormwater discharge to waters of the state. No other permits from Table 3-3 are applicable to this project.

A State Environmental Policy Act (SEPA) Environmental Checklist for the BV/BS system was submitted with the final CAP (Ecology 2018a). A SEPA Environmental Checklist for the Shallow Soil Interim Action Work was provided to Ecology in April 2019. Ecology has approved both checklists. No additional SEPA checklists are required.

6. ADDITIONAL COMPONENTS OF CLEANUP ACTION PLAN

The following sections describe the other components of the selected remedy.

6.1 Excavation of Impacted Surface Soil

As described in Sections 2.2.2 and 3.4.1, surface soil within the western half of SSA-2 and the entire SSA-3 will require excavation during Phase 2 of the Shallow Soil Interim Action (Figure 2-2).

Removal of these areas will be performed by WSDOT after rail realignment is complete and the soil and infrastructure beneath the former rail lines are accessible. If asbestos-containing-material pipe removal is necessary, an NOI permit from the Spokane Regional Clean Air Agency will be obtained prior to removal. Excavation may extend deeper than 15 feet bgs if high-concentration source material is encountered, and it is readily accessible for removal without resorting to shoring or other substantive measures. All excavations will be backfilled (and compacted) with clean imported fill soil. WSDOT's remediation contractor will transport the impacted soil and other waste materials to an appropriately permitted landfill for disposal.

ERM will observe the cleanup activities, collect confirmation and performance soil samples, and report the results of the Phase 2 Surface Soil Interim Action work to Ecology. Compliance sampling in the excavation walls will be performed prior to backfill to ensure the lateral extent of impacted soil exceeding the CULs was removed. Soil samples will be collected in the excavation bottom to document the level of impacts, if any, left in place. Per the IAWP (ERM 2018), an addendum to the *Interim Action Completion Report* (ERM 2019a) will be submitted for Phase 2 of the Surface Soil Interim Action. The Phase 2 work will be performed in accordance with Amendment No. 1 to AO No. 9188 (Ecology 2018b) and the IAWP. Section 7.1 of the CAP and Exhibit C of the CD also describe the requirements of the work. Because Phase 2 of the Surface Soil Interim Action is described in the IAWP, details of the plan for cleanup of these inaccessible soils are not repeated in this EDR.

6.2 Manual LNAPL Removal

LNAPL will be removed manually from a network of monitoring wells in the high and medium RTF areas during groundwater monitoring events. Based on past performance, manual removal of LNAPL from the monitoring well network yields approximately 1.5 gallons of LNAPL per monitoring event. Therefore, this action will result in removal of approximately 6 gallons of LNAPL per year. Although this opportunistic removal of LNAPL is helpful, it is only 0.001 percent of the estimated mobile LNAPL volume in the medium and high RTF areas that will be removed through enhanced NSZD. Details regarding the removal process are presented in the CMP.

6.3 Natural Source Zone Depletion

NSZD will be utilized to address residual LNAPL and any remaining mobile LNAPL in areas not undergoing active remediation (i.e., the low RTF area and sections of the medium and high RTF areas as the BV/BS system is shut down). The rate of LNAPL depletion by NSZD is expected to be sufficient to eliminate the residual mobile LNAPL in these areas within the 20-year RTF. However, monitoring will be performed to verify the occurrence and rate of NSZD in these areas. Because NSZD monitoring is not meaningful in areas where active remediation enhances biodegradation rates, it will only be performed in areas that are not affected by active remediation. As shown on Figure 5-1, the BV system is expected to impact most of the low RTF area; therefore, NSZD monitoring will not be performed until portions of the system are shut down and NSZD alone is responsible for degrading the residual mobile LNAPL in these areas.

To evaluate saturated zone NZSD, groundwater samples will be analyzed for NSZD parameters. Four methods for evaluating vadose zone NSZD may be implemented at the site: thermal monitoring, diffusion gradient, carbon traps, and/or carbon flux chambers. Details of the NSZD monitoring are provided in the CMP.

6.4 Institutional Controls and Environmental Covenant

As described in Section 3.4.4 of this document, institutional controls will be put in place to limit or prohibit activities that may interfere with the integrity of the remedy or result in exposure to hazardous substances. It is expected that cleanup of surface soil to applicable CULs at standard points of compliance cleanup will be achieved prior to implementation of any institutional controls; therefore, institutional controls will not be required to assure continued protection of human health and the environment from the surface soil. However, groundwater is expected to have concentrations of TPH-D/HO exceeding the applicable CULs.

Institutional controls will include an Environmental Covenant prohibiting the extraction of impacted groundwater for purposes other than remediation, and restricting future activities and uses of the Site as agreed to by Ecology and BNSF. An initial draft of an Environmental Covenant for implementation following completion of the surface soil cleanup is included as Appendix F. The Environmental Covenant will be consistent with WAC 173-340-440, RCW 64.70 (Uniform Environmental Covenants Act), and any policies or procedures specified by Ecology. The Environmental Covenant will be finalized, executed, and recorded with the office of the Spokane County Auditor following Ecology's review and approval. BNSF will provide Ecology with the original recorded Environmental Covenant within 30 days of the recording date.

As conditions change (e.g., the footprint of impacted groundwater exceeding applicable CULs decreases or is eliminated), the Environmental Covenant may be amended in accordance with Ecology guidance.

7. SCHEDULE

There has been one modification to the schedule in Exhibit C of the CD. In a letter dated 3 August 2020, Ecology approved a 9 July 2020 request to design and perform Task B2, the SPT, in conjunction with the Steam Enhanced Extraction Pilot Test (Task B9), if triggered.

Table 7-1 presents the anticipated task schedule for the engineering design, procurement, and construction of the BV/BS system. The schedule begins with submittal of the draft EDR and extends through start-up and acceptance of the BV/BS system. The schedule is aligned with WSDOT's current anticipated schedule for NSC construction work, but is subject to change based on WSDOT's actual completion of the NSC construction.

8. REFERENCES

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ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

FIGURES

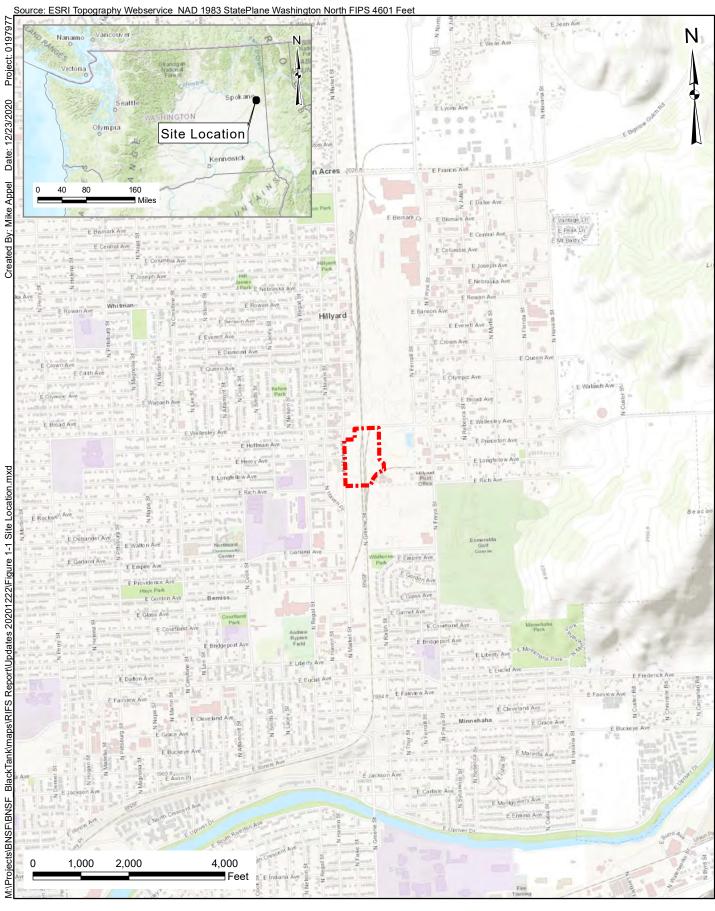
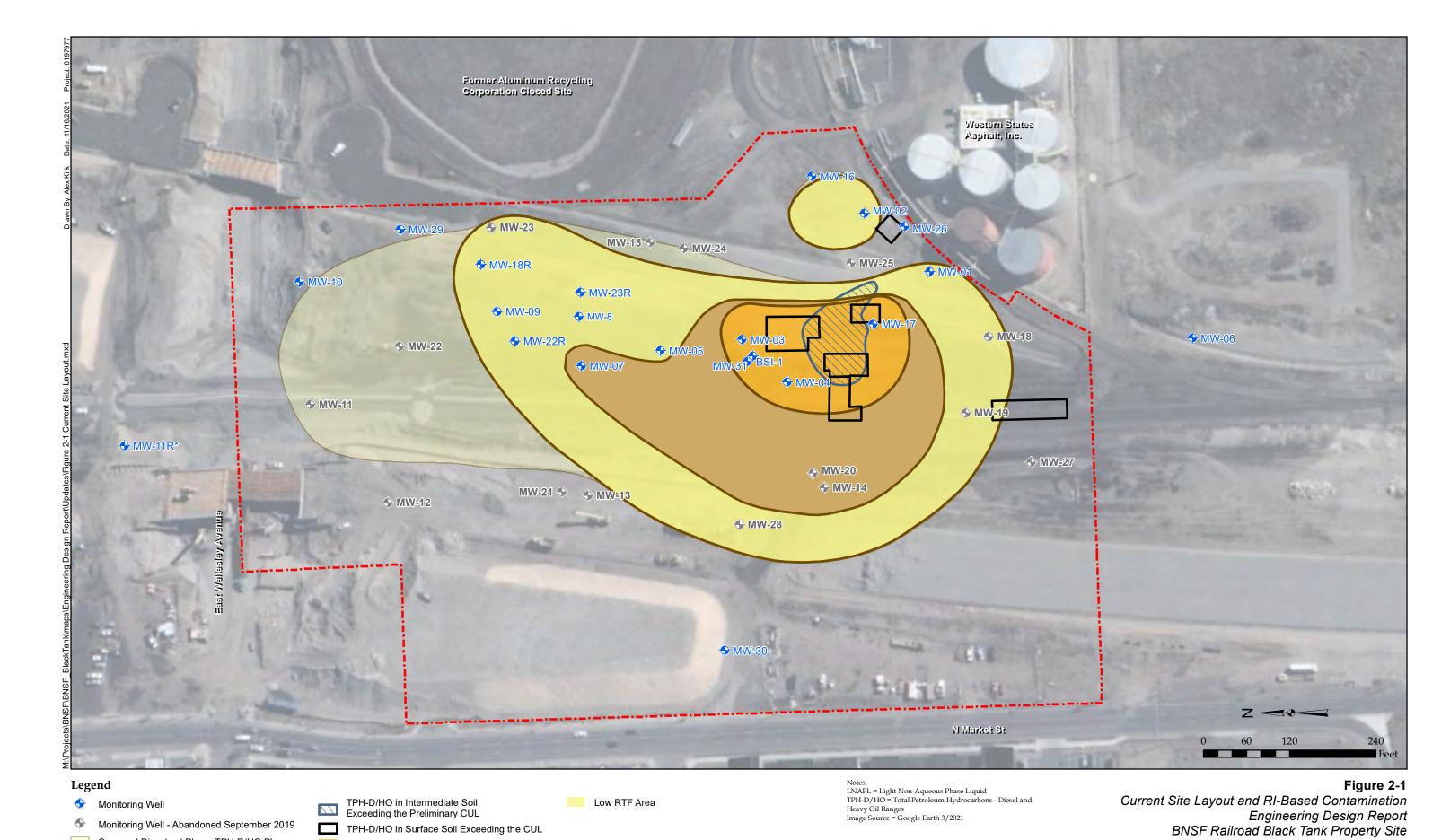






Figure 1-1
Site Location and Boundary
BNSF Railway Black Tank Property Site
Spokane, Washington



Seasonal Dissolved Phase TPH-D/HO Plume

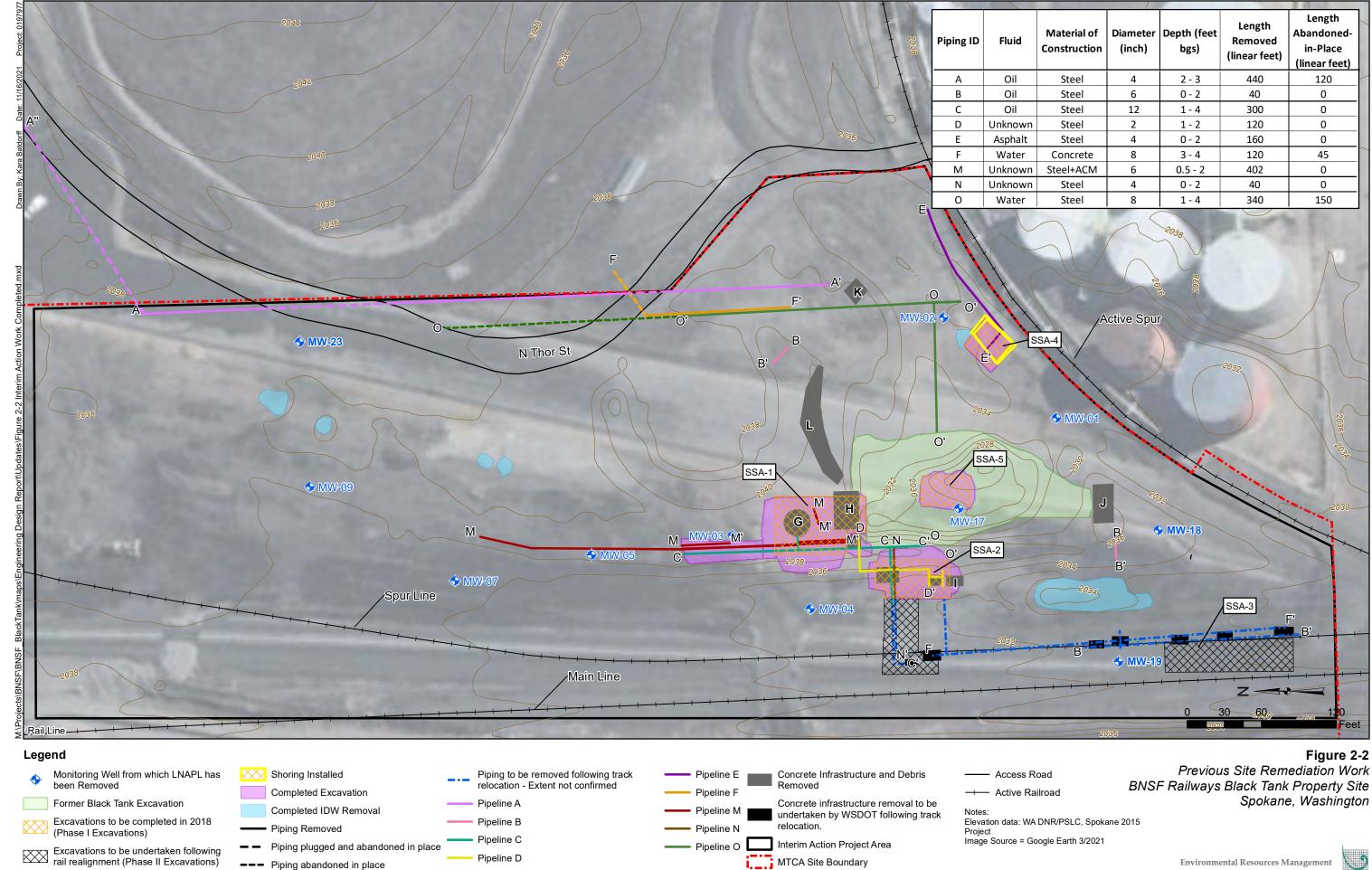
Site Boundary

High RTF Area

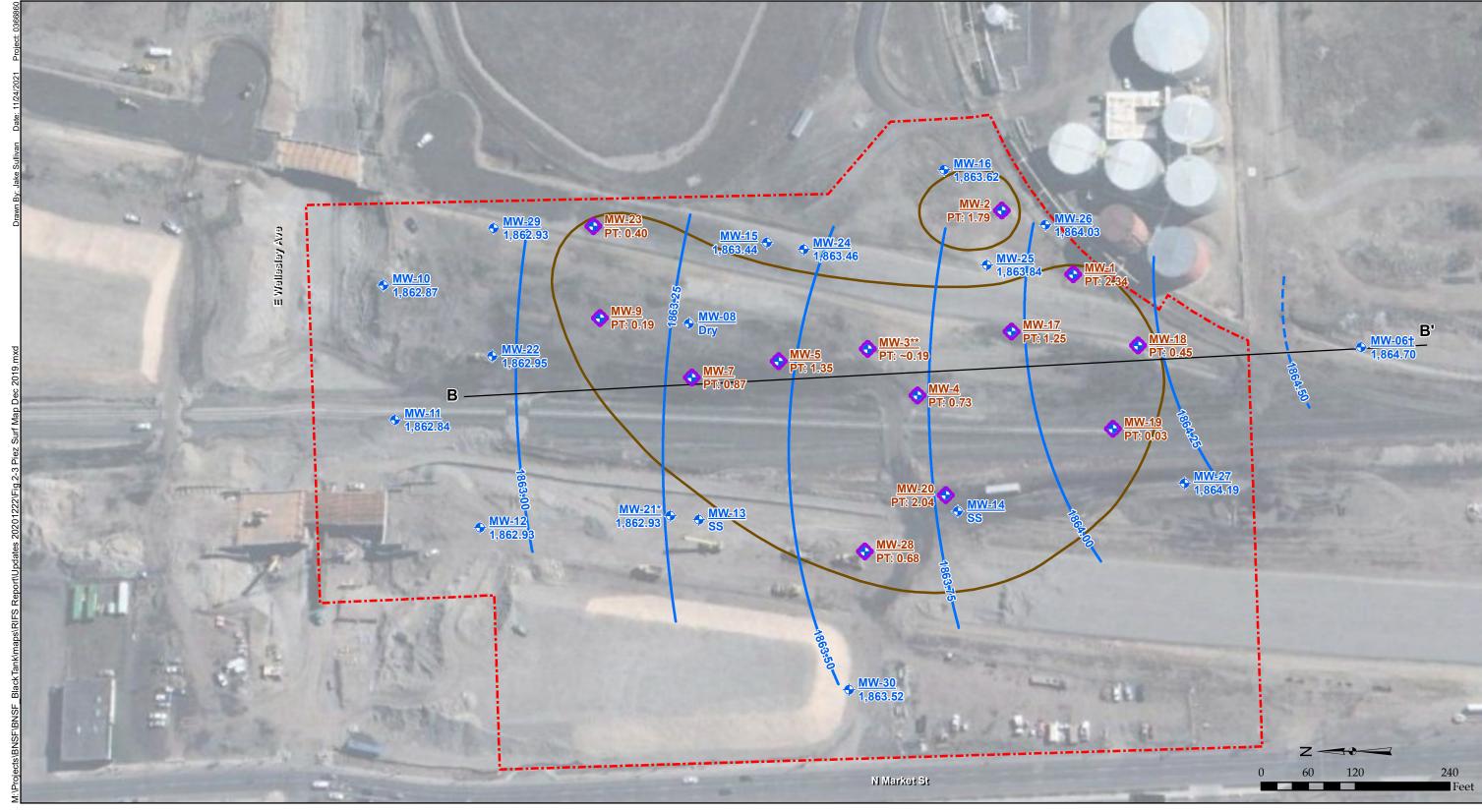
Medium RTF Area

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Spokane, Washington



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Legend

Monitoring Well with LNAPL

Monitoring Well without LNAPL Approximate Extent of LNAPL

 Cross Section Line Groundwater Contour Inferred Groundwater Contour BNSF Black Tank Site Boundary

- * = MW-21 was not used for contouring.

 ** = Water level not measured due to high viscosity of LNAPL.
- † = Biased low due to submerged screen greater than 1 foot.

Dry = No water detected or water level was below screened interval.

LNAPL = Light Non-Aqueous Phase Liquid.

NM = Not Measured.

PT = LNAPL thickness in feet.

SS = Well screened below the water table; screen is submerged. Not used for Contouring.

All elevations in Feet Above Mean Sea Level (ft. AMSL). Contour Interval = 0.25 feet.

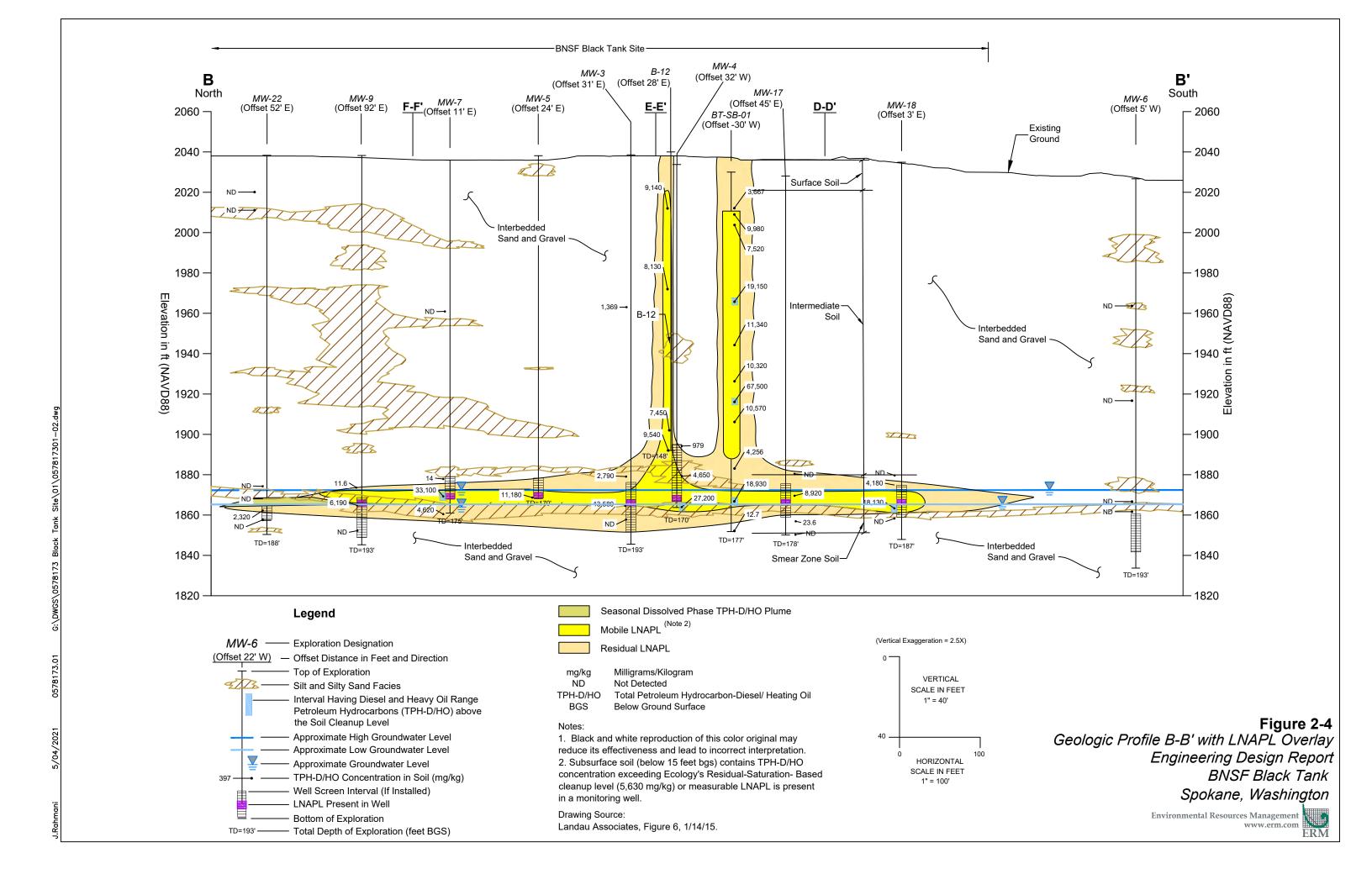
Aerial Photo: Esri World Imagery Webservice, Spokane Image Consortium, 2018.

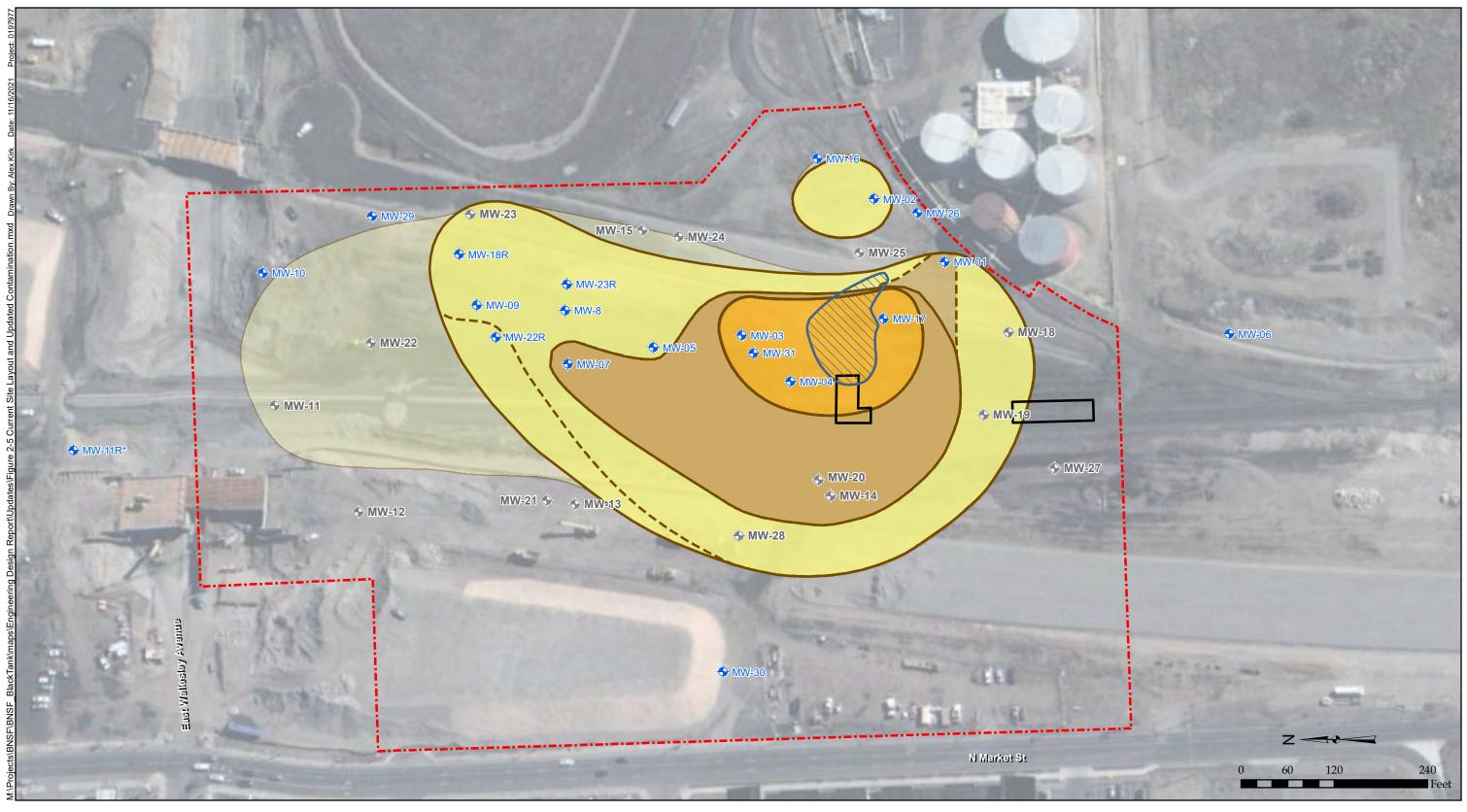
Figure 2-3 Piezometric Surface Map Engineering Design Report December 2019 BNSF Black Tank

Spokane, Washington

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Legend

Monitoring Well

Monitoring Well - Abandoned September 2019

-- September 2020 Boundary RTF

High RTF Area

Medium RTF Area

Low RTF Area

Seasonal Dissolved Phase TPH-D/HO Plume

Residual TPH-D/HO in Surface Soil Exceeding the CUL

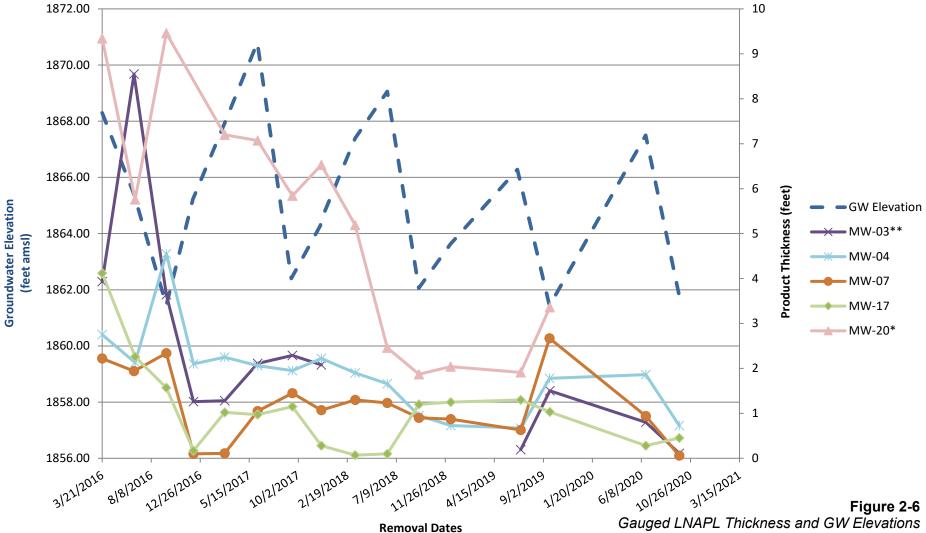
Site Boundary

TPH-D/HO in Intermediate Soil Exceeding the Preliminary CUL

Notes: LNAPL = Light Non-Aqueous Phase Liquid TPH-D/HO = Total Petroleum Hydrocarbons - Diesel and Heavy Oil Ranges Image Source = Google Earth 3/2021

Figure 2-5 Current Site Layout and Updated Site Contamination Engineering Design Report BNSF Black Tank Spokane, Washington

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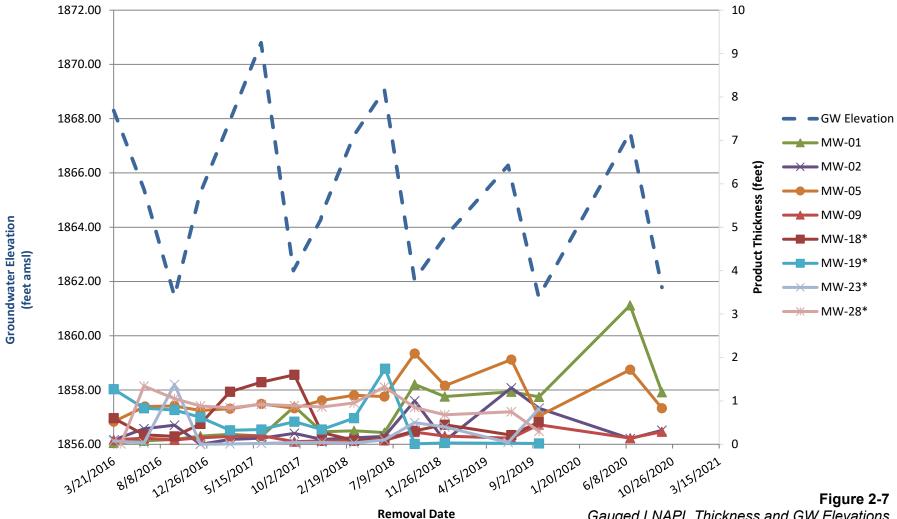


- · · LNAPL removed on this date.
 - ** LNAPL Thickness not gauged in MW-03 between 12/2017 and 9/2020 due to tar-like viscosity.
 - * MW-20 abandoned in September 2019.

(3/16 through 9/20) LNAPL > 1 foot thick Engineering Design Report BNSF Black Tank Site Spokane, Washington

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ERM



- · - · NAPL removed on this date.

* MW-18, MW-19, MW-23, and MW-28 abandoned September 2019.

Gauged LNAPL Thickness and GW Elevations
(3/16 through 9/20)
LNAPL < 1 Foot Thick
Engineering Design Report
BNSF Black Tank Site
Spokane, Washington





Monitoring Well - Abandoned

September 2020 Boundary RTF

Cut Locations

Cut Line BNSF Black Tank Site Boundary High RTF Area —— Railroad Curb/Roadway Medium RTF Area Detectable Warning Surface Low RTF Area

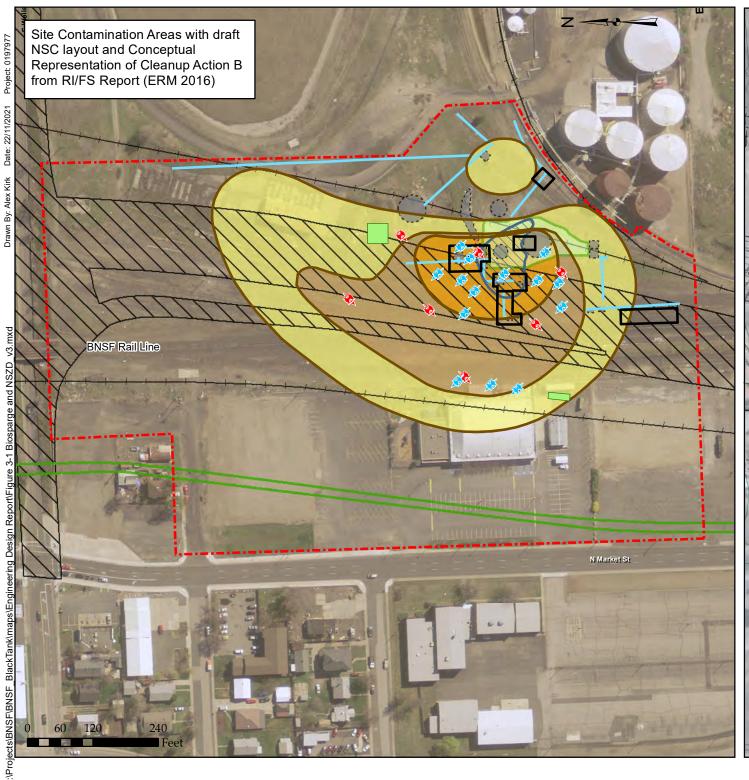
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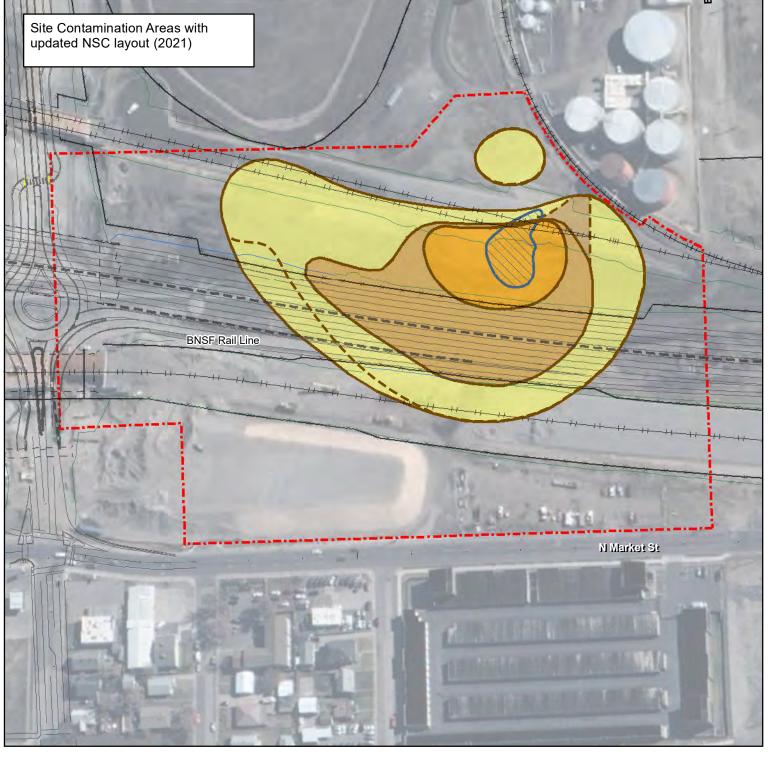
Alignment subject to change
CUL = Cleanup Level
LNAPL = Light Non-Aqueous Phase Liquid
RTF: Restoration Timeframe
NSC = North Spokane Corridor
TPH-D/HO = Combined Diesel and Heavy
Oil-Range Petroleum Hydrocarbons
CLIL = 5 360 milliograms per kilogram CUL = 5,360 milligrams per kilogram
WSDOT = Washington State Department of Transportation Aerial Photo: Google Earth 3/2021

and Decommissioned Monitoring Wells, Engineering Design Report BNSF Black Tank Spokane, Washington

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≅Legend

Monitoring Well

Monitoring Well - Abandoned September 2019

Proposed Biosparge Well

Proposed Bioventing Injection Well

Proposed Mechanical Equipment Facility

- September 2020 Boundary RTF

High RTF Area

Medium RTF Area Low RTF Area

Existing Piping (Petroleum and Chemical Solution), June 2016

DOT Proposed Railroad Realignment Option - 2014

Proposed BNSF Black Tank Site Boundary

TPH-D/HO in Intermediate Soil Exceeding the Preliminary CUL

Approximate Lateral Limits of Surface Soil Cleanup Areas

DOT Proposed Highway
Alignment Option - 2014

DOT Proposed Pedestrian Pathway - 2014

Former Black Tank Excavation

Historical Aboveground Storage Tank, Sump or Pump House

Notes: CUL = Cleanup Level

DOT = Washington State Department of Transportation LNAPL = Light Non-Aqueous Phase Liquid

NSZD = Natural Source Zone Depletion

ROI = Radius of Influence RTF = Restoration Timeframe

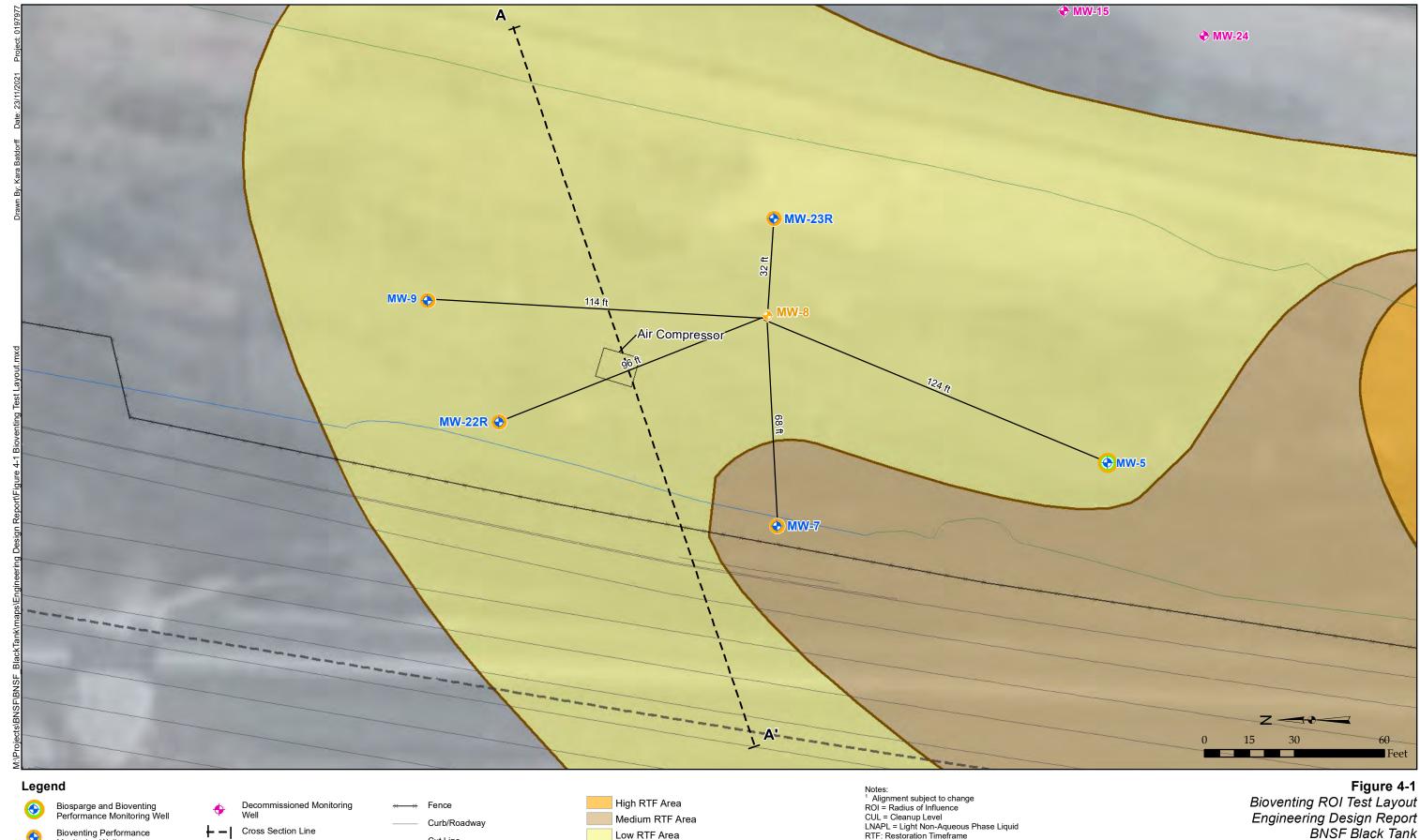
TPH-D/HO = Combined Diesel and Heavy Oil-Range Petroleum Hydrocarbons

Preliminary CUL = 13,600 milligrams per kilogram
Aerial Photo: USGS, April 2012, Google Earth 3/2021

Figure 3-1

Conceptual Representation of Cleanup Action B And Changes in NSC Layout (NSZD and Biosparging/Bioventing) BNSF Black Tank Spokane, Washington





Bioventing Performance Monitoring Well

Bioventing Injection Well

Monitoring Well to be Retained

├ - Cross Section Line

Cut Line

— Fill Line

BNSF Black Tank Site Boundary

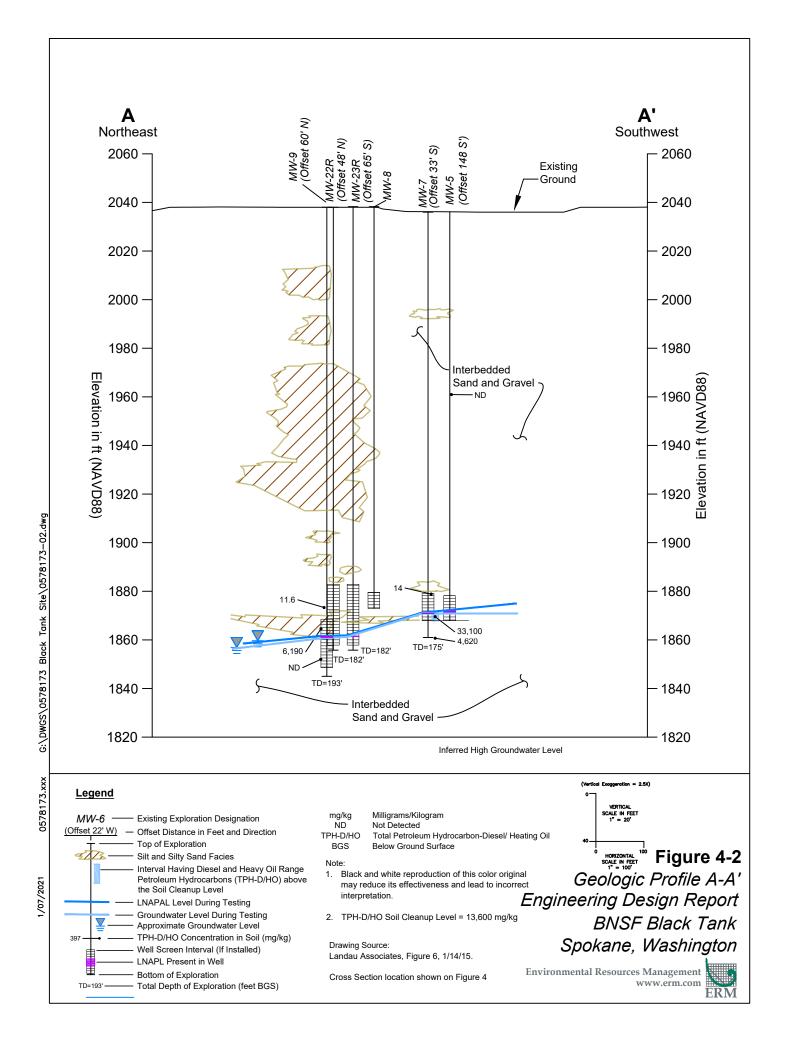
— — Barrier

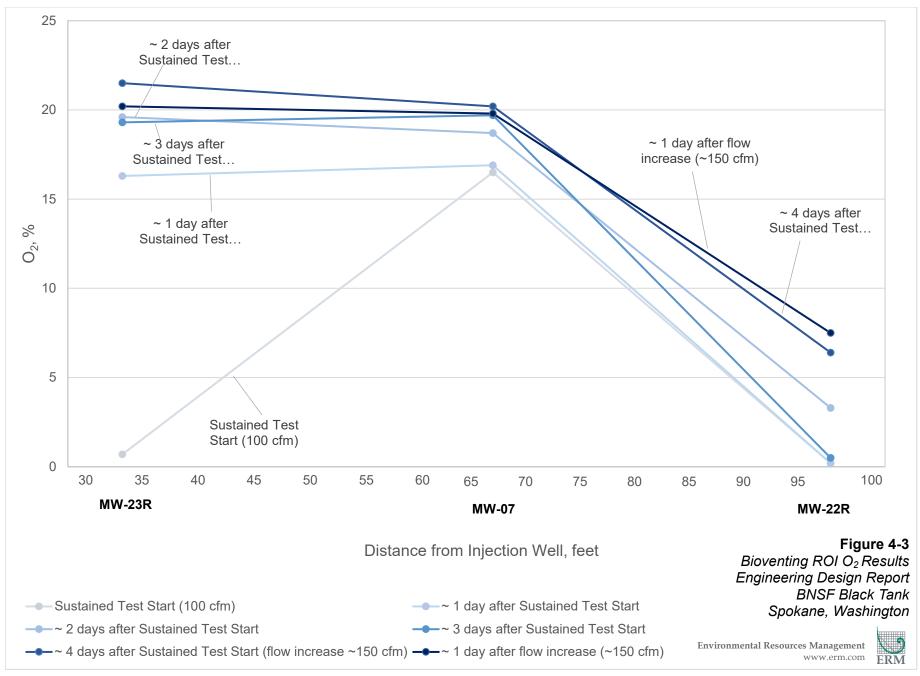
Notes:

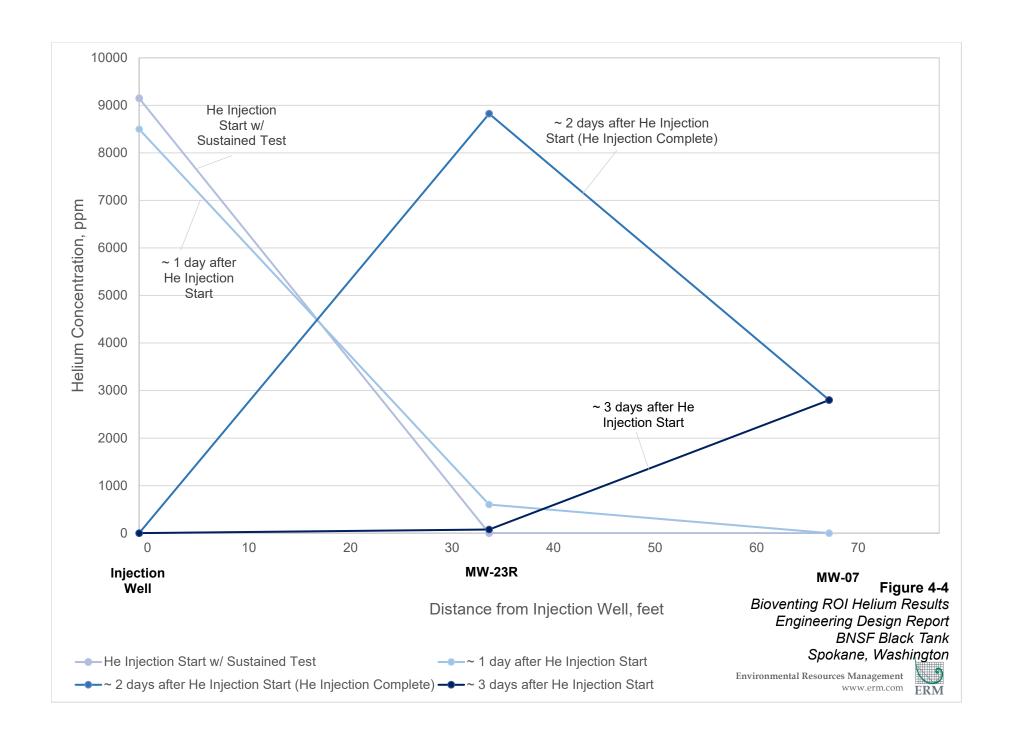
Alignment subject to change
ROI = Radius of Influence
CUL = Cleanup Level
LNAPL = Light Non-Aqueous Phase Liquid
RTF: Restoration Timeframe
NSC = North Spokane Corridor
TPH-D/HO = Combined Diesel and Heavy
Oil-Range Petroleum Hydrocarbons
CUL = 5,360 milligrams per kilogram
WSDOT = Washington State Department of
Transportation
Aerial Photo: Google Earth 3/2021.

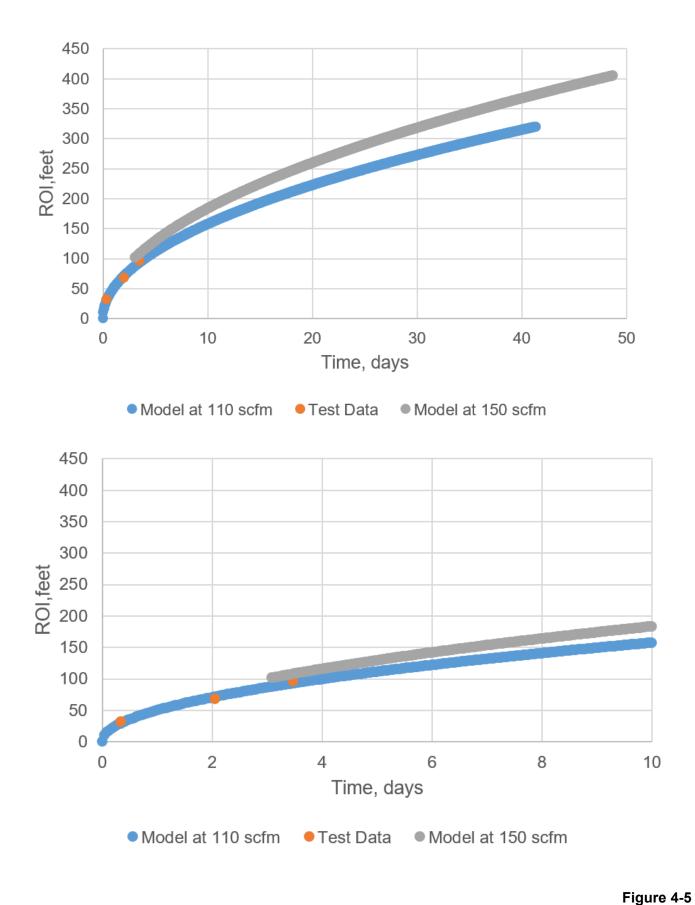
Figure 4-1 Engineering Design Report BNSF Black Tank Spokane, Washington

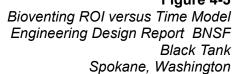




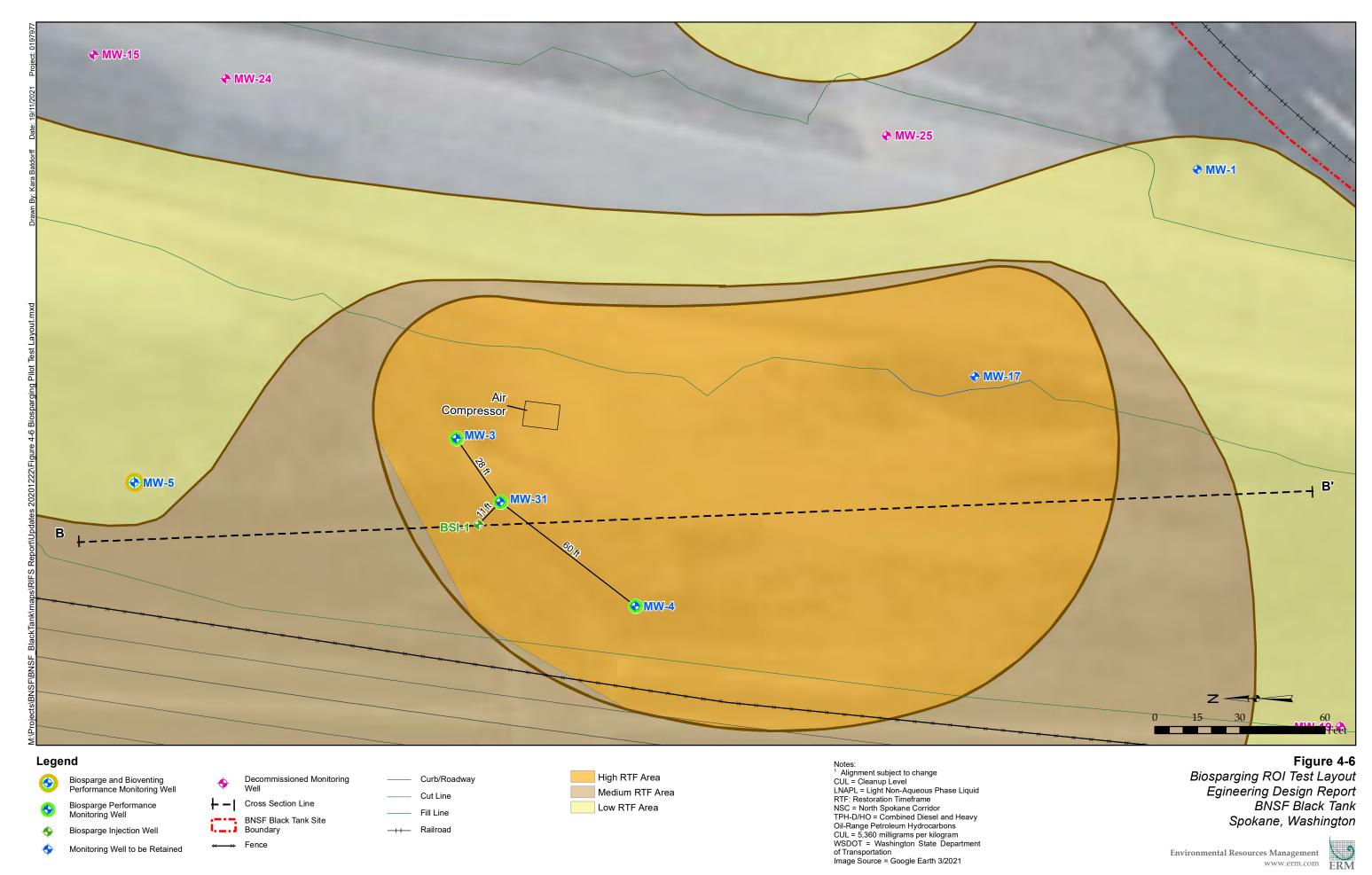








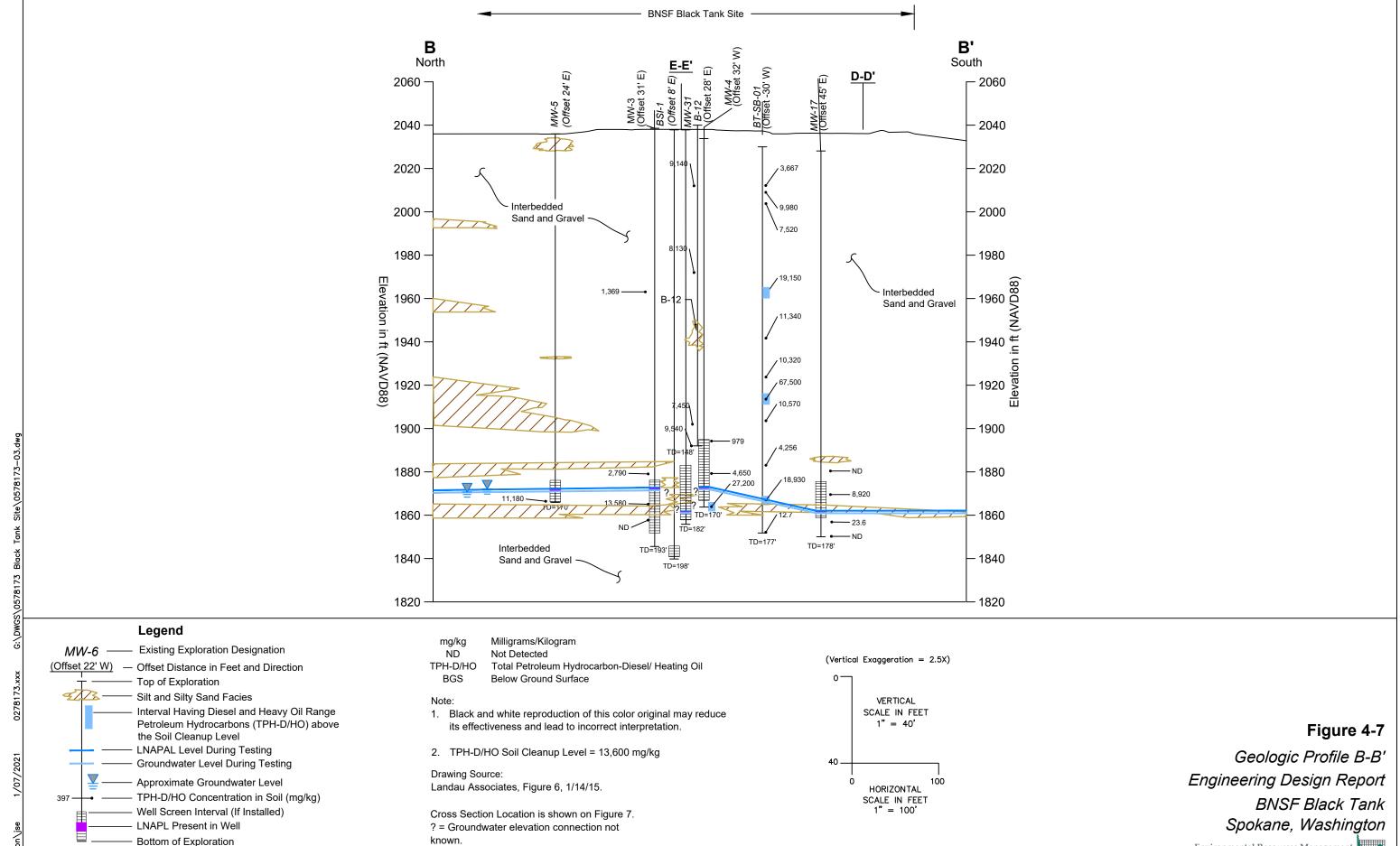




*--- Fence

Monitoring Well to be Retained

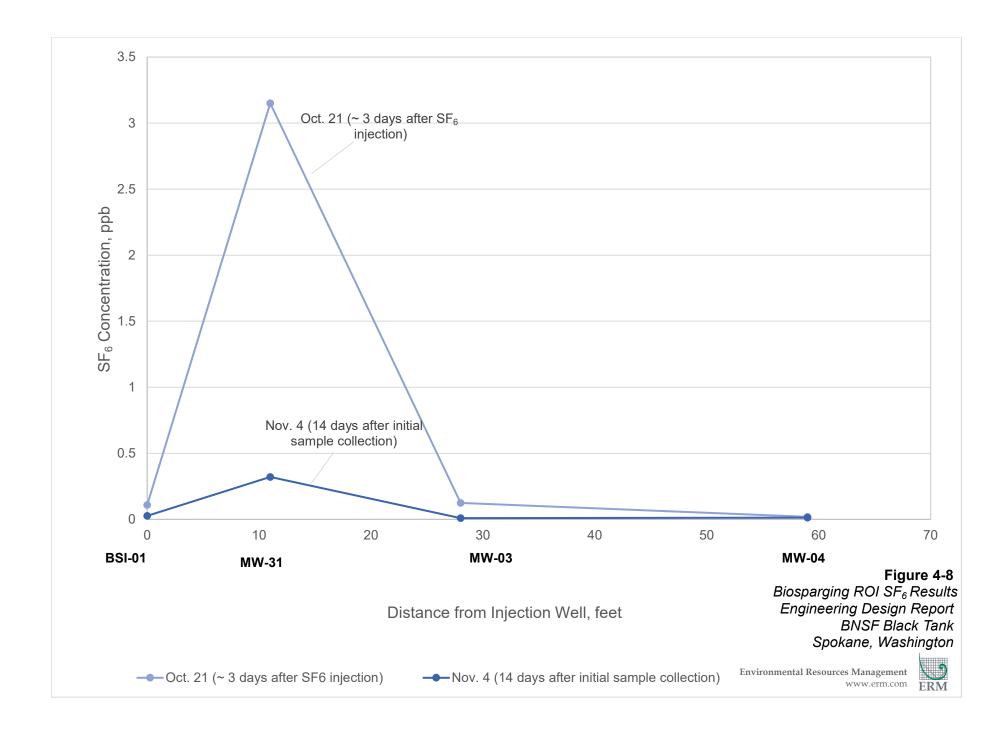
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Total Depth of Exploration (feet BGS)

Environmental Resources Management www.erm.com





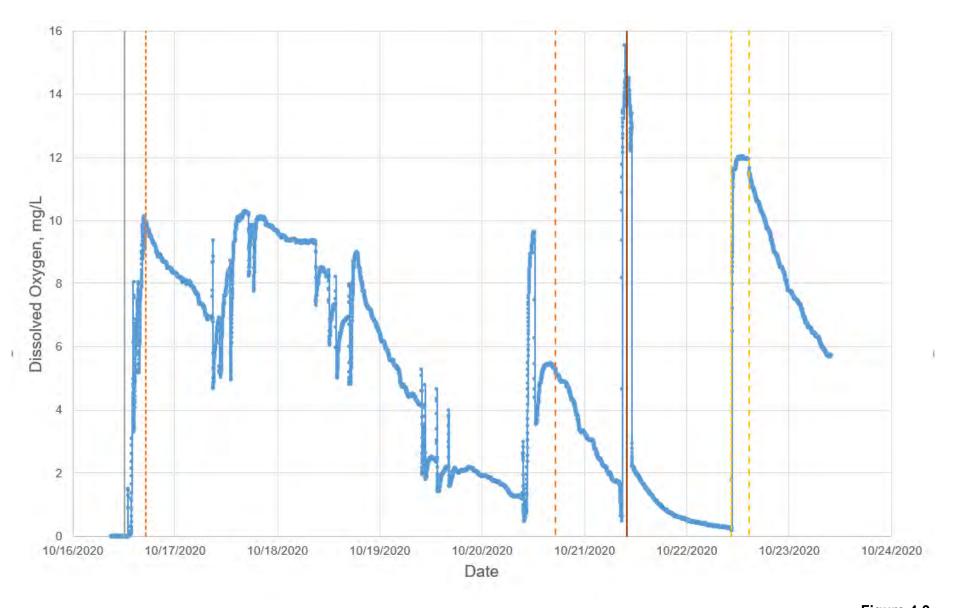




Figure 4-9
Biosparging ROI Dissolved Oxygen Results
Engineering Design Report
BNSF Black Tank
Spokane, Washington





Fill Line

---- Railroad

Legend

Bioventing Injection Well Screen Location

Well head location for Bioventing Injection Angle Well

- Bioventing Injection Angle Well

(🔰 125' ROI

O 150' ROI

- September 2020 Boundary RTF High RTF Area

Low RTF Area

-- Barrier

Guardrail Medium RTF Area

— Curb/Roadway

Detectable Warning Surface

BNSF Black Tank Site Boundary -- Bridge — Cut Line

Notes: LNAPL = Light Non-Aqueous Phase Liquid ROI = Radius of Influence RTF = Restoration Timeframe

Figure 5-1
Bioventing System Well Layout
Engineering Design Report
BNSF Black Tank Spokane, Washington



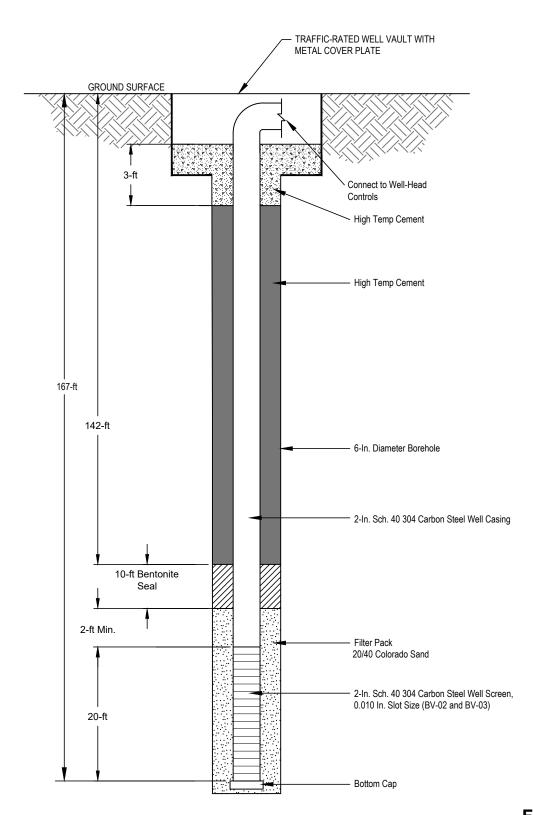


Figure 5-2 Vertical Bioventing Well Construction Diagram Engineering Design Report Black Tank Site Spokane, WA

NOTE:
BV-04 and BV-05 - APPROXIMATE WELL DEPTH AND
SCREEN INTERVAL. TOTAL DEPTH OF WELL AND
SCREEN INTERVAL WILL BE DETERMINED IN THE FIELD.

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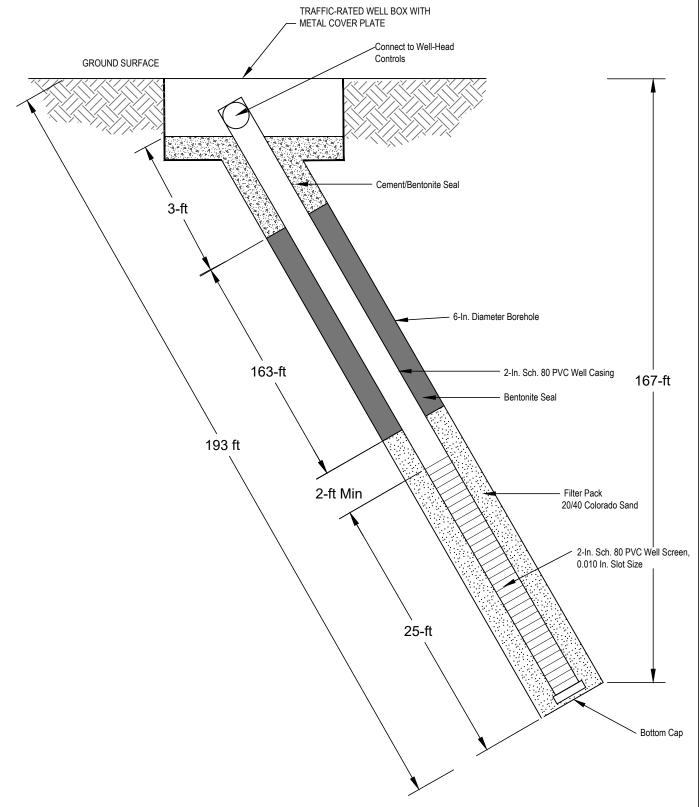


Figure 5-3 Angled Bioventing Well Construction Diagram Engineering Design Report Black Tank Site Spokane, WA

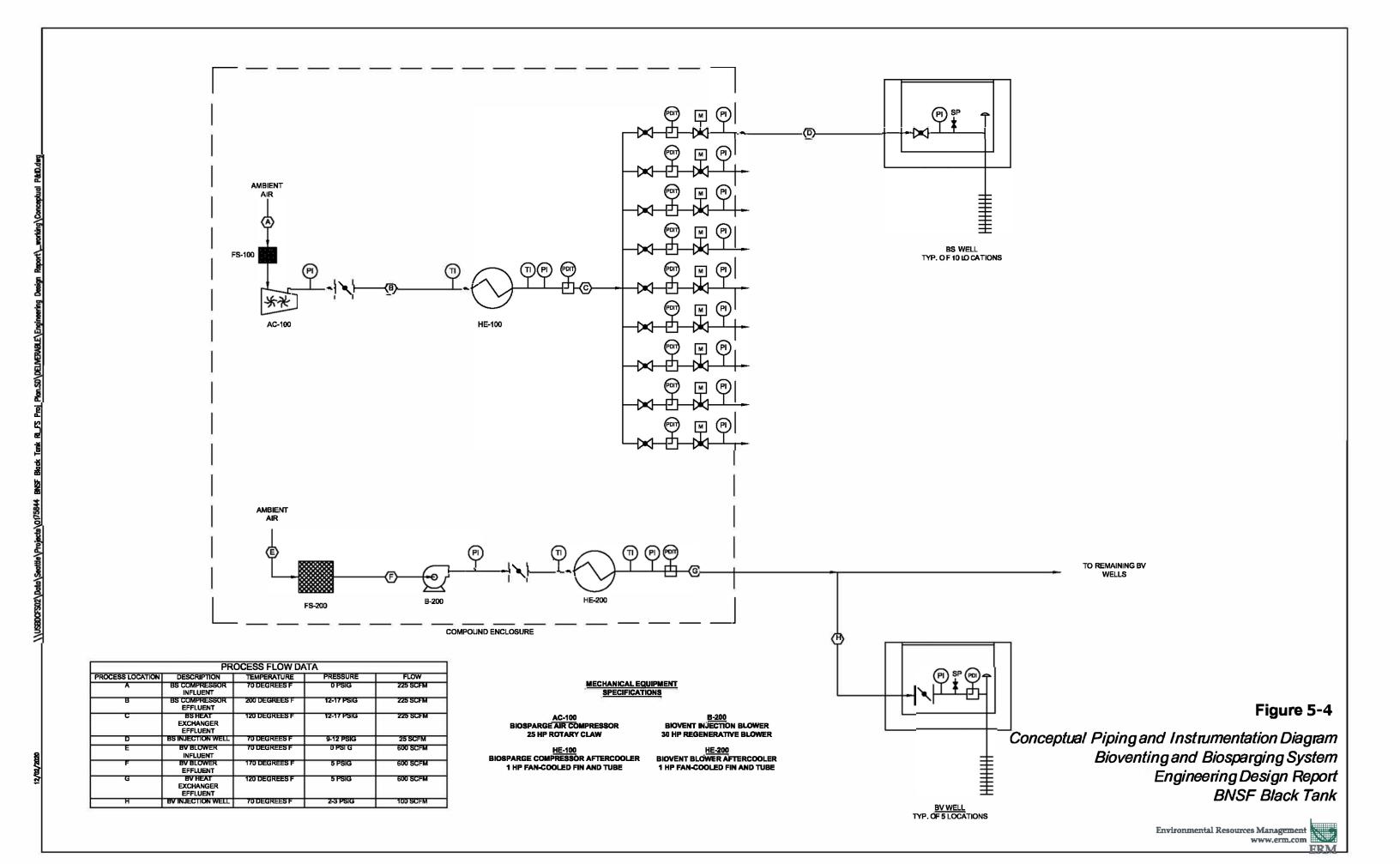
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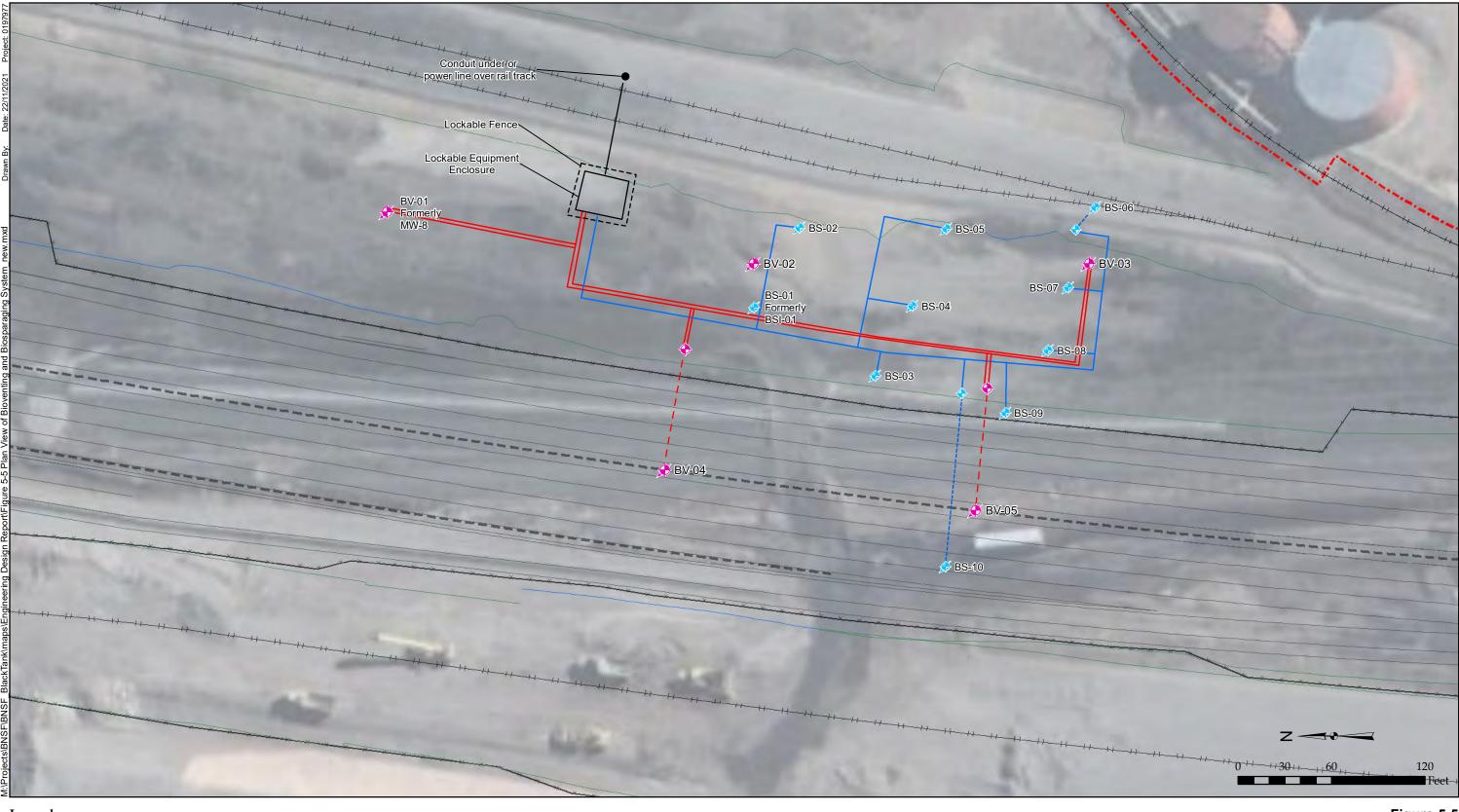
ERM

NOTE:

NOT TO SCALE

BV-04 and BV-05 - APPROXIMATE WELL DEPTH AND SCREEN INTERVAL. TOTAL DEPTH OF WELL AND SCREEN INTERVAL WILL BE DETERMINED IN THE FIELD.





Legend

Bioventing Injection Well Screen Location

Bioventing Injection Well Head Location

Biosparge Injection Well Screen Location Biosparge Injection Well Head Location ---- Biosparage Angle Well

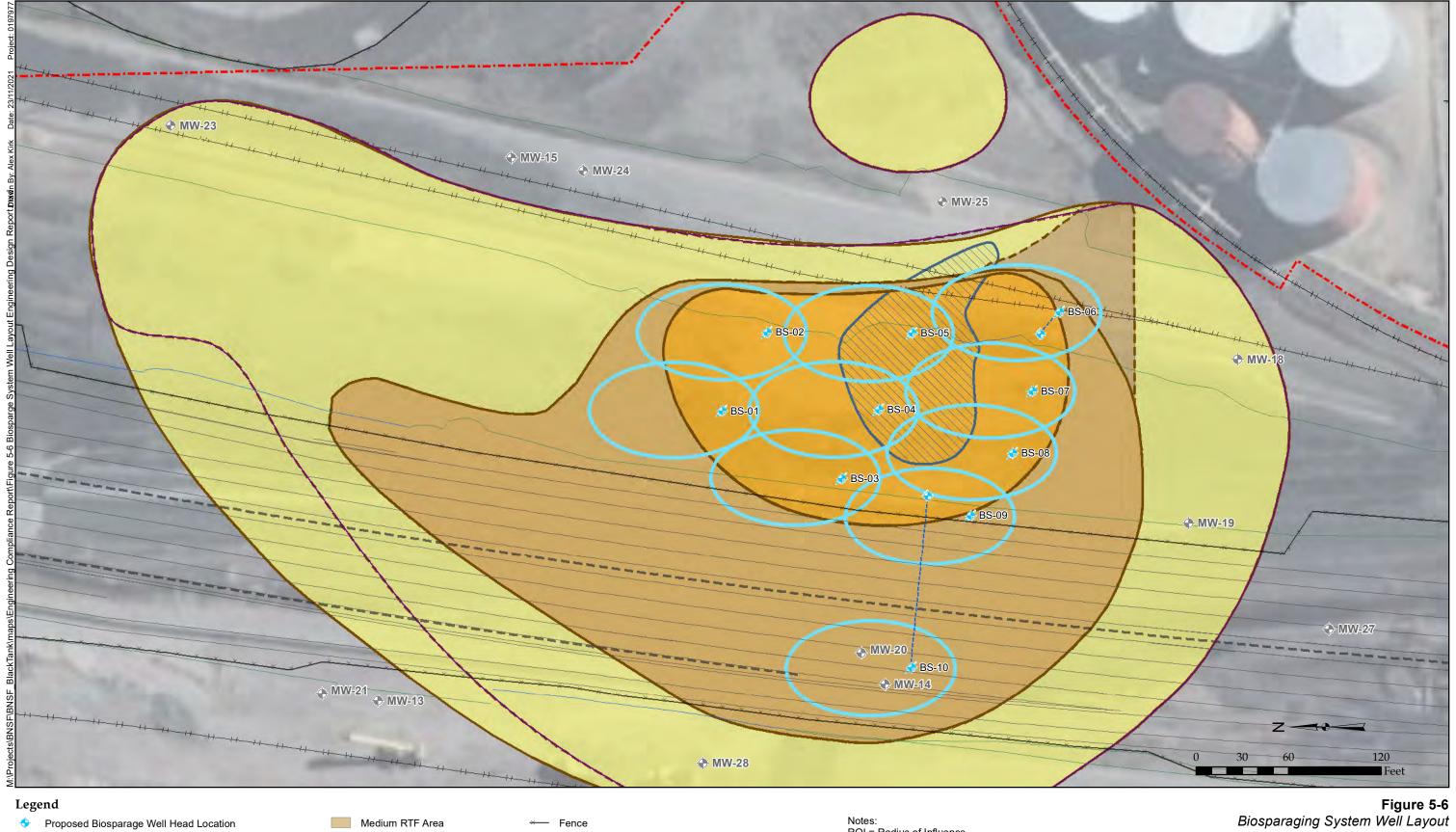
- - Guardrail

 Bioventing 30° Angle Well Curb/Roadway Biosparage Air Injection Conveyance Piping Detectable Warning Surface Bioventing Air Injection/Extraction Conveyance Piping -- Bridge — Cut Line -- Barrier *- Fence --- Fill Line

---- Railroad

Figure 5-5 Plan View of Bioventing and Biosparging System Engineering Design Report BNSF Black Tank Spokane, Washington





Proposed Biosparge Well Screen Location

---- Biosparage Angle Well

Biosparge Well ROI with Groundwater Gradient Influence

-- September 2020 Boundary RTF

High RTF Area

Low RTF Area Proposed BNSF Black Tank

Curb/Roadway — Cut Line

Site Boundary TPH-D/HO in Intermediate Soil Exceeding the Preliminary CUL

--- Fill Line

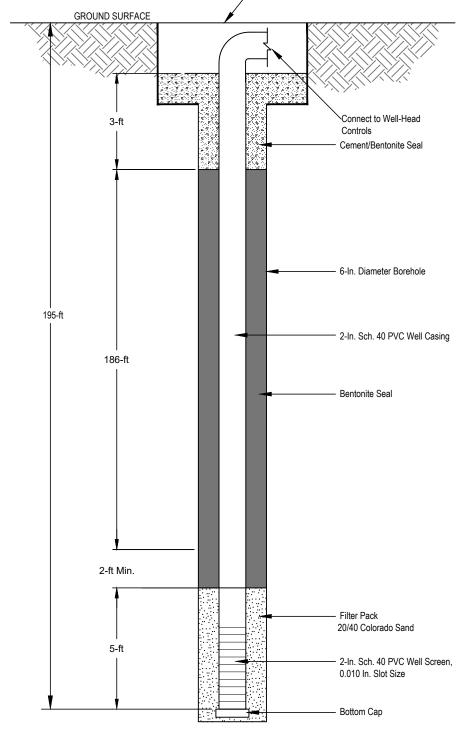
── Railroad

-- Barrier

ROI = Radius of Influence RTF = Restoration Timeframe Biosparaging System Well Layout Engineering Design Report BNSF Black Tank Spokane, Washington



Z.Avrukin



TRAFFIC-RATED WELL BOX WITH METAL COVER PLATE

NOTE:

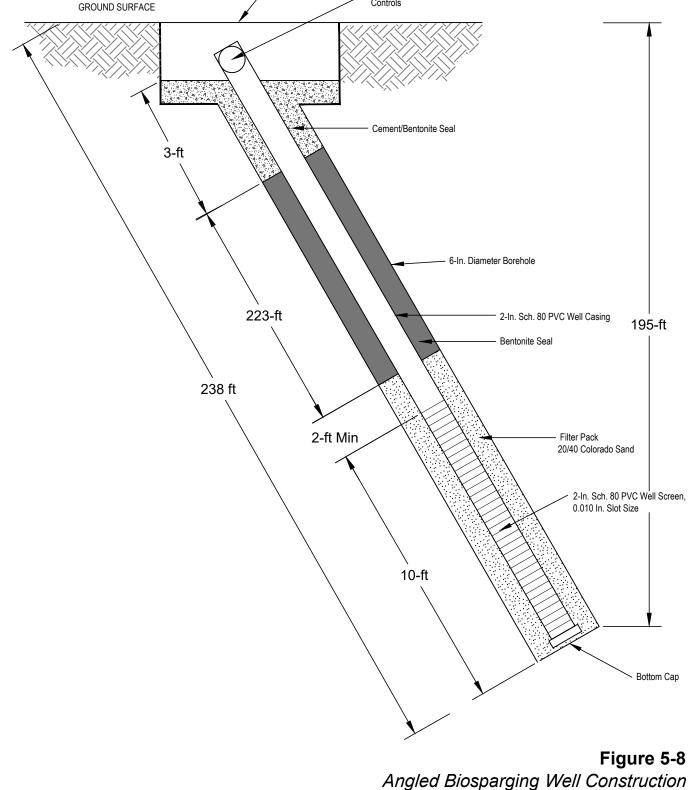
APPLICABLE TO VERTICAL WELLS AND ANGLED WELLS UP TO 7 DEGREES FROM VERTICAL.

APPLICABLE TO BS-02 THROUGH BS-09. LOW ANGLED WELL (BS-06) USE SCH. 80 PVC WELL CASING . APPROXIMATE WELL DEPTH AND SCREEN INTERVAL. TOTAL DEPTH OF WELL AND SCREEN INTERVAL WILL BE DETERMINED IN THE FIELD.

Figure 5-7
Vertical & Low Angled Biosparging Well
Construction Diagram

Engineering Design Report Black Tank Site Spokane, WA

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TRAFFIC-RATED WELL BOX WITH METAL COVER PLATE

Connect to Well-Head

Controls

Diagram Engineering Design Report Black Tank Site Spokane, WA

Environmental Resources Management www.erm.com

ERM

NOT TO SCALE

BV-10 - APPROXIMATE WELL DEPTH AND SCREEN INTERVAL. TOTAL DEPTH OF WELL AND SCREEN

INTERVAL WILL BE DETERMINED IN THE FIELD.

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

TABLES

Table 1-1
Engineering Design Report Requirements
Engineering Design Report
BNSF Black Tank
Spokane, WA

WAC 173-340-400 4(a) Requirement	Section
(i) Goals of the cleanup action including specific cleanup or performance requirements;	3.1, 3.2
(ii) General information on the facility including a summary of information in the remedial investigation/feasibility study updated as necessary to reflect the current conditions;	2.1, 2.2
(iii) Identification of who will own, operate, and maintain the cleanup action during and following construction;	3.3
(iv) Facility maps showing existing site conditions and proposed location of the cleanup action;	Figures 2-5, 5-1 & 5-6
(v) Characteristics, quantity, and location of materials to be treated or otherwise managed, including groundwater containing hazardous substances;	2.4
(vi) A schedule for final design and construction;	7.0
(vii) A description and conceptual plan of the actions, treatment units, facilities, and processes required to implement the cleanup action including flow diagrams;	3.4, 5.2, 6.1, 6.2 , 6.3 & associated figures
(viii) Engineering justification for design and operation parameters, including:	4.1
(A) Design criteria, assumptions and calculations for all components of the cleanup action;	4.1, 5.1, 6.1, 6.2 & 6.3
(B) Expected treatment, destruction, immobilization, or containment efficiencies and documentation on how that degree of	5.2.2, 5.2.3, 5.3.2
effectiveness is determined; and	& 6.4
(C) Demonstration that the cleanup action will achieve compliance with cleanup requirements by citing pilot or treatability test data, results from similar operations, or scientific evidence from the literature;	4.1
(ix) Design features for control of hazardous materials spills and accidental discharges (for example, containment structures, leak detection devices, run-on and runoff controls);	5.2.4
(x) Design features to assure long-term safety of workers and local residences (for example, hazardous substances monitoring devices, pressure valves, bypass systems, safety cutoffs);	5.2.4, 5.2.5
(xi) A discussion of methods for management or disposal of any treatment residual and other waste materials containing hazardous substances generated as a result of the cleanup action;	5.2.6
(xii) Facility specific characteristics that may affect design, construction, or operation of the selected cleanup action, including:	2.6, 2.7 & 5.1
(A) Relationship of the proposed cleanup action to existing facility operations;	2.6 & 5.1
(B) Probability of flooding, probability of seismic activity, temperature extremes, local planning and development issues; and	2.6 & 5.1
(C) Soil characteristics and groundwater system characteristics;	2.3.3, 2.3.5 & 5.1
(xiii) A general description of construction testing that will be used to demonstrate adequate quality control;	5.3.1
(xiv) A general description of compliance monitoring that will be performed during and after construction to meet the requirements of WAC 173-340-410;	5.3.2
(xv) A general description of construction procedures proposed to assure that the safety and health requirements of WAC 173-340-810 are met;	5.4
(xvi) Any information not provided in the remedial investigation/feasibility study needed to fulfill the applicable requirements of the State Environmental Policy Act (chapter 43.21C RCW);	5.5
(xvii) Any additional information needed to address the applicable state, federal and local requirements including the substantive requirements for any exempted permits; and property access issues which need to be resolved to implement the cleanup action;	5.5
(xviii) For sites requiring financial assurance and where not already incorporated into the order or decree or other previously submitted document, preliminary cost calculations and financial information describing the basis for the amount and form of financial assurance and, a draft financial assurance document;	NA
(xix) For sites using institutional controls as part of the cleanup action and where not already incorporated into the order or decree or other previously submitted documents, copies of draft restrictive covenants and/or other draft documents establishing these institutional controls; and	6.4
(xx) Other information as required by the department.	NA

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Table 2-1
Estimated Mobile LNAPL Volume
Engineering Design Report
BNSF Black Tank Site
Spokane, Washington

Area (acre)	Area - 2016 (acres)	Area - 2019 (acres)	Residual ¹ LNAPL (above smear zone)		Mobile LNAPL - 2016		Mobile LNAPL - 2019 (Using Mean Thickness)		Mobile LNAPL - 2019 (Using Mean + 1SD Thickness)		Change 2016 to 2019 (Using Mean Thickness)		Change 2016 to 2019 (Using Mean + 1SD Thickness)	
			gallons	gal/acre	gallons	gal/acre	gallons	gal/acre	gallons	gal/acre	gallons	gal/acre	gallons	gal/acre
High RTF	0.8	0.8	N/A	N/A	66,000	83,000	21,000	26,000	29,000	36,000	-45,000	-57,000	-37,000	-47,000
Medium RTF	1.7	1.8	N/A	N/A	84,000	49,000	57,000	32,000	74,000	41,000	-27,000	-17,000	-10,000	-8,000
Low RTF	3.5	3.4	21300	6100	9,000	2,600	12,000	3,600	27,000	8,000	3,000	1,000	18,000	5,400
Totals	6.0	6.0			159,000	27,000	90,000	15,000	130,000	22,000	-69,000	-12,000	-29,000	-5,000

Abbreviations:

gal/acre = gallons per acre

LNAPL = Light Non-Aqueous Phase Liquid

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Table 2-2 Groundwater and LNAPL Level Measurements March 2016 through September 2020 Engineering Design Report BNSF Black Tank Site Spokane, Washington

Monitoring Well	Measurement Date	Collector	Total Depth	TOC Elevation	Depth to Product	Depth to Groundwater	Gauged LNAPL Thickness	Corrected Depth to Groundwater	LNAPL Elevation	Groundwater Elevation	Corrected Groundwater Elevation	Submerged Screen?	Comment
Well	Date		feet	feet amsl	feet	feet	feet	feet	feet amsl	feet amsl	feet amsl	Y/N	
MW-01	6/20/2020	ERM	NM	2036.07	168.10	171.30	3.20	168.45	1867.97	1864.77	1867.62	No	
MW-01	9/24/2020	ERM	NM	2036.07	173.78	174.98	1.20	173.91	1862.29	1861.09	1862.16	No	Product thickness & GW depth/elevation corrected because product too viscous to allow accurate measurement. Original LNAPL thickness = 2.32 feet. Corrected thickness is based on product volume removed.
MW-02	6/20/2020	ERM	NM	2037.07	169.45	169.58	0.13	169.46	1867.62	1867.49	1867.61	No	
MW-02	9/24/2020	ERM	NM	2037.07	175.06	175.38	0.32	175.10	1862.01	1861.69	1861.97	No	Product thickness & GW depth/elevation corrected because product too viscous to allow accurate measurement. Original LNAPL thickness = 0.88 feet. Corrected thickness is based on product volume removed.
MW-03	6/20/2020	ERM	NM	2040.89	168.30	169.10	0.80	168.39	1872.59	1871.79	1872.50	No	
MW-03	9/24/2020	ERM	NM	2040.89	168.49	168.60	0.11	168.50	1872.4	1871.79	1872.39	No	W. W. C
MW-04	6/20/2020	ERM	NM	2033.59	159.24	161.10	1.86	159.44	1874.35	1872.49	1874.15	No	Well interior coated with LNAPL precluded collection of depth to LNAPL using LNAPL interface probe; however, depth to groundwater was collected because groundwater probe is smaller diameter. Product thickness/depth/elevation based on volume product removed and GW elevation.
MW-04	9/24/2020	ERM	NM	2033.59	161.20	161.93	0.73	161.28	1872.39	1871.66	1872.31	No	Product thickness & GW depth/elevation corrected because product too viscous to allow accurate measurement. Original LNAPL thickness = 1.6 feet. Corrected thickness is based on product volume removed.
MW-05	6/20/2020	ERM	NM	2040.87	168.89	ND	1.72	ND	1871.98	ND	ND	NA	Product thickness based on volume product removed because product too viscous (tarry) to allow accurate measurement. No water observed. Original LNAPL thickness could not be measured.
MW-05	9/24/2020	ERM	NM	2040.87	169.01	169.84	0.83	169.10	1871.86	1871.03	1871.77	No	Product thickness & GW depth/elevation corrected because product too viscous to allow accurate measurement. Original LNAPL thickness = 1.7 feet. Corrected thickness is based on product volume removed.
MW-06	6/20/2020	ERM	NM	2029.48	ND	160.98	ND	160.98	ND	1868.50	1868.50	Yes	
MW-06	9/24/2020	ERM	NM	2029.48	ND	166.64	ND	166.64	ND	1863.06	1862.84	Yes	
MW-07	6/20/2020	ERM	NM	2036	165.11	166.05	0.94	165.21	1870.89	1869.95	1870.79	No	
MW-07	9/24/2020	ERM	NM	2036	165.15	165.21	0.06	165.16	1870.85	1870.79	1870.84	No	
MW-09	6/20/2020	ERM	NM	2040.60	173.51	173.65	0.14	173.53	1867.09	1866.95	1867.07	No	
MW-09	9/24/2020	ERM	NM	2040.60	179.28	179.57	0.29	179.31	1861.32	1861.03	1861.29	No	
MW-10 MW-10	6/20/2020 9/24/2020	ERM ERM	NM NM	2038.07	ND ND	171.23 176.97	ND ND	171.23 176.97	ND ND	1866.84 1861.1	1866.84 1861.10	No Yes	
MW-16	6/20/2020	ERM	NM	2036.70	ND ND	169.20	ND ND	169.20	ND	1867.50	1867.50	No	
MW-16	9/24/2020	ERM	NM	2036.70	ND ND	174.92	ND ND	174.92	ND	1861.78	1861.78	No	
MW-17	6/20/2020	ERM	NM	2030.78	162.83	163.11	0.28	162.86	1867.95	1867.67	1867.92	No	
MW-17	9/24/2020	ERM	NM	2030.78	168.70	169.15	0.45	168.75	1862.08	1861.63	1862.03	No	
MW-22R	9/24/2020	ERM	NM	2039.85	ND	178.56	ND	178.56	ND	1861.285	1861.29	No	
MW-23R	9/24/2020	ERM	NM	2039.94	178.46	178.47	0.01	178.46	1861.479	1861.469	1861.48	No	
MW-26	6/20/2020	ERM	NM	2035.63	ND	167.75	ND	167.75	ND	1867.88	1867.88	Yes	
MW-26	9/24/2020	ERM	NM	2035.63	ND	173.50	ND	173.50	ND	1862.13	1862.13	No	
MW-29	6/20/2020	ERM	NM	2041.05	ND	174.18	ND	174.18	ND	1866.87	1866.87	No	
MW-29	9/24/2020	ERM	NM	2041.05	ND ND	179.91	ND ND	179.91	ND	1861.14	1861.14	No	
MW-30	6/20/2020 9/24/2020	ERM ERM	NM NM	2041.25	ND ND	173.87	ND ND	173.87 179.58	ND	1867.38	1867.38	No	
MW-30 MW-31	9/24/2020 9/24/2020	ERM ERM	NM NM	2041.25	ND 176.76	179.58 176.78	ND 0.02	179.58 176.76	ND 1861.45	1861.67 1861.43	1861.67 1861.45	No No	
BSI-1	9/24/2020	ERM	NM	2038.21	ND	178.33	ND	178.33	ND	1860.249	1860.25	Yes	Well casing not at final elevation; depth to groundwater is approximate.

Erroneous measurement revised based on volume of LNAPL removed from the well.

Abbreviations: amsl = above mean sea level

Corrected product thickness evaluation:

NA = data not available

For unconfined observations, no correction to approximate product thickness was performed.

ND = not detected

For perched observations, corrected product thicknesses were calculated by calculating the difference between the product elevation and top of silt facies elevation.

NM = not measured

TOC = top of casing

For confined observations, corrected product thicknesses were calculated by calculating the difference between bottom of silt facies and groundwater elevation.

Table 2-3
Construction Details of Decommissioned Monitoring Wells to be Decommissioned
Engineering Design Report
BNSF Black Tank Site
Spokane, Washington

Well Identification	Well Completion	Washington State Plane Coordinates, North Zone (NAD 83, feet)		Borehole Depth (feet	Ground Surface Elevation (NAVD	Top of Casing Elevation	Screen Inter	val Depth to	Screen Length	Screen Slot Size	Filter Pack Depth to	
identification	Date	Easting	Northing	bgs)	88, feet)	(NAVD 88, feet)	Тор	Bottom	(feet) (inches)		Тор	Bottom
			i torumg				(feet bgs)	(feet bgs)			(feet bgs)	(feet bgs)
MW-07	23-Jul-08	274277.4	2494723.7	175.0	2036.46	2036.00	158.5	168.5	10.0	0.020	156.0	169.0
MW-11	29-May-09	274654.5	2494670.0	197.0	2037.98	2037.67	167.0	197.0	30.0	0.010	164.9	197.0
MW-12	9-Jan-06	274546.6	2494533.1	198.0	2038.21	2038.21	167.0	197.0	30.0	0.010	163.5	198.0
MW-13	9-Mar-06	274268.2	2494543.2	197.0	2039.54	2039.21	182.0	197.0	15.0	0.010	180.0	197.0
MW-14	9-Jun-06	273939.3	2494553.9	217.0	2039.26	2038.84	197.0	217.0	20.0	0.010	195.0	217.0
MW-15	28-May-09	274181.6	2494895.5	197.0	2037.66	2037.40	166.0	196.0	30.0	0.010	163.0	196.0
MW-18	17-Oct-13	273710.3	2494764.6	187.0	2034.90	2037.67	161.2	176.2	15.0	0.010	158.2	177.6
MW-19	11-Nov-13	273742.2	2494658.5	178.0	2030.91	2030.43	160.3	175.3	15.0	0.010	157.2	176.6
MW-20	29-Oct-13	273954.7	2494574.2	188.0	2039.48	2039.11	167.5	182.5	15.0	0.010	164.6	183.9
MW-21	1-Nov-13	274304.5	2494547.9	188.0	2039.39	2039.04	176.3	186.3	10.0	0.010	173.4	187.8
MW-22	5-Nov-13	274530.8	2494751.0	188.0	2038.32	2041.20	170.7	180.7	10.0	0.010	167.8	182.1
MW-23	28-Oct-13	274402.7	2494916.3	188.0	2038.68	2041.49	172.0	182.0	10.0	0.010	169.1	183.3
MW-24	12-Nov-13	274135.0	2494887.1	183.4	2037.77	2040.41	168.1	183.1	15.0	0.010	165.2	183.4
MW-25	14-Nov-13	273902.1	2494866.9	178.2	2033.17	2036.14	162.9	177.9	15.0	0.010	160.9	178.2
MW-27	27-Sep-14	273651.14	2494589.65	187.0	2038.36	2038.15	173.5	183.5	10.0	0.010	171.8	184.0
MW-28	7-Apr-16	274057.40	2494502.77	187.0	2040.36	2042.76	165.0	180.0	15.0	0.010	162.0	160.0
MW-29	9-Apr-16	274529.23	2494913.79	183.0	2038.54	2041.05	168.0	183.0	15.0	0.010	183.0	165.0

bgs = below ground surface

Table 3-1 Groundwater Cleanup Levels Engineering Design Report BNSF Black Tank Spokane, WA

Analyte	Cleanup Level (ug/L)	Basis for Cleanup Level		
TPH-D/HO	500	MTCA Method A		
Benzo(a)Pyrene	0.1	MTCA Method A		
Benzo(a)Pyrene TEQ	0.1	MTCA Method A		
LNAPL	No Detectable LNAPL	WAC 173-340-360(2)(c)(ii)		

ug/L = micrograms per liter
TPH-D/HO = total petroleum hydrocarbons as diesel/heavy oil
TEQ = toxic equivalency quotient
LNAPL = light, non-aqueous-phase liquid
MTCA = Model Toxics Control Act

Table 3-2 Soil Cleanup Levels Engineering Design Report BNSF Black Tank Spokane, WA

Analyte	Cleanup Level (mg/kg)	Basis for Cleanup Level
Surface Soil (surface to 15 feet bgs)		
TPH-D/HO	2000	MTCA Method A
Benzo(a)Pyrene	0.1	MTCA Method A
Benzo(a)Pyrene TEQ	0.1	MTCA Method A
Naphthalene	5	MTCA Method A
Total Naphthalenes	5	MTCA Method A
Subsurface Soil (below 15 feet bgs)		
TPH-D/HO	5630	Residual Saturation*
LNAPL		
LNAPL	No Detectable LNAPL	WAC 173-340-360(2)(c)(ii)

^{*}Residual Saturation was determined empirically as the lowest median soil TPH value from soil borings that contained soil contamination from the ground surface to groundwater.

bgs = below ground surface
LNAPL = light, non-aqueous-phase liquid
mg/kg = milligrams per kilogram
MTCA = Model Toxics Control Act
TPH-D/HO = total petroleum hydrocarbons as diesel/heavy oil
TEQ = toxic equivalency quotient
WAC = Washington Administrative Code

Table 3-3
Applicable or Relevant and Appropriate Requirements For the Cleanup Action
Engineering Design Report
BNSF Black Tank
Spokane, WA

Jurisdiction		Summary of ARARs							
	Municipal Code 10.08D	Nuicances (Noise and Dust)							
0" 10 1	Municipal Code 17E.010	Critical Aquifer Rechage Areas - Aquifer Protection							
City of Spokane	Municipal Code 17D.060	Stormwater Management Regulations							
	Municipal Code 17D.090	Land Disturbing Activities (TESC and Grading)							
	Ch. 18.104 RCW	Water Well Construction							
	Ch. 173-160 WAC	Minimum Standards for Construction and Maintenance of							
	Ch. 173-162 WAC	Rules & Regulations Governing the Licensing of Well							
	Ch. 173-303 WAC	Dangerous Waste Management							
	Ch. 173-304 WAC	Solid Waste Handling Standards							
	Ch. 70.105D RCW	Model Toxics Control Act							
	Ch. 173-340 WAC	MTCA Cleanup Regulation							
State of Washington Regulations	Ch. 173-350 WAC	Solid Waste Handling Standards							
rtegulations	Ch. 43.21C RCW	State Environmental Policy Act							
	Ch. 197-11 WAC	SEPA Rules							
	Ch. 70.94 RCW	Washington Clean Air Act							
	Ch. 43.21A RCW	General Regulations for Air Pollution							
	Ch. 173-400 WAC	General Regulations for Air Pollution							
	Ch. 173-460 WAC	Controls for New Sources of Air Pollution							
	Ch. 173-470 WAC	Ambient Air Quality Standards for Particulate Matter							
	29 CFR 1910	Occupational Safety and Health Act							
	42 USC 7401	Clean Air Act of 1977							
Federal Regulations	40 CFR 50	National Ambient Air Quality Standards							
	40 CFR 141	Drinking Water Regulations							
	40 CFR 260-268	Hazardous Waste Regulations (RCRA)							

ARAR = applicable or relevant and appropriate requirement

CFR = Code of Federal Regulations

MTCA = Model Toxics Control Act

RCRA = Resource Conservation and Recovery Act

RCW = Revised Code of Washington

SEPA = State Environmental Policy Act

TESC = Temporary Erosion and Sediment Control

USC = U.S. Code

WAC = Washington Administrative Code

Table 4-1
Bioventing Pilot Test Oxygen Data
Engineering Design Report
BNSF Black Tank
Spokane, WA

				Distanc	e from Injec	ction (ft)		
			32	68	96	114	124	
Start time	Injection Pressure (psi)	Injection dP (in. w.c.)	MW-23R O2 %	MW-7 O2 %	MW-22R O2 %	MW-9 O2 %	MW-5 O2 %	Notes
10/6/2020 15:50	0	0.1	0.7	16.5	0.2	0	12.2	Sustained Test Start (110 cfm)
10/6/2020 17:11	0	0.46	1.8	16.7	0.5	0	12.4	
10/7/2020 9:32	1	1.1	6.3	17.3	0.4	0	12.3	
10/7/2020 11:51	1.25	1.63	13.8	16.8	0.6	0	12.1	
10/7/2020 13:22	1.5	3	2.9	16.7	0.2	0	12.1	
10/7/2020 16:03	1.5	3	16.3	16.9	0.2	0	12.2	~ 1 day after Sustained Test Start
10/8/2020 9:32	2	3	17.6	17.4	2.3	0	12.3	
10/8/2020 11:49	1.8	3	14.7	17.3	1.2	0	11.9	
10/8/2020 14:45	1.8	3	16.2	17.8	1.6	0	12	
10/8/2020 16:38	1.8	3	19.6	18.7	3.3	0	12.5	~ 2 days after Sustained Test Start
10/9/2020 9:11	1.8	3	19.3	19	11.7	0	11.9	
10/9/2020 14:08	1.8	3	16.1	19.6	1.7	0	12.1	
10/9/2020 15:32	1.8	3	17.8	19.2	1.2	0	11.7	
10/9/2020 16:55	1.8	3	19.3	19.7	0.5	0	12.1	~ 3 days after Sustained Test Start
10/10/2020 9:00	1.8	3	18.8	20.1	1.7	0	12	
10/10/2020 11:10	1.8	3	18.9	20.3	1.4	0	12.2	
10/10/2020 19:30	3	5	21.5	20.2	6.4	0	12.6	~ 4 days after Sustained Test Start (flow increase ~150 cfm)
10/11/2020 8:05	3.2	5.2	21.6	20.8	5.8	0	12.6	
10/11/2020 12:05	3.2	5.2	20.2	19.8	7.5	0	12	~ 1 day after flow increase (~150 cfm)
10/11/2020 18:25	0	0	15.1	20.7	0.7	0	12.3	
10/12/2020 10:40	0	0	16	20.4	6	0	12.3	
10/12/2020 13:35	0	0	8.5	20.4	2.4	0	12.5	
10/12/2020 16:35	0	0	8.4	20.6	2.7	0	12.5	
10/13/2020 7:50	0	0	1.4	20.7	0.2	0	12.3	
10/13/2020 13:00	0	0	0.8	20.2	0.2	0	12.3	

CH4 = Methane

CO2 = Carbon dioxide

in. w.c. = inches water column

L/min = Liters per minute

O2 = Oxygen

psi = Pounds per square inch

Table 4-2 Bioventing Pilot Test Helium Data Engineering Design Report BNSF Black Tank Spokane, WA

ookane, WA				Dista	ince from I	njection (ft)	1		
•			0	32	68	96	114	124	
Start time	Injection Pressure (psi)	Injection dP (in. w.c.)	Injection Helium Concentration (ppmv)	MW-23R Helium (ppmv)	MW-7 Helium (ppmv)	MW-22R Helium (ppmv)	MW-9 Helium (ppmv)	MW-5 Helium (ppmv)	Notes
10/6/2020 15:50	0	0.1	9150	0	0	0	0	0	He Injection Start w/ Sustained Test
10/6/2020 17:11	0	0.46	9500	0	0	0	0	0	
10/7/2020 9:32	1	1.1	9325	0	0	0	0	0	
10/7/2020 11:51	1.25	1.63	6800	0	0	0	0	0	
10/7/2020 13:22	1.5	3	12475	0	0	0	0	0	
10/7/2020 16:03	1.5	3	8500	600	0	0	0	0	~ 1 day after He Injection Start
10/8/2020 9:32	2	3	9725	1625	0	0	0	0	
10/8/2020 11:49	1.8	3	8725	1275	0	0	0	0	
10/8/2020 14:45	1.8	3	0	3950	1450	0	0	0	
10/8/2020 16:38	1.8	3	0	8825	2800	0	0	0	~ 2 days after He Injection Start (He Injection Complete)
10/9/2020 9:11	1.8	3	0	175	3525	0	0	0	
10/9/2020 14:08	1.8	3	0	0	3475	0	0	0	
10/9/2020 15:32	1.8	3	0	0	2525	0	0	0	
10/9/2020 16:55	1.8	3	0	75	2800	0	0	0	~ 3 days after He Injection Start
10/10/2020 9:00	1.8	3	0	125	875	0	0	0	
10/10/2020 11:10	1.8	3	0	0	775	0	0	0	
10/10/2020 19:30	3	5	0	25	150	0	0	0	
10/11/2020 8:05	3.2	5.2	0	0	0	0	0	0	
10/11/2020 12:05	3.2	5.2	0	75	0	0	0	0	
10/11/2020 18:25	0	0	0	0	0	0	0	0	
10/12/2020 10:40	0	0	0	0	100	0	0	0	
10/12/2020 13:35	0	0	0	0	100	0	0	0	
10/12/2020 16:35	0	0	0	0	150	0	0	0	
10/13/2020 7:50	0	0	0	0	25	0	0	0	
10/13/2020 13:00	0	0	0	0	0	0	0	0	

Notes: CH4 = Methane CO2 = Carbon dioxide in. w.c. = inches water column L/min = Liters per minute O2 = Oxygen psi = Pounds per square inch

Table 4-3
Biosparing Pilot SF⁶ Test Data
Engineering Design Report
BNSF Black Tank
Spokane, WA

ID	Distance from Injection	Sample Date	Reported Concentration (ppb)	Measured Concentration (ppb)
BSI-1-CO-DUP-102120	-	16-Oct-20	30.6	30.6
BSI-1-CO-102120	-	16-Oct-20	31.9	31.9
BSI-01-102120	0	21-Oct-20	0.108	0.108
MW-31-102120	11	21-Oct-20	3.15	3.15
MW-03-102120	28	21-Oct-20	0.124	0.124
MW-04-102120	59	21-Oct-20	<lod< td=""><td>0.019</td></lod<>	0.019
Field Blank	-	21-Oct-20	0.140	0.14
MW-07-102120	~250	21-Oct-20	<lod< td=""><td>0.016</td></lod<>	0.016
MW-22R-102120	~330	21-Oct-20	<lod< td=""><td>0.023</td></lod<>	0.023
BSI-1-110420	0	4-Nov-20	0.026	0.026
MW-31-110420	11	4-Nov-20	0.321	0.321
MW-03-110420	28	4-Nov-20	<lod< td=""><td>0.009</td></lod<>	0.009
MW-04-110420	59	4-Nov-20	<lod< td=""><td>0.013</td></lod<>	0.013
Field Blank	-	4-Nov-20	<lod< td=""><td>0.012</td></lod<>	0.012
MW-07-110420	~250	4-Nov-20	<lod< td=""><td>0.014</td></lod<>	0.014
MW-22R-110420	~330	4-Nov-20	<lod< td=""><td>0.014</td></lod<>	0.014

LOD = Limit of detection (25 ppt)

LOQ = Limit of quantitation (40 ppt)

ppb = Parts per million

ppt = Parts per trillion

^{1.} Data validator reviewed results and concluded that the SF6 detection in the field blank does not indicate cross-contamination in the investigative samples because three of the associated investigative samples are non-detect for SF6. Therefore, none of the results are qualified as having blank contamination.

^{2.} During sampling of MW-05 for SF6, the connector on the bailer broke, leaving the bailer in the bottom of the well, where it blocked access to the groundwater in the well. The bailer could not be immediately retrieved so no groundwater samples were collected from MW-05 for SF6 analysis. Retrieval will be attempted on 11/16/20 and if successful, the well will be sampled for SF6. Wells immediately downgradient of MW-05 (MW-07 and MW-22R) were sampled and showed no detectable SF6.

Table 4-4
Viscosity Data - Field and Laboratory
Engineering Design Report
BNSF Black Tank
Spokane, WA

		Pre-Design Inve	RI Data			
Well ID	Field Sample Temperature (°C)	Field Sample Test Duration (sec)	Estimated Field Viscosity (cSt) ¹	Lab Viscosity at 40°C (cSt)	Lab Viscosity at 21°C (cSt)	Converted Lab Viscosity at 40°0 (cSt) ⁴
MW-01	12.70	8.32	3,500	412	2,070	500
MW-02	12.00	8.25	4,300	460	-	-
MW-03	17.10	10.54	5,500	871	5,570	1,000
MW-04	17.50	7.97	3,700	655	-	-
MW-05	13.60	4.40	1,800	281	-	-
MW-07	12.80	2.38	750	141	-	-
MW-09	_2	_2	_2	47	-	-
MW-17	13.20	4.12	1,100	192	-	-
MW-18	-	-	-	-	1,660	390
MW-31	20.40	16.59	-	_3	-	_

°C= degrees celsius

sec = seconds

cSt = centistoke

- = Not sampled

- 1 = Converted lab viscosity to field viscosity at field tempuratures using Bunker C viscosity vs. temperature graph (Figure D-2) (WinGD 2018)
- 2 = Not enough sample to conduct field test.
- 3 = Viscosity test fails at 40 and 100° C . Sample too thick and fails to flow adequately to fill viscosity cell.
- 4 = Converted lab viscosity at 21°C to lab viscosity at 40°C using Bunker C viscosity vs. temperature graph (Figure D-3) (WinGD 2018)

Table 4-5
LNAPL Recovery Test Transmissivity
Engineering Design Report
BNSF Black Tank
Spokane, WA

Well ID	In(R _{oi} /r _w)	b _n (ft)	ρ _r	s _n (ft)	Q _n (ft³/day)	2020 Recovery Test T _n (ft ² /day)	2016 Recovery Test T _n (ft²/day)
MW-01	4.6	1.19	0.96	0.05	2.9E-05	4.5E-04	
MW-02	4.6	0.32	0.96	0.01	9.8E-06	5.6E-04	
MW-03	4.6	0.26	0.98	0.01	3.2E-04	4.5E-02	6.2E-04
MW-04	4.6	0.72	0.98	0.01	4.8E-05	2.4E-03	1.5E-02
MW-05	4.6	0.83	0.98	0.02	1.9E-04	8.6E-03	
MW-07	4.6	2.02	0.98	0.04	2.8E-04	5.1E-03	9.8E-02
MW-09	4.6	0.11	0.98	0.00	3.1E-04	1.0E-01	
MW-17	4.6	1.00	0.98	0.02	1.5E-03	5.6E-02	3.6E-02
MW-31	4.6	0.21	0.98	0.00	5.8E-05	1.0E-02	

Transmissivity is calculated using Equation 16.

LNAPL drawdown time is calculated using Equation 17.

The value of the term $ln(R_o/r_w)$ can be assumed to equal 4.6 with the introduction of little additional error.

Abbreviations:

ft3/day = cubic feet per day

ft2/day = square feet per day

ITRC = Interstate Technology and Regulatory Council

 $ITRC-Low = 0.1 \ ft^2/day = Low\ end\ of\ ITRC\ transmissivity\ range\ that\ indicates\ hydraulic\ recovery\ of\ LNAPL\ is\ not\ practicable.$

ITRC- High = 0.8 ft²/day High end of ITRC transmissivity range that indicates hydraulic recovery of LNAPL is not practicable.

LNAPL = light non-aqueous phase liquid

Equation 16 - ASTM E2856-11^{ε1}, Manual Skimming Test Transmissivity

$$T_n = \frac{Q_n \ln(\frac{R_{oi}}{r_w})}{2\pi s_n}$$

Equation 17 - ASTM E2856-11^{£1}, Maximum unconfined LNAPL drawdown

$$s_{n_unconfined} = b_n (1 - \rho_r)$$

Equations 16 & 17 Variables

 T_n = LNAPL Transmissivity (ft²/day)

Q_n = measured LNAPL recovery rate (ft³/day)

R_{oi} = radius of influence (ft)

 r_w = well radius (ft)

s_n = LNAPL drawdown at time t (ft)

 b_n = LNAPL thickness (ft) ρ_r = LNAPL specific density

Table 5-1 New Well Construction Details Engineering Design Report BNSF Black Tank Spokane, WA

						Screen	Interval		Screen	Filter	Pack				
			Borehole	Soil Sampling	Borehole	Dep	th to	Screen	Slot	Dep	th to	Well			
Well	Pre-Clearance	Drilling	Diameter	Interval	Depth	Тор	Bottom	Length	Size	Тор	Bottom	Diameter		Well Construction Material	s
Identification	Method	Method	(inches)	For Logging	(feet bgs)	(feet bgs)	(feet bgs)	(feet)	(inches)	(feet bgs)	(feet bgs)	(inches)	Screen	Riser	Protective Cover
BV-02 & BV-03	Air Knife	Sonic	6	Continuous	167	147	167	20	0.010	145	167	2	Schedule 40 304 SS	Schedule 40 carbon steel	Well Vault
BV-04 & BV-05*	Air Knife	Sonic	6	Continuous	167	147	167	25 **	0.010	145	167	2	Schedule 80 PVC	Schedule 80 PVC	Well Vault
BS-02 to BS-05, BS-07 to BS-09	Air Knife	Sonic	6	Continuous	195	190	195	5	0.010	188	195	2	Schedule 40 PVC	Schedule 40 PVC	Well Vault
BS-06 & BS-10*	Air Knife	Sonic	6	Continuous	195	190	195	5	0.010	188	195	2	Schedule 80 PVC	Schedule 80 PVC	Well Vault

Notes:

* = borehole length will be longer than depth for the angled wells

** = Screen length of these two angled wells is 25 ft. Because angled well the screen will cover the vertical interval of 147 ft bgs to 167 ft bgs
bgs = below ground surface
N/A = Not Applicable

Table 7-1
Schedule for Engineering Design, Procurement and Construction of BV/BS System Engineering Design Report
BNSF Black Tank
Spokane, WA

Task	Estimated Duration	Schedule
Submit Revised Draft EDR and accompanying documents to Ecology		2Q 2021
Complete rerouting of active rail lines on the Site		3Q 2021
Complete Phase 2 of the Surface Soil Interim Action	90 days	3Q - 4Q 2021
Submit Addendum to the Surface Soil Interim Action Completion Report	60 days	4Q 2021
Ecology review and approve Draft EDR and accompanying documents		10/27/2021
Submit Final EDR w/ accompanying documents to Ecology		11/26/2021
Ecology approve Final EDR		11/26/2021
Submit Construction Plans & Specifications to Ecology	60 days + 30 days	1Q 2022
Ecology approve Construction Plans & Specifications		2Q 2022
North Spokane Corridor (NSC) project grading complete (1)		3Q 2022
Initiate construction of BV/BS system including verification that BV/BS system is aligned with NSC as-built configuration		3Q - 4Q 2022
Construct BV/BS system including well network and including procure equipment, materials, & services.	240	3Q 2022 – 3Q 2023
Start-up and Acceptance of BV/BS System	60 days	3Q - 4Q 2023

(1) Timeframe based on information provided on the NSC website and in May 2021 conversations with WSDOT.

NA = Not applicable TBD = To be determined Q = quarter

EDR = Engineering Design Report

BV/BS = biovent/biosparge

NSC = North Spokane Corridor

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

APPENDIX A LNAPL CONDITIONS ASSESSMENT

www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021

APPENDIX A: LNAPL CONDITIONS ASSESSMENT

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Acronyms and Abbreviations

ERM Environmental Resources Management

LNAPL light non-aqueous phase liquid

MW Monitoring Well

SM Silty sands and silt-mixtures

SP Poorly-graded sands and gravelly sands, little or no fines

www.erm.com Project No.: 0578173 Client: BNSF, HOOL November 2021

1. INTRODUCTION

ERM (ERM-West, Inc.) evaluated hydrostratigraphic and gauged light non-aqueous phase liquid (LNAPL) thickness data from the Black Tank site to determine whether the LNAPL is under confined, unconfined, or perched conditions, and if those conditions have resulted in gauged LNAPL thicknesses that are exaggerated relative to the formation thickness of mobile LNAPL. Research by Kirkman et al. (2013) and Reyenga and Hawthorne (2015) shows that confined and perched conditions can result in exaggerated gauged LNAPL thicknesses. This evaluation was undertaken because lenses or beds of silt and silty sand facies identified at the Site in the depth interval between the seasonal high and low groundwater table could potentially produce confined and/or perched LNAPL (ERM 2017). To assess whether exaggerated gauged LNAPL thicknesses exist at the Site and to make appropriate corrections to those thicknesses, ERM used the techniques presented in Hawthorne (2011) and Kirkman et al. (2013) to evaluate LNAPL and groundwater level data collected from the Site since March 2016.

2. LNAPL EVALUATION APPROACH

ERM prepared hydrostratigraphs showing LNAPL (air/LNAPL interface) elevation, groundwater (LNAPL/water interface) elevation, and gauged LNAPL thickness data for each monitoring well containing measurable LNAPL (MW-01 through MW-05, MW-07, MW-09, MW-17 through MW-20, MW-23, and MW-28). The position of the well screen and any layers of silt and silty sand facies identified in the screened interval are also shown on the hydrostratigraphs. Visually assessing these data on the hydrostratigraph helps identify potential confined, unconfined, and perched LNAPL conditions (Hawthorne 2011; Kirkman et al. 2013). Data collected since March 2016 was used for this evaluation because the data were collected using better field measurement technique and were cross-checked using LNAPL removal data. Data collected prior to March 2016 are suspect because the techniques for: (1) measuring the LNAPL-groundwater interface and (2) identifying and correcting erroneous gauged LNAPL thickness data had not yet been refined. LNAPL recovery from these wells was performed following each monitoring event, except June and September 2016. For this evaluation, it is assumed that gauged LNAPL thicknesses recover to levels representative of the formation thickness of mobile LNAPL between monitoring events. This assumption appears to be valid because the period between monitoring events has ranged from one quarter to more than a year for some wells, yet the hydrostratigraphs show that gauged LNAPL thicknesses are generally stable. Nonetheless, this assumption can be re-evaluated in the future as additional LNAPL thickness data is collected.

Field measurement of the LNAPL/water interface continues to be challenging in wells where the LNAPL is highly viscous, the well interior is coated with LNAPL, and/or the LNAPL thickness exceeds approximately 2 feet because it is difficult to get the water level probe to pass through the LNAPL before the protective ice coating melts and the LNAPL fouls the probe. In some wells (e.g., MW-03, MW-04, and MW-05), the LNAPL has been too viscous to collect LNAPL/water interface data. When possible, ERM determines the LNAPL thickness for these wells based on the volume of LNAPL removed from the well. ERM also crosschecks the gauged LNAPL thicknesses from other wells noted to contain viscous LNAPL or as having measurement problems by comparing the field LNAPL thickness to calculated LNAPL thickness value based on the volume of LNAPL removed from the well. If the field LNAPL thickness exceeds twice the calculated LNAPL thickness, the field LNAPL thickness is considered erroneous and replaced with the calculated LNAPL thickness. The Addendum for Modified Product Gauging Method, RI/FS Project Plan (ERM 2016), presents the updated field methodology used to collect gauged LNAPL thickness data, and the groundwater monitoring reports from 2016 through 2020 present the gauged LNAPL thickness data used in this report and the technique for identifying and correcting erroneous gauged LNAPL thickness data.

The silt and silty sand facies may be a potential confining layer for the LNAPL without being a confining layer for groundwater. Soils containing a higher percentage of silt and/or clay have smaller pore sizes and LNAPL movement may be inhibited in these soils because it cannot overcome their pore entry pressure (Kirkman et al. 2013). Because water does not have the same capillary limitations as LNAPL, some fine-grained materials may not be a confining layer for groundwater while being a confining layer for LNAPL (Hawthorne et al. 2011a). Layers of the silt and silty sand facies, as described in the *Work Plan to Determine Deep Contamination Cleanup Action Design Parameters* (ERM 2020), and layers of sand with silt (i.e., SM or SP/SM) are illustrated on the hydrostratigraphs because both may contain a sufficiently high percentage of silt and/or clay to be a potential confining layer for LNAPL.

ERM visually assessed each hydrostratigraph for perched, confined, and unconfined conditions. Perched LNAPL conditions exist when a monitoring well is constructed with its screen above and within a confining layer, LNAPL rests above the confining layer, and the groundwater level is within or below the confining layer. Under these conditions, the well will act as a sump, and LNAPL will accumulate in the well until it equilibrates with the top of the LNAPL interval (Hawthorne et al. 2011b). This results in an exaggerated LNAPL thickness if the bottom of the monitoring well is deeper than the top of the confining layer. Confined LNAPL conditions exist when LNAPL is trapped beneath a confining layer and pressurized. If a monitoring well is constructed with its screen within and below the confining layer, the well will act as a pressure relief valve for the confined LNAPL, and LNAPL will accumulate and rise in the well until it equalizes with atmospheric pressure, resulting in an exaggerated gauged LNAPL thickness (Hawthorne et al. 2011a). Unconfined LNAPL is neither trapped below nor resting above a confining layer. The gauged LNAPL thickness in a well under unconfined conditions is representative of the LNAPL thickness in the formation.

Because groundwater levels at the site fluctuate up to 9.6 feet in a year and layers of the silt and silty sand facies occur partially or wholly within the groundwater fluctuation interval, LNAPL conditions at a well can vary from unconfined to confined or perched as a result of changes in groundwater level. Therefore, ERM assessed the conditions at each monitoring well during each monitoring event and identified LNAPL thickness measurements for correction, as needed, to reflect the conditions during that monitoring event.

The approach used to assess the LNAPL conditions from the hydrostratigraphs and determine an appropriate LNAPL thickness correction, if needed, is summarized below:

- Unconfined LNAPL conditions—Neither the LNAPL level nor the groundwater level are situated within a potential confining layer. No potential for exaggeration exists; therefore, no LNAPL thickness correction is needed.
- Perched LNAPL conditions—The LNAPL level is situated above a potential confining layer and the groundwater level is below the top of the potential confining layer. Acceptable methods for correcting the LNAPL thickness are to use: (1) the difference between the LNAPL elevation and the elevation of the top of the confining layer, or (2) the LNAPL thickness measurement from the most recent monitoring event showing unconfined conditions at the monitoring well (Reyenga and Hawthorne 2015).
- Confined LNAPL conditions—The LNAPL level is within a potential confining layer and the groundwater level is below the same potential confining layer. Acceptable methods for correcting the LNAPL thickness are to use: (1) the difference between the groundwater elevation and the elevation of the bottom of the confining layer, or (2) the LNAPL thickness measurement from the most recent monitoring event showing unconfined conditions at the monitoring well (Reyenga and Hawthorne 2015).

Where both the LNAPL level and the groundwater level are within a potential confining layer, the LNAPL condition may be either perched or confined. The condition cannot be discerned from this information alone, but generally can be discerned using stratigraphic and previous LNAPL condition data for the well. Neither Hawthorne (2011) nor Kirkman et al. (2013) provide a method for directly correcting the gauged LNAPL thickness measurement for this situation. However, comparison to historical LNAPL thickness measurements can be used to develop a corrected LNAPL thickness. Specifically, the corrected LNAPL thickness should be the lessor of: (1) the gauged LNAPL thickness from the monitoring event, (2) the most recent corrected LNAPL thickness based on documented unconfined, confined, or perched LNAPL conditions, or (3) the lowest gauged LNAPL thickness if all monitoring events have shown LNAPL and groundwater levels within the potential confining layer.

This evaluation presents ERM's interpretation of the LNAPL conditions at for each monitoring event at each Site monitoring well based on the data currently available and the evaluation techniques provided by Hawthorne (2011) and Kirkman et al. (2013). However, evaluation of LNAPL conditions at the site is an iterative process that may result in revised interpretations over time. As more LNAPL data are collected and new evaluation techniques are used, our interpretation of the LNAPL conditions at individual monitoring well locations may evolve.

3. RESULTS OF LNAPL EVALUATION AND LNAPL THICKNESS CORRECTION

The LNAPL conditions for each monitoring well containing LNAPL during the March 2016 through September 2020 monitoring events are summarized on Table A-1, and the hydrostratigraphs used to interpret the LNAPL conditions are presented on Figures A-1 through A-13.

Monitoring wells MW-01, MW-04, MW-05, MW-09, MW-17, MW-19, MW-23, and MW-28 show no evidence of exaggerated gauged LNAPL thicknesses. These wells have either no confining layer or only leaky confining layers within their normal groundwater fluctuation intervals. Therefore, they show unconfined or leaky confined conditions throughout all monitoring events (Table A-1). No confining layers are situated within the normal groundwater fluctuation intervals at MW-1 (Figure A-1), MW-5 (Figure A-5), and MW-23 (Figure A-12); therefore, unconfined LNAPL conditions consistently occurred at these wells during all of the monitoring events (Table A-1). Only potential leaky confining layers are situated in the normal groundwater fluctuation intervals at MW-4 (Figure A-4), MW-9 (Figure A-7), MW-17 (Figure A-8), MW-19 (Figure A-9), and MW-28 (Figure A-13); therefore, unconfined or leaky confined LNAPL conditions consistently occurred at these wells during all of the monitoring events (Table A-1).

Monitoring wells MW-2, MW-3, MW-7, and MW-18 have confining layers near the bottom of their normal groundwater fluctuation intervals and LNAPL above the confining layers (Figures A-2, A-3, A-6, and A-9). Occasionally, groundwater levels are below the top of the confining layer, resulting in perched LNAPL conditions and exaggerated gauged LNAPL thicknesses at these wells (Table A-1 and Figures A-2, A-3, A-6, and A-9). This is particularly obvious in MW-07 and MW-18. Monitoring well MW-20 has relatively thick confining layers throughout most of its normal groundwater fluctuation interval and LNAPL between and within the confining layers (Figure A-11). Groundwater levels at this well are generally below the top of the lower confining layer, resulting in mostly perched LNAPL conditions and exaggerated gauged LNAPL thicknesses (Table A-1 and Figure A-11). However, one monitoring event showed a very high water table condition resulting in the LNAPL being trapped below an overlying confining layer. This produced confined LNAPL conditions and exaggerated gauged LNAPL thicknesses (Table A-1 and Figure A-11).

Potential corrected LNAPL thickness values for perched or confined conditions exhibited at MW-2, MW-3, MW-07, MW-18, and MW-20 are displayed on their hydrostratigraphs (Figures A-2, A-3, A-6, A-9, and A-11). For MW-02, MW-03, MW-07, and MW-18, this analysis helps explain some or all of the anomalous spikes in gauged LNAPL thickness in these wells by showing that they correlate with perched LNAPL conditions resulting from low groundwater levels (Figures A-2, A-3, A-6, and A-9). Thus, the gauged LNAPL thickness data obtained from these wells during unconfined conditions is representative of the LNAPL thicknesses in the formation at these wells and the occasional exaggerated thickness observed during perched conditions is not representative (Figures A-2, A-3, A-6 and A-9). For MW-20, this analysis helps explain the anomalously high gauged LNAPL thicknesses reported from a well situated a considerable distance from the LNAPL source. The consistent perched or confined LNAPL conditions resulting from the thick confining layers observed throughout the groundwater fluctuation interval at this well location likely results in consistently exaggerated gauged LNAPL thicknesses. The actual formation thickness of LNAPL at this location cannot be readily determined from the well data; however, we can reasonably say that it is less than the thinnest LNAPL measurement reported for the well (1.87 feet).

ERM believes the potential corrected LNAPL thickness data present a more realistic representation of changes over time in the formation LNAPL thickness at MW-02, MW-03, MW-07, MW-18, and MW-20. The potential corrected data reduces both spatial and temporal anomalies in the LNAPL thickness data by eliminating exaggerated thicknesses. However, correcting the exaggerated thicknesses results in relatively modest changes in the reported LNAPL thicknesses and the changes do not affect the footprint of any of the LNAPL restoration timeframe (RTF) areas.

MW-02, MW-03, and MW-07 are potential performance monitoring wells for the Site, and because they occasionally exhibited perched conditions, the LNAPL conditions at those wells and potential corrections to gauged LNAPL thickness data from those wells should be considered for future evaluation of gauged LNAPL thickness performance monitoring data. The other two wells that exhibited perched and/or confined conditions (MW-18 and MW-20) were decommissioned and are not available for future performance monitoring.

4. REFERENCES

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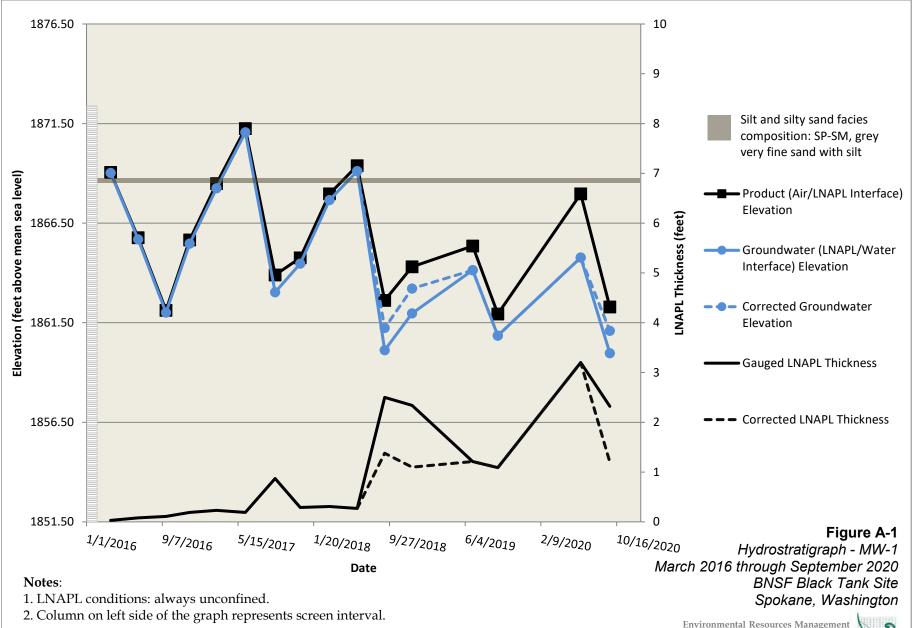
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Table A-1 Summary of Hydrostratigraphic Assessment Engineering Design Report BNSF Black Tank Spokane, WA

Monitoring Well		LNAPL Condition										
wonitoring weil	March-16	June-16	September-16	December-16	March-17	June-17	September-17	December-17	March-18	June-18	September-18	December-18
MW-01	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined
MW-02	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Perched	Unconfined
MW-03	Perched	Perched	Perched	Unconfined	Unconfined	Unconfined	Perched	Perched	Not measured	Not measured	Not measured	Not measured
MW-04	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer
MW-05	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined
MW-07	Perched	Perched	Perched	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined
MW-09	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer
MW-17	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined	Unconfined
MW-18	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Perched	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined
MW-19	Unconfined	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined	Unconfined within leaky confined layer	Unconfined	Unconfined	Unconfined within leaky confined layer	Unconfined	Unconfined	Unconfined within leaky confined layer	Unconfined within leaky confined layer
MW-20	Perched	Perched	Perched	Perched	Perched	Perched	Perched	Perched	Perched	Confined	Perched	Perched
MW-23	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined	Unconfined
MW-28	Unconfined within leaky confined layer	Unconfined	Unconfined	Unconfined	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Unconfined	Unconfined			

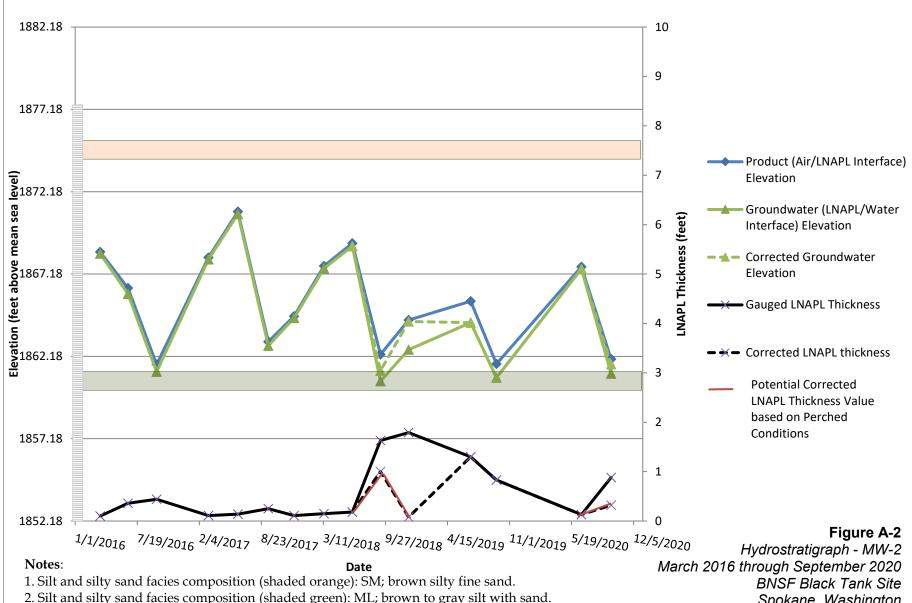
Table A-1 (continued) Summary of Hydrostratigraphic Assessment Engineering Design Report BNSF Black Tank Spokane, WA

Monitoring Well	LNAPL Condition								O-mode Maked	
wonitoring weil	June-19	September-19	June-20	September-20	Unconfined	Confined	Perched	Needed	Correction Method	
MW-01	Unconfined	Unconfined	Unconfined	Unconfined	Always	Not Observed	Not Observed	None	Correction not needed; therefore, gauged product thickness used	
MW-02	Unconfined	Perched	Unconfined	Unconfined	Mostly	Not Observed	Occasionally	Occasionally	Lesser of most recent unconfined gauged measurement or corrected measurement taken during perched conditions.	
MW-03	Not measured	Unconfined	Unconfined	Unconfined	Occasionally	Not Observed	Occasionally	Occasionally	Lesser of most recent unconfined gauged measurement or corrected measurement taken during perched conditions.	
MW-04	Unconfined within leaky confined layer	Always	Not Observed	Not Observed	None	Correction not needed as unconfined conditions are observed within the leaky confined layer; therefore, gauged product thickness used.				
MW-05	Unconfined	Unconfined	Unconfined	Unconfined	Always	Not Observed	Not Observed	None	Correction not needed; therefore, gauged product thickness used	
MW-07	Unconfined	Perched	Unconfined	Perched	Mostly	Not Observed	Occasionally	Occasionally	Lesser of most recent unconfined gauged measurement or corrected measurement taken during perched conditions.	
MW-09	Unconfined within leaky confined layer	Unconfined	Unconfined within leaky confined layer	Unconfined	Always	Not Observed	Not Observed	None	Correction not needed as unconfined conditions are observed within the leaky confined layer; therefore, gauged product thickness used.	
MW-17	Unconfined within leaky confined layer	Unconfined	Unconfined within leaky confined layer	Unconfined	Always	Not Observed	Not Observed	None	Correction not needed as unconfined conditions are observed within the leaky confined layer; therefore, gauged product thickness used.	
MW-18	Unconfined	Unconfined	Not measured	Not measured	Mostly	Not Observed	Occasionally	Occasionally	Lesser of most recent unconfined gauged measurement or corrected measurement taken during perched conditions.	
MW-19	Not measured	Unconfined within leaky confined layer	Not measured	Not measured	Always	Not Observed	Not Observed	None	Correction not needed as unconfined conditions are observed within the leaky confined layer; therefore, gauged product thickness used.	
MW-20	Perched	Perched	Not measured	Not measured	Never	Occasionally	Mostly	Always	Lesser of most recent unconfined gauged measurement or corrected measurement taken during perched or confined conditions.	
MW-23	Unconfined	Unconfined	Not measured	Not measured	Always	Not Observed	Not Observed	None	Correction not needed; therefore, gauged product thickness used	
MW-28	Unconfined within leaky confined layer	Unconfined within leaky confined layer	Not measured	Not measured	Always	Not Observed	Not Observed	None	Correction not needed as unconfined conditions are observed within the leaky confined layer; therefore, gauged product thickness used.	



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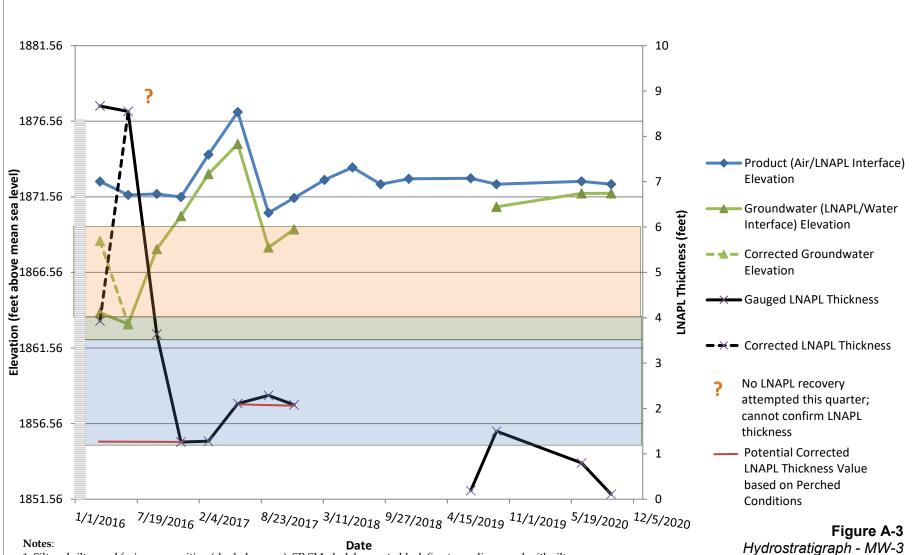
2. Silt and silty sand facies composition (shaded green): ML; brown to gray silt with sand.

- 2. LNAPL conditions: mostly unconfined; occassionally perched.
- 4. Column on left side of graph represents screen interval.

Spokane, Washington

Environmental Resources Management





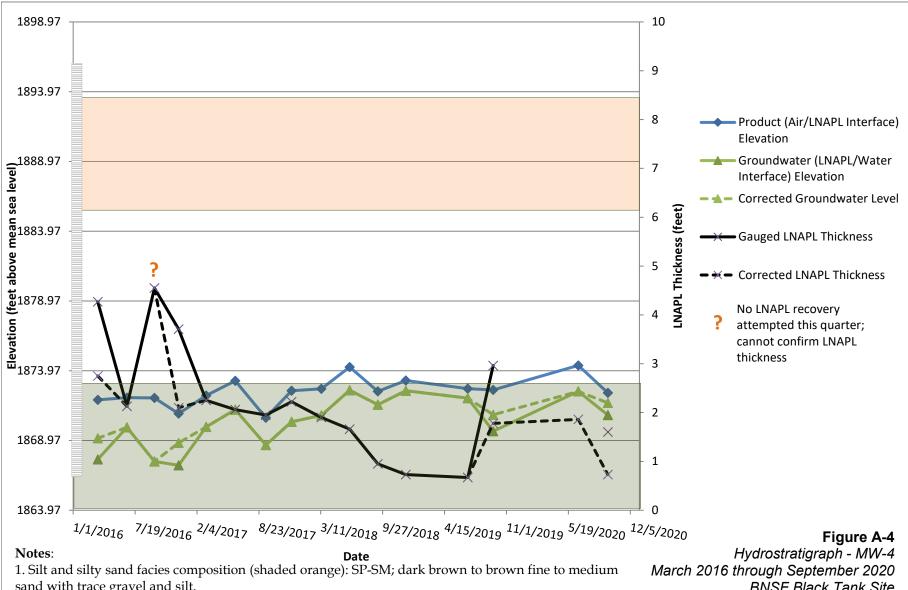
1. Silt and silty sand facies composition (shaded orange): SP-SM; dark brown to black fine to medium sand with silt.

- 2. Silt and silty sand facies composition (shaded green): ML; grayish green silt with sand.
- 3. Silt and silty sand facies composition (shaded blue): SP-SM; grayish green fine to medium sand with silt.
- 4. LNAPL conditions: ocassionally unconfined, ocassionally perched.
- 5. Column on the left side of graph represents screen interval.
- 6. LNAPL thickness and groundwater elevation not measured between 12/2017 and 9/2019 due to tar-like viscosity. LNAPL thickness from 6/2019 is based on volume removed.

Hydrostratigraph - MW-3
March 2016 through September 2020
BNSF Black Tank Site
Spokane, Washington

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sand with trace gravel and silt.

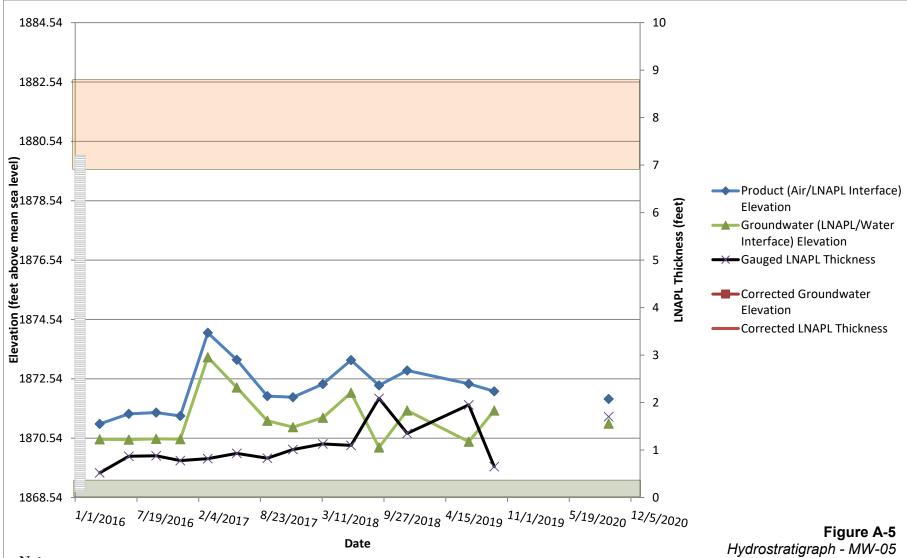
2. Silt and silty sand facies composition (shaded green): SP-SM; dark brown to black fine to medium sand with silt, SM; dark brown to black fine sand, ML; dark brown to black silt with sand.

- 3. LNAPL conditions: Unconfined within leaky confining layer.
- 4. Column on the left side of graph represents screen interval.

BNSF Black Tank Site Spokane, Washington

Environmental Resources Management



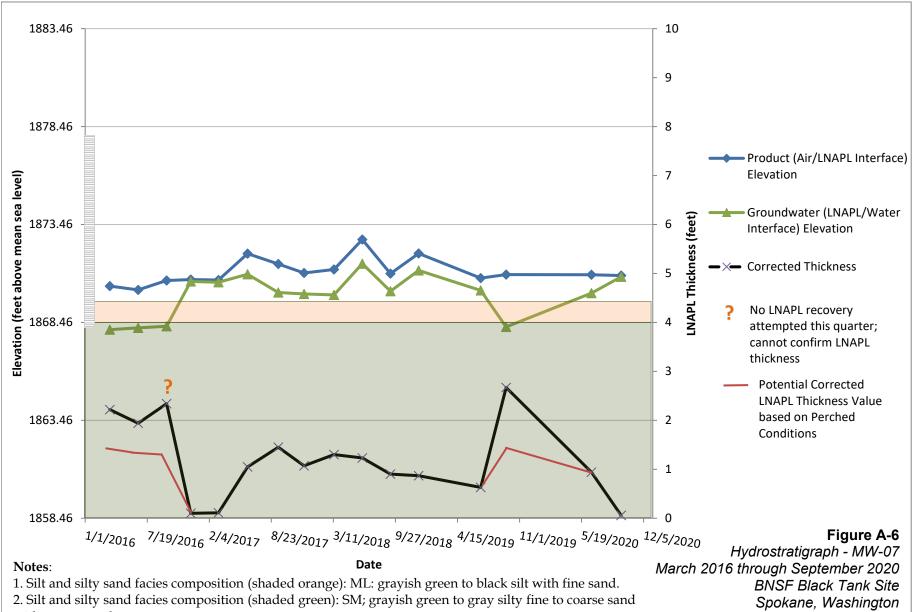


1. Silt and silty sand facies composition (shaded orange): SP-SM; Gray fine to medium sand with silt.

- 2. Silt and silty sand facies composition (shaded green): ML; dark brown silt.
- 3. LNAPL conditions: always unconfined.
- 4. Column on left side of graph represents screen interval.

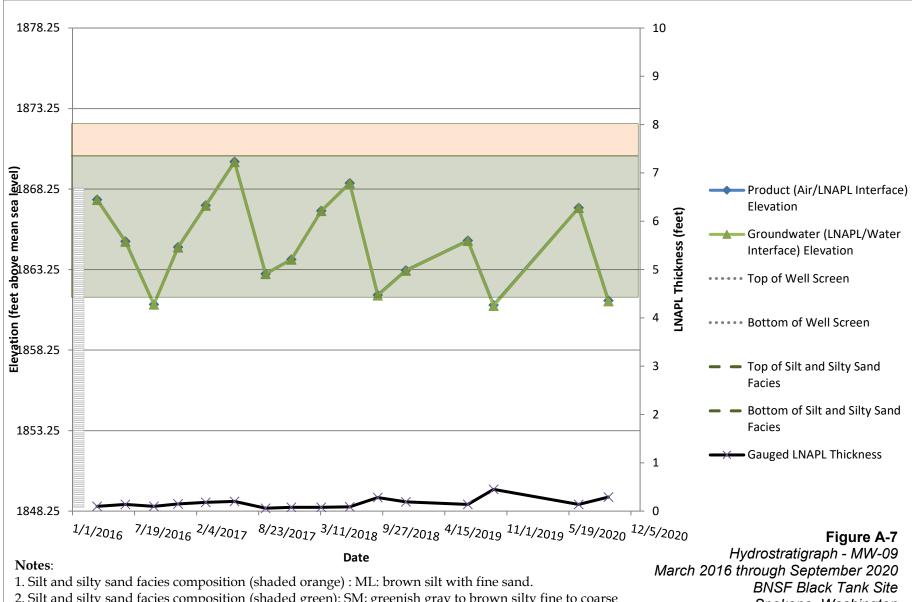
Hydrostratigraph - MW-05
March 2016 through September 2020
BNSF Black Tank Site
Spokane, Washington





- with trace gravel.
- 3. LNAPL onditions: mostly unconfined, ocassionally perched.
- 4. Column on the left side of graph represents screen interval.

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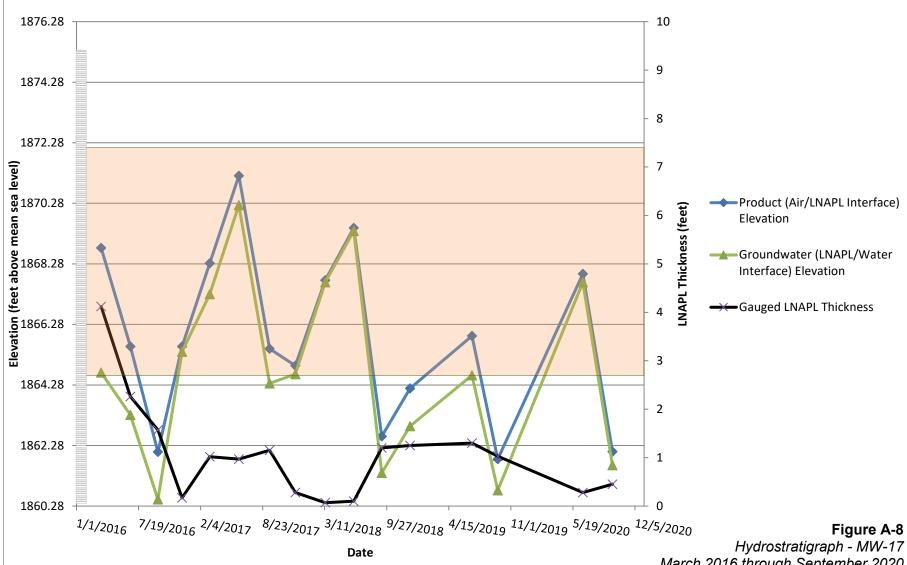


2. Silt and silty sand facies composition (shaded green): SM; greenish gray to brown silty fine to coarse sand with gravel.

- 3. LNAPL conditions: Unconfined, generally within leaky confining layer.
- 4. Column on the left side of graph represents screen interval.

Spokane, Washington





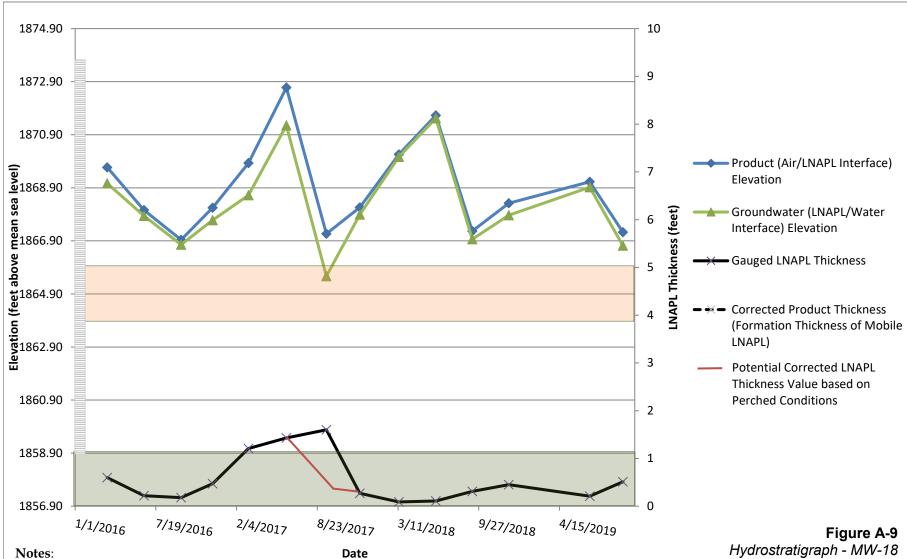
1. Silt and silty sand facies composition (shaded orange): SP/SM; gray silty medium sand with silt.

2. LNAPL conditions: Unconfined, occassionally within leaky confining layer.

3. Column on left side of graph represents screen interval.

Hydrostratigraph - MW-17
March 2016 through September 2020
BNSF Black Tank Site
Spokane, Washington



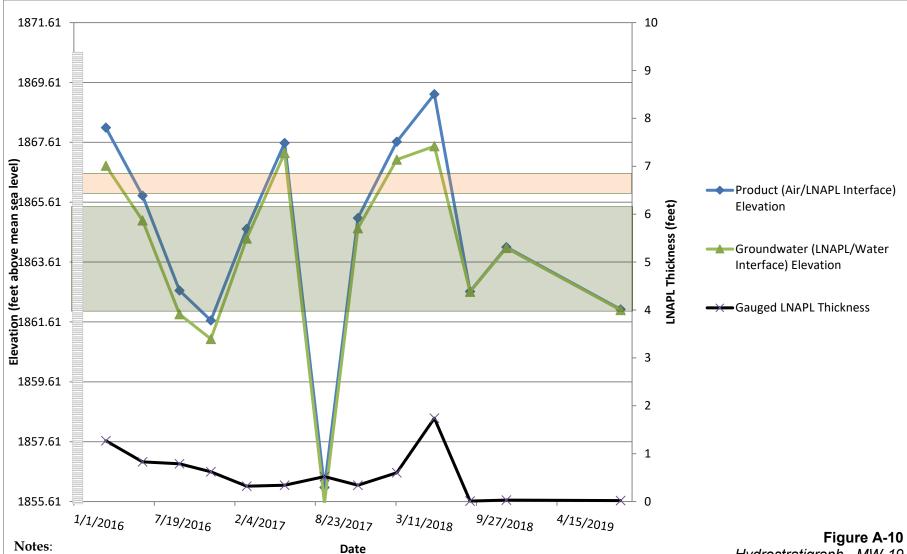


2. Silt and silty sand facies composition (shaded green): SW-SM; dark grayish brown, well graded sand with silt.

- ${\it 3.\ LNAPL\ conditions:\ Mostly\ unconfined,\ occassionally\ perched.}$
- 4. Column on left side of the graph represents screen interval.

Hydrostratigraph - MW-18 March 2016 through September 2019 BNSF Black Tank Site Spokane, Washington

Environmental Resources Management www.erm.com

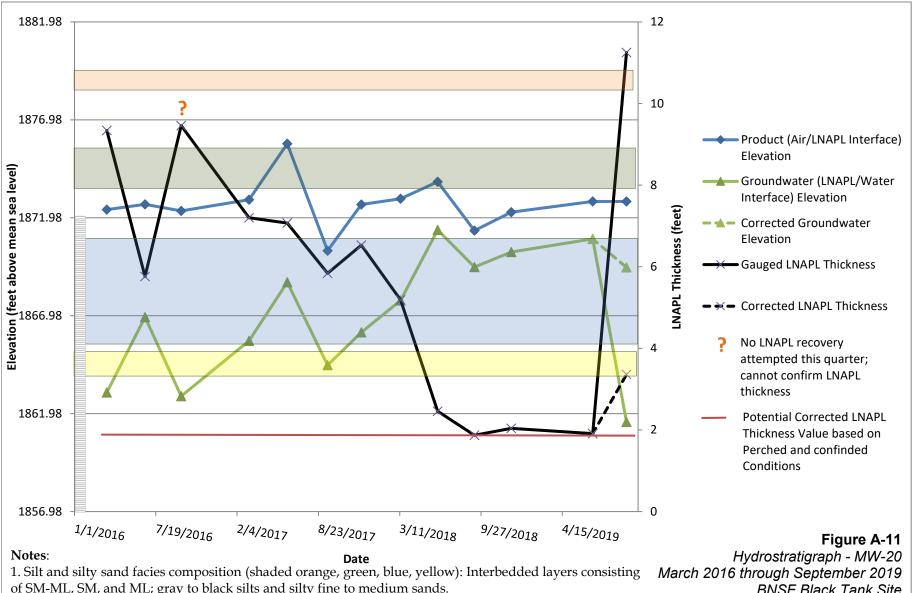


1. Silt and silty sand facies composition (shaded orange): SM-ML; Gray silt and silty sand laminations.

- 2. Silt and silty sand facies composition (shaded green): ML; yellowish gray silt, grading to SM-ML and SM; gray silty fine sand.
- 3. LNAPL conditions: Unconfined, occassionally within leaky confining layer.
- 4. Column on left side of the graph represents screen interval.
- 5. September 2017 groundwater elevation likely erroneous.

Hydrostratigraph - MW-19
March 2016 through September 2019
BNSF Black Tank Site
Spokane, Washington

Environmental Resources Management www.erm.com

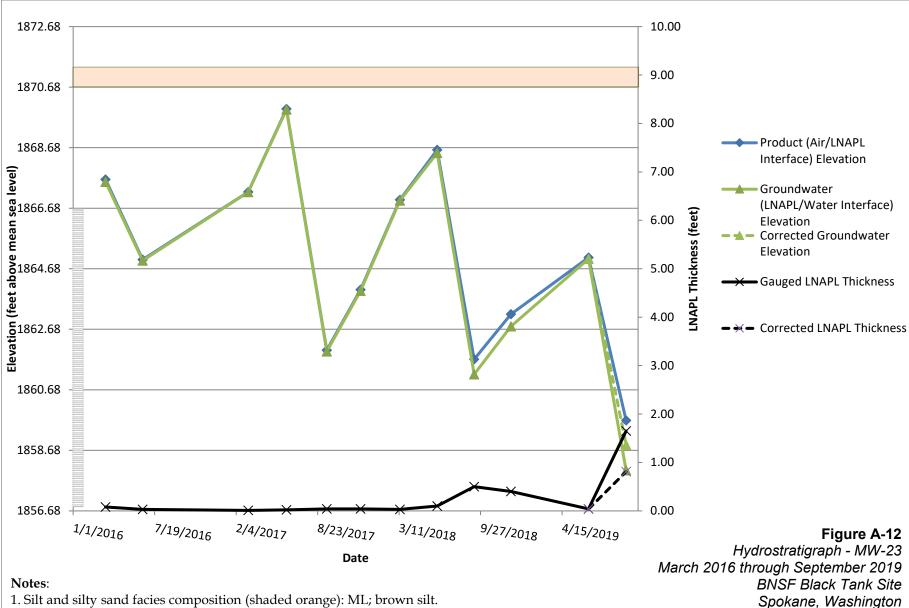


of SM-ML, SM, and ML; gray to black silts and silty fine to medium sands.

- 2. LNAPL conditions: Perched and occasionally confined.
- 3. Column on left side of graph represents screen interval.

BNSF Black Tank Site Spokane, Washington

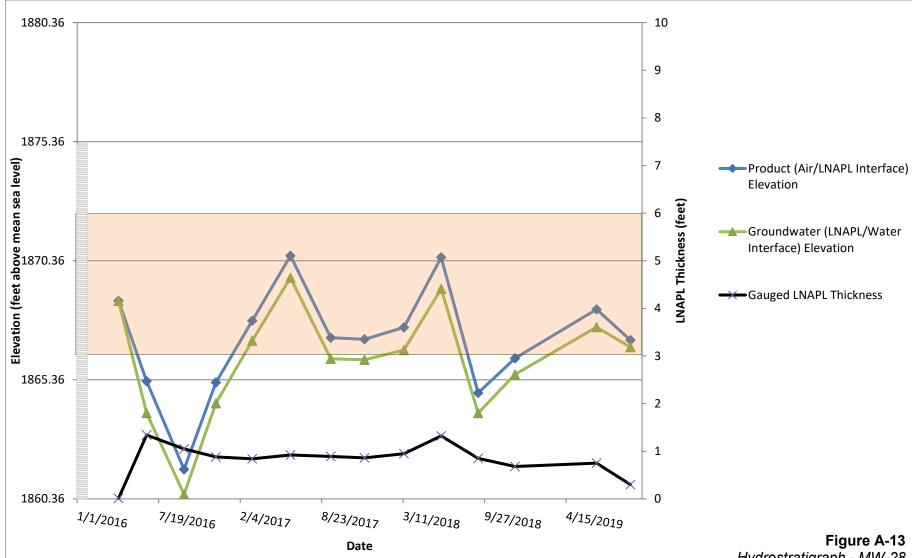
> **Environmental Resources Management** ERM



1. Silt and silty sand facies composition (shaded orange): ML; brown silt.

- 2. LNAPL Conditions: Always unconfined.
- 3. Column on left side of column represents screen interval.

Environmental Resources Management www.erm.com ERM



- 1. Silt and silty sand facies composition (shaded orange): SM; brown silt and silty fine sand.
- 2. LNAPL conditions: Unconfined, occassionally within leaky confining layer.
- 3. Column on left side of the graph represents screen interval.

Figure A-13

Hydrostratigraph - MW-28

March 2016 through September 2019

BNSF Black Tank Site

Spokane, Washington

Environmental Resources Management www.erm.com

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property								
APPENDIX B	WELL DEVELOPMENT FORMS AND BORING LOGS							

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WELL DEVELOPMENT FORMS

www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021

WELL DEVELOPMENT/PURGE FORM WELL NUMBER ______MW-22R LOCATION MW-22R AIR TEMPERATURE ~ 800 F CLIENT BNSF, Husky DEVELOPMENT CONTRACTOR EWE CASING VOLUME (GAL.) 0.98 PROJECT NAME/NUMBER Black Tank 2020 Well Install DEVELOPMENT METHOD Surge/bail, pump FIELD SUPERVISOR MC TOTAL GALS. REMOVED 36 WELL CONSTRUCTION DATA (Y/N) DEPTH TO WATER AT END 178.45 btoc 178.46 btoc DEPTH TO WATER AT START TO @ Start 184.15' btoc TD @ End 184.15' btoc TIME DURATION (MIN) SUSPEND. TURBIDITY VOL. DEPTH DISCHRG. TEMP. COND. REMARKS TO RATE SOLIDS mS/cm 00 HRS/MIN SURGE BAIL PUMP GAL. WATER (gpm) mg/L NTU Disposition of water, etc.) 1345 1408 178.41 Over 800 NTU 0718 18,19 0.298 178.44 ~1.5 7.41 0.211 612 0808 178.45 19.47 0.322 7.56 0,209 9/10/2020 SIGNATURE ERM-WEST, INC. plp/1.00/12.95

0.83 K 60 mm 49.8

VELL CONSTRUCT	MBER						DEVELOPME				EWE	DATE 9/9-10/2020 AIR TEMPERATURE 280°		
	PROJECT NAME/NUMBER Black Tank 2020 Well Install WELL CONSTRUCTION DATA (Y/N) Y							ENT METHOI)	Surge/bail, pu	mp	CASING VOLUME (GAL.) 0,8		
	ION DATA (Y	/N)		Y			FIELD SUPER	RVISOR		MC		TOTAL GALS, REMOVED 68		
EPTH TO WATER	AT START		1	78.3=	1' btoo		DEPTH TO W	ATER AT EN	(D	178.	40' btoc			
					5,17' bto	_	TI	000	79-	183.1	8' btoc			
TIME	DUR			VOL.		DISCHRG. RATE	TEMP.	COND.	pН	SUSPEND. SOLIDS	TURBIDITY	REMARKS		
HRS/MIN	SURGE	BAIL	PUMP	GAL.	WATER	(gpm)		1 of Cir	-	₩g/L	NTU	Disposition of water, etc.)		
1400	X	X		0	/		-	-	-	-	-			
1505	*	+	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	6	178.38	-	-	-	-		-	over 800 NTU		
0830	1		X	48	178.39	20,5	16.96	0,480	7.28	0.312	34,8			
Move to MW	1-31			. 10	17 0. 21				7.00	D. 31 -	-			
Move to MW			X	48	178.39	20.6	_	_	_	_	_			
1128			X	/	-		17.83	0.461	7.70	0,300	800+			
1133			X	68	17840		16.25	0,451		0.296	50.5 7.8			
11.10				90	110 10		16.11	0. (21	1.51	0.610	7,0			
	1													
						1								
	-										,	, , , , , , , , , , , , , , , , , , , ,		
											7			
										/				
										(m)	m) /	-		
										V	9/10	/2020		
	OVO:	VD II	1		(PD)	09/10/202					//			
	SIGNATU	RE				07/10/202	0					ERM-WEST, INC. plp/1.00/12.95		

WELL NUMBER MW-31 LOCATION MW-3 DATE 09/10/2020 DEVELOPMENT CONTRACTOR EWE AIR TEMPERATURE ~86° F CLIENT BNSF, Husky CASING VOLUME (GAL.) 1.16 PROJECT NAME/NUMBER Black Tank 2020 Well Install DEVELOPMENT METHOD Surge/bail, pump FIELD SUPERVISOR MC TOTAL GALS, REMOVED 35 WELL CONSTRUCTION DATA (Y/N) Y DEPTH TO WATER AT START 176.62 5toc DEPTH TO WATER AT END 176.65' 540C TO @ start - 183,32' btoc TO @ End- 183,32' btoc pH TIME DURATION (MIN) VOL. DEPTH DISCHRG. COND. TEMP. SUSPEND. TURBIDITY REMARKS TO RATE SOLIDS HRS/MIN SURGE BAIL PUMP GAL. WATER mg/L NTU Disposition of water, etc.) (gpm) 0950 176.62 1025 800+ 21.0 1030 1036 18.54 0.557 0.356 BODT 16.90 0.307 185 1041 0.472 7.64 1045 16.37 0.431 7.72 51.8 15,280 7.75 19.9 1053 16.70 0.419 0.272 11101 35 176.65 16.41 0.407 7.72 0.265 9.6 9/10/2020 SIGNATURE ERM-WEST, INC. plp/1.00/12.95

WELL DEVELOPMENT/PURGE FORM

30

HRS/MIN SURGE BAIL PUMP GAL. WATER (gpm) -		EWE											WELL NUMBER
174,33 150 DEPTH TO WATER AT END 174,33	CASING VOLUME (GAL) ~ 1	ımp	Surge/bail, pu)	NT METHOI	DEVELOPME	_	Well Install	nk 2020 V	Black Ta		UMBER	PROJECT NAME/N
174,33 150 DEPTH TO WATER AT END 174,33	TOTAL GALS, REMOVED ~55		MC		VISOR	FIELD SUPER			Y		//N)	TION DATA (Y	WELL CONSTRUCT
TIME DURATION (MIN) VOL. DEPTH DISCHRG. TEMP. COND. pH SUSPEND. TURE SOLIDS HRS/MIN SURGE BAIL PUMP GAL. WATER (gpm) — - mg/L 1445 X X D 178.33 / mg/L 1450 5	toc	37 btoc	174	D	ATER AT EN	DEPTH TO W		s' bloc	74.33	1.		AT START	DEPTH TO WATER
HRS/MIN SURGE BAIL PUMP GAL. WATER (gpm) -			- NC	29	OPE	75		V C	- 1	Start	@	TD	
1445 X X O 178.33 1450 1452 PUMP MALFUNCTION. 1605 X S / ~1 1610 X / / 18.77 0.305 7.84 0.188 = 1610 1616 X / / 16.28 0.223 9.58 0.145 = 1622 1622 X / / 15.0/ 0.207 9.35 0.135 4	Y REMARKS	TURBIDITY		pH	COND.	ТЕМР.			VOL.	MIN)	ATION (DUR	TIME
1452 PUMP MALFUNCTION	Disposition of water, etc.)	NTU	mg/L		-		(gpm)	WATER	GAL.	PUMP	BAIL	SURGE	HRS/MIN
1452 PUMP MALFUNCTION 1605 1605 X S / ~1 18,77 0,305 9.84 0.198 = 1616 X / / 16.28 0.223 9.58 0.198 = 1612 X / / 15.0/ 0.207 9.35 0.135 4 1544 X / / 15.6/ 0.208 9.22 0.135 3				/	/	/	/	178.33			X	X	
1605				-									
1611				-			-	N			P M	PUM	
1616							~1	/	5				
1622 × 1 / 15.01 0.207 9.35 0.135 4		794						/	-				
1644 / / / 15.61 0.208 9.22 0,135 3		26,2						-	/,	_		-	
15.61 0.21 9.18 0.134 6 × 55 174.37 16.11 0.21 9.18 0.134	_	44,3						-	,				
7 38 177.37		34.0						174 17	55				11.00
		0,6	0.137	7.18	0.611	10.11		177,24	20				(021)
										1	9		
		, ,					10						
				1									
	1												
	2/2/2	MA										-	
	9/21/2020	N. P.											

NC- Not collected. TD is 7200', only have 200' water level tape

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

BORING LOGS

www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021

PAGE 1 OF 5

	CLIEN	NT BNSF	and	Husky	/ Oil			PROJECT NAME Black Tank Site		
	PROJ	ECT NUM	IBER	0175	5844			PROJECT LOCATION Spokane, WA		
	DATE	STARTE	D 9/	11/20		COMPLETED 9/	/29/20	GROUND ELEVATION 2038.579 feet	HOLE SIZE	6 inches
	CONT	RACTOR	Env	rironm	ental W	est		GROUND WATER LEVELS:		
	EQUII	PMENT _	Sonic					$\overline{igspace}$ at time of drilling $\underline{\ \ \ }$ 166.00 ft /	Elev 1872.	.58 ft
	LOGG	ED BY	Matt C	Crande	ell	CHECKED BY _	Dave Edwards	AT END OF DRILLING		
	NOTE	S Pre-cl	leared	to 8 f	eet bgs			AFTER DRILLING		
ł		Z	. 0							
NK 2016-2020.GPJ	O DEPTH (ft)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG			AL DESCRIPTION	Elevation (ft)	WELL DIAGRAM Steel well monument
)20\10 - OCTOBER\BNSF BLACK TA	 - 5	VAC CLEARANCE				Gray sandy fine ~12 inches. Fil	e-coarse GRAVE II.	L with subrounded cobbles and boulders to	2030.6	Cement Seal
H DESKTOP ITEMS\20	 - 10 			GW		3.5 inches. Loc 2.0	ose, moist.	coarse sand and subrounded cobbles to coarse sand and subrounded cobbles to coarse, subrounded.	2026.6	
TOP_CURRENT MONT	 15 	8-18	100			Loose, moist.				2-inch Blank Carbon Steel Casing
0 16:20 - C:\USERS\MATT.CRANDELL\DESK	20 25	18-28	70	SW		28.0			2010.6	
GENERAL BH / TP / WELL - GINT STD US GDT - 12/8/20 16:20 - C. (USERS/MATT, CRANDELL/DESKTOP] CHRENT MONTH DESKTOP ITEMS/2020/10 - OCTOBER/BNSF BLACK TANK 2016-2020. GPJ 40-2020.	30 - 35 	28-38	20			Coarse GRAVE sand. Poor rec	covery. Loose, m		2000.6	
GENERA	40			GW		Gray fine-coars 3.5 inches. Loc		coarse sand and subrounded cobbles to		

PAGE 2 OF 5

1	NT BNS					PROJECT LOCATION Spokane WA			
1	ECT NUI				COMPLETED 0/00/00	PROJECT LOCATION Spokane, WA	101 E 01	7E 6 inches	
	STARTE						1ULE SIZ	b inches	
1					/est		Class 407	0.50.4	
1	PMENT _				CUECKED BY Davis Edwards				
	S <u>Pre-c</u>				CHECKED BY _Dave Edwards				
NOTE		Teared	1 10 6 1	eet bgs		AFTER DRILLING			
0 (#) OEPTH	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		RIAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM	
45	38-48	40	GW		Gray fine-coarse GRAVEL with 3.5 inches. Loose, moist. <i>(con</i> No cobbles at 44 feet bgs.	coarse sand and subrounded cobbles to tinued)			
55	48-58	100	SW		Gray gravelly fine-very coarse toose, moist.	SAND. Gravel is fine-coarse, subrounded.	1988.6	Slough/cave-in of formation	
60	58-68	50	GW		Gray fine-coarse GRAVEL with	SAND with subrounded cobbles to 3.5 subrounded. Loose, wet. Moisture from drill	1975.6		
			sw		Gray fine-coarse GRAVEL with	coarse sand. Loose, moist.	1970.6		
70	- 68-78	85	GW	7	3.0 Gray grayelly fine-very coarse	SAND with subrounded cobbles to 4 inches.	1965.6		
75 			sw		Gravel is fine-coarse, subround	ded. Loose, moist.			
80				· · · · · · · · · · · · · · · · · · ·		Continued Next Page)	<u> </u>		

PAGE 3 OF 5

CLII	ENT BNS	F and	Husky	Oil		PROJECT NAME Black Tank Site		
PRO	JECT NU	MBER	0175	5844		PROJECT LOCATION Spokane, WA		
DAT	E START	ED 9/	11/20		COMPLETED 9/29/20	GROUND ELEVATION 2038.579 feet	HOLE S	IZE 6 inches
CON	NTRACTO	R Env	/ironm	ental Wes	st	GROUND WATER LEVELS:		
EQL	JIPMENT	Sonic				$\overline{igspace}$ at time of drilling $\underline{\ \ \ }$ 166.00 ft /	Elev 18	72.58 ft
LOG	GED BY	Matt 0	Crande	ell	CHECKED BY _Dave Edwards	AT END OF DRILLING		
NOT	TES Pre-	cleared	to 8 f	eet bgs		AFTER DRILLING		
	Z							
ANK 2016-2020.GPJ		RECOVERY %	U.S.C.S.	GRAPHIC LOG		IAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
020/10 - OCTOBERIBNSF BLACK TA	- - - 78-88	100	SW	88.	Gravel is fine-coarse, subround	SAND with subrounded cobbles to 4 inches. ed. Loose, moist. (continued)	1950.6	
ONTH DESKTOP ITEMS/2(_ _ _ _ 88-98	70	GW	93.	Gray fine-coarse GRAVEL with coarse sand. Loose, moist. Granitic boulder from 93 to 94.5 feet bgs. Hard drilling.			
§ - - -	+			94.	.5		1944.1	
95 84 95	_				Gray fine-coarse GRAVEL with			
ä			GW					
GENERAL BH / TP / WELL - GINT STD US.GDT - 12/8/20 16:20 - C.:USERS/MATT. CRANDELL/DESKTOP CURRENT MONTH DESKTOP\ TEMS\;2020.0710 - OCTOBER\;BNSF BLACK TANK 2016-2020.6PJ Contact Contact	98-118	90	SW	98.		y coarse SAND, trace subrounded cobbles	1940.6	- Bentonite Grout Seal
)			``				



	CLIEN	IT BNSF	= and	Husky	Oil		PROJECT NAME Black Tank Site		
		ECT NUM							
		STARTE				COMPLETED 9/29/20		HOLE S	IZE 6 inches
		RACTOR					-		
		PMENT					\subseteq AT TIME OF DRILLING <u>166.00 ft /</u>	Elev 18	72.58 ft
	LOGG	ED BY	Matt C	Crande	ell	CHECKED BY Dave Edwards			
		S Pre-c					AFTER DRILLING		
		Z	_						
NK 2016-2020.GPJ	(f) (f)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		RIAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
OP ITEMS\2020\10 - OCTOBER\BNSF BLACK TA	125	118-138	100	sw		to 5 inches. Loose, moist. (co	e SAND with fine subrounded gravel. Loose.	1914.6	
SKTOP_CURRENT MONTH DESKT	SP SP 135					138.0 Grayish brown fine-coarse SA	ND trace silt. Loose, moist	1900.6	
L/DES	 140			SW		140.0	ND, trace sit. Loose, moist.	1808 6	
GENERAL BH / TP / WELL - GINT STD US.GDT - 12/8/20 16:20 - C:\USERS\MATT.CRANDELL\DESKTOP_CURRENT MONTH DESKTOP ITEMS\2020:0110 - OCTOBER\BNSF BLACK TANK 2016-2020.GPJ	145	5 - - - - - - - - - - - - - - - - - - -		SP SP		Grayish brown medium-coarse wet. Moisture from drill head so the same standard grain size change to fine the same standard grain brown fine-coarse SA grayish brown fine-coarse SA grayish brown fine-coarse SA	e-medium. ND, trace silt. Loose, moist. e SAND with fine subrounded gravel. Loose,	1898.6 1886.6	
155 M M M M M M M M M						Wet. Moistare nom dim nead			

PAGE 5 OF 5

CLIENT BNSF and Husky Oil PROJECT NAME Black Tank Site												
PROJ	ECT NUI	MBER	017	5844		PROJECT LOCATION Spokane, WA						
DATE	STARTE	D 9/	11/20		COMPLETED 9/29/20	GROUND ELEVATION 2038.579 feet	HOLE SIZE 6 inc	hes				
CONT	TRACTOR	R Env	rironm	ental W	/est	GROUND WATER LEVELS:						
EQUI	PMENT	Sonic				$\sqrt{2}$ AT TIME OF DRILLING 166.00 ft /	Elev 1872.58 ft					
LOGG	SED BY	Matt 0	Crando	ell	CHECKED BY _Dave Edwards	AT END OF DRILLING						
	S Pre-c					AFTER DRILLING						
	Z											
NK 2016-2020.GPJ 90 (ft)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG	MATER	IAL DESCRIPTION	Elevation (ft)	L DIAGRAM				
4			SP	1	161.0 Moderate black LNAPL staining	and odor. Top of smear zone.	1877.6					
Black silty fine SAND with moderate LNAPL staining and odor. Stiff, wet. SM 165 Black fine SAND with silt. Very heavy LNAPL staining and odor. Slightly												
7-01/02	Black fine SAND with silt. Very heavy LNAPL staining and odor. Slightly stiff, wet. Top of current LNAPL surface.											
170 H H H H H H H H H H H H H H H H H H H	158-178	90	SM	1	LNAPL staining and odor. 174.0 Black fine SAND with silt. Very	Black silty fine sand with greenish gray laminated silt lenses. Moderate LNAPL staining and odor. 1864.6 Black fine SAND with silt. Very heavy LNAPL staining and odor. Slightly 1863.6						
Ϋ́Υ _			ML		<u>175.5</u> \ stiff, wet.	Heavy LNAPL staining and odor.	1863.1					
3[]			SP			bunded gravel. Heavy LNAPL staining and	1					
180				1	178.0 odor. Loose, wet.	0 odor. Loose, wet. 1860.6 Black fine-coarse SAND with fine subrounded gravel. Heavy LNAPL						
MAII.CKANDEL					LNAPL staining ceases, odor re	mains at 181 feet bgs.						
28 185 185 - 185 		400	SW		LNAPL odor ceases at approxir	nately 185 feet bgs.		- Bentonite Pellet Seal				
17/8/7	178-198	100										
190	-				· ·	or replaced because difficulti						
195 195 195 195 195					seal from 3 to 98 feet be compliant with the well re	encountered during well construction resulted in the well seal from 3 to 98 feet below ground surface being noncompliant with the well regulations. A log for the repaired or replaced well will be provided when the work is completed.						
NEKAL E			SP		Dark gray fine SAND with silt, tr moist.	ace fine subrounded gravel. Very hard,	1840.6	► Slough/cave-in of formation				
В					Bottom of	borehole at 198.0 feet.						

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3547-0359	IT DNO			. 0:1		PROJECT NAME OF L.T. L.C.				
1							PROJECT NAME Black Tank Site PROJECT LOCATION Spokane, WA			
1	ECT NUI				COMPLETED 8/21/20	-	ארו ב פי	7E 6 inches		
1							IULE SI	LE 0 mories		
1					est		Elov 100	SA 25 ft		
1	PMENT _				CHECKED BY Davis Educands					
1					CHECKED BY Dave Edwards					
NOTE	S Pre-c	cleared	to 8 1	eet bgs		AFTER DRILLING				
(t) OEPTH (t)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		RIAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM		
 - 45	38-48	85	GW		1.0	parse GRAVEL. Loose, moist. (continued) arse SAND, trace silt. Gravel is fine-coarse.	1995.8			
50	48-58	80	GW	5:	Medium brown sandy fine to co	arse SAND, trace subrounded cobbles to 4 Loose, moist.	1991.8 1986.8			
65	58-68	100		6	3.0	ine-coarse GRAVEL. Loose, moist.	1971.8			
70 75 	68-78	40	GW	7/	medium-coarse sand. Loose, i	VEL and COBBLES to 5 inches, with moist. arse SAND, trace silt. Gravel is fine-coarse.	1961.8			
80	-		sw		Loose, moist.	arse orino, trace sitt. Graver is illie-coarse.				



CLIEN	NT BNS	F and	Husky	/ Oil			PROJECT NAME Black Tank Site			
PROJ	ECT NUM	IBER	0175	5844			PROJECT LOCATION Spokane, WA			
DATE	STARTE	D 8/	19/20			COMPLETED <u>8/21/20</u>	GROUND ELEVATION 2039.845 feet	HOLE SI	IZE 6 inches	
CONT	RACTOR	R Env	/ironm	ental \	Vest		GROUND WATER LEVELS:			
EQUI	PMENT	Sonic					$\sqrt{2}$ AT TIME OF DRILLING _175.50 ft /	Elev 186	64.35 ft	
LOGG	ED BY	Matt C	Crande	ell		CHECKED BY Dave Edwards				
	S Pre-c						AFTER DRILLING			
							·	1		
© DEPTH (ft)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		MATER	RIAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM	
 - 85	78-88	100	SW SM SW ML		84.0	Brown SILT lens. Medium brown gravelly fine-coal Loose, moist.	arse SAND, trace silt. Gravel is fine-coarse. arse SAND, trace silt. Gravel is fine-coarse.	1958.8 1958.3 1955.8 1955.3		
90	Grayish brown fine-coarse G trace silt. Higher percentage of SILT a						AVEL with subrounded cobbles to 4 inches, 0.5 feet bgs, possible small silt lens.	1941.8		
-			GP	000	98.0	Multi-colored fine GRAVEL, tra	ce coarse gravel.		_	
100	†			\ <u>\</u>	99.0	Brown coarse SAND.		1940.8		
100	1		SP		101.0			1938.8		
			N 41			Brown laminated SILT. Moist,	stiff.			
[98-108	100	ML		103.0			1936.8		
105	90-100	100	SP		108.0	Brown very coarse SAND. Sand grain size decreases to n sequence from 98 to 103 feet b	nedium-coarse. Overall fining downwards gs.	1931.8	ш	
110			GP		100.0	Grayish brown sandy fine GRA from drill head water.	VEL, sand is coarse. Loose, wet. Moisture	1931.8	ш	
<u>-</u>	108-118	80			113.0	D	ODAVISIA I	1926.8		
<u>-</u>	-		6	0]	Brown tine-coarse SAND and fi	ne-coarse GRAVEL. Loose, moist.			
115 SW (\$\cdot\)										
} -	-				116.0	Gray gravelly fine-coarse SANI) trace silt. Loose, dry	1923.8		
120			sw			Gray gravery line-warse SAINL	2, 11406 SIII. 120056, UI Y.		ш	



ENI BN2	F and	Husky	/ Oil		PROJECT NAME Black Tank Site			
JECT NU	MBER	0175	5844		PROJECT LOCATION Spokane, WA			
E STARTE	ED 8/	19/20		COMPLETED _8/21/20	GROUND ELEVATION 2039.845 feet	HOLE S	IZE 6 inches	
ITRACTO	R Env	/ironm	ental V	West	GROUND WATER LEVELS:			
					∇ AT TIME OF DRILLING 175.50 ft	/ Elev 18	64.35 ft	
_			ell	CHECKED BY Dave Edwards				
	1	1						
SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION Grav gravelly fine-coarse SAND, trace silt. Loose, dry. (continued)			
118-128	85	SW		Gray gravelly fine-coarse SANI	D, trace silt. Loose, dry. (continued)			
_ _ _ 128-138	100	ML			ine gravel. Moist, loose.	1905.8 1905.3		
_ _ _ 138-148	100	sw					- Bentonite	
_ _ _ 148-158	100	SM		Brown fine-medium SAND with	silt. Loose, moist.	1886.8 1886.3 1881.8	Pellet Seal	
	DJECT NUI TE STARTE NTRACTOR JIPMENT GGED BY GES Pre-CO 118-128	DJECT NUMBER TE STARTED 8/ NTRACTOR Env DJIPMENT Sonic GGED BY Matt C TES Pre-cleared - 118-128 85 - 128-138 100 - 138-148 100 - 148-158 100	SECT NUMBER	NTRACTOR Environmental Name of the process of the p	STARTED 8/19/20 COMPLETED 8/21/20	PROJECT LOCATION Spokane, WA Spokane,	PROJECT LOCATION Spokane, WA	



	827-0391										
	CLIEN	NT BNS	F and	Husky	/ Oil		PROJECT NAME Black Tank Site				
	PROJ	ECT NU	MBER	017	5844		PROJECT LOCATION Spokane, WA				
	DATE	STARTE	D 8/	19/20		COMPLETED 8/21/20	GROUND ELEVATION 2039.845 feet I	HOLE SI	IZE 6	inch	nes
		RACTOR									
		PMENT					\triangle AT TIME OF DRILLING _175.50 ft/	Flev 186	34 35	ft	
		SED BY			all.	CHECKED BY _ Dave Edwards					
	NOTE	S Pre-c	learec	1 10 6 1	eet bg	5	AFTER DRILLING				
NK 2016-2020.GPJ	DEPTH (ft)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		RIAL DESCRIPTION	Elevation (ft)	V	VELI	_ DIAGRAM
NDELLIDESKTOP_CURRENT MONTH DESKTOP ITEMS\2020\10 - OCTOBER\BNSF BLACK TANK 2016-2020.GPJ	 165	158-168	100	SW		Brown fine-coarse SAND, trace 165.5 166.0 Brown medium SAND Medium brown SILT. Moist, s	ce silt. Loose, moist. (continued)	1874.3 1873.8		*	-20/40 Pioneer Silica Sand
OP ITEMS\2020\1	 170			ML SM		Coarsening downwards seque trace coarse sand. Wet, sligh smear zone.	ence to 168 feet bgs: Gray silty fine SAND, tly stiff. LNAPL odor, slight staining. Top of	1871.8			
ENT MONTH DESKTO	 175	168-182	100			staining and odor.	ND, trace silt. Wet, loose. Heavy LNAPL		4		- 0.010 Inch Slotted Well Screen
NDESKTOP_CURR	 			SW		Top of current LNAPL surface	ning, wet at 175.5 feet bgs. No gravel, no silt. 9 feet bgs, odor continues to bottom of boring				
DELL	180					at 182 feet bgs.	reet bgs, oddr continues to bottom or boning	<u>}</u>			
RAN						182.0		1857.80			- Slough/cave-in of formation
E.		ļ					of borehole at 182.0 feet.	1007.00	nosou	1070	or iornation
GENERAL BH / TP / WELL - GINT STD US.GDT - 12/8/20 16:20 - C:\USERS\MATT.CR/											

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CUI	ENT BNS	F and	Huek	, Oil		PROJECT NAME Black Tank Site		
- 1								
					COMPLETED 9/2/20			7F 6 inches
1					st		<u> </u>	<u> </u>
- 1	JIPMENT			1011tai 1100		$\sqrt{2}$ AT TIME OF DRILLING 173.5	0 ft / Flev 186	6 41 ft
- 1	_			-II	CHECKED BY Dave Edwards			
	TES Pre-c				ONEONED DI <u>Dave Edwards</u>	AFTER DRILLING		
	_	l	1001	l l		74 12K 3K42LIKO		
NK 2016-2020.GPJ A DEPTH (#)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG	MATER	IAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
20/10 - OCTOBER\BNSF BLACK TAI	- - - 38-48 - -	50	GW	48.0	cobbles to 4 inches, trace silt. I	e GRAVEL with very coarse sand and Moist, loose. <i>(continued)</i>	1991.9	
TOP_CURRENT MONTH DESKTOP ITEMS\\20	- - - 48-58	0		58.0	No recovery. Driller reports a b sample; lithology loose enough down the entire 10 feet of the same sample.		1981.9	
8/20 16:20 - C:\USERS\MATT.CRANDELL\DESK\	58-68	70			Brownish-gray sandy fine-coars cobbles to 4 inches, trace silt. I	e GRAVEL with very coarse sand and Moist, loose.		
GENERAL BH / TP / WELL - GINT STD US.GDT - 12/8/20 16:20 - C.:USERS/MATT.CRANDELL/DESKTOP_CURRENT MONTH DESKTOP_TEMS\;2020.020.010 - OCTOBER\;BNSF BLACK TANK 2016-2020.6PJ P DEPTH	- - - - - - - - -	50	GW					



8	CKIVI	p		20 102 00				
CL	IENT BNS	F and	Husky	Oil		PROJECT NAME Black Tank Site		
PR	OJECT NUI	MBER	0175	844		PROJECT LOCATION Spokane, WA		
DA	TE STARTE	D _8/3	31/20		COMPLETED 9/2/20	GROUND ELEVATION 2039.908 feet	HOLE S	ZE 6 inches
СО	NTRACTOR	R Env	/ironm	ental West		GROUND WATER LEVELS:		
EQ	UIPMENT _	Sonic				$\sqrt{2}$ AT TIME OF DRILLING 173.50 ft.	/ Elev 186	66.41 ft
LO	GGED BY	Matt 0	Crande	ell	CHECKED BY Dave Edwards	AT END OF DRILLING		
NO	TES Pre-c	leared	to 9 f	eet bgs		AFTER DRILLING		
	Z							
E E	(ff) SAMPLE IDENTIFICATION	RY %	ν.	일			Elevation (ft)	
6-2020.GPJ DEPTH		RECOVERY	U.S.C.S.	GRAPHIC LOG	MATER	IAL DESCRIPTION	atio	WELL DIAGRAM
016-2 D	\& Fi	ECC		R _			Elev	
¥ 80) 🖺	~						
Χ⊢				17 × 1	Brownish-gray sandy fine-coars cobbles to 4 inches, trace silt. I	e GRAVEL with very coarse sand and		
BLAC					cobbles to 4 illeries, trace sitt.	worst, roose. (continueu)		
RNS -	78-88	80						
計。 。								
85 85	2							
ŏ-	1							
120/1	1							
MS/2(GW					
<u> </u>)							
ŽΙ-	_							
DES	-							
Ĭ-	88-98	70						
ĭ 5 95	, -							
AN S	_							
5				97.0			1942.9	
한			SW	98.0	Grayish brown gravelly fine-coa Moist, loose.	rse SAND. Gravel is fine, subrounded.	1941.9	
DEST	-				Brownish-gray sandy fine-coars	e GRAVEL with very coarse sand and	_	
10	0				cobbles to 4 inches, trace silt. I	Moist, loose.		
RANI -	-			XX				
5	+							
S/MA	98-108	70	GW					
H 10	5							
6:20								
8/20	+			108.0		e subrounded gravel, trace medium sand.	1931.9	
12/14			GP	000	Wet, loose. Moisture from drill		1000.0	
11	<u> </u>			110.0	Brownish-gray sandy fine-coars	e GRAVEL with very coarse sand and	1929.9	
SN a	1				cobbles to 4 inches, trace silt. I	Moist, loose.		
TST	108-118	80	GW					
5	100-110	00	GVV					
<u> </u>	5							
<u></u> }_	4			116.0		rse SAND. Gravel is fine, subrounded.	1923.9	
₩	-				Moist, loose.	ise Sand. Graver is line, subfourtided.		
- RAL	+		SW		Trace subrounded cobbles to 5	inches from 118 to 128 feet bgs.		
GENERAL BH / TP / WELL - GINT STD US.GDT - 12/8/20 16:20 - C.: USERS/MATT. CRANDELL/DESKTOP! CURRENT MONTH DESKTOP ITEMS/2020/10 - OCTOBER/BNSF BLACK TANK 2016-2020.GPJ 1	0					Ç		



CLIE	NT BNS	F and	Husky	/ Oil		PROJECT NAME Black Tank Site		
PRO	JECT NUI	MBER	0175	5844		PROJECT LOCATION Spokane, WA		
DAT	E STARTE	D _8/	31/20		COMPLETED 9/2/20	GROUND ELEVATION 2039.908 feet	HOLE SIZ	ZE 6 inches
CON	TRACTOR	R Env	/ironm	ental \	West	_ GROUND WATER LEVELS:		
EQU	IPMENT	Sonic				$\underline{\nabla}$ AT TIME OF DRILLING <u>173.50 ft</u>	/ Elev 186	6.41 ft
1	_			ell	CHECKED BY Dave Edwards			
	ES Pre-c					AFTER DRILLING		
	Z							
NK 2016-2020.GPJ 150 170 170 170 170 170	<u>,</u> E	RECOVERY %	U.S.C.S.	GRAPHIC LOG	MATE	RIAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
125 - 125	_ _ _ 118-128	80	SW		Brownish gray gravelly fine-co Moist, loose. (continued)	arse SAND. Gravel is fine, subrounded.	1911.9	
130 130	- - 128-138	80	SP		Brown fine-medium SAND. M	oist, loose		
135 24 25	_		SM		Brown silty fine SAND.		1904.9	
5 <u>5</u> -	-		SP		137.0 138.0 Brown fine-medium SAND. M	oist loose	1902.9	
100 - C:005EKS/MA I : CKANDELLIDESKI	- - 138-148	90	SW		Brown gravelly fine-coarse SA Moist, loose.	ND, trace fine subrounded gravel and silt.	1901.9	- Bentonite
150 - 150 -		90	SP		Brown fine-medium SAND, tra	ce silt. Moist, loose.	1890.9	Pellet Seal



824.503865388468466								
CLIENT _E					·			
PROJECT					PROJECT LOCATION Spokane, WA			
DATE STA						HOLE S	SIZE 6 inch	es
1			nental \	Vest				
EQUIPMEN					✓ AT TIME OF DRILLING 173.50 ft /			
				CHECKED BY Dave Edwards				
NOTES _P	re-cleared	to 9	feet bg	S	AFTER DRILLING			
DEPTH (ft) (T) SAMPLE SAMPLE (DENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG	MATER	IIAL DESCRIPTION	Elevation (ft)	WELL	. DIAGRAM
165	168 90	SP		Brown fine-medium SAND, trac Faint LNAPL odor at 160 feet b Moderate LNAPL staining and o	te silt. Moist, loose. <i>(continued)</i> gs, odor increases to 167 feet bgs.			-20/40 Pioneer Silica Sand
170	182 100	ML		170.5 171.0 Bluish gray SILT with heavy bla of current LNAPL surface.	ick LNAPL staining on underside of silt. Tope	1869.4 1868.9		-0.010 Inch Slotted Well Screen
180				bgs. 182.0	odor, color change to dark gray at 179 feet borehole at 182.0 feet.	1857.9		- Slough/cave-in of formation

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				125-462-8					
CLI	ENT BNS	F and	Husky	/ Oil		PROJECT NAME Black Tank Site			
PRO	DJECT NUM	MBER	017	5844		PROJECT LOCATION Spokane, WA			_
DA	TE STARTE	D 9/2	2/20		COMPLETED 9/8/20	GROUND ELEVATION 2038.21 feet	HOLE SI	ZE 6 inc	hes
co	NTRACTOR	R Env	/ironm	ental Wes	st	GROUND WATER LEVELS:			
EQI	JIPMENT _	Sonic				$\overline{2}$ AT TIME OF DRILLING $\underline{163.50}$ ft /	Elev 187	4.71 ft	
LO	GGED BY	Matt 0	Crande	ell	CHECKED BY Dave Edwards	AT END OF DRILLING			
	TES Pre-c					AFTER DRILLING			
-			1						
O DEPTH	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		IAL DESCRIPTION	Elevation (ft)	WEL	L DIAGRAM Steel well monument
MO-OCTOBER/BNSF BLACK TAI	VAC CLEARANCE				Brown fine-coarse SAND, trace	subangular fine gravel and silt. Fill.			⋖ Cement Seal
GENERAL BH/TP/WELL -GINT STD US.GDT - 128/20 16:20 - C:\USERS\MATT.CRANDELL\DESKTOP\\ CURRENT MONTH DESKTOP\\ COCTOBER\BNSF BLACK TANK 2016-2020.GFJ\\ COCTOBER\BNSF BLACK TANK 2016-2020.FT\\ COCTOBER\BNSF BLACK TANK 2016-2020.FT\	- - 8-18	50	GW	8.0	subrounded cobbles to 3 inches	nded GRAVEL with fine-medium sand with s. Wet, loose.	2030.2	4	─ 2-inch Blank PVC Casing
20 16:20 - C:\(\)\(\)\(\)\(\)\(\)\(\)\(\)\(\)\(\)\(\	- - 18-28	40	sw	25.0	Brown gravelly fine-very coarse	SAND. Gravel is fine, subrounded.	2013.2		- Bentonite Grout Seal
1/ TP / WELL - GINT STD US.GDT - 12/8/,	- - 28-38	50	GW	36.0 0	subrounded cobbles to 3 inches	nded GRAVEL with fine-medium sand with s. Wet, loose.	2002.2		
a a			GP	38.0			2000.2		
GENERA GENERA 40			GW		Light brown fine-coarse subrour subrounded cobbles to 3 inches	nded GRAVEL with fine-medium sand with s. Wet, loose.			

	PROJ DATE CONT EQUII	ECT NUM STARTE RACTOR PMENT	MBER ED 9/3 R Env	017 <u></u> 2/20 /ironm	5844 nental West	COMPLETED	9/8/20	PROJECT LOCATION Spokane, WA GROUND ELEVATION 2038.21 feet GROUND WATER LEVELS:	HOLE SIZ	I.71 ft
- 1		SED BY _ S _Pre-c				CHECKED BY	Dave Edwards	AT END OF DRILLING _ AFTER DRILLING		
ŀ		SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG			RIAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
- GINT STD US.GDT - 12/8/20 16:20 - C.\USERS\MATT.CRANDELL\DESKTOP_CURRENT MONTH DESKTOP ITEMS\2020\10 - OCTOBER\BNSF BLACK TANK 2016-2020.GFJ	40 45 50 55 60 65 65 65 65	38-48 - 38-48 - 48-58 - 58-68	50 60	GW		subrounded	cobbles to 3 inch	unded GRAVEL with fine-medium sand with es. Wet, loose. (continued)		
GENERAL BH / TP / WELL - GINT STD US.GDT - 12/8.	70 75 80	68-78	90							



CLIE	DNC	Eand	الماما	, Oil			DDO IECT NAME Plack Took Site		
1	JECT NU								
						COMPLETED 0/8/20	GROUND ELEVATION 2038.21 feet	HOI E SI	7F 6 inches
1						3/3/20		I IOLL OI	<u> </u>
1	PMENT _			Cittai VV	COL		∇ AT TIME OF DRILLING 163.50 ft /	Flev 187	74 71 ft
1	_			الد		CHECKED BY Dave Edwards			
1						Dave Edwards	AFTER DRILLING		
NOTE		leared	1001	l l			AI TER BRIEFING		
OBPTH (ft)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG			IAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
85	78-88	70	GW		34.0 35.5	Dark grayish brown gravelly fine subrounded. Moist, loose.	e-coarse SAND, trace silt. Gravel is fine, GRAVEL with fine-medium sand with	1954.2 1952.7	
90 95	88-98	70	GW		94.0 95.0	Granitic boulder from 94 to 95 for Brownish gray fine-coarse SANI loose.	eet bgs. D with fine subrounded gravel. Moist,	1944.2 1943.2	
100 105	98-108	70	SW		08.0			1930.2	
<u> </u> -			GW			Medium gray fine-medium sand Wet, loose. Moisture from drill h	y subrounded GRAVEL. Sand is coarse. nead water.		
110	108-128	95	SW		10.0		D with fine subrounded gravel. Moist,	1928.2	
120	1								

0,4743,00				120 102				
1	NT BNS							
1	JECT NUM					PROJECT LOCATION Spokane, WA		
1	STARTE						HOLE SIZE	6 inches
1				ental We	st			
1	PMENT _					_ ∇ AT TIME OF DRILLING _163.50 f		
1					CHECKED BY Dave Edwards			
NOTE	S Pre-c	leared	to 8 1	eet bgs		AFTER DRILLING	ı	
(ft) DEPTH	SAMPLE	RECOVERY %	U.S.C.S.	GRAPHIC LOG		RIAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
	-		SW		Brownish gray fine-coarse SA loose. <i>(continued)</i> 3.0	ND with fine subrounded gravel. Moist,	1915 2	
125	- - - -		sw		$\frac{3.5}{4}$ 4 inch granitic boulder at 123	feet bgs. ND with fine subrounded gravel. Moist,	1915.2 1914.7	
130				12	9.0 Granitic boulder from 129 to 1	34 feet bgs. Very hard drilling.	1909.2	
				13	4.0 Light grayish brown fine-medi	IM SAND Moiet loose	1904.2	
135	- - 128-148	75			Light grayion blown line incar	am o, ivo. Moist, 1000c.		
140	-							
145			SW					
								- Bentonite
150	-							Pellet Seal
S	- - -				4.0			
155	-			15		AND with fine subrounded gravel. Moist,	1884.2	
<u> </u>	446.100	00	SP					
160	148-168	90						

8.773		_						
1	T BNS					PROJECT NAME Black Tank Site		
1	ECT NUM				001015	PROJECT LOCATION Spokane, WA		NITE 0: 1
1	STARTE					<u></u>	HOLE	SIZE 6 inches
1				ental V	Vest	_		
1	MENT _				OUEQUED DV D E L	✓ AT TIME OF DRILLING 163.50 ft /		
1					CHECKED BY _Dave Edwards			
NOTES	S Pre-c	leared	1081	eet bg	S	AFTER DRILLING		
(ft) (16)	SAMPLE IDENTIFICATION	RECOVERY %	U.S.C.S.	GRAPHIC LOG		IAL DESCRIPTION	Elevation (ft)	WELL DIAGRAM
 165			SP		loose. (continued) Moderate LNAPL odor and stain Heavy black LNAPL staining an surface at 163.5 feet bgs. Finin 165.5 166.5 Sand grain size change to fine a		1872. 1871.	7 :
-			SP		Black silty fine SAND with lamin and odor. Wet, moderately stiff	nated silt lenses. Heavy LNAPL staining	1870.2	1: 1-1: 1
			CM		Black fine SAND with heavy LN	APL staining and odor. Wet, loose.	1070.2	
170			SM		Black silty fine SAND with lamin and odor. Wet, moderately stiff	nated silt lenses. Heavy LNAPL staining	1868.2	2 :
175	168-180	100	SW		odor.	ne subrounded gravel. LNAPL staining and feet bgs, odor continues to total depth of	1858.	0.010 Inch Slotted Well Screen
						borehole at 180.0 feet.	,	

APPENDIX C RADIUS OF INFLUENCE TEST DATA

Figure C-1	Bioventing Pressure vs. Flow
Table C-1	Bioventing Pilot Test Vapor Data
Figure C-2	Biosparge Pressure vs. Flow
Table C-2	Sparging Pilot Test Vapor Data
Table C-3	Bioventing Pilot Test Oxygen Data
Table C-4	Bioventing Pilot Test Helium Data
Table C-5	Biosparing Pilot SF 6 Test Data
Figure C-3	Bioventing ROI O2 Results
Figure C-4	Bioventing ROI Helium Results
Figure C-5	Biosparging ROI SF ₆ Results
Figure C-6	Biosparging ROI Dissolved Oxygen Results at MW-31
Photo Loa	• • •

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ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

BIOVENTING TEST DATA

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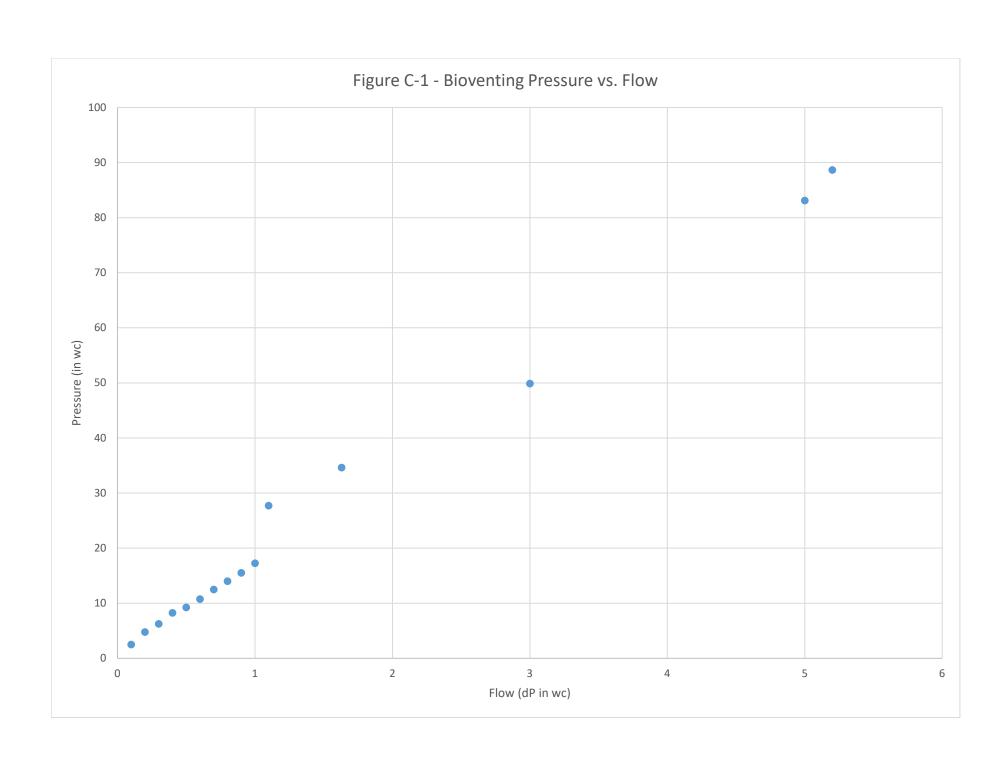


Table C-1
Bioventing Pilot Test Vapor Data
BNSF Black Tank
Spokane. WA

Spokane, WA				Distance	from Injec	tion (ft)											
				32	68	96	114	124									
Start time	Injection Pressure (psi)	Injection dP (in. w.c.)	Injection Helium Concentration (ppmv)	MW-5 Helium (ppmv)	MW-7 Helium (ppmv)	MW-9 Helium (ppmv)	MW-22R Helium (ppmv)	MW-23R Helium (ppmv)	MW-5 CH4 %	MW-7 CH4 %	MW-9 CH4 %	MW-22R CH4 %	MW-23R CH4 %	MW-5 CO2 %	MW-7 CO2 %	MW-9 CO2 %	MW-22R CO2 %
10/6/2020 15:50	0	0.1	9150	0	0	0	0	0	0.3	0.4	1.7	3	3.2	6.2	3.1	4.2	3.3
10/6/2020 17:11	0	0.46	9500	0	0	0	0	0	0.4	0.4	1.8	3	3	6	3	4.2	3
10/7/2020 9:32	1	1.1	9325	0	0	0	0	0	0.3	0.3	1.7	3.1	2	6.8	3.1	4.8	3.5
10/7/2020 11:51	1.25	1.63	6800	0	0	0	0	0	0.4	0.4	1.9	3.3	0.7	6.5	3	4.6	3.3
10/7/2020 13:22	1.5	3	12475	0	0	0	0	0	0.4	0.5	1.8	3.2	2.8	6.4	3	4.5	3.2
10/7/2020 16:03	1.5	3	8500	0	0	0	0	600	0.5	0.6	1.9	3.6	0.7	6.2	2.8	4.4	3.1
10/8/2020 9:32	2	3	9725	0	0	0	0	1625	0.3	0.3	1.7	2.7	0.6	6.7	3.1	4.8	3.4
10/8/2020 11:49	1.8	3	8725	0	0	0	0	1275	4.3	0.4	1.8	3	0.9	6.6	3	4.6	3.3
10/8/2020 14:45	1.8	3	0	0	1450	0	0	3950	0.4	3	1.8	2.8	0.8	6.4	3.1	4.5	3.1
10/8/2020 16:38	1.8	3	0	0	2800	0	0	8825	0.3	0.3	1.7	2.5	0.4	6.2	3	4.5	3.1
10/9/2020 9:11	1.8	3	0	0	3525	0	0	175	0.3	0.3	1.8	1	0.4	6.9	2.9	4.8	2.8
10/9/2020 14:08	1.8	3	0	0	3475	0	0	0	0.5	0.4	1.9	3	0.8	6.3	2.5	4.4	3.1
10/9/2020 15:32	1.8	3	0	0	2525	0	0	0	0.4	0.3	1.8	2.7	0.5	6.3	2.3	4.4	3.1
10/9/2020 16:55	1.8	3	0	0	2800	0	0	75	0.3	0.3	1.8	2.9	0.4	6.4	2.6	4.5	3.2
10/10/2020 9:00	1.8	3	0	0	875	0	0	125	0.2	0.3	1.7	2.7	0.5	6.5	2	4.6	3.3
10/10/2020 11:10	1.8	3	0	0	775	0	0	0	0.3	0.3	1.8	2.9	0.6	6.3	1.8	4.5	3.2
10/10/2020 19:30	3	5	0	0	150	0	0	25	0.3	0.3	1.7	1.8	0.3	6.8	1.6	4.8	3.2
10/11/2020 8:05	3.2	5.2	0	0	0	0	0	0	0.2	0.3	1.8	1.9	0.2	7.2	1.2	5.1	3.5
10/11/2020 12:05	3.2	5.2	0	0	0	0	0	75	0.2	0.2	1.8	1.5	0.2	7.1	1.1	4.9	3.3
10/11/2020 18:25	0	0	0	0	0	0	0	0	0.2	0.3	1.8	2.7	0.8	7	1.3	5	3.7
10/12/2020 10:40	0	0	0	0	100	0	0	0	0.1	0.3	1.7	1.9	0.7	6.7	1.4	4.7	3.3
10/12/2020 13:35	0	0	0	0	100	0	0	0	0.3	0.2	1.7	2.6	1.4	6.8	1.4	4.7	3.5
10/12/2020 16:35	0	0	0	0	150	0	0	0	0.4	0.3	1.8	2.4	1.5	6.9	1.4	4.8	3.5
10/13/2020 7:50	0	0	0	0	25	0	0	0	0.2	0.3	1.8	3.1	2.1	6.9	1.6	4.9	3.7
10/13/2020 13:00	0	0	0	0	0	0	0	0	0.2	0.2	1.8	3.1	2.2	7	1.4	4.8	3.7

Notes:

CH4 = Methane

CO2 = Carbon dioxide

in. w.c. = inches water column

L/min = Liters per minute

O2 = Oxygen

psi = Pounds per square inch

Table C-1
Bioventing Pilot Test Vapor Data
BNSF Black Tank
Spokane, WA

Start time	Injection Pressure (psi)	MW-23R CO2 %	MW-5 O2 %	MW-7 O2 %	MW-9 O2 %	MW-22R O2 %	MW-23R O2 %	MW-5 Flow (L/min)	MW-7 Flow (L/min)	MW-9 Flow (L/min)	MW-22R Flow (L/min)	MW-23R Flow (L/min)	MW-5 Pressure (in w.c.)	MW-7 Pressure (in w.c.)	MW-9 Pressure (in w.c.)	MW-22R Pressure (in w.c.)
10/6/2020 15:50	0	5.6	12.2	16.5	0	0.2	0.7	0	0	0	0	0	0	0	0	0
10/6/2020 17:11	0	5.6	12.4	16.7	0	0.5	1.8	0	0	0	0	0	0	0	0	0
10/7/2020 9:32	1	5.9	12.3	17.3	0	0.4	6.3	0	0	0	0	0	0	0	0	0
10/7/2020 11:51	1.25	4.2	12.1	16.8	0	0.6	13.8	0	0	0	0	0	0	0	0	0
10/7/2020 13:22	1.5	6.2	12.1	16.7	0	0.2	2.9	0	0	0	0	0	0	0	0	0
10/7/2020 16:03	1.5	3.4	12.2	16.9	0	0.2	16.3	0	0	0	0	0	0	0	0	0
10/8/2020 9:32	2	1.7	12.3	17.4	0	2.3	17.6	0	0	0	0	0	0	0	0	0
10/8/2020 11:49	1.8	2.3	11.9	17.3	0	1.2	14.7	0	0	0	0	0	0	0	0	0
10/8/2020 14:45	1.8	1.8	12	17.8	0	1.6	16.2	0	0	0	0	0	0	0	0	0
10/8/2020 16:38	1.8	0.7	12.5	18.7	0	3.3	19.6	0	0	0	0	0	0	0	0	0
10/9/2020 9:11	1.8	0.5	11.9	19	0	11.7	19.3	0	0	0	0	0	0	0	0	0
10/9/2020 14:08	1.8	1.4	12.1	19.6	0	1.7	16.1	0	0	0	0	0	0	0	0	0
10/9/2020 15:32	1.8	0.8	11.7	19.2	0	1.2	17.8	0	0	0	0	0	0	0	0	0
10/9/2020 16:55	1.8	0.5	12.1	19.7	0	0.5	19.3	0	0	0	0	0	0	0	0	0
10/10/2020 9:00	1.8	1	12	20.1	0	1.7	18.8	0	0	0	0	0	0	0	0	0
10/10/2020 11:10	1.8	0.9	12.2	20.3	0	1.4	18.9	0	0	0	0	0	0	0	0	0
10/10/2020 19:30	3	0.2	12.6	20.2	0	6.4	21.5	0	0	0	0	0	0	0	0	0
10/11/2020 8:05	3.2	0.2	12.6	20.8	0	5.8	21.6	0	0	0	0	0	0	0	0	0
10/11/2020 12:05	3.2	0.1	12	19.8	0	7.5	20.2	0	0	0	0	0	0	0	0	0
10/11/2020 18:25	0	1.5	12.3	20.7	0	0.7	15.1	0	0	0	0	0	0	0	0	0
10/12/2020 10:40	0	1.5	12.3	20.4	0	6	16	0	0	0	0	0	0	0	0	0
10/12/2020 13:35	0	3.6	12.5	20.4	0	2.4	8.5	0	0	0	0	0	0	0	0	0
10/12/2020 16:35	0	3.7	12.5	20.6	0	2.7	8.4	0	0	0	0	0	0	0	0	0
10/13/2020 7:50	0	6.1	12.3	20.7	0	0.2	1.4	0	0	0	0	0	0	0	0	0
10/13/2020 13:00	0	6.4	12.3	20.2	0	0.2	0.8	0	0	0	0	0	0	0	0	0

Notes:

CH4 = Methane

CO2 = Carbon dioxide

in. w.c. = inches water column

L/min = Liters per minute

O2 = Oxygen

psi = Pounds per square inch

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

BIOSPARGING TEST DATA

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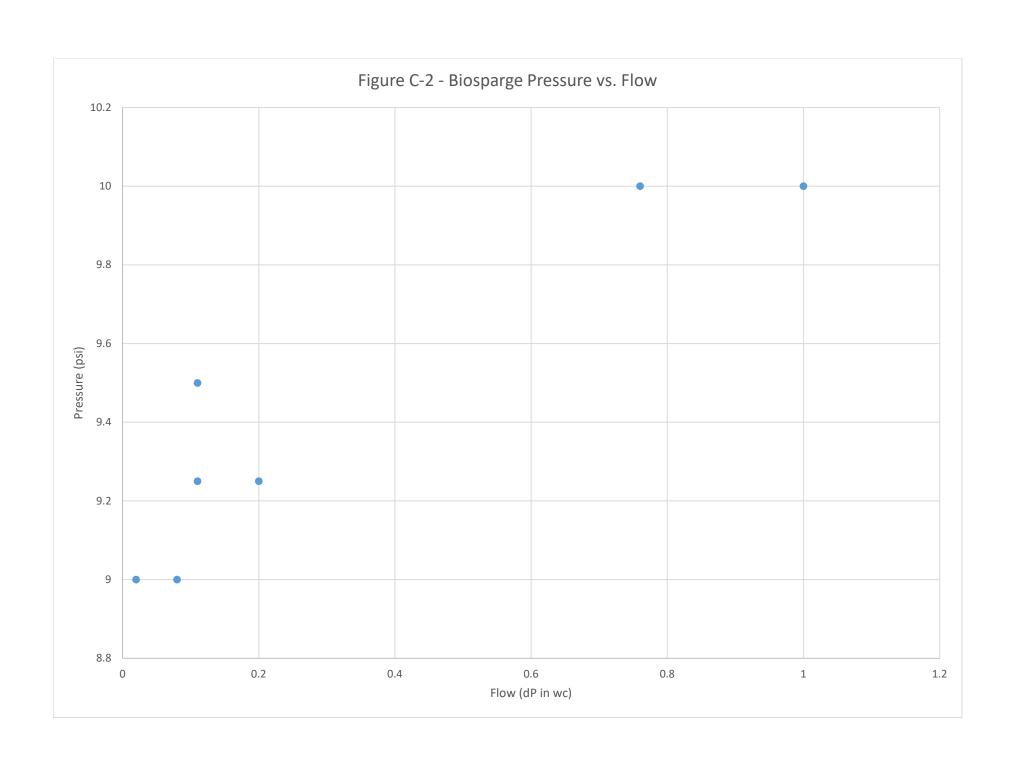


Table C-2
Sparging Pilot Test Vapor Data
BNSF Black Tank
Spokane, WA

Distance from Injection (ft)

•					32	59	133	11																				
Start time	Injection DP (in wc)	Injection He (ppm)	Droccuro	Injection SF6 (L/min)	MW-03 CH4 (%)	MW-04 CH4 (%)	MW-05 CH4 (%)	MW-31 CH4 (%)	MW-03 He (ppm)	MW-04 He (ppm)	MW-05 He (ppm)	MW-31 He (ppm)	MW-03 flow (L/min)	MW-04 flow (L/min)	MW-05 flow (L/min)	MW-31 flow (L/min)	MW-03 presure (in wc)	MW-04 presure (in wc)	MW-05 presure (in wc)	MW-31 pressure (psi)	MW-03 CO2 (%)	MW-04 CO2 (%)	MW-05 CO2 (%)	MW-31 CO2 (%)	MW-03 O2 (%)	MW-04 O2 (%)	MW-05 O2 (%)	MW-31 O2 (%)
10/16/2020 10:31	0	0	0	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	16.9	8.4	7	13.6	0	10.3	12.4	2.9
10/16/2020 12:58	0.02	9100	9	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	10	16.6	10.4	6.9	14.6	0	7.8	12.4	1.2
10/16/2020 14:13	0.08	10300	9	0	0.2	0.2	0.2	0.4	0	0	0	0	0	0	0	0	0	0	0	0	16.6	8.7	7	15.1	0	9.7	12.3	0.7
10/16/2020 15:19	0.2	10400	9.25	0	0.2	0.2	0.2	0.4	0	0	0	0	0	0	0	0	0	0	0	0	16.7	7.5	7	14.8	0	11.2	12.1	1.3
10/16/2020 16:31	0.76	10925	10	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	16.7	8.6	6.9	14	0	9.4	12.2	2.8
10/17/2020 8:46	0.11	8800	9.5	0.5	0.2	0.2	0.2	0.6	0	0	0	0	0	0	0	0	0	0	0	0	16.8	6.5	7	15.2	0	12.8	12.5	1.3
10/17/2020 10:28	0.11	12000	9.5	0.5	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	16.4	6.9	6.8	15.3	0	11.8	11.9	0.2
10/17/2020 12:50	0.11	12500	9.5	0.5	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	10	16.3	6	6.7	15.3	0	13	12.1	0.6
10/17/2020 17:16	0.11	10000	9.5	0.5	0.2	0.2	0.2	0.8	0	0	0	0	0	0	0	0	0	0	0	0	16.4	6.4	6.8	14.4	0	12.8	12.5	3
10/18/2020 9:00	0.11	8900	9.5	0.5	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	16.9	6.5	7	15.4	0	13	12.6	0.9
10/18/2020 12:22	0.11	11175	9.5	0.5	0.2	0.2	0.2	0.4	0	0	0	0	0	0	0	0	0	0	0	0	17	7.7	7.1	15.9	0	10.8	12.3	0.6
10/18/2020 13:54	0.11	11025	9.5	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	16.9	5.9	7	15.5	0	13.6	12.4	0.8
10/19/2020 8:32	0.11	9725	9.5	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	10	17.1	7.3	7.1	16	0	11.7	12.4	0.4
10/19/2020 9:36	0.13	0	9.25	0	0.2	0.2	0.2	0.4	0	0	0	0	0	0	0	0	0	0	0	0	16.4	7.6	6.8	15.2	0	10.8	12	0.3
10/19/2020 13:56	0.13	0	9.25	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	16.2	7.9	6.5	15	0	9.9	12.2	0.5
10/19/2020 16:34	0.13	0	9.25	0	0.2	0.2	0.2	0.4	0	0	0	0	0	0	0	0	0	0	0	0	16.3	7.6	6.7	15.1	0	10.4	12.9	0.5
10/20/2020 10:20	0.11	0	9.5	0	0.2	0.2	0.2	0.4	0	0	0	0	0	0	0	0	0	0	0	0	16.8	7.5	7	15.7	0	11.5	12.8	0.3
10/20/2020 12:33	0.11	0	9.25	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	16.3	8	6.7	15	0	10.3	12.3	0.4
10/21/2020 8:23	0	0	0	0	0.2	0.2	0.2	0.5	0	0	0	0	0	0	0	0	0	0	0	0	17.2	7.8	7.2	16.1	0	11.7	12.6	0.2

Notes:

CH4 = Methane

CO2 = Carbon dioxide

in. w.c. = inches water column

L/min = Liters per minute

O2 = Oxygen

psi = Pounds per square inch

Table C-3 Bioventing Pilot Test Oxygen Data Engineering Design Report BNSF Black Tank Spokane, WA

-				Distanc	e from Injec	ction (ft)		
			32	68	96	114	124	
Start time	Injection Pressure (psi)	Injection dP (in. w.c.)		MW-7 O2 %	MW-22R O2 %	MW-9 O2 %	MW-5 O2 %	Notes
10/6/2020 15:50	0	0.1	0.7	16.5	0.2	0	12.2	Sustained Test Start (110 cfm)
10/6/2020 17:11	0	0.46	1.8	16.7	0.5	0	12.4	
10/7/2020 9:32	1	1.1	6.3	17.3	0.4	0	12.3	
10/7/2020 11:51	1.25	1.63	13.8	16.8	0.6	0	12.1	
10/7/2020 13:22	1.5	3	2.9	16.7	0.2	0	12.1	
10/7/2020 16:03	1.5	3	16.3	16.9	0.2	0	12.2	~ 1 day after Sustained Test Start
10/8/2020 9:32	2	3	17.6	17.4	2.3	0	12.3	
10/8/2020 11:49	1.8	3	14.7	17.3	1.2	0	11.9	
10/8/2020 14:45	1.8	3	16.2	17.8	1.6	0	12	
10/8/2020 16:38	1.8	3	19.6	18.7	3.3	0	12.5	~ 2 days after Sustained Test Start
10/9/2020 9:11	1.8	3	19.3	19	11.7	0	11.9	
10/9/2020 14:08	1.8	3	16.1	19.6	1.7	0	12.1	
10/9/2020 15:32	1.8	3	17.8	19.2	1.2	0	11.7	
10/9/2020 16:55	1.8	3	19.3	19.7	0.5	0	12.1	~ 3 days after Sustained Test Start
10/10/2020 9:00	1.8	3	18.8	20.1	1.7	0	12	
10/10/2020 11:10	1.8	3	18.9	20.3	1.4	0	12.2	
10/10/2020 19:30	3	5	21.5	20.2	6.4	0	12.6	~ 4 days after Sustained Test Start (flow increase ~150 cfm)
10/11/2020 8:05	3.2	5.2	21.6	20.8	5.8	0	12.6	
10/11/2020 12:05	3.2	5.2	20.2	19.8	7.5	0	12	~ 1 day after flow increase (~150 cfm)
10/11/2020 18:25	0	0	15.1	20.7	0.7	0	12.3	
10/12/2020 10:40	0	0	16	20.4	6	0	12.3	
10/12/2020 13:35	0	0	8.5	20.4	2.4	0	12.5	
10/12/2020 16:35	0	0	8.4	20.6	2.7	0	12.5	
10/13/2020 7:50	0	0	1.4	20.7	0.2	0	12.3	
10/13/2020 13:00	0	0	0.8	20.2	0.2	0	12.3	

Notes:
CH4 = Methane
CO2 = Carbon dioxide
in. w.c. = inches water column
L/min = Liters per minute
O2 = Oxygen
psi = Pounds per square inch

Table C-4 Bioventing Pilot Test Helium Data Engineering Design Report BNSF Black Tank

Spokane, WA				Dista	nce from I	njection (ft)		•	
			0	32	68	96	114	124	
Start time	Injection Pressure (psi)	Injection dP (in. w.c.)	Injection Helium Concentration (ppmv)	MW-23R Helium (ppmv)	MW-7 Helium (ppmv)	MW-22R Helium (ppmv)	MW-9 Helium (ppmv)	MW-5 Helium (ppmv)	Notes
10/6/2020 15:50	0	0.1	9150	0	0	0	0	0	He Injection Start w/ Sustained Test
10/6/2020 17:11	0	0.46	9500	0	0	0	0	0	
10/7/2020 9:32	1	1.1	9325	0	0	0	0	0	
10/7/2020 11:51	1.25	1.63	6800	0	0	0	0	0	
10/7/2020 13:22	1.5	3	12475	0	0	0	0	0	
10/7/2020 16:03	1.5	3	8500	600	0	0	0	0	~ 1 day after He Injection Start
10/8/2020 9:32	2	3	9725	1625	0	0	0	0	
10/8/2020 11:49	1.8	3	8725	1275	0	0	0	0	
10/8/2020 14:45	1.8	3	0	3950	1450	0	0	0	
10/8/2020 16:38	1.8	3	0	8825	2800	0	0	0	~ 2 days after He Injection Start (He Injection Complete)
10/9/2020 9:11	1.8	3	0	175	3525	0	0	0	
10/9/2020 14:08	1.8	3	0	0	3475	0	0	0	
10/9/2020 15:32	1.8	3	0	0	2525	0	0	0	
10/9/2020 16:55	1.8	3	0	75	2800	0	0	0	~ 3 days after He Injection Start
10/10/2020 9:00	1.8	3	0	125	875	0	0	0	
10/10/2020 11:10	1.8	3	0	0	775	0	0	0	
10/10/2020 19:30	3	5	0	25	150	0	0	0	
10/11/2020 8:05	3.2	5.2	0	0	0	0	0	0	
10/11/2020 12:05	3.2	5.2	0	75	0	0	0	0	
10/11/2020 18:25	0	0	0	0	0	0	0	0	
10/12/2020 10:40	0	0	0	0	100	0	0	0	
10/12/2020 13:35	0	0	0	0	100	0	0	0	
10/12/2020 16:35	0	0	0	0	150	0	0	0	
10/13/2020 7:50	0	0	0	0	25	0	0	0	
10/13/2020 13:00	0	0	0	0	0	0	0	0	

Notes: CH4 = Methane CO2 = Carbon dioxide in. w.c. = inches water column L/min = Liters per minute O2 = Oxygen psi = Pounds per square inch

Table C-5

Biosparing Pilot SF⁶ Test Data
Engineering Design Report
BNSF Black Tank
Spokane, WA

ID	Distance from Injection	Sample Date	Reported Concentration (ppb)	Measured Concentration (ppb)
BSI-1-CO-DUP-102120	-	16-Oct-20	30.6	30.6
BSI-1-CO-102120	-	16-Oct-20	31.9	31.9
BSI-01-102120	0	21-Oct-20	0.108	0.108
MW-31-102120	11	21-Oct-20	3.15	3.15
MW-03-102120	28	21-Oct-20	0.124	0.124
MW-04-102120	59	21-Oct-20	<lod< td=""><td>0.019</td></lod<>	0.019
Field Blank	-	21-Oct-20	0.140	0.14
MW-07-102120	~250	21-Oct-20	<lod< td=""><td>0.016</td></lod<>	0.016
MW-22R-102120	~330	21-Oct-20	<lod< td=""><td>0.023</td></lod<>	0.023
BSI-1-110420	0	4-Nov-20	0.026	0.026
MW-31-110420	11	4-Nov-20	0.321	0.321
MW-03-110420	28	4-Nov-20	<lod< td=""><td>0.009</td></lod<>	0.009
MW-04-110420	59	4-Nov-20	<lod< td=""><td>0.013</td></lod<>	0.013
Field Blank	-	4-Nov-20	<lod< td=""><td>0.012</td></lod<>	0.012
MW-07-110420	~250	4-Nov-20	<lod< td=""><td>0.014</td></lod<>	0.014
MW-22R-110420	~330	4-Nov-20	<lod< td=""><td>0.014</td></lod<>	0.014

Notes:

LOD = Limit of detection (25 ppt)

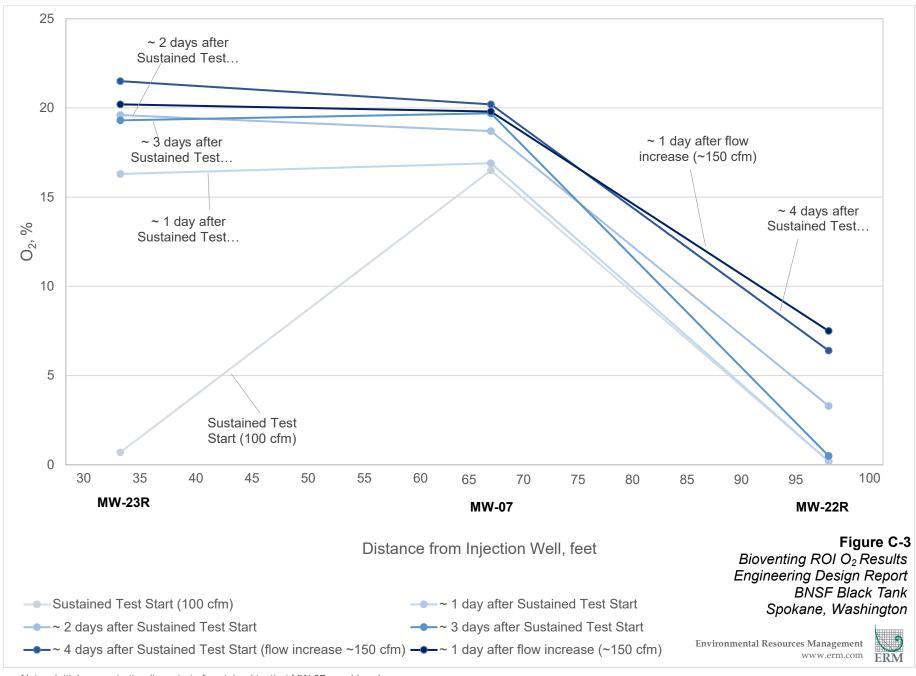
LOQ = Limit of quantitation (40 ppt)

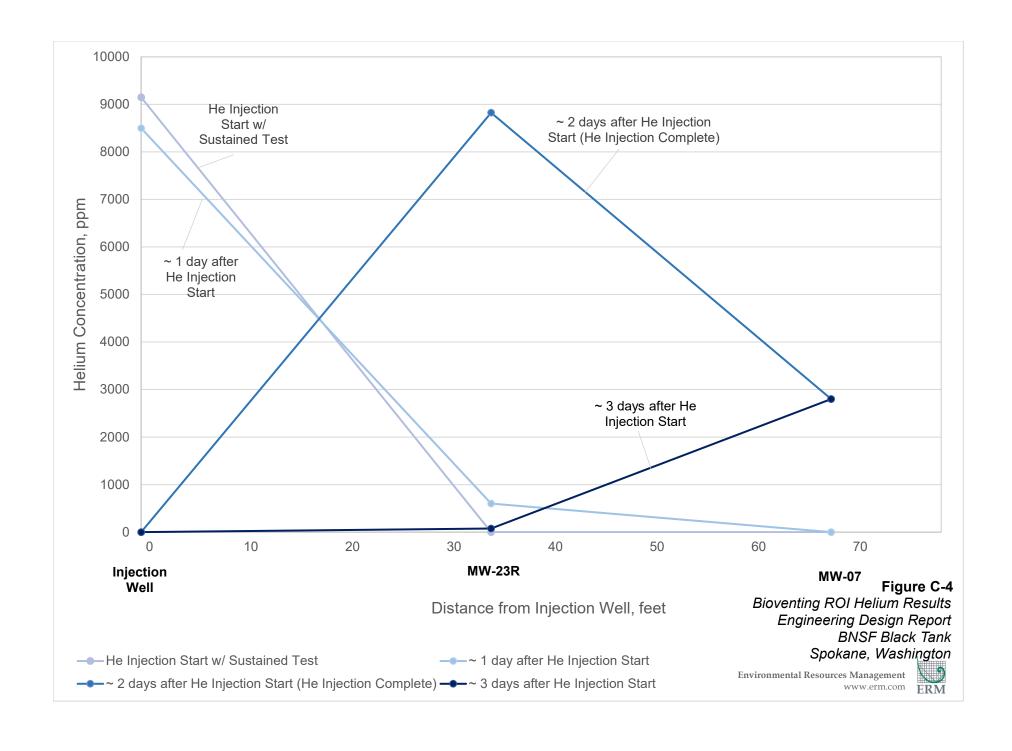
ppb = Parts per million

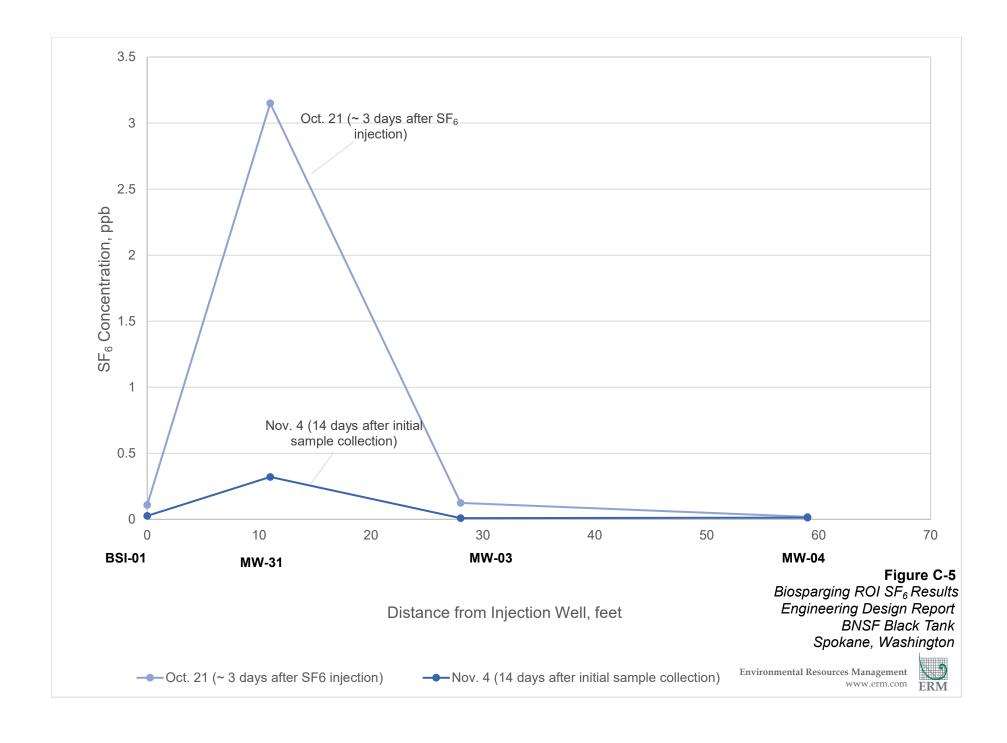
ppt = Parts per trillion

^{1.} Data validator reviewed results and concluded that the SF6 detection in the field blank does not indicate cross-contamination in the investigative samples because three of the associated investigative samples are non-detect for SF6. Therefore, none of the results are qualified as having blank contamination.

^{2.} During sampling of MW-05 for SF6, the connector on the bailer broke, leaving the bailer in the bottom of the well, where it blocked access to the groundwater in the well. The bailer could not be immediately retrieved so no groundwater samples were collected from MW-05 for SF6 analysis. Retrieval will be attempted on 11/16/20 and if successful, the well will be sampled for SF6. Wells immediately downgradient of MW-05 (MW-07 and MW-22R) were sampled and showed no detectable SF6.







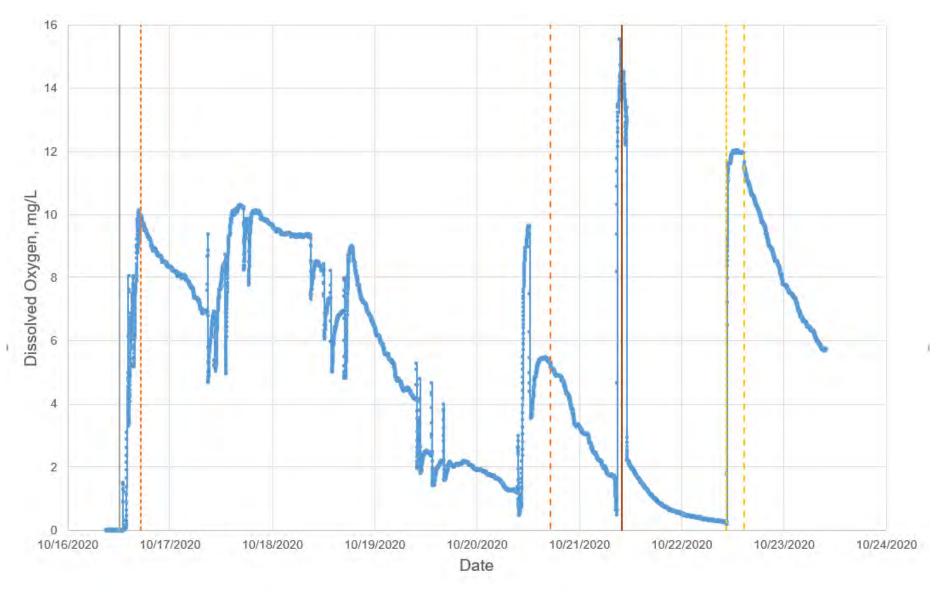


Figure C-6
Biosparging ROI Dissolved Oxygen Results at MW-31
Engineering Design Report
BNSF Black Tank
Spokane, Washington

es Management www.erm.com

Environmental Resources Management www.erm.com ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

PHOTOLOG

www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021



Photo 1: BS ROI set up



Photo 2: Manifold

BNSF Black Tank Spokane, WA January 2021



Appendix C Page 1



Photo 3: Wellhead

BNSF Black Tank Spokane, WA January 2021



APPENDIX D UPDATED BASELINE LNAPL PARAMETER DATA

Figure D-1 Mobile LNAPL Thickness - September 2019 Table D-1 Estimated Mobile LNAPL Volume Table D-2 LNAPL Thickness Variation Analysis

Figure D-2 Converted Field Viscosities
Figure D-3 Converted RI Viscosities
Figure D-4 Viscosity-Test Duration Correlation
LNAPL Viscosity Lab Report

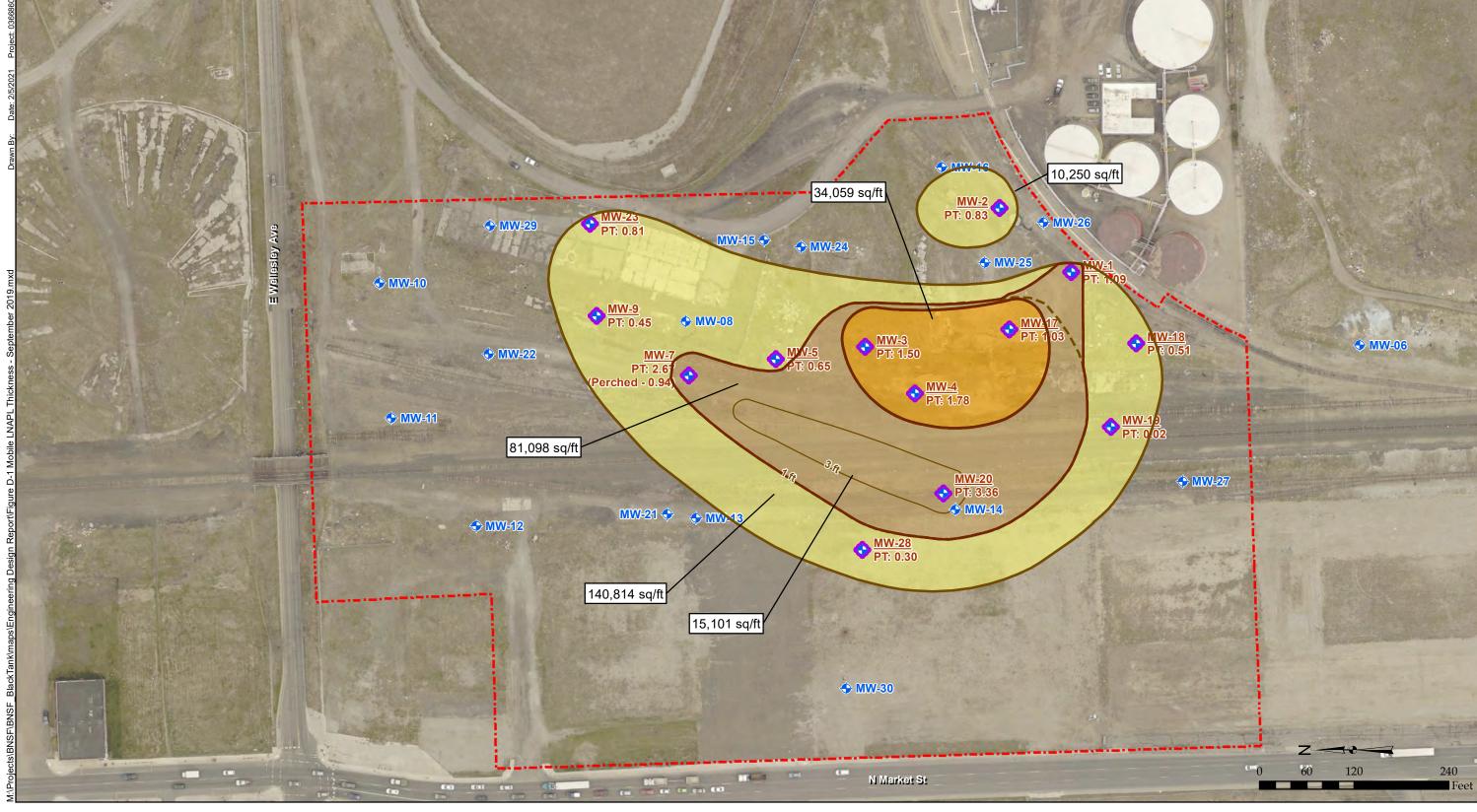
Table D-3 LNAPL Skimming Test Data

www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

MOBILE LNAPL ANALYSIS

www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021



Legend

Monitoring Well with LNAPL

Monitoring Well without LNAPL

LNAPL 2019 Contour

-- LNAPL 2016 Contour

High RTF Area Medium RTF Area Low RTF Area BNSF Black Tank Site Boundary

*MW-21 was not used for contouring.

** MW-22 water level is anomalous and is used for contouring

† = Biased low due to submerged screen greater than 1 foot.

Dry = No water detected or water level was below screened interval.

LNAPL = Light Non-Aqueous Phase Liquid.

NM = Not Measured.

PT = LNAPL thickness in feet.

SS = Well screened below the water table; screen is submerged. Not used for Contouring. All elevations in Feet Above Mean Sea Level (ft. AMSL).

Contour Interval = 0.25 feet.

Aerial Photo: Esri World Imagery Webservice, Spokane Image Consortium, 2018.

Figure D-1 Mobile LNAPL Thickness - September 2019 Engineering Design Report BNSF Black Tank

Environmental Resources Management



Spokane, Washington

Table D-1
Estimated Mobile LNAPL Volume - September 2019
Engineering Design Report
BNSF Black Tank
Spokane, WA

LNAPL Thickness Interval	Mean LNAPL Thickness (ft)	Area (sq ft)	Area (acres)	Estimated % Mobile LNAPL Saturation	Porosity	Mobile LNAPL Thickness using Mean Thickness (ft)	Estimated Mobile LNAPL Volume using Mean Thickness (gal)	Mean + 1SD LNAPL Thickness (ft)	Mobile LNAPL Thickness using Mean + 1SD Thickness (ft)	Estimated Mobile LNAPL Volume using Mean + 1SD Thickness (gal)
High RTF										
1 - 3 feet	2.00	34,059	0.8	10%	0.421	0.08	21,454	2.68	0.11	28,709
Totals		34,059	0.8				21,454			28,709
Medium RTF										
1 - 3 feet	2.00	65,997	1.5	10%	0.421	0.08	41,572	2.68	0.11	55,631
3 - 3.36 ft	3.18	15,101	0.3	10%	0.421	0.13	15,124	3.86	0.16	18,341
Totals		81,098	1.9				56,696			73,972
Low RTF										
0-1 feet	0.50	146,405	3.4	5%	0.421	0.01	11,528	1.18	0.02	27,122
Totals		146,405	3.4				11,528			27,122
All RTF Areas						_			_	_
Totals		261,562.0	6.0				89,677			129,804

Notes:

1SD = one standard deviation

One standard deviation above and below the average contains 68% of the data

ft = feet

sq ft = square feet

gal = gallons

LNAPL = Light Non-Aqueous Phase Liquid

Table D-2 LNAPL Thickness Variation Analysis Engineering Design Report BNSF Black Tank Spokane, WA

	Mean Thickness	Standard Deviation		
Monitoring Well	(2017 - 2020)	(2017 - 2020)		
	feet	feet		
MW-01	0.95	0.84		
MW-02	0.38	0.42		
MW-03	1.30	0.86		
MW-04	1.56	0.61		
MW-05	1.20	0.48		
MW-07	1.02	0.67		
MW-09	0.18	0.11		
MW-17	0.76	0.48		
MW-18	0.62	0.59		
MW-19	0.44	0.53		
MW-20	4.35	2.24		
MW-23	0.20	0.28		
Mean	Mean Standard Deviation			

Notes:

Averages for wells abanbon in Sept 2019 (MW-18, MW-19, MW-20, and MW-23) are only through that date.

ERM Page 1 of 1 PN0178173 2/11/2021

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

VISCOSITY ANALYSIS

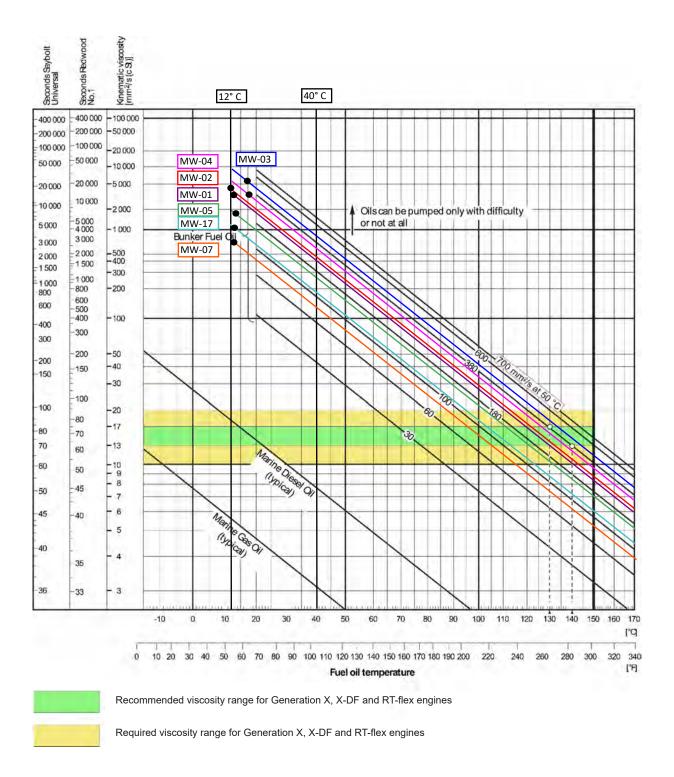
www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021

Diesel engine fuels

Diesel engine fuels

Fig 1 Viscosity / Temperature diagram

• = field sample temperature



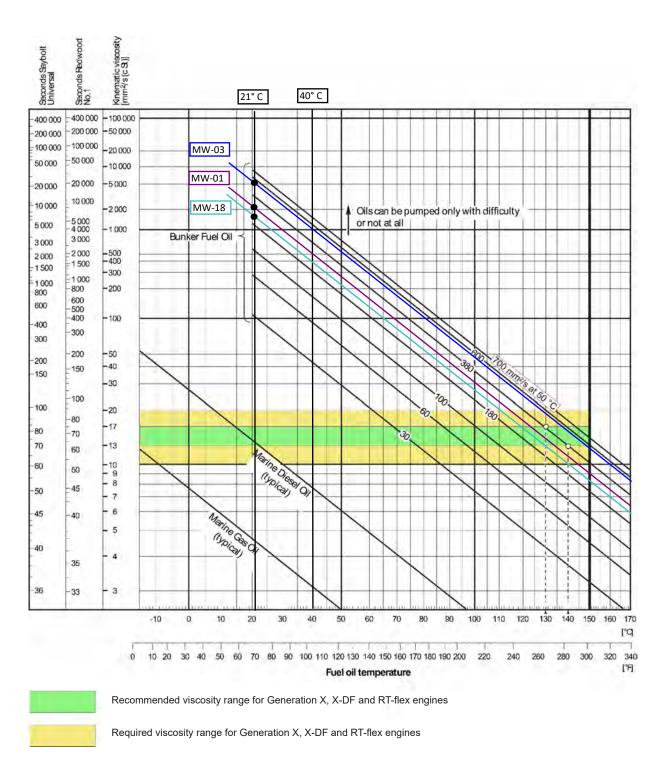
00099

Diesel engine fuels

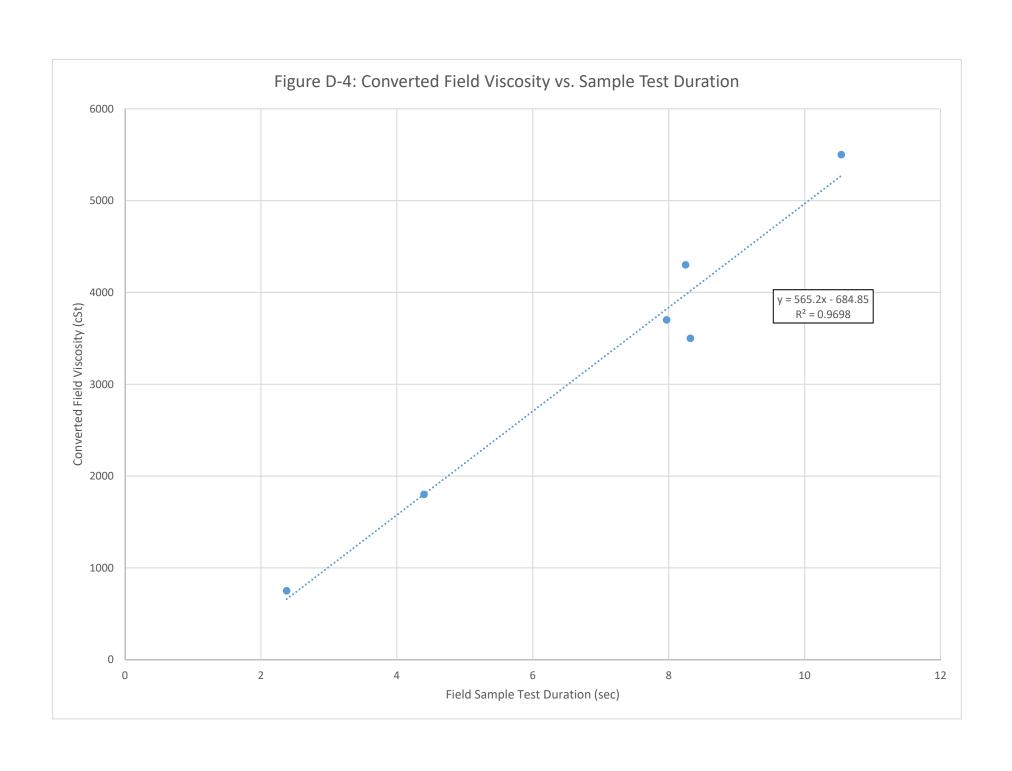
Diesel engine fuels

Fig 1 Viscosity / Temperature diagram

= RI lab sample temperature



00099



(612)607-1700



October 13, 2020

Emily Ponaski ERM West Inc. 1050 SW 6th Ave Suite 1650 Portland, OR 97204

RE: Project: 559481 Black Tank

Pace Project No.: 10533594

Dear Emily Ponaski:

Enclosed are the analytical results for sample(s) received by the laboratory on September 29, 2020. The results relate only to the samples included in this report. Results reported herein conform to the applicable TNI/NELAC Standards and the laboratory's Quality Manual, where applicable, unless otherwise noted in the body of the report.

Some analyses were subcontracted outside of the Pace Network. The test report from the external subcontractor is attached to this report in its entirety.

The test results provided in this final report were generated by each of the following laboratories within the Pace Network:

If you have any questions concerning this report, please feel free to contact me.

Sincerely,

DENNI GROSS

Jennifer Gross jennifer.gross@pacelabs.com (612)607-1700 Project Manager

Enclosures

cc: Todd McGovern, ERM West Inc.







SAMPLE SUMMARY

Project: 559481 Black Tank

Pace Project No.: 10533594

Lab ID	Sample ID	Matrix	Date Collected	Date Received
10533594001	MW-2-LNAPL	Non Aqueous	09/25/20 11:15	09/29/20 08:50
10533594002	MW-1-LNAPL	Non Aqueous	09/25/20 11:57	09/29/20 08:50
10533594003	MW-7-LNAPL	Non Aqueous	09/25/20 14:06	09/29/20 08:50
10533594004	MW-17-LNAPL	Non Aqueous	09/25/20 13:03	09/29/20 08:50
10533594005	MW-5-LNAPL	Non Aqueous	09/25/20 15:06	09/29/20 08:50
10533594006	MW-9-LNAPL	Non Aqueous	09/28/20 08:35	09/29/20 08:50
10533594007	MW-4-LNAPL	Non Aqueous	09/28/20 09:52	09/29/20 08:50
10533594008	MW-31-LNAPL	Non Aqueous	09/28/20 11:13	09/29/20 08:50
10533594009	MW-3-LNAPL	Non Aqueous	09/28/20 12:01	09/29/20 08:50

REPORT OF LABORATORY ANALYSIS



Pace Analytical www.pacelabs.com

1700 Elm Street - Suite 200 Minneapolis, MN 55414 (612)607-1700

PROJECT NARRATIVE

Project: Pace Project No.:	
Method: Description: Client: Date:	

This data package has been reviewed for quality and completeness and is approved for release.

REPORT OF LABORATORY ANALYSIS

CHAIN-OF-CUSTODY / Analytical Request Document

The Chain-of-Custody is a LEGAL DOCUMENT. All relevant fields must be completed accurately.

(N/Y) Samples Samples 000 S L00 200 9 00 200 000 WO#: 10533594 SAMPLE CONDITIONS (N/A) 9 Cooler ŏ Sealed Regulatory Agency State / Location WA / Spokane (N/Y) Received on Resid Page: TEMP in C TIME 8 DATE Signed: 09/28/2626 Requested Analysis Filtered (Y/N DATE me Laboration 45P 907 27.76 KLISO) SIN Address: ermwestaccountspayable@erm.com X jennifer.gross@pacelabs.com, ACCEPTED BY / AFFILIATION WYTPHDx (With Silica Gel) WYTPHDx (No Silica Gel) N/A JaeT seavisnA Matt Gardell 1 Methanol Attention: Accounts Payable Preservatives Na2S2O3 Company Name: ERM West HOBN Pace Project Manager ЮН Invoice Information: Z ниоз H2SO4 Pace Quote: 1500 Section C TIME Unpreserved # OF CONTAINERS SAMPLER NAME AND SIGNATURE McGovern tod-megovern Ochn con 128/20 SIGNATURE of SAMPLER: SAMPLE TEMP AT COLLECTION PRINT Name of SAMPLER: DATE TIME 8CACE TANK \$59481 ENO DATE TODA RELINGUISHED BY / AFFILIATION 9/25/1406 1157 835 TIME 19hz 1115 9/25 | 303 9152 1526 9/28 952 9/28/11/3 1/26 | 1/26 | START Report To: Guzanne Dolbeng. 3/12 1 की Required Project Information: S (G=GRAB C=COMP) SAMPLE TYPE urchase Order #: 2 MATRIX CODE (see valid codes to left) Z Project Name: Section B Copy To: CODE WY WY SP P WY OP OP OF MATRIX
Drinking Water
Water
Waster
Waster
Product
Soil/Soild
Oil
Wipe
An An
Other
Tissue 10 Day Standard Turn MW-17-LNAPL MW-I- LN APL ADDITIONAL COMMENTS One Character per box. (A-Z, 0-9 / , -) Sample Ids must be unique MW-7-LNAP 1218 3rd Avenue, Suite 1412 MW-5-CNAPL MNG-3-LNAP L MW-4-LABPL MW-31-LNAPL Fax MW-7-LNAPP MW-9-LN APL SAMPLE ID Seattle, WA 9810 (425)773-3788 Required Client Information: Requested Due Date Address: 9 7 ထ Phone: 5 6 Email: က F 10 12 Page 4 of 15 # MBTI

ace Analytical`

Document Name:

Sample Condition Upon Receipt (SCUR) - MN

Document No.:

Page 1 of 1

Pace Analytical Services -

Document Revised: 12Aug2020

Minneapolis ENV-FRM-MIN4-0150 Rev.01 Sample Condition **Client Name:** Project #: WO#: 10533594 **Upon Receipt** Due Date: 10/06/20 USPS Courier: Fed Ex □UPS Client Pace Commercial SpeeDee CLIENT: ERM_WA See Exceptions 1456 **Tracking Number:** ENV-FRM-MIN4-0142 No Biological Tissue Frozen? ☐Yes ☐No ▼N/A **≯**No. **Custody Seal on Cooler/Box Present?** Seals intact? Yes Bubble Wrap Bubble Bags Yes **Packing Material:** None Other: Temp Blank? ☐ T1(0461) ☐ T2(1336) ☐ T3(0459) Wet Type of Ice: Blue None Thermometer: Melted ☐ T4(0254) ☐ T5(0489) Did Samples Originate in West Virginia? ☐Yes ∠ No Were All Container Temps Taken? ☐ Yes □No 🗖 N/A Temp should be above freezing to 6°C Cooler Temp Read w/temp blank: **Average Corrected** See Exceptions ENV-FRM-MIN4-0142 Temp (no temp blank ☐1 Container Cooler Temp Corrected w/temp blank:_ **Correction Factor:** °C only): _ 3. // ℃ Date/Initials of Person Examining Contents: USDA Regulated Soil: (N/A, water sample/Other:__ Did samples originate in a quarantine zone within the United States: AL, AR, CA, FL, GA, Did samples originate from a foreign source (internationally, including ID, LA. MS, NC, NM, NY, OK, OR, SC, TN, TX or VA (check maps)? ØNo Hawaii and Puerto Rico)? ☐Yes ∠No If Yes to either question, fill out a Regulated Soil Checklist (F-MN-Q-338) and include with SCUR/COC paperwork. COMMENTS: Yes Chain of Custody Present and Filled Out? □No 1. Chain of Custody Relinquished? **⊠**Yes □No 2. Yes □No Sampler Name and/or Signature on COC? □n/a 3. Yes Samples Arrived within Hold Time? □No 4. ☐ Fecal Coliform ☐ HPC ☐ Total Coliform/E coli ☐ BOD/cBOD ☐ Hex Chrome No Short Hold Time Analysis (<72 hr)? □Yes ☐ Turbidity ☐ Nitrate ☐ Nitrite ☐ Orthophos ☐ Other_ Mo **Rush Turn Around Time Requested?** ☐Yes 6. Sufficient Volume? **⊠**Yes □No 7. Z Yes Z Yes **Correct Containers Used?** □No 8. □<u>No</u> -Pace Containers Used? **Z**Yes □No 9. Containers Intact? ⊠N/A 10. Is sediment visible in the dissolved container? Yes No Field Filtered Volume Received for Dissolved Tests? ☐ Yes □No 11. If no, write ID/ Date/Time on Container Below: Is sufficient information available to reconcile the samples See Exception ENV-FRM-MIN4-0142 to the COC? Yes □No Matrix: Water Soil Oil Other_ All containers needing acid/base preservation have been 12. Sample # □Yes □No ZÍN/A checked? ■ NaOH ☐ HNO₃ All containers needing preservation are found to be in H₂SO₄ Zinc Acetate ☐Yes compliance with EPA recommendation? (HNO₃, H₂SO₄, <2pH, NaOH >9 Sulfide, NaOH>10 Cyanide) Positive for Res. Yes See Exception □No □Yes ENV-FRM-MIN4-0142 Exceptions: VOA, Coliform, TOC/DOC Oil and Grease, No Chlorine? pH Paper Lot# DRO/8015 (water) and Dioxin/PFAS Res. Chlorine 0-6 Roll 0-6 Strip 0-14 Strip Extra labels present on soil VOA or WIDRO containers? □Yes □No ØŊ/A 13. See Exception Z<u>N/A</u> Headspace in VOA Vials (greater than 6mm)? □Yes □No ENV-FRM-MIN4-0140 Trip Blank Present? ☐Yes □No ☑N/A Trip Blank Custody Seals Present? Yes ΠNo Pace Trip Blank Lot # (if purchased): Field Data Required? Yes No CLIENT NOTIFICATION/RESOLUTION Person Contacted: Date/Time: Comments/Resolution:

ENNI (-PROSS Date: **Project Manager Review:** Note: Whenever there is a discrepancy affecting North Carolina compliance samples, a copy of this form will be sent to the North Carolina DEHNR Certification Office (i.e. out of hold, incorrect preservative, out of temp, incorrect containers).

Page 5 of 15

09/30/20



Document Name:

Sample Condition Upon Receipt (SCUR) Exception Form

Document No.: ENV-FRM-MIN4-0142 Rev.01 Document Revised: 04Jun2020 Page 1 of 1

Pace Analytical Services -Minneapolis

SCUR Exceptions:						Wo	rkord	ler #:		
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Sample ID	Type o Preserv		Date Adjusted	Time Adjusted	Amoun t Added (mL)	Lot # Added	pH After	In Comp		Initials
								□Yes	□No	
								Yes	No	
								□Yes		
								Yes	□No	
Comments:	L	<u> </u>		1	I			J		
						. "				

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4

Greensburg, PA 15601 Ph: 724-850-5600 Fax:

Pace Analytical Services

Email: Rachel.Christner@pacelabs.com

Pace Analytical Services/Minneapolis, MN

FINAL REPORT

559481 BLACK TANK

MW-2-LNAPL

This report and the data within has completed QA/QC review



Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Pace Analytical Services Lube Supplier Level 3 Level 4

2019 Unknown Make: UNKNOWN Make: UNKNOWN Lube Type Model: UNKNOWN Model: UNKNOWN Unknown Serial No: UNKNOWN

Client Sample# MW-2-LNAPL PO# 10533594

Kinematic Viscosity - ASTM D445 40C - new			
Т	Test Code: VISC40 new / Method: D445, SOP 0107		
Result Date	10/08/2020		
Viscosity @40C	460.06 cSt		

Tracking #	414052-1
Client Sample #	MW-2-LNAPL
Sample Date	09/25/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 460.06 cSt

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4

Greensburg, PA 15601 Ph: 724-850-5600 Fax:

Email: Rachel.Christner@pacelabs.com

FINAL REPORT

This report and the data within has completed QA/QC review



Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Pace Analytical Services

Pace Analytical Services/Minneapolis, MN 559481 BLACK TANK MW-1-LNAPL

Pace Analytical Services

2019

Unknown Lube Type Unknown

PO# 10533594

Lube Supplier

Level 4

Make: UNKNOWN Model: UNKNOWN Serial No: UNKNOWN

Client Sample# MW-1-LNAPL

Kinematic Viscosity - ASTM D445 40C - new			
Т	est Code: VISC40 new / Method: D445, SOP 0107		
Result Date	10/08/2020		
Viscosity @40C	412.06 cSt		

Tracking #	414052-2
Client Sample #	MW-1-LNAPL
Sample Date	09/25/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 412.06 cSt

Level 3

Make: UNKNOWN

Model: UNKNOWN

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4 Greensburg, PA 15601

Ph: 724-850-5600 Fax:

Email: Rachel.Christner@pacelabs.com

FINAL REPORT

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Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Pace Analytical Services

Pace Analytical Services/Minneapolis, MN 559481 BLACK TANK MW-7-LNAPL

Pace Analytical Services

Client Sample# MW-7-LNAPL

2019

Unknown Lube Type Unknown

PO# 10533594

Lube Supplier

Level 4

Make: UNKNOWN Model: UNKNOWN Serial No: UNKNOWN

Kinematic Viscosity - ASTM D445 40C - new				
_	est Code: VISC40 new / Method: D445, SOP 0107			
Result Date	10/08/2020			
Viscosity @40C	141.18 cSt			

Tracking #	414052-3
Client Sample #	MW-7-LNAPL
Sample Date	09/25/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 141.18 cSt

Level 3

Make: UNKNOWN

Model: UNKNOWN

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4

Greensburg, PA 15601 Ph: 724-850-5600 Fax:

Pace Analytical Services

Email: Rachel.Christner@pacelabs.com

Pace Analytical Services/Minneapolis, MN

FINAL REPORT

559481 BLACK TANK

MW-17-LNAPL

Viscosity @40C

This report and the data within has completed QA/QC review



Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Level 4

Lube Supplier

Unknown Make: UNKNOWN Make: UNKNOWN Lube Type Model: UNKNOWN Model: UNKNOWN Unknown Serial No: UNKNOWN

Level 3

Client Sample# MW-17-LNAPL PO# 10533594

Kinematic Viscosity - ASTM D445 40C - new			
т	est Code: VISC40 new / Method: D445, SOP 0107		
Result Date	10/08/2020		

192.38 cSt

2019

Tracking #	414052-4
Client Sample #	MW-17-LNAPL
Sample Date	09/25/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 192.38 cSt

Authorized Signature

Analyst:

Date: 10/08/2020

Pace Analytical Services

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4

Greensburg, PA 15601 Ph: 724-850-5600 Fax:

Email: Rachel.Christner@pacelabs.com

FINAL REPORT

This report and the data within has completed QA/QC review



Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Pace Analytical Services

Pace Analytical Services/Minneapolis, MN 559481 BLACK TANK MW-5-LNAPL

Pace Analytical Services

2019

Client Sample# MW-5-LNAPL PO# 10533594

Lube Supplier Level 3

Unknown Make: UNKNOWN Lube Type Model: UNKNOWN Unknown

Level 4

Make: UNKNOWN Model: UNKNOWN Serial No: UNKNOWN

Kinematic Viscosity - ASTM D445 40C - new

Test Code: VISC40 new / Method: D445, SOP 0			
Result Date	10/08/2020		
Viscosity @40C	280.76 cSt		

Tracking #	414052-5
Client Sample #	MW-5-LNAPL
Sample Date	09/25/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 280.76 cSt

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4 Greensburg, PA 15601

Ph: 724-850-5600 Fax:

Email: Rachel.Christner@pacelabs.com

FINAL REPORT

This report and the data within has completed QA/QC review



Lube Supplier

Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Pace Analytical Services

Pace Analytical Services/Minneapolis, MN 559481 BLACK TANK MW-9-LNAPL

Pace Analytical Services

2019

Unknown Lube Type Unknown

Client Sample# MW-9-LNAPL PO# 10533594

Level 3 Level 4

Make: UNKNOWN Make: UNKNOWN Model: UNKNOWN Model: UNKNOWN Serial No: UNKNOWN

Kinematic Viscosity - ASTM D445 40C - new				
T	est Code: VISC40 new / Method: D445, SOP 0107			
Result Date	10/08/2020			
Viscosity @40C	46.889 cSt			

Tracking #	414052-6
Client Sample #	MW-9-LNAPL
Sample Date	09/28/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 46.889 cSt

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4 Greensburg, PA 15601

Ph: 724-850-5600 Fax:

Email: Rachel.Christner@pacelabs.com

FINAL REPORT

This report and the data within has completed QA/QC review



Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Pace Analytical Services

Pace Analytical Services/Minneapolis, MN 559481 BLACK TANK MW-4-LNAPL

Pace Analytical Services

2019

Client Sample# MW-4-LNAPL

Lube Supplier

Unknown Lube Type Unknown

PO# 10533594

Level 4

Make: UNKNOWN Model: UNKNOWN Serial No: UNKNOWN

Kinematic Viscosity - ASTM D445 40C - new

Test Code: VISC40 new / Method: D445, SOP 0107 Result Date 10/08/2020 Viscosity @40C 655.38 cSt

Tracking #	414052-7
Client Sample #	MW-4-LNAPL
Sample Date	09/28/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 655.38 cSt

Level 3

Make: UNKNOWN

Model: UNKNOWN

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4

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Email: Rachel.Christner@pacelabs.com

FINAL REPORT

MW-31-LNAPL

This report and the data within has completed QA/QC review



Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

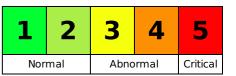
Pace Analytical Services Lube Supplier Level 3

Pace Analytical Services/Minneapolis, MN 2019 Unknown Make: UNKNOWN 559481 BLACK TANK Model: UNKNOWN

> Lube Type Level 4

Client Sample# MW-31-LNAPLUnknown PO# 10533594

Make: UNKNOWN Model: UNKNOWN Serial No: UNKNOWN



Tracking #	414052-8
Client Sample #	MW-31-LNAPL
Sample Date	09/28/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	
Mach Cond	·
Lube Cond	

General Diagnostic Notes

Viscosity fails at 40C and 100C. Sample too thick. Sample fails to flow adequately to fill viscosity cell.

Kinematic Viscosity - ASTM D445 40C - new			1	Test Code: VISC40 new / Method: D445, SOP 010			
Sample Date		09/28/2020					
Result Date		10/08/2020	REF				
Tracking #		414052-8	LOWER		UPPER		
Viscosity @40C	cSt	-	-		-		
Comments for section							

Viscosity fails at 40C and 100C.

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

Pace Analytical Services Contact: Rachel Christner Address: Pace Analytical Services 1638 Roseytown Road Suites 2,3 & 4 Greensburg, PA 15601

Ph: 724-850-5600 Fax:

Email: Rachel.Christner@pacelabs.com

FINAL REPORT

This report and the data within has completed QA/QC review



Fuels & Lubrication Lab 1801 Route 51 South Jefferson Hills, PA 15025

Pace Analytical Services

Pace Analytical Services/Minneapolis, MN 559481 BLACK TANK MW-3-I NAPI

Pace Analytical Services

2019

Unknown Lube Type Unknown

Lube Supplier

Level 3

Make: UNKNOWN Model: UNKNOWN Level 4

Make: UNKNOWN Model: UNKNOWN Serial No: UNKNOWN

Client Sample# MW-3-LNAPL

PO# 10533594

Kinematic Viscosity - ASTM D445 40C - new Test Code: VISC40 new / Method: D445, SOP 0107

Result Date 10/08/2020 Viscosity @40C 870.52 cSt

Tracking #	414052-9
Client Sample #	MW-3-LNAPL
Sample Date	09/28/2020
Received Date	10/02/2020
Time on Oil	
Time on Unit	

General Diagnostic Notes

Viscosity @ 40 C: 870.52 cSt

Authorized Signature

Analyst:

Date: 10/08/2020

Shannan Burnett

ENGINEERING DESIGN REPORT

BNSF Railway Black Tank Property

TRANSMISSIVITY ANALYSIS

www.erm.com Version: 1.0 Project No.: 0578173 Client: BNSF, Husky November 2021

Table D-3

LNAPL Skimming Test Field Data Summary - September 2020

Engineering Design Repot

BNSF Black Tank

Spokane, WA

	Initial C	onditions	Recover	y Event 1	Recover	y Event 2	Recover	y Event 3	Recover	y Event 4	LNAPL
Well ID	Gauged LNAPL Thickness (feet)	LNAPL Volume Removed (ml)	Elapsed Time (min)	LNAPL Volume Removed (ml)	Discharge Rate, Q _n (ft³/day)						
MW-01	1.19	740	4421	10	4196	20	25889	15			2.9E-05
MW-02	0.32	200	4474	10	4204	20	25886	5			9.8E-06
MW-03	0.26	160	1566	30	4111	190	16020	100			3.2E-04
MW-04	0.72	450	1609	10	4234	10	16039	15			4.8E-05
MW-05	0.83	515	4291	280	1238	10	4306	20	26176	100	1.9E-04
MW-07	2.02	1260	4331	40	2729	10	28721	160			2.8E-04
MW-09	0.11	70	1622	10							3.1E-04
MW-17	1.00	625	4365	290	1378	20	4128	90	24612	735	1.5E-03
MW-31	0.21	130	2885	80	1495	40	8771	75	8695	10	5.8E-05

Notes:

Elapsed time is the time between LNAPL recovery events.

Grey-shaded cells indicate no LNAPL recovered.

Conversion factors:

1,440 minutes/day

28,320 ml/ft³

Abbreviations:

ft³/day = cubic feet per day

LNAPL = light non-aqueous phase liquid

ml = milliliters

ENGINEERING DESIGN REPORT BNSF Railway Black Tank Property

APPENDIX E DESIGN CALCULATIONS

www.erm.com Version: 2.0 Project No.: 0578173 Client: BNSF, Husky November 2021

Appendix E Design Calculations - Theoretical BV ROI Engineering Design Report BNSF Black Tank Spokane, WA

Equation 2-27 (USACOE 2002)	Where:			
_		Input Value	Units	
$+-\frac{\pi r^2bn}{2}$	Flow (Q)	110	cfm	
t = —	Increased flow	150	cfm	
Q	Thickness (b)	60	ft	
	Porosity (n)	0.34	-	
	Time (t)	various	days	
	O ₂ Radius (r)	various	ft	
Theoretical Cumulative			Pilot	: T

		- 2	 	
	Theoretical Cumulative		Pilo	t Test
Time (days)	Injected Volume	O ₂ Radius (ft)	Time (days)	O ₂ Radius (ft)
0.00	0	0		
0.04	6600	10	0.34	32
0.08	13200	14	2.05	68
0.13	19800	18	3.48	96
0.17	26400	20		
0.21	33000	23		
0.25	39600	25		
0.29	46200	27		
0.33	52800	29		
0.38	59400	30		
0.42	66000	32		
0.46	72600	34		
0.50	79200	35		
0.54	85800	37		
0.58	92400	38		
0.63	99000	39		
0.67	105600	41		
0.71	112200	42		
0.75	118800	43		
0.79	125400	44		
0.83	132000	45		
0.88	138600	47		
0.92	145200	48		
0.96	151800	49		
1.00	158400	50		
1.04	165000	51		
1.08	171600	52		
1.13	178200	53		
1.17	184800	54		
1.21	191400	55		
1.25	198000	56		

1.29	204600	57				
1.33	211200	57				
1.38	217800	58				
1.42	224400	59				
1.46	231000	60				
1.50	237600	61				
1.54	244200	62				
1.58	250800	63				
1.63	257400	63				
1.67	264000	64				
1.71	270600	65				
1.75	277200	66				
1.79	283800	67				
1.83	290400	67				
1.88	297000	68				
1.92	303600	69				
1.96	310200	70				
2.00	316800	70				
2.04	323400	71				
2.08	330000	72				
2.13	336600	72				
2.17	343200	73				
2.21	349800	74				
2.25	356400	75				
2.29	363000	75				
2.33	369600	76				
2.38	376200	77				
2.42	382800	77				
2.46	389400	78				
2.50	396000	79				
2.54	402600	79				
2.58	409200	80				
2.63	415800	81				
2.67	422400	81				
2.71	429000	82				
2.75	435600	82				
2.79	442200	83				
2.83	448800	84				
2.88	455400	84				
2.92	462000	85				
2.96	468600	86				
3.00	475200	86				
					Cumulative	O ₂ Radius
3.04	481800	87	Increased Flow	Time (days)	Injected Volume	(ft)
3.08	488400	87		3.0	657000	101
3.13	495000	88		3.1	666000	102
3.17	501600	88		3.1	675000	103
3.21	508200	89		3.2	684000	103
3.25	514800	90		3.2	693000	104

3.29	521400	90	3.3	702000	105
3.33	528000	91	3.3	711000	105
3.38	534600	91	3.3	720000	106
3.42	541200	92	3.4	729000	107
3.46	547800	92	3.4	738000	107
3.50	554400	93	3.5	747000	108
3.54	561000	94	3.5	756000	109
3.58	567600	94	3.5	765000	109
3.63	574200	95	3.6	774000	110
3.67	580800	95	3.6	783000	111
3.71	587400	96	3.7	792000	111
3.75	594000	96	3.7	801000	112
3.79	600600	97	3.8	810000	112
3.83	607200	97	3.8	819000	113
3.87	613800	98	3.8	828000	114
3.92	620400	98	3.9	837000	114
3.96	627000	99	3.9	846000	115
4.00	633600	99	4.0	855000	116
4.04	640200	100	4.0	864000	116
4.08	646800	100	4.0	873000	117
4.12	653400	101	4.1	882000	117
4.17	660000	101	4.1	891000	118
4.21	666600	102	4.2	900000	119
4.25	673200	102	4.2	909000	119
4.23	679800	103	4.2	918000	120
4.29					
	686400	103	4.3	927000	120
4.38	693000	104	4.3	936000	121
4.42	699600	104	4.4	945000	121
4.46	706200	105	4.4	954000	122
4.50	712800	105	4.5	963000	123
4.54	719400	106	4.5	972000	123
4.58	726000	106	4.5	981000	124
4.63	732600	107	4.6	990000	124
4.67	739200	107	4.6	999000	125
4.71	745800	108	4.7	1008000	125
4.75	752400	108	4.7	1017000	126
4.79	759000	109	4.8	1026000	127
4.83	765600	109	4.8	1035000	127
4.88	772200	110	4.8	1044000	128
4.92	778800	110	4.9	1053000	128
4.96	785400	111	4.9	1062000	129
5.00	792000	111	5.0	1071000	129
5.04	798600	112	5.0	1080000	130
5.08	805200	112	5.0	1089000	130
5.13	811800	113	5.1	1098000	131
5.17	818400	113	5.1	1107000	131
5.21	825000	113	5.2	1116000	132
5.25	831600	114	5.2	1125000	132
5.29	838200	114	5.3	1134000	133
5.33	844800	115	5.3	1143000	134

5.38	851400	115	5.3	1152000	134
5.42	858000	116	5.4	1161000	135
5.46	864600	116	5.4	1170000	135
5.50	871200	117	5.5	1179000	136
5.54	877800	117	5.5	1188000	136
5.58	884400	117	5.5	1197000	137
5.63	891000	118	5.6	1206000	137
5.67	897600	118	5.6	1215000	138
5.71	904200	119	5.7	1224000	138
5.75	910800	119	5.7	1233000	139
5.79	917400	120	5.8	1242000	139
5.83	924000	120	5.8	1251000	140
5.88	930600	121	5.8	1260000	140
5.92	937200	121	5.9	1269000	141
5.96	943800	121	5.9	1278000	141
6.00	950400	122	6.0	1287000	142
6.04	957000	122	6.0	1296000	142
6.08	963600	123	6.0	1305000	143
6.13	970200	123	6.1	1314000	143
6.17	976800	123	6.1	1323000	144
6.21	983400	124	6.2	1332000	144
6.25	990000	124	6.2	1341000	145
6.29	996600	125	6.3	1350000	145
6.33	1003200	125	6.3	1359000	146
6.38	1009800	126	6.3	1368000	146
6.42	1016400	126	6.4	1377000	147
6.46	1023000	126	6.4	1386000	147
6.50	1029600	127	6.5	1395000	148
6.54	1036200	127	6.5	1404000	148
6.58	1042800	128	6.5	1413000	148
6.63	1049400	128	6.6	1422000	149
6.67	1056000	128	6.6	1431000	149
6.71	1062600	129	6.7	1440000	150
6.75	1069200	129	6.7	1449000	150
6.79	1075800	130	6.8	1458000	151
6.83	1082400	130	6.8	1467000	151
6.88	1089000	130	6.8	1476000	152
6.92	1095600	131	6.9	1485000	152
6.96	1102200	131	6.9	1494000	153
7.00	1108800	132	7.0	1503000	153
7.04	1115400	132	7.0	1512000	154
7.08	1122000	132	7.0	1521000	154
7.13	1128600	133	7.1	1530000	155
7.17	1135200	133	7.1	1539000	155
7.21	1141800	133	7.2	1548000	155
7.25	1148400	134	7.2	1557000	156
7.29	1155000	134	7.3	1566000	156
7.33	1161600	135	7.3	1575000	157
7.38	1168200	135	7.3	1584000	157
7.42	1174800	135	7.4	1593000	158

7.46	1181400	136	7.4	1602000	158
7.50	1188000	136	7.5	1611000	159
7.54	1194600	137	7.5	1620000	159
7.58	1201200	137	7.5	1629000	159
7.63	1207800	137	7.6	1638000	160
7.67	1214400	138	7.6	1647000	160
7.71	1221000	138	7.7	1656000	161
7.75	1227600	138	7.7	1665000	161
7.79	1234200	139	7.8	1674000	162
7.83	1240800	139	7.8	1683000	162
7.88	1247400	140	7.8	1692000	162
7.92	1254000	140	7.9	1701000	163
7.96	1260600	140	7.9	1710000	163
8.00	1267200	141	8.0	1719000	164
8.04	1273800	141	8.0	1728000	164
8.08	1280400	141	8.0	1737000	165
8.13	1287000	142	8.1	1746000	165
8.17	1293600	142	8.1	1755000	165
8.21	1300200	142	8.2	1764000	166
8.25	1306800	143	8.2	1773000	166
8.29	1313400	143	8.3	1782000	167
8.33	1320000	144	8.3	1791000	167
8.38	1326600	144	8.3	1800000	168
8.42	1333200	144	8.4	1809000	168
8.46	1339800	145	8.4	1818000	168
8.50	1346400	145	8.5	1827000	169
8.54	1353000	145	8.5	1836000	169
8.58	1359600	146	8.5	1845000	170
8.63	1366200	146	8.6	1854000	170
8.67	1372800	146	8.6	1863000	170
8.71	1379400	147	8.7	1872000	171
8.75	1386000	147	8.7	1881000	171
8.79	1392600	147	8.8	1890000	172
8.83	1399200	148	8.8	1899000	172
8.88	1405800	148	8.8	1908000	173
8.92	1412400	148	8.9	1917000	173
8.96	1419000	149	8.9	1926000	173
9.00	1425600	149	9.0	1935000	174
9.04	1432200	149	9.0	1944000	174
9.08	1438800	150	9.0	1953000	175
9.13	1445400	150	9.1	1962000	175
9.17	1452000	151	9.1	1971000	175
9.21	1458600	151	9.2	1980000	176
9.25	1465200	151	9.2	1989000	176
9.29	1471800	152	9.3	1998000	177
9.33	1478400	152	9.3	2007000	177
9.38	1485000	152	9.3	2016000	177
9.42	1491600	153	9.4	2025000	178
9.46	1498200	153	9.4	2034000	178
9.50	1504800	153	9.5	2043000	179

9.54	1511400	154	9.5	2052000	179
9.58	1518000	154	9.5	2061000	179
9.63	1524600	154	9.6	2070000	180
9.67	1531200	155	9.6	2079000	180
9.71	1537800	155	9.7	2088000	180
9.75	1544400	155	9.7	2097000	181
9.79	1551000	156	9.8	2106000	181
9.83	1557600	156	9.8	2115000	182
9.87	1564200	156	9.8	2124000	182
9.92	1570800	157	9.9	2133000	182
9.96	1577400	157	9.9	2142000	183
10.00	1584000	157	10.0	2151000	183
10.04	1590600	158	10.0	2160000	184
10.08	1597200	158	10.0	2169000	184
10.13	1603800	158	10.1	2178000	184
10.17	1610400	159	10.1	2187000	185
10.21	1617000	159	10.2	2196000	185
10.25	1623600	159	10.2	2205000	185
10.29	1630200	159	10.3	2214000	186
10.33	1636800	160	10.3	2223000	186
10.38	1643400	160	10.3	2232000	187
10.42	1650000	160	10.4	2241000	187
10.46	1656600	161	10.4	2250000	187
10.50	1663200	161	10.5	2259000	188
10.54	1669800	161	10.5	2268000	188
10.58	1676400	162	10.5	2277000	188
10.63	1683000	162	10.6	2286000	189
10.67	1689600	162	10.6	2295000	189
10.71	1696200	163	10.7	2304000	190
10.75	1702800	163	10.7	2313000	190
10.79	1709400	163	10.8	2322000	190
10.83	1716000	164	10.8	2331000	191
10.88	1722600	164	10.8	2340000	191
10.92	1729200	164	10.9	2349000	191
10.96	1735800	165	10.9	2358000	192
11.00	1742400	165	11.0	2367000	192
11.04	1749000	165	11.0	2376000	193
11.08	1755600	166	11.0	2385000	193
11.13	1762200	166	11.1	2394000	193
11.17	1768800	166	11.1	2403000	194
11.21	1775400	166	11.2	2412000	194
11.25	1782000	167	11.2	2421000	194
11.29	1788600	167	11.3	2430000	195
11.33	1795200	167	11.3	2439000	195
11.38	1801800	168	11.3	2448000	195
11.42	1808400	168	11.4	2457000	196
11.46	1815000	168	11.4	2466000	196
11.50	1821600	169	11.5	2475000	197
11.54	1828200	169	11.5	2484000	197
11.58	1834800	169	11.5	2493000	197

11.63	1841400	170	11.6	2502000	198
11.67	1848000	170	11.6	2511000	198
11.71	1854600	170	11.7	2520000	198
11.75	1861200	170	11.7	2529000	199
11.79	1867800	171	11.8	2538000	199
11.83	1874400	171	11.8	2547000	199
11.88	1881000	171	11.8	2556000	200
11.92	1887600	172	11.9	2565000	200
11.96	1894200	172	11.9	2574000	200
12.00	1900800	172	12.0	2583000	201
12.04	1907400	173	12.0	2592000	201
12.08	1914000	173	12.0	2601000	201
12.13	1920600	173	12.1	2610000	202
12.17	1927200	173	12.1	2619000	202
12.21	1933800	174	12.2	2628000	202
12.25	1940400	174	12.2	2637000	203
12.29	1947000	174	12.3	2646000	203
12.33	1953600	175	12.3	2655000	204
12.38	1960200	175	12.3	2664000	204
12.42	1966800	175	12.4	2673000	204
12.46	1973400	175	12.4	2682000	205
12.50	1980000	176	12.5	2691000	205
12.54	1986600	176	12.5	2700000	205
12.58	1993200	176	12.5	2709000	206
12.63	1999800	177	12.6	2718000	206
12.67	2006400	177	12.6	2727000	206
12.71	2013000	177	12.7	2736000	207
12.75	2019600	178	12.7	2745000	207
12.79	2026200	178	12.8	2754000	207
12.83	2032800	178	12.8	2763000	208
12.88	2039400	178	12.8	2772000	208
12.92	2046000	179	12.9	2781000	208
12.96	2052600	179	12.9	2790000	209
13.00	2059200	179	13.0	2799000	209
13.04	2065800	180	13.0	2808000	209
13.08	2072400	180	13.0	2817000	210
13.12	2079000	180	13.1	2826000	210
13.17	2085600	180	13.1	2835000	210
13.21	2092200	181	13.2	2844000	211
13.25	2098800	181	13.2	2853000	211
13.29	2105400	181	13.2	2862000	211
13.33	2112000	182	13.3	2871000	212
13.37	2118600	182	13.3	2880000	212
13.42	2125200	182	13.4	2889000	212
13.46	2131800	182	13.4	2898000	213
13.50	2138400	183	13.5	2907000	213
13.54	2145000	183	13.5	2916000	213
13.58	2151600	183	13.5	2925000	214
13.62	2158200	184	13.6	2934000	214
13.67	2164800	184	13.6	2943000	214
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13.75	2178000	184	13.7	2961000	215
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14.17	2244000	187	14.1	3051000	218
14.21	2250600	187	14.2	3060000	219
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14.33	2270400	188	14.3	3087000	219
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14.58	2310000	190	14.5	3141000	221
14.62	2316600	190	14.6	3150000	222
14.67	2323200	190	14.6	3159000	222
14.71	2329800	191	14.7	3168000	222
14.75	2336400	191	14.7	3177000	223
14.79	2343000	191	14.7	3186000	223
14.83	2349600	191	14.8	3195000	223
14.87	2356200	192	14.8	3204000	224
14.92	2362800	192	14.9	3213000	224
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15.00	2376000	193	15.0	3231000	225
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15.17	2402400	194	15.1	3267000	226
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15.42	2442000	195	15.4	3321000	228
15.46	2448600	195	15.4	3330000	228
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19	9.13	3029400	217	19.1	4122000	254
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19	9.83	3141600	221	19.8	4275000	258
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29.67	4699200	271	29.6	6399000	316
29.71	4705800	271	29.7	6408000	316
29.75	4712400	271	29.7	6417000	316
29.79	4719000	271	29.8	6426000	317
29.83	4725600	272	29.8	6435000	317
29.88	4732200	272	29.8	6444000	317
29.92	4738800	272	29.9	6453000	317
29.96	4745400	272	29.9	6462000	318
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30.21	4785000	273	30.2	6516000	319
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30.29	4798200	274	30.3	6534000	319
30.33	4804800	274	30.3	6543000	320

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30.42	4818000	274	30.4	6561000	320
30.46	4824600	274	30.4	6570000	320
30.50	4831200	275	30.5	6579000	320
30.54	4837800	275	30.5	6588000	321
30.58	4844400	275	30.5	6597000	321
30.63	4851000	275	30.6	6606000	321
30.67	4857600	275	30.6	6615000	321
30.71	4864200	275	30.7	6624000	321
30.75	4870800	276	30.7	6633000	322
30.79	4877400	276	30.8	6642000	322
30.83	4884000	276	30.8	6651000	322
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30.92	4897200	276	30.9	6669000	323
30.96	4903800	277	30.9	6678000	323
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31.08	4923600	277	31.0	6705000	323
31.13	4930200	277	31.1	6714000	324
31.17	4936800	278	31.1	6723000	324
31.21	4943400	278	31.2	6732000	324
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31.33	4963200	278	31.3	6759000	325
31.38	4969800	278	31.3	6768000	325
31.42	4976400	279	31.4	6777000	325
31.46	4983000	279	31.4	6786000	325
31.50	4989600	279	31.5	6795000	326
31.54	4996200	279	31.5	6804000	326
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31.67	5016000	280	31.6	6831000	326
31.71	5022600	280	31.7	6840000	327
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31.79	5035800	280	31.8	6858000	327
31.83	5042400	280	31.8	6867000	327
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31.92	5055600	281	31.9	6885000	328
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32.75	5187600	285	32.7	7065000	332
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32.88	5207400	285	32.8	7092000	333
32.92	5214000	285	32.9	7101000	333
32.96	5220600	285	32.9	7110000	333
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33.04	5233800	286	33.0	7128000	333
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33.13	5247000	286	33.1	7146000	334
33.17	5253600	286	33.1	7155000	334
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33.54	5313000	288	33.5	7236000	336
33.58	5319600	288	33.5	7245000	336
33.63	5326200	288	33.6	7254000	336
33.67	5332800	288	33.6	7263000	337
33.71	5339400	289	33.7	7272000	337
33.75	5346000	289	33.7	7281000	337
33.79	5352600	289	33.8	7290000	337
33.83	5359200	289	33.8	7299000	337
33.88	5365800	289	33.8	7308000	338
33.92	5372400	290	33.9	7317000	338
33.96	5379000	290	33.9	7326000	338
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34.13	5405400	290	34.1	7362000	339
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35.00	5544000	294	35.0	7551000	343
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35.13	5563800	295	35.1	7578000	344
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35.25	5583600	295	35.2	7605000	344
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35.38	5603400	296	35.3	7632000	345
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35.96	5695800	298	35.9	7758000	348
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36.04	5709000	298	36.0	7776000	348
36.08	5715600	299	36.0	7785000	349
36.13	5722200	299	36.1	7794000	349
36.17	5728800	299	36.1	7803000	349
36.21	5735400	299	36.2	7812000	349
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36.42	5768400	300	36.4	7857000	350
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36.50	5781600	300	36.5	7875000	351
36.54	5788200	301	36.5	7884000	351
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36.79	5827800	302	36.8	7938000	352
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36.92	5847600	302	36.9	7965000	353
36.96	5854200	302	36.9	7974000	353
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37.08	5874000	303	37.0	8001000	353
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37.50	5940000	304	37.5	8091000	355
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37.58	5953200	305	37.5	8109000	356
37.63	5959800	305	37.6	8118000	356
37.67	5966400	305	37.6	8127000	356
37.71	5973000	305	37.7	8136000	356
37.75	5979600	305	37.7	8145000	356
37.79	5986200	306	37.8	8154000	357
37.83	5992800	306	37.8	8163000	357
37.88	5999400	306	37.8	8172000	357
37.92	6006000	306	37.9	8181000	357
37.96	6012600	306	37.9	8190000	357
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38.04	6025800	307	38.0	8208000	358
38.08	6032400	307	38.0	8217000	358
38.13	6039000	307	38.1	8226000	358
38.17	6045600	307	38.1	8235000	358
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38.42	6085200	308	38.4	8289000	360
38.46	6091800	308	38.4	8298000	360
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38.54	6105000	309	38.5	8316000	360
38.58	6111600	309	38.5	8325000	360
38.63	6118200	309	38.6	8334000	361
38.67	6124800	309	38.6	8343000	361
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38.83	6151200	310	38.8	8379000	362
38.88	6157800	310	38.8	8388000	362
38.92	6164400	310	38.9	8397000	362
38.96	6171000	310	38.9	8406000	362
39.00	6177600	310	39.0	8415000	362
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39.46	6250200	312	39.4	8514000	364
39.50	6256800	312	39.5	8523000	365
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39.58	6270000	313	39.5	8541000	365
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39.67	6283200	313	39.6	8559000	365
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39.75	6296400	313	39.7	8577000	366
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39.87	6316200	314	39.8	8604000	366
39.92	6322800	314	39.9	8613000	367
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40.17	6362400	315	40.1	8667000	368
40.21	6369000	315	40.2	8676000	368
40.25	6375600	315	40.2	8685000	368
40.29	6382200	316	40.2	8694000	368
40.33	6388800	316	40.3	8703000	369
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40.42	6402000	316	40.4	8721000	369
40.46	6408600	316	40.4	8730000	369
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40.62	6435000	317	40.6	8766000	370
40.67	6441600	317	40.6	8775000	370
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40.87	6474600	318	40.8	8820000	371
40.92	6481200	318	40.9	8829000	371
40.96	6487800	318	40.9	8838000	371
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41.46	6567000	320	41.4	8946000	374
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41.67	6600000	321	41.6	8991000	375
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41.83	6626400	322	41.8	9027000	375
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42.71	6765000	325	42.7	9216000	379
42.75	6771600	325	42.7	9225000	379
42.79	6778200	325	42.7	9234000	380
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44.17	6996000	330	44.1	9531000	386
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44.37	7029000	331	44.3	9576000	387
44.42	7035600	331	44.4	9585000	387
44.46	7042200	331	44.4	9594000	387
44.50	7048800	332	44.5	9603000	387
44.54	7055400	332	44.5	9612000	387
44.58	7062000	332	44.5	9621000	387
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44.67	7075200	332	44.6	9639000	388
44.71	7081800	332	44.7	9648000	388
44.75	7088400	333	44.7	9657000	388
44.79	7095000	333	44.7	9666000	388
44.83	7101600	333	44.8	9675000	389
44.87	7108200	333	44.8	9684000	389
44.92	7114800	333	44.9	9693000	389

44.96	7121400	333	44.9	9702000	389
45.00	7128000	333	45.0	9711000	389
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45.21	7161000	334	45.2	9756000	390
45.25	7167600	334	45.2	9765000	390
45.29	7174200	335	45.2	9774000	391
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45.67	7233600	336	45.6	9855000	392
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45.75	7246800	336	45.7	9873000	392
45.79	7253400	336	45.7	9882000	393
45.83	7260000	337	45.8	9891000	393
45.87	7266600	337	45.8	9900000	393
45.92	7273200	337	45.9	9909000	393
45.96	7279800	337	45.9	9918000	393
46.00	7286400	337	46.0	9927000	394
46.04	7293000	337	46.0	9936000	394
46.08	7299600	337	46.0	9945000	394
46.12	7306200	338	46.1	9954000	394
46.17	7312800	338	46.1	9963000	394
46.21	7319400	338	46.2	9972000	394
46.25	7326000	338	46.2	9981000	395
46.29	7332600	338	46.2	9990000	395
46.33	7339200	338	46.3	9999000	395
46.37	7345800	339	46.3	10008000	395
46.42	7352400	339	46.4	10017000	395
46.46	7359000	339	46.4	10026000	396
46.50	7365600	339	46.5	10035000	396
46.54	7372200	339	46.5	10044000	396
46.58	7378800	339	46.5	10053000	396
46.62	7385400	339	46.6	10062000	396
46.67	7392000	340	46.6	10071000	396
46.71	7398600	340	46.7	10080000	397
46.75	7405200	340	46.7	10089000	397
46.79	7411800	340	46.7	10098000	397
46.83	7418400	340	46.8	10107000	397
46.87	7425000	340	46.8	10116000	397
46.92	7431600	341	46.9	10125000	397
46.96	7438200	341	46.9	10134000	398
47.00	7444800	341	47.0	10143000	398
.,	, 111000	J-1	77.0	131-13000	330

47.04	7451400	341	47.0	10152000	398
47.08	7458000	341	47.0	10161000	398
47.12	7464600	341	47.1	10170000	398
47.17	7471200	341	47.1	10179000	399
47.21	7477800	342	47.2	10188000	399
47.25	7484400	342	47.2	10197000	399
47.29	7491000	342	47.2	10206000	399
47.33	7497600	342	47.3	10215000	399
47.37	7504200	342	47.3	10224000	399
47.42	7510800	342	47.4	10233000	400
47.46	7517400	342	47.4	10242000	400
47.50	7524000	343	47.5	10251000	400
47.54	7530600	343	47.5	10260000	400
47.58	7537200	343	47.5	10269000	400
47.62	7543800	343	47.6	10278000	400
47.67	7550400	343	47.6	10287000	401
47.71	7557000	343	47.7	10296000	401
47.75	7563600	344	47.7	10305000	401
47.79	7570200	344	47.7	10314000	401
47.83	7576800	344	47.8	10323000	401
47.87	7583400	344	47.8	10332000	402
47.92	7590000	344	47.9	10341000	402
47.96	7596600	344	47.9	10350000	402
48.00	7603200	344	48.0	10359000	402
48.04	7609800	345	48.0	10368000	402
48.08	7616400	345	48.0	10377000	402
48.12	7623000	345	48.1	10386000	403
48.17	7629600	345	48.1	10395000	403
48.21	7636200	345	48.2	10404000	403
48.25	7642800	345	48.2	10413000	403
48.29	7649400	345	48.2	10422000	403
48.33	7656000	346	48.3	10431000	403
48.37	7662600	346	48.3	10440000	404
48.42	7669200	346	48.4	10449000	404
48.46	7675800	346	48.4	10458000	404
48.50	7682400	346	48.5	10467000	404
48.54	7689000	346	48.5	10476000	404
48.58	7695600	347	48.5	10485000	404
48.62	7702200	347	48.6	10494000	405
48.67	7708800	347	48.6	10503000	405
48.71	7715400	347	48.7	10512000	405
48.75	7722000	347	48.7	10521000	405

ERM Page 24 of 24 PN 0178173 2/11/2021

BNSF Railway Black Tank Property **DRAFT ENVIRONMENTAL COVENANT APPENDIX F**

ENGINEERING DESIGN REPORT

www.erm.com Version: 1.0 Project No.: 0578173 Client: BNSF, Husky November 2021

After Recording Return

Original Signed Covenant to: 1

Jeremy Schmidt, P.E.

Toxics Cleanup Program

Department of Ecology

Eastern Regional Office

4601 N. Monroe Street

Spokane, WA 99205-1295

Environmental Covenant

(For MTCA Sites – August 20, 2015 Version)

Grantors: BNSF Railway Company (BNSF) ²

Grantee: State of Washington, Department of Ecology (hereafter "Ecology")

Brief Legal Description: Generally congruent with property located at 3202 East Wellesley

Avenue in Spokane, Spokane County, Washington

Tax Parcel Nos.: BNSF parcels #35032.4401 and #35032.4501 and

Cross Reference: Consent Decree (CD) No. 19203114-32 (2019) between BNSF, Marathon Oil

Company, and Ecology. Marathon Oil Company's obligations for this project are being performed by Husky Oil Operations Limited (HOOL).

Prior to CD was Agreed Order No. 9188.

RECITALS 3

- **a.** This document is an environmental (restrictive) covenant (hereafter "Covenant") executed pursuant to the Model Toxics Control Act ("MTCA"), chapter 70.105D RCW, and Uniform Environmental Covenants Act ("UECA"), chapter 64.70 RCW.
- **b.** The Property that is the subject of this Covenant is part or all of a site commonly known as "BNSF Railway Black Tank Property Site," Facility Site Identification (ID) No. 98615712. The Property is legally described in Exhibit A, and illustrated in Exhibit B, both of which are attached (hereafter "Property"). If there are differences between these two Exhibits, the legal description in Exhibit A shall prevail.
- c. Portions of the Property are the subject of remedial action conducted under MTCA. Exhibit C illustrates the portions of the Property affected by this Covenant (hereafter the "Affected Property"). This Covenant is required because residual contamination will remain on the Affected Property during and after completion of remedial actions. Specifically, the following principal contaminants will remain on the Affected Property: ⁴

¹ Some counties keep the original Covenant, others don't. If the signed original is available, it must be sent to Ecology. If the signed original is not available, send a legible copy to Ecology.

² The Grantor of a Covenant typically is the fee simple land owner of the property. The Grantor may also include holders of other property interests such as a holder of an easement, right-of-way, mineral right, lien, or mortgage.

³ This section is primarily used to describe this document and its purpose. It should not be used for substantive binding provisions.

⁴ List the contaminants for the associated media. If more than a few are present, list the top three to five for each medium.

NOTE: BNSF is working to refine the extent of property impacted and evaluating various ownership interests.

Medium	Principal Contaminants Present			
Soil	TPH-D/HO, LNAPL, cadmium, cPAHs, and naphthalenes			
Groundwater	TPH-D/HO, LNAPL			
Surface Water/Sediment	None			

- **d.** It is the purpose of this Covenant to restrict certain activities and uses of the Property to protect human health and the environment and the integrity of remedial actions conducted at the site. Records describing the extent of residual contamination and remedial actions conducted are available through Ecology. This includes a Remedial Investigation and Feasibility Study (RI/FS), Cleanup Action Plan (CAP), Surface Soil Interim Action Completion Report, and Engineering Design Report.
- **e.** This Covenant grants Ecology certain rights under UECA and as specified in this Covenant. As a Holder of this Covenant under UECA, Ecology has an interest in real property, however, this is not an ownership interest which equates to liability under MTCA or the Comprehensive Environmental Response, Compensation, and Liability Act, 42 U.S.C. § 9601 *et seq.* The rights of Ecology as an "agency" under UECA, other than its' right as a holder, are not an interest in real property.

COVENANT

BNSF, as Grantor ⁵ and FEE SIMPLE, EASEMENT OR OTHER owner of the Property hereby grants to the Washington State Department of Ecology, and its successors and assignees, the following covenants. Furthermore, it is the intent of the Grantors that such covenants shall supersede any prior interests the GRANTORS have in the property and run with the land and be binding on all current and future owners of any portion of, or interest in, the Property.

Section 1. General Restrictions and Requirements.

The following general restrictions and requirements shall apply to the Property:

- **a. Interference with Remedial Action**. The Grantors shall not engage in any activity on the Property that may impact or interfere with the remedial action and any operation, maintenance, inspection or monitoring of that remedial action without prior written approval from Ecology.
- **b. Protection of Human Health and the Environment**. The Grantors shall not engage in any activity on the Property that may threaten continued protection of human health or the environment without prior written approval from Ecology. This includes, but is not limited to, any activity that results in the release of residual contamination that was contained as a part of the remedial action or that exacerbates or creates a new exposure to residual contamination remaining on the Property.

-

⁵ If there is more than one Grantor, use the term "Grantors" here and throughout this document.

- **c. Continued Compliance Required.** Grantors shall not convey any interest in any portion of the Property without providing for the continued adequate and complete operation, maintenance and monitoring of remedial actions and continued compliance with this Covenant.
- **d.** Leases. Grantors shall restrict any lease for any portion of the Property to uses and activities consistent with this Covenant and notify all lessees of the restrictions on the use of the Property.
- **e. Preservation of Reference Monuments.** Grantors shall make a good faith effort to preserve any reference monuments and boundary markers used to define the areal extent of coverage of this Covenant. Should a monument or marker be damaged or destroyed, Grantors shall have it replaced by a licensed professional surveyor within 30 days of discovery of the damage or destruction.

Section 2. Specific Prohibitions and Requirements.

In addition to the general restrictions in Section 1 of this Covenant, the following additional specific restrictions and requirements shall apply to the Property.

- **a.** Land use BNSF's property is zoned Light Industrial, and WSDOT's easement is zoned Center and Corridor Core, which allows for many types of uses including commercial, office, residential, and parks.
- b. Containment of soil/waste materials Any drilling or excavation on the site is prohibited for purposes other than (i) remediation or (ii) construction, operation or maintenance of the North Spokane Corridor (NSC) Project or interstate railroad facilities.
- c. Stormwater facilities To minimize the potential for mobilization of contaminants remaining in the soil and groundwater on the Property, no stormwater infiltration facilities or ponds shall be constructed over any portion of the LNAPL plume (the high, medium, and low RTF areas) on the affected Property as illustrated in Exhibit C. All stormwater catch basins, conveyance systems, and other appurtenances located over these areas shall be of water-tight construction.
- d. Vapor/gas controls Not applicable.
- e. Groundwater use The groundwater beneath the property remains contaminated and shall not be extracted for any purpose other than temporary construction dewatering, investigation, monitoring or remediation. Drilling of a well for any water supply purpose is strictly prohibited. Groundwater extracted from the Affected Property for any purpose shall be considered potentially contaminated and any discharge of this water shall be done in accordance with state and federal law.
- f. Sediments Not applicable.
- g. Monitoring Any monitoring of groundwater for purposes other than remediation is prohibited. Several groundwater monitoring wells and vapor monitoring wells are located on the Affected Property to monitor the performance of the remedial action. The Grantors shall maintain clear access to these devices and protect them from damage. The Grantors shall report to Ecology within forty-eight (48) hours of the discovery of any damage to any monitoring device. Unless Ecology approves of an alternative plan in writing, the Grantors shall promptly repair the damage and submit a report documenting this work to Ecology within thirty (30) days of completing the repairs.
- **h.** Other. Not applicable.

Section 3. Access.

a. The Grantors shall maintain clear access to all remedial action components necessary to construct, operate, inspect, monitor and maintain the remedial action.

- b. The Grantors freely and voluntarily grants Ecology and its authorized representatives, upon reasonable notice for security and safety purposes and subject to Ecology and its authorized representatives' compliance with railroad safety procedures, the right to enter the Property at reasonable times to evaluate the effectiveness of this Covenant and associated remedial actions, and enforce compliance with this Covenant and those actions, including the right to take samples, inspect any remedial actions conducted on the Property, and to inspect related records.
- **c.** No right of access or use by a third party to any portion of the Property is conveyed by this instrument.

Section 4. Notice Requirements.

- **a. Conveyance of Any Interest.** The Grantors, when conveying any interest in the Affected PROPERTY DESCRIBED AND ILLUSTRATED IN EXHIBIT C, including but not limited to title, easement, leases, and security or other interests, must:
 - i. Provide written notice to Ecology of the intended conveyance at least thirty (30) days in advance of the conveyance.⁶
 - **ii**. Include in the conveying document a notice in substantially the following form, as well as a complete copy of this Covenant:
 - NOTICE: THIS PROPERTY IS SUBJECT TO AN ENVIRONMENTAL COVENANT GRANTED TO THE WASHINGTON STATE DEPARTMENT OF ECOLOGY ON DATE AND RECORDED WITH THE SPOKANE COUNTY AUDITOR UNDER RECORDING NUMBER RECORDING NUMBER RECORDING NUMBER RECORDING NUMBER OF WHICH IS ATTACHED TO THIS DOCUMENT.
 - **iii.** Unless otherwise agreed to in writing by Ecology, provide Ecology with a complete copy of the executed document within thirty (30) days of the date of execution of such document.
- **b. Reporting Violations.** Should the Grantors become aware of any violation of this Covenant, Grantors shall promptly report such violation in writing to Ecology.
- **c. Emergencies.** For any emergency or significant change in site conditions resulting in a violation of this Covenant, the Grantors are authorized to respond to such an event in accordance with state and federal law. The Grantors must notify Ecology in writing of the event and response actions planned or taken as soon as practical but no later than within 24 hours of the discovery of the event.

Publication Number: 15-09-054 Attachment C page 4 Revised: December 22, 2016

⁶ Ecology may waive this notice provision for some units at a Property where the anticipated use is a multi-tenant/owner building where some owners or tenants are unlikely to be exposed to residual contamination. For example: upper story apartments or condominiums, or commercial tenants in a strip mall, with limited rights to use the grounds under and around the building (such as for parking).

If Ecology agrees to such a waiver, the circumstances of the waiver must be detailed in paragraph 4.a.i. In addition to the specific circumstances, this provision must include the following statement: "Waiver of this advance notice to Ecology for these transactions does not constitute waiver of this notice for the entire Property nor a waiver of the requirement in Section 4.a.ii. to include this notice in any document conveying interest in the Property."

d. Notification procedure. Any required written notice, approval, reporting or other communication shall be personally delivered or sent by first class mail to the following persons. Any change in this contact information shall be submitted in writing to all parties to this Covenant. Upon mutual agreement of the parties to this Covenant, an alternative to personal delivery or first class mail, such as e-mail or other electronic means, may be used for these communications.



Shane DeGross

BNSF Railway Company

605 Puyallup Avenue
Tacoma, WA 98134

(253) 591-2567
Shane DeGross@BNSF.com

Environmental Covenants Coordinator
Washington State Department of Ecology
Toxics Cleanup Program
P.O. Box 47600
Olympia, WA 98504 – 7600
(360) 407-6000
ToxicsCleanupProgramHQ@ecy.wa.gov

Section 5. Modification or Termination.

- **a.** Grantors must provide written notice and obtain approval from Ecology at least sixty (60) days in advance of any proposed activity or use of the Property in a manner that is inconsistent with this Covenant. ⁷ For any proposal that is inconsistent with this Covenant and permanently modifies an activity or use restriction at the site: ⁸
- i. Ecology must issue a public notice and provide an opportunity for the public to comment on the proposal; and
- ii. If Ecology approves of the proposal, the Covenant must be amended to reflect the change before the activity or use can proceed.
- **b.** If the conditions at the site requiring a Covenant have changed or no longer exist, then the Grantors may submit a request to Ecology that this Covenant be amended or terminated. Any amendment or termination of this Covenant must follow the procedures in MTCA and UECA and any rules promulgated under these chapters.⁹

Section 6. Enforcement and Construction.

- **a.** This Covenant is being freely and voluntarily granted by the Grantors.
- **b.** Within ten (10) days of execution of this Covenant, Grantors shall provide Ecology with an original signed Covenant and proof of recording and a copy of the Covenant and proof of recording to others required by RCW 64.70.070.
- c. Ecology shall be entitled to enforce the terms of this Covenant by resort to specific performance or legal process. All remedies available in this Covenant shall be in addition to any and all remedies at law or in equity, including MTCA and UECA. Enforcement of the terms of this Covenant shall be at the discretion of Ecology, and any forbearance, delay or omission to exercise its rights under this Covenant in the event of a breach of any term of this Covenant is

⁷ Example of inconsistent uses are using the Property for a use not allowed under the covenant (i.e. mixed residential and commercial use on a property restricted to industrial uses), OR drilling a water supply well when use of the groundwater for water supply is prohibited by the covenant.

⁸ An example of an activity that is unlikely to be considered a permanent modification is a proposal to disturb a cap to repair an existing underground utility that passes through the site. However, installing a new underground utility within a capped area would be a permanent change.

⁹ As time passes, the original grantor and other signers of the Covenant may no longer exist as viable entities. This provision is intended to allow future amendments or termination of the Covenant without Ecology having to seek court authorization, as provided by RCW 64.70.100.

not a waiver by Ecology of that term or of any subsequent breach of that term, or any other term in this Covenant, or of any rights of Ecology under this Covenant.

- **d.** The Grantors shall be responsible for all costs associated with implementation of this Covenant. Furthermore, the Grantors, upon request by Ecology, shall be obligated to pay for Ecology's costs to process a request for any modification or termination of this Covenant and any approval required by this Covenant.
- **e.** This Covenant shall be liberally construed to meet the intent of MTCA and UECA.
- f. The provisions of this Covenant shall be severable. If any provision in this Covenant or its application to any person or circumstance is held invalid, the remainder of this Covenant or its application to any person or circumstance is not affected and shall continue in full force and effect as though such void provision had not been contained herein.
- **g.** A heading used at the beginning of any section or paragraph or exhibit of this Covenant may be used to aid in the interpretation of that section or paragraph or exhibit but does not override the specific requirements in that section or paragraph.
- Neither Ecology nor Grantors intend to include any third party beneficiaries with enforcement rights under this Covenant.
- g. By signing this Covenant, the BNSF does not intend to affect the scope of existing preemption under federal law, including but not limited to the Interstate Commerce Commission Termination Act, 49 U.S.C. § 100501.

[GRANTOR'S SIGNATURE BLOCK FOR ORIGINAL COVENANTS]

Each person who signs must have a separate signature block and applicable notary acknowledgment. Repeat as many times as necessary.

Holders of other property interests must either sign the amended Covenant as a GRANTOR or sign the subordination agreement in Exhibit D.

The undersigned Grantor warrants he/she holds the title [to the Property] OR [to an (Easement/Right of Way/etc.) on the Property] and has authority to execute this Covenant.

	EXECUTED this	day of	 , 20
	[Signature]		
oy:	PRINTED NAME]		
Γitle:			

Insert one of the following, as applicable after each signature. See example format on page after next:

INDIVIDUAL ACKNOWLEDGMENT
CORPORATE ACKNOWLEDGMENT
REPRESENTATIVE ACKNOWLEDGEMENT

INDIVIDUAL ACKNOWLEDGMENT

STATE OF	
COUNTY OF	_
personally appeared before me, ackn	
	Notary Public in and for the State of Washington ¹⁰ Residing at My appointment expires
CT ATE OF	CORPORATE ACKNOWLEDGMENT
STATE OFCOUNTY OF	
On this day of	, 20, I certify that
	knowledged that he/she is the
by free and voluntary act and deed of	e within and foregoing instrument, and signed said instrument of said corporation, for the uses and purposes therein mentioned authorized to execute said instrument for said corporation.
	Notary Public in and for the State of Washington ¹⁵
	Residing at My appointment expires
STATE OFCOUNTY OF	REPRESENTATIVE ACKNOWLEDGEMENT
personally appeared before me, act that he/she was authorized to	, 20, I certify that
PARTY BEING REPRESENTED to be and purposes mentioned in the instru	the free and voluntary act and deed of such party for the uses
	Notary Public in and for the State of Washington ¹⁵ Residing at
	My appointment expires

¹⁰ Where landowner is located out of state, replace with appropriate out-of-state title and location.

Publication Number: 15-09-054 Attachment C page 8 Revised: December 22, 2016

[ECOLOGY'S SIGNATURE BLOCK]

The Department of Ecology, hereby accepts the status as GRANTEE and HOLDER of the above Environmental Covenant.

STATE OF WASHINGTON	
DEPARTMENT OF ECOLOGY	
[Signature]	
by: [Printed name]	
Title:	
Dated:	
	STATE ACKNOWLEDGMENT
STATE OF	_
COUNTY OF	_
On this day of	_, 20, I certify that
personally appeared before me, acknowledge	_, 20, 1 certify that d that he/she is the
	and foregoing instrument, and signed said instrument by
	and purposes therein mentioned, and on oath stated that
he/she was authorized to execute said instrun	nent for said state agency.
	Notary Public in and for the State of Washington
	Residing at
	Residing at
	My appointment expires

Exhibit A

LEGAL DESCRIPTION



NOTE: BNSF is working to refine the extent of property impacted and is evaluating various ownership interests.

Parcel Number: 35032.4501

MINNEHAHA ADDITION NORTH BEG AT NE COR OF LT 6, BK 108, TH S ALG E LNS OF STS 6 & 7 OF BK 108 TO SE COR OF LT 7, TH W ALG S LN OF LT 7 & ALG SD S LN EXTENDED W ACROSS RALPH ST TO SE COR OF BK 109, THS ALG E LN OF BK 109 EXTENDED S ACROSS HOFFMAN AVE TO NE COR BK 102, TH CONTINUING S ALG E LN OF BK 102 TO SE COR OF BK 102, TH W ALG S LN OF BK 102 TO SE COR OF LT 7 IN SD BK 102, TH S ALG E LN OF SDLT 7 EXTENDED S ACROSS HEROY AVE TO NE COR OF LT 3 OF BK 95, TH CONTINUING S ALG E LNS OF LTS 3, 4, 5, 6 & 7 OF BK 95 & ACROSS LONGFELLOW AVE TO NE COR OF LT 3 OF BK 88 & CONTINUING S ALG E LNS OF LTS3, 4, 5, 6 & 7 OF BK 88 & ACROSS RICH AVE TO NE COR OF LT 3 OF BK 81, TH W ALG N LN OF LT 3 TO NW COR OF SD LT 3 OF BK 81, TH N ACROSS RICH AVE TO SW COR OF BK 88, TH CONTINUING N ALG W LNS OF BKS88, 95, 102 & 109 A ALG SD W LNS EXTENDED ACROSS LONGFELLOW AVE & HEROY AVE & HOFFMAN AVE TO NW COR OF BK 109, TH E ALG N LN OF BK 109 & ALG SD N LN EXTENDED ACROSS RALPH ST TO NW COR OF LT 6 OF BK108 & ALG N LN OF SD LT 6 TO POB & INCLUDING ALL STREETS & AVENUES & ALLEYS CONTAINED WITHIN THE ABOVE DESCRIPTION

Parcel Number: 35032.4401

MINNEHAHA ADDITION NORTH BEG AT NE COR OF LT 6, BLK 108; TH S ALG E LNS OF LTS 6 & 7 OF BLK 108 TO SE COR OF LT 7: TH W ALG S LN OF LT 7 & ALG SD S LN EXTENDED W ACROSS RALPH ST TO SE COR OF BLK 109;TH S ALG E LN OF BLK 109 EXTENDED S ACROSS HOFFMAN AVE TO NE COR OF BLK 102; TH CONTINUING S ALG E LN OF BLK 102 TO SE COR OF BLK 102; TH W ALG S LN OF BLK 102 TO SE COR OF LT 7 IN SD BLK 102; TH SALG E LN OF SD LT 7 EXTENDED S ACROSS HEROY AVE TO NE COR OF LT 3 OF BLK 95; TH CONTINUING S ALG E LNS OF LOTS 3, 4, 5, 6, & 7 OF BLK 95 & ACROSS LONGFELLOW AVE TO NE COR OF LT 3 OF BLK 88 & CONTINUING S ALG E LNS OF LTS 3. 4. 5. 6. & 7 OF BLK 88 & ACROSS RICH AVE TO NE COR OF LT 3 OF BLK 81; TH W ALG N LN OF LT 3 TO NW COR OF SD LT 3 & OF SD BLK 81; TH S ALG W LN OF BLK 81 TO SW COR OFBLK 81; TH E ALG S LN OF SD BLK TO SE COR OF SD BLK 81; TH N ALG E LN OF BLK 81 & ALG SD E LN EXTENDED N ACROSS RICH AVE TO SE COR OF BLK 88; TH E ALG S LN EXTENDED E OF BLK 88 ACROSS RALPH ST TO SWCOR OF BLK 89: TH CONTINUING E ALG S LNS OF BLKS 89 & 90 & ALG SD S LNS EXTENDED ACROSS THOR ST TO SE COR OF BLK 90; TH N ALG E LNS OF BLKS 90, 93, 104, & 107 & ALG SD E LNS EXTENDED ACROSS LONGFELLOWAVE & HEROY AVE & HOFFMAN AVE TO NE COR OF BLK 107; TH W ALG N LNS OF BLKS 107 & 108 & ALG SD N LNS EXTENDED ACROSS THOR ST TO NE COR OF LT 6 OF BLK 108 & POB & INCLUDING ALL THE STREETS & AVENUES &ALLEYS CONTAINED WITHIN THE ABOVE DESCRIPTION NOW FULLY VACATED.

Exhibit B

PROPERTY MAP



NOTE: BNSF is working to refine the extent of property impacted and is evaluating various ownership interests.

Parcel No. 35032.4401



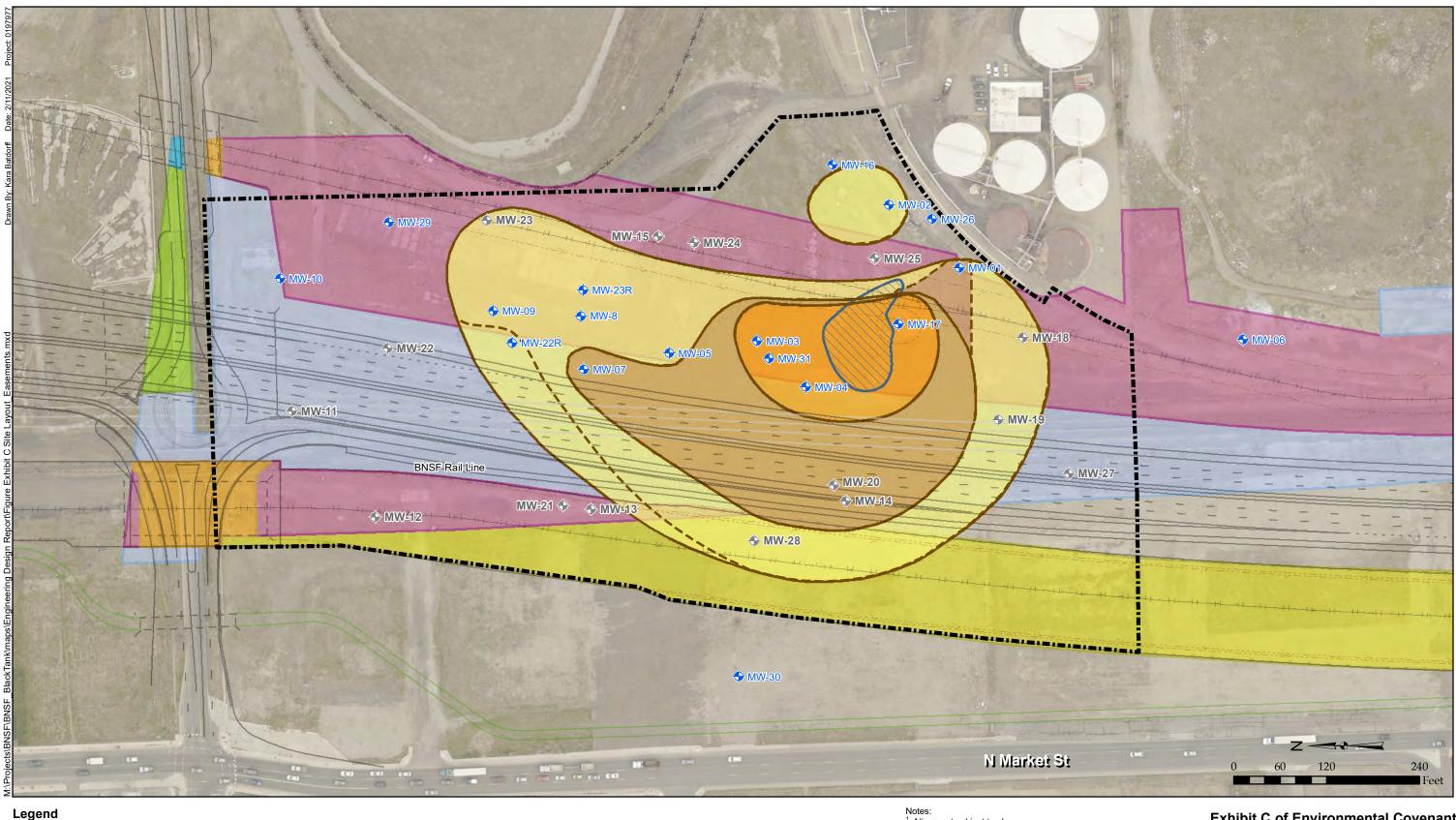
Parcel No. 35032.4501



Exhibit C

MAP ILLUSTRATING LOCATION OF RESTRICTIONS





Monitoring Well

Monitoring Well - Abandoned September 2019

TPH-D/HO in Intermediate Soil Exceeding the CUL

High RTF Area Medium RTF Area Low RTF Area

Location of Restrictions Alta Area

BN Leasing Crossing Easement BN Leasing Perm. Easement

BNSF Crossing Easement

BNSF Perm. Easement to

— — Bridge

---- Ditch

--- Fence

Pedestrian Pathway

Curb/Sidewalk

BNSF TCE

WSDOT Proposed Highway Alignment (2018 Version)

Proposed Railroad Alignment - Centerline

Notes:

¹ Alignment subject to change
CUL = Cleanup Level
LNAPL = Light Non-Aqueous Phase Liquid
RTF: Restoration Timeframe NSC = North Spokane Corridor TPH-D/HO = Combined Diesel and Heavy Oil-Range Petroleum Hydrocarbons CUL = 5,360 milligrams per kilogram WSDOT = Washington State Department of Transportation Aerial Photo: Spokane Image Consortium, 2018.

Exhibit C of Environmental Covenant

Site Layout and Location of Restrictions Engineering Design Report BNSF Black Tank Spokane, Washington

> es Management www.erm.com ERM **Environmental Resources Management**



Exhibit D

SUBORDINATION AGREEMENT

KNOW ALL PERSONS, That [HOLDER'S NAME], the owner and holder of that certain
[INSTRUMENT – E.G. EASEMENT/ROW/MORTGAGE/ETC.] bearing the date the day
of[MONTH], [YEAR], executed by[NAME OF PERSON THAT GRANTED THE INTEREST
BEING SUBORDINATED],[LEGAL STATUS OF ORIGINAL GRANTOR – E.G. LANDOWNER,
CORPORATE OFFICER, ETC.], and recorded in the office of the County Auditor of
[COUNTY] County, State of Washington, on[DATE], under Auditor's File Number
, does hereby agree that said Instrument shall be subordinate to the interest of the
State of Washington, Department of Ecology, under the environmental (restrictive) covenant
dated[Date], executed by[Name of Person Signing this Subordination
AGREEMENT], and recorded in[COUNTY] County, Washington under Auditor's File
Number
[SIGNATURE]
by:[Printed name]
Title:
Dated:

Insert one of the following, as applicable. See example format on next page:

INDIVIDUAL ACKNOWLEDGMENT CORPORATE ACKNOWLEDGMENT

REPRESENTATIVE ACKNOWLEDGEMENT

INDIVIDUAL ACKNOWLEDGMENT

STATE OF COUNTY OF	
personally appeared before me, acknowledg	, 20, I certify that
	Notary Public in and for the State of Washington ¹¹ Residing at My appointment expires
STATE OF	CORPORATE ACKNOWLEDGMENT
of the corporation that executed the within by free and voluntary act and deed of said c	
	Notary Public in and for the State of Washington ¹⁶ Residing at My appointment expires
STATE OF	REPRESENTATIVE ACKNOWLEDGEMENT
personally appeared before me, acknowle that he/she was authorized to execut	dged that he/she signed this instrument, on oath stated ate this instrument, and acknowledged it as the AUTHORITY of [NAME OF e and voluntary act and deed of such party for the uses
	Notary Public in and for the State of Washington ¹⁶ Residing at My appointment expires

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¹¹ Where landowner is located out of state, replace with appropriate out-of-state title and location.