# Technical Memorandum

- To: David South, Washington State Department of Ecology
- Copies: Paul Agid and Don Robbins, Port of Seattle
  - From: Matt Woltman
  - **Date:** August 6, 2010

Project No: POS-LLA

Re: Deep Monitoring Well Installation and Sampling and Analysis Plan for the Lora Lake Apartments Parcel Phase of the Site Remedial Investigation

# INTRODUCTION

This technical memorandum presents the proposed protocols for installing up to three deep monitoring wells at the Lora Lake (LL) Apartments Parcel, as part of the LL Apartments Parcel Phase of the Site Remedial Investigation to be completed in summer 2010. These additional monitoring wells are required per recent Washington State Department of Ecology (WSDOE) and public comments to the Draft Remedial Investigation/Feasibility Study (RI/FS) Work Plan. The purpose of completing this additional deep well investigation is to provide further information regarding subsurface geologic and hydrogeologic conditions at the LL Apartments Parcel, and to also identify if dense non-aqueous phase liquid (DNAPL) contamination has formed at depths below the vertical extents of previous LL Apartments Parcel investigations.

The proposed well installation program calls for installation of a minimum of two deep monitoring wells, with conditional installation of a third deep well pending review of data and observations collected during installation of the first two wells. The process for this deep monitoring well installation sequence is described within this technical memorandum. This technical memorandum also presents the proposed approach for collection and laboratory analysis of soil and groundwater samples as part of the deep well installation and development activities.

This proposed approach for installing the deep monitoring wells has been developed based on a recent geology/hydrogeology evaluation completed by Aspect Consulting (Aspect) with focus on the LL Apartments Parcel and surrounding areas (Aspect 2010). This evaluation was completed to provide a summary of known geologic/hydrogeologic conditions within the vicinity of the LL Apartments Parcel, and to identify potential locations and depths where subsurface confining units may be encountered. A brief

summary of the Aspect Consulting geology/hydrogeology evaluation is provided herein and a copy of the evaluation technical memorandum is provided in Attachment 1.

This deep-well technical memorandum has been prepared to serve as an addendum to the Sampling and Analysis Plan/Quality Assurance Project Plan (SAQ/QAPP), included as Appendix B of the current Lora Lake Apartments RI/FS Work Plan (Floyd|Snider 2010). Relevant information regarding project organization and responsibility; laboratory quality assurance objectives; sample handling and custody documentation; data reduction, validation and management; corrective actions; waste management; and data reporting is provided in the RI/FS Work Plan SAP/QAPP.

# GEOLOGY/HYDROGEOLOGY EVALUATION SUMMARY

The purpose of the geology/hydrogeology evaluation was to estimate the potential target depths for the first confining unit based on the review of existing geologic and groundwater information in the near vicinity of the Site (as it is described in the current Lora Lake Apartments RI/FS Work Plan), per WSDOE comments to the RI/FS Work Plan. Most of the geology/hydrogeology data used in the evaluation were obtained from a groundwater study commissioned by the Port of Seattle (Port) that describes in detail regional geology and groundwater conditions within a 42-square-mile study area surrounding the Seattle-Tacoma International Airport (STIA; Aspect and S.S. Papadopulos 2005). The groundwater study was conducted under an Agreed Order with WSDOE and represents the most current and comprehensive understanding of the geologic and hydrogeologic framework in the area surrounding the STIA (Aspect 2010). The LL Apartments Parcel is included within the northwest region of the groundwater study data collection boundary.

In summary, the Puget Lowland is underlain at depth by volcanic and sedimentary bedrock, and is filled to the present day land surface with both glacial and non-glacial deposits (non-glacial or inter-glacial deposits are those derived between periods of glaciation) that occurred during the Quaternary Period (within the last 2 million years; Aspect 2010). These glacial and non-glacial deposits (from youngest to oldest) are commonly referred to as recent post-glacial deposits. Fraser Glacial Deposits (also known as Vashon Drift), and Pre-Fraser deposits. Located within these general glacial and non-glacial deposits are sub-units of fine and coarse-grained materials that make up the regional aquifer and confining layer system.

The geology/hydrogeology evaluation examined boring logs from many deep investigations and monitoring well installations that have been completed within the vicinity of the LL Apartments Parcel (and overall Site). Although no deep investigations or monitoring well installations have been completed within the current boundaries of the LL Apartments Parcel, interpretation of the available information suggests the following site stratigraphy, from youngest to oldest units:

• **Post-glacial Deposits.** These materials are commonly identified to be fill, topsoil, recent alluvium, or lacustrine deposits that were deposited following

the most recent period of glaciation. Based on development history at the LL Apartments Parcel, a unit of fill is known to be present in the southern and central areas of the property, as it was placed as part of development of the former apartment building complex. The fill is known to have a variable thickness of up to 15 feet throughout the property, and is absent in the northern portion of the property. Based on recent explorations at the property, a sand/gravel unit is located beneath the fill (and at the surface in the northern portion of property) and is assumed to be either a recent alluvium or recessional outwash deposit. Based on the locations of previous explorations at the property, it is unclear if borings have extended below this sand/gravel unit and into the upper portion of the units below. Therefore the thickness of this unit is unknown.

• Fraser Glacial Deposits. These glacial deposits are located beneath the post-glacial deposits and contain sub-units of both sand/gravel and silt/clay materials. The Fraser Glacial Deposits consist of outwash (recessional and advance), till, and glaciolacustrine (Lawton Clay) deposits. Typically, outwash deposits are associated with aquifers and till and glaciolacustrine deposits represent confining layers.

Borings at the LL Apartments Parcel may extend into the upper portions of a recessional outwash deposit associated with the Fraser Glaciation; however, this deposit is similar in appearance and density to the recent alluvium deposit described above and it is currently unknown whether existing property borings have encountered this unit.

Underlying the recessional outwash unit is the Vashon Glacial Till. This unit represents the first potential confining layer for the LL Apartments Parcel; however, no borings have been completed at the property to sufficient depths to identify if this till unit is present. According to interpretation of deeper off-property boring logs, the Vashon Glacial Till unit (if present) is expected to be encountered at depths below ground surface (bgs) of approximately 15 to 35 feet (in the northwest area of the LL Apartments Parcel) to 45 to 65 feet (in the eastern and southeastern area of the parcel).

The Vashon Advance Outwash unit underlies the Vashon Glacial Till and is a sand/gravel aquifer unit that is expected to be approximately 35 to 40 feet in thickness at the LL Apartments Parcel. Underlying the Vashon Advance Outwash unit is the Lawton Clay, which represents a second potential confining layer and the bottom of the Fraser Glacial Deposits. The Lawton Clay (if present) is expected to be encountered at depths of approximately 80 to 100 bgs (in the northwest area of the parcel) and 95 to 115 feet bgs (in the eastern and southeastern area of the parcel).

• **Pre-Fraser Glacial Deposits.** Similar to the Fraser Glacial Deposits, the Pre-Fraser Glacial Deposits are also composed of alternating aquifer and confining layer units extending down to the bedrock contact. Less information is known about these older glacial deposits, as fewer explorations have encountered them due to their increased depth; however, it is expected that the next potential confining layer would be a Pre-Fraser fine-grained deposit that may be located at depths exceeding 200 feet bgs at the LL Apartments Parcel.

The summary stratigraphic interpretation described above represents the known deposits and sub-units within the geologic record for this region. However, glacial advance and recessional processes can often completely erode stratigraphic units or deposits, resulting in gaps in the geologic record. Although the aquifers and confining layers discussed above have been identified in borings near the LL Apartments Parcel, it is possible that these units may not be present beneath the parcel. A detailed discussion of the stratigraphic interpretation and potential for erosion of stratigraphic units at and around the LL Apartments Parcel is presented in the geology/hydrogeology evaluation technical memorandum included as Attachment 1 (Aspect 2010).

# DEEP MONITORING WELL INSTALLATION PROGRAM

The following section presents details of the proposed deep monitoring well installation program, including protocols for collection and analysis of soil and groundwater samples.

# Monitoring Well Location and Depth

Up to three deep monitoring wells (MW-15 through MW-17) are proposed for installation at the LL Apartments Parcel at the locations shown on Figure 1. Monitoring wells will be co-located with proposed primary soil borings and installed at the following locations:

- MW-15 will be co-located with Primary Soil Boring PSB-15 and installed adjacent to existing Monitoring Well MW-5. The proposed location for MW-15 will serve as a downgradient groundwater contamination monitoring point within the known extent of groundwater contamination at the LL Apartments Parcel. Additionally, the co-location of MW-15 with existing Monitoring Well MW-5 will allow for vertical gradients to be measured between the shallow and deep aquifers.
- MW-16 will be co-located with Primary Soil Boring PSB-9 to the southwest of the assumed source area for soil and groundwater DNAPL-forming chemical contamination. The proposed location of MW-16 will serve as a source area monitoring location.
- MW-17 will be a conditional deep monitoring well to be co-located with Primary Soil Boring PSB-17 and installed adjacent to existing Monitoring Well MW-4. The proposed location for MW-17 will serve as a downgradient groundwater contamination monitoring point outside the known extent of groundwater contamination at the LL Apartments Parcel. Additionally, the co-location of MW-17 with existing Monitoring Well MW-4 will allow for vertical

gradients to be measured in the shallow and deep aquifers, and proposed locations for the three deep monitoring wells provide a configuration to evaluate horizontal flow gradients and directions in the deep aquifer unit.

As a conditional monitoring well, MW-17 will be installed only if DNAPL chemicals are found in samples taken from MW-15 or MW-16. The Port will share well installation data (soil boring logs, field screening results, and chemical data) with WSDOE during installation of MW-15 and MW-16 so that a collaborative decision can be made regarding the need to install MW-17.

Deep monitoring wells will be installed to a depth where the first confining layer is encountered. The confining layer will be defined as a low permeability material (i.e., silt, clay, or till) with a minimum thickness of 5 feet. According to the stratigraphic description provided above and in the geology/hydrogeology evaluation (Aspect 2010), the uppermost potential confining layer (Vashon Glacial Till), if present, would be encountered at approximately 15 to 35 feet bgs in the northwest area of the parcel and 45 to 65 feet bgs in the eastern and southeastern area of the parcel. If the Vashon Glacial Till unit is absent, the next confining layer (Lawton Clay) would be encountered at approximately 80 to 100 feet bgs in the northwest area of the parcel and 95 to 115 feet bgs in the eastern and southeastern area of the parcel and 95 to 115 feet bgs in the eastern and southeastern area of the parcel and 95 to 125 feet bgs in the eastern and southeastern area of the parcel and 95 to 125 feet bgs in the eastern and southeastern area of the parcel and 95 to 125 feet bgs in the eastern and southeastern area of the parcel and 95 to 125 feet bgs in the eastern and southeastern area of the parcel. In the event neither confining unit is encountered, borings will be terminated at a maximum depth of 125 feet bgs, 10 feet below the deepest predicted surface of the Lawton Clay unit, and deep monitoring wells will be installed to that maximum depth.

Deep Monitoring Wells MW-15 through MW-17 will be installed in numerical sequence. If no confining layer and no DNAPL contamination are encountered during the installation of Monitoring Wells MW-15 and MW-16, then the Port and WSDOE will discuss the need for installing MW-17.

# DEEP MONITORING WELL BORINGS AND SOIL SAMPLING

# Geologic Logging

Deep monitoring well soil borings will be completed using a sonic drilling methodology at the locations shown on Figure 1. This methodology will allow for a detailed inspection of the geologic structure and identification of discontinuous low permeability zones where DNAPL could accumulate at locations above the first confining layer. Soil core intervals can be arranged in vertical sequence so that direct observation of the stratigraphic sequence at each deep monitoring well soil boring location can be determined and to assist with accurate identification of monitoring well screen intervals.

All soil boring drilling activities will be monitored by a field technician and soil borings will be continuously logged throughout the entire length of the exploration. Interpretations of soil conditions will be made according to the United Soil Classification

System (USCS) and will be recorded on the soil boring log. Photographs of the soil boring core intervals will also be obtained.

# Deep Monitoring Well Soil Boring Sampling

#### Field Screening

The deep monitoring well soil cores will be field screened to identify intervals that are potentially contaminated with volatile constituents by the following methods:

- Using a photoionization detector (PID)
- Conducting sheen tests
- Recording visual and olfactory signs of contamination.

Soil will also be screened for the presence of staining, sheens, odors, or anthropogenic materials (i.e., slag, metal fragments, woody debris, etc.). Soil field screening procedures will be conducted according to the methods described in the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

#### Soil Sampling

Soil samples will be collected from the core barrel samplers at approximate 5-foot-depth intervals of interest, as identified by results of the field screening methods described above. The co-located primary soil borings, drilled prior to the installation of the deep monitoring wells, will be used to determine the shallowest depth at which soil samples will be logged and collected from the deep monitoring well soil borings. For the deep borings, geologic logging of the soil cores will start at the ground surface. Soil samples will be collected starting at the sampling interval below the deepest soil sample collected at the co-located primary soil boring. For example, if the maximum depth of soil sample collected for potential analytical testing associated with deep Monitoring Well MW-15 will be at 25 feet bgs. Deep soil borings will be logged, and soil samples collected for analytical testing or archiving, to a depth where a confining layer is encountered or a maximum depth of 125 feet bgs, whichever occurs first.

Similar to soil logging procedures, soil samples will also be described and classified according to the USCS and photographed. As part of soil sampling activities, the following information will be recorded on the soil boring log:

- Date, time, and name of the person logging the sample
- Weather conditions
- Soil sample location number
- Soil sample depth and soil description

- Soil sample recovery
- Presence of debris
- PID readings
- Presence of sheen or any other indications of contamination such as odor

Soil samples will be collected for analytical testing or archiving (at the identified sample location) directly from the core barrel sampler using U.S. Environmental Protection Agency (USEPA) Method 5035A for volatile organic compounds (VOCs). This preservation method uses a Teflon corer to collect an undisturbed, sealed sample that minimizes loss of volatiles during sampling and transport. The remainder of soil within the designated sample interval will be placed into a decontaminated stainless steel bowl for homogenization.

# Soil Sample Handling

Following homogenization, the sample material will be placed into laboratory-supplied sample containers with the lids tightly sealed, labeled, and placed in a cooler on ice. Standard chain-of-custody procedures will be implemented for all sampling events. These sampling procedures described above are consistent with the soil sampling and analysis methods presented in the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd Snider 2010).

#### Decontamination

Field sampling equipment used in the collection of soil samples from soil borings (i.e., stainless steel bowl and spoons, etc.) will be decontaminated by washing with an Alconox and tap water wash, and rinsing with deionized water. All field sampling equipment and drilling equipment will be decontaminated according to the procedures described in the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

#### **Quality Control Sample Collection**

Additional quality control soil samples will be collected to verify that field collection and laboratory analytical procedures are completed in an acceptable manner. Specifics regarding requirements for collection of quality control soil samples are presented in Section 4.0 and Section 6.0 of the existing SAQ/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

# DEEP MONITORING WELL INSTALLATION AND DEVELOPMENT

# **Deep Monitoring Well Installation**

Deep monitoring wells will be installed following the "Minimum Standards for Construction and Maintenance of Wells" from WAC 173-160. Monitoring wells will be completed by Cascade Drilling of Woodinville, Washington at well locations shown in Figure 1. All wells will be constructed of 2-inch diameter, flush-threaded, Schedule 40 PVC well casing and screen. Well screen assemblies will consist of a 10-foot length of 0.010-inch (10-slot), flush-threaded, machine-slotted, Schedule 40 PVC set in a 10/20 sand or equivalent silica sand filter pack. Deep monitoring well screens will be 10 feet in length, with bottoms set at the contact with the identified confining layer, or, in the absence of a confining layer, the 125 foot bgs maximum boring depth. The well design includes a 0.5-foot long flush-threaded, Schedule 40 PVC sump with a flush-threaded end cap. The sand filter pack will be a pre-pack filter that will be installed between the well casing and the drill casing as the casing is withdrawn. A weighted tape will be used to monitor filter pre-pack placement and depth during installation. The sand filter pack will extend 3 feet above the top of the screened interval. A minimum 2-foot thick seal of hydrated bentonite chips will be installed in the annular space immediately above the sand filter pack and hydrated with potable water if installed above the water table. The remainder of the annular space will be sealed with bentonite grout or hydrated bentonite chips to within 1 foot of the ground surface.

The monitoring wells will be secured with a flush-to-ground locking steel protective monument with expansion seals on the well casing to minimize the potential for rain/surface water entering the monument. Installed wells will be labeled with a permanent marker on the well casing on the well cover of flush mounts. All newly installed monitoring wells will be surveyed by a licensed surveyor.

#### Deep Monitoring Well Development

Development activities, including purging and surging, will be performed on each newly installed deep monitoring well to remove water and fines from the well casing, filter pack, and surrounding formation. This will be done to remove water and fines in the formation disrupted by well installation, and to establish a hydraulic connection between each well and the surrounding aquifer matrix. The goal of well development is to allow groundwater representative of the formation to flow into the well.

All deep monitoring wells will be developed according to the procedures presented in Section 8.3.4 of the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010). Well development will be completed by continuous pumping at a steady rate using a pump that is capable of completing well development activities depending on the finished depth of the deep soil boring. Wells will be developed using the described methodologies or equivalents at least 48 hours following well installation. Well development equipment will be decontaminated by pumping clean water through the pump and washing to the satisfaction of the field technical staff.

Based on field screening observations made during well installation and analysis of initial soil sample data, samples may be collected using the sump prior to well development in order to determine potential waste characterization issues and disposal alternatives.

# GROUNDWATER SAMPLING

Groundwater samples will be collected from all deep monitoring wells during the three quarterly monitoring events as described in the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010). Deep monitoring wells will be purged and sampled using low-flow procedures with a pump suitable for deep groundwater sample collection and disposable polyethylene tubing. Samples will be collected sufficiently close to the well bottom to be representative of groundwater or DNAPL near the contact with the low permeability unit (if present).

Field procedures including measuring the depth to water, purging deep monitoring wells, groundwater sample collection, and sampling equipment decontamination will be completed as described in Section 8.5.1 of the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

# **Quality Control Sample Collection**

Quality control soil samples will be collected according to the procedures described in the existing SAQ/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

# DEEP MONITORING WELL SLUG TEST AND SURVEYING

After completing the installation and development of the deep monitoring wells, slug tests will be conducted at each deep monitoring well location to estimate the aquifer hydraulic conductivity in the vicinity of the well. Pneumatic slug test methods may be required if the aquifer formation is very permeable. Hydraulic conductivity data will be collected to assist with future evaluation of groundwater contaminant migration and potential transport modeling, if determined to be necessary. Slug tests will be completed according to the procedures described in Section 8.6 of the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

Additionally, all deep monitoring well locations will be surveyed for reference and mapping purposes according to the requirements specified in the Agreed Order for the Site (as defined in the current Lora Lake Apartments RI/FS Work Plan). Requirements for monitoring well survey procedures are provided in Section 8.7 of the existing SAP/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

# LABORATORY ANALYSIS PROGRAM

Soil and groundwater samples collected from each of the proposed deep monitoring well will be analyzed for the following constituent groups using the methods presented below. For soil and groundwater samples, the chemical analyses will be performed by Analytical Resources, Incorporated (ARI).

### Deep Monitoring Well Installation Soil Samples

The deep monitoring well installation soil sample collected at the confining layer contact, if present, or at the bottom of the boring, will be immediately analyzed for the following constituents by the methods indicated below:

- Pentachlorophenol (PCP) by USEPA Method 8041
- Tetrachloroethene (PCE), trichloroethene (TCE), dichloroethene (DCE), and 1,2-dichloroethane (1,2-DCA) by USEPA Method 8260C
- Total organic carbon (TOC) by Plumb 1981

All other soil samples collected during installation of the deep monitoring wells will be archived at the analytical laboratory. Following receipt of analytical data, a coordination meeting will be scheduled with WSDOE to discuss the following:

- Review of soil data from the co-located primary soil borings. A review of these data will be conducted to evaluate the need for additional laboratory analyses of archived deep monitoring well soil samples in the event that the vertical extent of contamination has not been bounded.
- Review of soil boring/well installation logs and results of field screening efforts to identify specific archived soil samples that may be beneficial for analytical testing.
- Review of soil sample results from the sample immediately analyzed at the bottom of the soil boring.

Results of this coordination meeting will be used to identify the need for analysis of additional archived soil samples for the constituents of concern (COCs) described above. Efficient coordination with WSDOE will be necessary in order to ensure that sample holding times do not expire for the archived soil samples selected for laboratory analysis.

#### Deep Monitoring Well Installation Groundwater Samples

The groundwater samples collected at the deep monitoring well locations will be analyzed for the following constituents by the methods indicated below:

• PCP by USEPA Method 8041

- PCE, TCE, DCE, and 1,2-DCA by USEPA Method 8260C-SIM
- Total suspended solids by SM 2540D
- pH by USEPA Method 150.1

Deep monitoring wells will be sampled according to the same schedule as the other monitoring wells that are being sampled at the LL Apartments Parcel.

# **Quality Control Sample Analyses**

Quality control (QC) groundwater samples will be collected according to the procedures described in the existing SAQ/QAPP document for the LL Apartments Parcel investigations (Floyd|Snider 2010).

# **Reporting Limits and Data Validation**

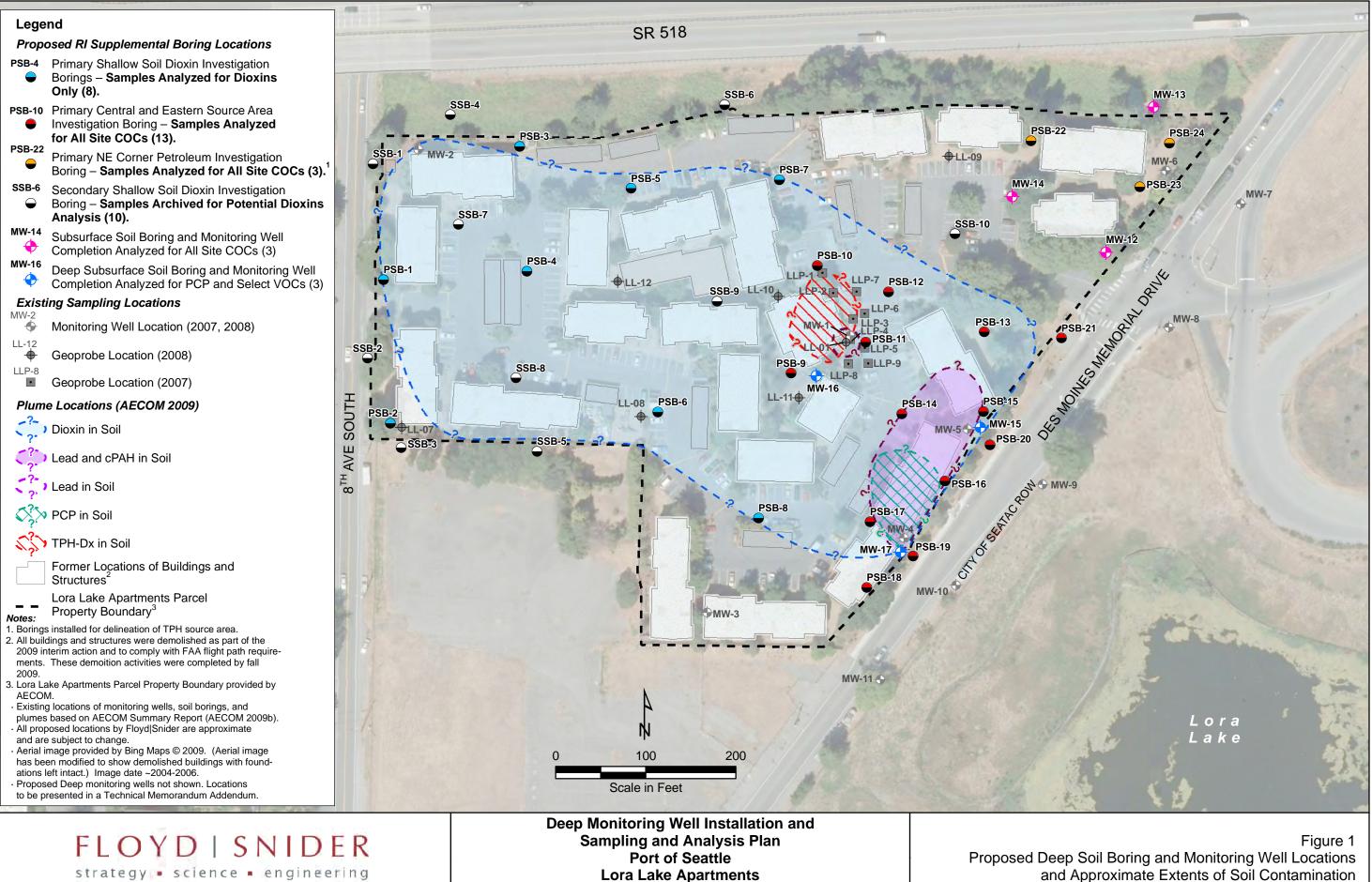
The analytical methods identified in this technical memorandum result in the lowest analytically achievable method detection limits and reporting limits or Practical Quantitation Limits (PQLs). The existing SAP/QAPP for the LL Apartments Parcel investigations presents the target reporting limits and the project data quality assurance criteria for each analytical method as performed by the analytical laboratories (Floyd|Snider 2010). These reporting limits are goals only, insofar as instances may arise where high sample concentrations, non-homogeneity of samples, or matrix interferences preclude achieving the desired reporting limit and associated QC criteria. In such instances, the laboratory will report the reason for any deviation from these reporting limits.

All data generated as part of the deep monitoring well installation program will be validated according to the same procedures described in Section 6.0 of the existing SAP/QAPP document (Floyd|Snider 2010).

# REFERENCES

- Aspect Consulting, LLC. 2010. Geology/Hydrogeology Technical Memorandum Lora Lake Apartment Parcel Remedial Investigation/Feasibility Study Work Plan Addendum. Prepared for Floyd|Snider. 21 July.
- Aspect Consulting and S. S. Papadopulos & Associates, Inc. (Aspect and S.S. Papadopulos). 2005. Seattle-Tacoma International Airport: Phase 1 Groundwater Study Report. Prepared for Port of Seattle. February.
- Floyd|Snider. 2010. *Final Remedial Investigation/Feasibility Study Work Plan*. Prepared for Port of Seattle. July.

Figure



**Burien**, Washington

File: F:\projects\POS-LLA\GIS\MXD\Deep MW Installation SAP\Figure 1 (Proposed Deep Soil Boring and MW Locations).mxd Date: 7/15/2010

and Approximate Extents of Soil Contamination

Attachment 1 Geology/Hydrogeology Technical Memorandum



# MEMORANDUM

Project No.: 090134-004-01

July 21, 2010

To: Matt Woltman – Floyd Snider Was Wash Fhilio droaeoloai 1038 2sed Geo 2sed Geo Jeremy Michael Shaha John Jacob Strunk From: Jeremy M. Shaha, LG John J. Strunk, LHG Senior Staff Hydrogeologist Senior Associate Geologist Re: Geology/Hydrogeology Technical Memorandum - Lora Lake Apartment Parcel **Remedial Investigation/Feasibility Study Work Plan Addendum** 

# Introduction

This technical memorandum provides an evaluation of the geologic and hydrostratigraphic conditions in the near vicinity of the Lora Lake Apartment Parcel (Site). The primary objective of this evaluation is to provide information and data on the geologic formations and groundwater conditions in support of developing protocols for deep monitoring well placement and design of an associated dense non-aqueous phase liquid (DNAPL) testing program. Information contained in this memorandum addresses comments received on the Draft Lora Lake Apartments Remedial Investigation/Feasibility Study Work Plan (Floyd|Snider, 2010) which, in general, required:

- An evaluation of aquifer boundaries and hydraulic connections;
- Description of groundwater flow components;
- Rationale for deep monitoring well screen placement; and
- DNAPL characterization.

Work Plan review comments from the Department of Ecology (Ecology) (Ecology, 2010) require that adequate site characterization occurs to the depth of the first confining unit underlying the water table aquifer and a sampling program be developed to evaluate if DNAPLs have accumulated on top of the confining unit. This technical memorandum provides estimates of the target depths for the first confining unit based on the review of existing geologic and groundwater information in the near vicinity of the Site. The bulk of the geology/hydrogeology data used in developing this technical memorandum is obtained from a Port of Seattle (Port) commissioned Groundwater Study (Aspect and S.S. Papadopulos, 2008) which describes in detail regional geology and groundwater conditions within a 42-square mile study area surrounding the Seattle-Tacoma International Airport (STIA). The Groundwater Study was conducted under an Ecology Agreed Order and represents the most current and comprehensive understanding of the geologic and hydrogeologic framework in the area surrounding STIA. The Site falls within the northwest region of the Groundwater Study data collection boundary.

# Site Description and Existing Characterization

The Site is located at 15001 Des Moines Memorial Drive in Burien, Washington. The Site occupies approximately 8.3 acres of currently vacant land that is bound to the north by State Route 518 (SR518), to the east and southeast by Des Moines Memorial Drive, to the west by 8th Avenue South, and to the south by an open area that was the former Seattle City Light Sunnydale substation and other open land parcels that have been cleared from prior industrial and commercial land use (Figure 1). Lora Lake, a secured Port-owned land parcel, is located to the southeast of the Site, across Des Moines Memorial Drive. Lora Lake is a man-made water body resulting from peat mining activities that occurred sometime from the late 1930s through the 1950s.

The Site is located in the Puget Lowland, within the Miller Creek Watershed, just northwest of STIA. The Site topography gradually slopes to the southeast, towards Lora Lake, with steeper slopes located adjacent to Des Moines Memorial Drive and from the Highway 518 embankment to the north. The existing Site topography was created as a result of construction associated with the former apartment building complex in 1987.

Several environmental investigations have been performed at the Site providing insight on shallow geology and groundwater conditions (Golder, 1987; GeoScience Management, 2008; ENSR|AECOM, 2008a, and ENSR|AECOM, 2008b). Soil borings and monitoring wells completed at the Site range in depth from approximately 15 to 30 feet below ground surface (bgs). These explorations have identified a discontinuous fill layer which overlays glacial outwash deposits. Detailed descriptions of these shallow geologic units in the context of deeper units and groundwater aquifer systems are presented in the following sections. Figure 2 presents the surficial geologic map (Booth and Waldron, 2002) showing that the Site is located within a recessional outwash (Qvr) channel that generally trends to the southwest on the west side of STIA.

A network of 11 groundwater monitoring wells were installed by AECOM in 2008. The wells are located within the Site boundary and east of Des Moines Memorial Drive at depths ranging between 15 to 30 feet bgs (AECOM, 2009). Appendix A provides the well logs for the monitoring wells installed at the Site and cross sections developed based on the lithologic interpretations of the well logs. Groundwater has been measured in the wells at depths of between 6 to 20 feet bgs and is present in the glacial outwash deposits and some fill materials. A southeast groundwater flow direction, across the Site towards Lora Lake, has been documented from groundwater level measurements collected between March and December 2008 (ENSR|AECOM, 2008b and AECOM, 2009).

# **Regional Geologic Setting**

The Puget Lowland is underlain at depth by volcanic and sedimentary bedrock, and is filled to the present-day land surface with both glacial and non-glacial sediments (non-glacial or inter-glacial sediments are those derived between periods of glaciation) deposited during the Quaternary Period (within the last 2 million years).

The Quaternary geologic history of the Puget Sound region is dominated by a succession of at least six dated and named periods of ice sheet glaciations. In the Puget Lowland, the most recent continental glacier was present as a lobe of ice that reached its maximum extent just south of Olympia during the Vashon stade (a short period of regional glacial advance) of the Fraser glaciation (a major period of regionwide glaciation). The Fraser glaciation locally occurred between about 13,000 and 15,000 years ago and consists of (from youngest to oldest): recessional lacustrine deposits (Qls), Vashon recessional outwash (Qvr), weathered Vashon glacial till (Qvtw), Vashon glacial till (Qvt), Vashon advance outwash (Qva), and transition beds (Qtb). Sediments that were deposited prior to the Vashon glaciation are collectively referred to as "pre-Fraser". These generally consist of alternating sequences of coarse- (C) and fine-grained (F) deposits. Table 1 provides the geologic unit descriptions based on a categorization system previously established by a Phase I Groundwater Study (Aspect and S.S. Papadopulos, 2008) conducted in the area.

During the Vashon stade, glacial ice was about 3,000 feet thick in the study area (Thorson, 1980). Sediments that were overridden by the glacier are termed glacially overconsolidated. The weight of the ice compacted the underlying sediments to a very dense or hard state. Sediments that were not glacially overridden and thus were not overconsolidated are termed normally consolidated. These sediments, such as recessional outwash and recent alluvium, are typically much less dense or hard.

During the glacial cycles, a tremendous amount of sediment was deposited in the lowland. However, during both the glacial and non-glacial cycles, some of the glacial sediment deposits were eroded and re-deposited elsewhere. Due to these cycles of deposition, erosion, and re-deposition, there may be gaps in the stratigraphic sequences, and very young deposits may rest on much older deposits. For example, in the vicinity of the Site, there may be areas where the till was eroded during the deposition of the recessional outwash, and the transition beds were eroded during the deposition of the advance outwash.

Geologic processes following the Vashon glaciation are dominated by erosion of the uplands and deposition of recent alluvium and lacustrine deposits in the valleys and water bodies of the Puget Lowland. Extensive filling of former wetlands and grading for construction projects has further modified the land surface. Sediments that were deposited after the Vashon stade of the Fraser glaciation (during the Holocene Epoch) are usually termed "recent" to identify their stratigraphic position relative to the older deposits. These deposits include (Table 1): fill (fill), topsoil (Qts), recent alluvium (Qal), and recent lacustrine deposits (Ql).

# Regional Hydrostratigraphy

In general, the coarse-grained (C) units composed of sand and gravel form the regional aquifer systems beneath the Site; whereas, the fine-grained (F) units composed of low permeability silt and clay form the aquitards, which impede groundwater flow. Table 1 provides the hydrostratigraphic unit designations for the geologic units discussed above. The following sections provide a brief

description of the respective geologic and hydrostratigraphic units (from oldest to youngest) present within the vicinity of the Site.

# Pre-Fraser Fine- and Coarse-Grained Units

Interconnected deposits of pre-Fraser coarse-grained units (Qpfc) are composed primarily of sand, gravel, or silty sand and are grouped as hydrostratigraphic units C2 through C6. Pre-Fraser fine-grained units (Qpff) are composed primarily of silt and clay, and where laterally extensive, are grouped as fine-grained hydrostratigraphic units F2 through F7. Both the F and C hydrostratigraphic units contain interbeds of other sediments.

# Transition Beds – Lawton Clay and Pre-Fraser Fine-Grained Deposits

Transition beds (Qtb) are the fine-grained deposits that mark the transition during a glacial advance from non-glacial lake deposition to glacial lake deposition. In the Seattle area, the Vashon advance glaciolacustrine (glacial lake) deposits are known as the Lawton Clay, and are commonly recognized as a distinctive geologic unit. For the hydrostratigraphy in the vicinity of the Site, the Vashon glaciolacustrine deposits (Lawton Clay) are grouped within the transition beds as the F2 unit because it is generally not possible to distinguish between Lawton Clay and fine-grained deposits of the Olympia beds in the boring and well logs. The F2 unit may also include some older glacial and non-glacial fine-grained deposits. The F2 unit overlies the C2 and older units.

The fine-grained Olympia bed non-glacial deposits contain silt, clay, and silt-sand mixtures, with scattered to abundant organics and thin interbeds of peat and organic-rich wetland deposits. Lawton Clay is composed of hard, gray, interbedded silt and clay with thin fine sand interbeds and scattered sand and gravel dropstones (sand or gravel particles that fell into fine-grained sediments after they melted out of floating glacial ice).

# Vashon Glacial Deposits

Sediments deposited during the Vashon glacial advance are collectively termed Vashon Drift. The main components of the Vashon Drift glacial sequence present within the vicinity of the Site are described below from oldest to youngest. These deposits date from the advance of the Vashon ice-sheet glacier about 15,000 years ago to the retreat of the ice about 13,500 years ago. The earliest Vashon deposit, the glaciolacustrine Lawton Clay was described above.

Advance Outwash (Qva) – Advance glaciofluvial (glacial meltwater) deposits of the Vashon Stade are also called Vashon advance outwash. A prominent sandy Vashon advance outwash unit, known in the Seattle area as the Esperance sand, occurs widely across the study area. Advance outwash present within the vicinity of the Site is generally very dense, brown to gray, homogeneous, clean (no appreciable silt or clay) to slightly silty, fine to medium sand, although some deposits are composed of sand and gravel. The advance outwash unit forms a regional aquifer which exhibits both unconfined and confined groundwater conditions. Where the C1 unit is confined and exposed at the surface through hillside erosion or downcutting by a natural drainage, such as in the Miller Creek drainage, discharge from the C1 unit may occur as seeps or wetlands or springs contributing to surface water baseflow.

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**Vashon Till (Qvt)** – This unit consists primarily of lodgement till, a very dense, gray mixture of clay, silt, sand, and gravel with cobbles that was deposited at the base of the Vashon glacier. Interbeds or lenses of water-worked outwash-like sand and gravel are present within the till. The interbeds and lenses commonly range from several inches to about 5 feet thick. Thin lacustrine lenses and boulders are also present in Vashon till.

Unweathered till generally exhibits very low permeability and forms an aquitard as hydrostratigraphic unit F1. Where low areas in the till surface are present, groundwater will perch atop this unit. Wetlands commonly occur in enclosed depressions on the till surface. Where Vashon till caps units such as the C1 unit, it may form a confining layer.

**Weathered Vashon Till (Qvtw)** – Where the upper surface of the till has been weathered, a subunit of the Vashon till, called weathered till (Qvtw) had been identified. Although typically not greater than 5 feet thick, this unit is brown to gray, and is looser and has higher permeability (and transmits water more rapidly) than unweathered till. It is usually grouped with the F1 hydrostratigraphic unit.

**Recessional Outwash** (**Qvr**) – Recessional outwash deposits consist of coarse-grained fluvial (river and stream) sediments (Qvr) associated with the retreat of the Vashon ice sheet. The fluvial sediments consist of sand, gravel, and silty sand. Recessional outwash is the only Vashon glacial deposit that was never overridden by glacial ice and compressed, and is therefore substantially less dense and softer than the older deposits.

Recessional outwash occurs as thin deposits on upland areas, typically less than 20 feet thick, and in broad low areas that were former recessional meltwater channels. Along the walls and bottoms of most major drainages within the vicinity of the Site, recessional outwash is more continuous and may exceed 30 feet in thickness. Recessional outwash may occur in the same geographic locations as recent alluvium (modern stream and river deposits). It has sedimentary characteristics similar to alluvium, and may be grouped with alluvium. In upland areas where drainage is poor, recessional outwash commonly forms a perched aquifer. Recessional outwash is included in the C0 hydrostratigraphic unit.

**Recessional Lacustrine Deposits (Qvrl)** - Fine-grained deposits associated with the recession of the Vashon ice sheet glacier are identified as recessional lacustrine deposits (Qvrl). These normally consolidated (not glacially overridden) glacial lake, pond, and kettle (a depression that formed by melting of buried glacial ice) deposits generally consist of silty to clayey sediments with interbedded silty fine sands. They are usually restricted to poorly-drained ice meltout kettles on the uplands, and as thin interbeds within coarse-grained recessional outwash deposits in the low-lying areas. Recessional lacustrine deposits are included in the F0 hydrostratigraphic unit.

# Recent (Holocene) Deposits

Holocene sediments are those that have been deposited since the disappearance of glacial ice following the last ice age. In the central Puget Lowland, this disappearance of ice occurred approximately 13,500 years ago. The sediments were deposited by non-glacial geologic processes that are largely active today, such as erosion, landslides, streams and rivers, and human activities such as excavating and filling. Because these sediments have not been glacially overridden, they are normally consolidated or slightly over consolidated, and are typically very loose to dense or soft to

very stiff. The most extensive Holocene deposits include recent alluvium and fill. Recent deposits are either C0 if coarse-grained, or F0 if fine-grained. There is no stratigraphic order among the recent deposits.

**Recent Alluvium (Qal)** – Recent alluvium consists of young stream and river (fluvial) sands and gravels and silty sands, commonly containing some interbedded silt and clay and organic matter. Thin ribbons of recent alluvium fill the ravines and creek bottoms that drain the upland, including portions of Miller Creek. The alluvium thickens where the creeks approach the shoreline.

**Recent Lacustrine Deposits (Ql)** – Recent deposits of lake (lacustrine), pond, floodplain and other low-energy or slack water bodies are generally composed of silt, clay, and silty fine sand. These fine-grained deposits commonly contain some organics and wood and may include peat. This geologic unit is included in the F0 hydrostratigraphic group.

**Topsoil** (**Qts**) – Topsoil consists of organic-enriched soil that typically marks the top of native soils or any formerly vegetated older surface. Although typically less than 2 feet thick, topsoil is commonly present below fill soils and is an important marker bed for delineating the thickness and extent of fill that covers the topsoil. Topsoil is included in the C0 unit.

**Artificial Fill (Fill)** – Fills of various thickness and composition are present within the vicinity of the Site. Fills are most common in low-lying areas around drainage bottoms. Fills may be composed of any mixture of fine or coarse grained soils. Fills are considered to be water bearing zones if coarse-grained and classified as C0. Fine grained fills, classified as F0, tend to exhibit poor water bearing capabilities.

# Site Hydrostratigraphy

In the vicinity of the Site, only Fraser glacial deposits are exposed at land surface (Figure 2). The Fraser glacial deposits consist primarily of recessional outwash (C0), which was deposited in what may have been a relatively large, southwest-northeast trending ancestral outwash channel. Adjacent to this channel, older Quaternary deposits consisting of till (F1) and advance outwash (C1) are exposed at the surface.

Several cross sections were developed by ENSR|AECOM (2008a and 2008b), based on 11 monitoring wells installed at the Site (Appendix A), in order to determine the subsurface stratigraphy in the vicinity of the Site. The cross sections indicate that up to 15 feet of fill (fill) is present over much of the Site, except within the northern region of the Site, where it is absent. The cross sections also indicate that recessional outwash (C0) is present to depths of at least 30 feet bgs. However, because all of the Site monitoring wells are completed in the recessional outwash (C0), at depths of less than 30 feet, they do not provide information on the presence/absence of deeper stratigraphic units. Therefore, an additional cross section was created for the Site, incorporating nearby wells completed at greater depths, in the older till (F1) and advance outwash (C1) units. Figure 3 presents the cross section map and relevant well locations and Figure 4 presents the additional cross section.

Based on Figure 4, recessional outwash (C0) is present at the surface in the vicinity of the Site, where fill is not present, and extends to depths of more than 15 feet bgs (northwestern property boundary), to more than 30 feet bgs (southeastern property boundary), based on the respective well logs. Although there are currently no existing deep wells (depths greater than 30 feet) at the Site, several wells located to the southeast of the Site (NDW-1A and NDW-4) indicate the presence of a relatively thick (between 25 and 35 feet) till unit (F1), near the east side of the Miller Creek valley. In addition, a well to the north of the Site (23N4E-20C1) also indicates the presence of an approximately 10-foot thick till unit (F1) exposed at the surface. If the till (F1) is continuous across the Site, as illustrated in Figure 4, the unit would be expected to occur at depths ranging between 15 (northwestern property boundary) and 45 (southeastern property boundary) feet bgs, or at elevations of between 290 and 265 feet NGVD 29, respectively. This range of elevations, based on site-specific interpretation of vicinity data (Figure 4), is slightly lower than those predicted by a structure contour map (Figure 5) developed as part of the Phase I Groundwater Study (Aspect and S.S. Papadopulos, 2008) for the top of the advance outwash (C1), which occurs immediately below the till (F1). The structure contour map predicts the top of the advance outwash (C1) to occur at an elevation of approximately 300 feet NGVD 29, meaning that the top of the till (F1) would occur at an elevation of more than 300 feet NGVD 29.

It also important to note that areas of the till (F1) may have been eroded during the deposition of the younger recessional outwash (C0). There is a nearby cross section (A-A'), developed as part of the Phase I Groundwater Study (Aspect and S.S. Papadopulos, 2008), which is located immediately to the north of the Site (Figure 3) and cuts across the recessional outwash channel. This cross section (Figure 6) indicates that the till (F1) may not be present at the center of the recessional outwash channel (vicinity of well B-211, 23N4E20B1) and that the recessional outwash (C0) may transition directly into the advance outwash (C1). Although Figure 6 indicates the potential for the till (F1) to be absent within the outwash channel, it is uncertain whether this is the case beneath the Site.

If the till (F1) is absent beneath the Site, the next significant confining layer that would be present beneath the site is the transition beds (F2). However, due to the limited deep (depths greater than 30 feet) well data in the vicinity of the Site, it is not possible to say with any certainty whether the transition beds (F2) are present. Based on the cross section through the Site (Figure 4), the transition beds (F2) are about 20 feet thick to the north of the Site (well 23N4E20C1) and are encountered at an elevation of approximately 225 feet NGVD 29. However, as illustrated in Figure 4, the unit is likely dipping slightly to the south-southeast, towards the center of the recessional outwash channel. Therefore, the transition beds (F2) would be expected to be encountered at a slightly lower elevation beneath the site. A structure contour map (Figure 7), developed as part of the Phase I Groundwater Study (Aspect and S.S. Papadopulos, 2008) for the top of the transition beds (F2), indicates that in the vicinity of the Site, the transition beds are expected to be encountered at an elevation of about 220 feet NGVD 29. This elevation for the top of the transition beds (F2) is very similar to that observed on the cross section located immediately to the north of the site (Figure 6). However, it is important to note that as with the till (F1), the transition beds (F2) may have been eroded during the deposition of the advance outwash (C1). If the transition beds (F2) are present beneath the Site, they would be expected to be encountered at depths of between 80 (southeastern property boundary) to 95 (north central property boundary) feet bgs.

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Underlying the transition beds (F2), if they are present beneath the Site, is a relatively thick (approximately 150 feet thick, based on Figure 6) unit of pre-Fraser coarse-grained deposits (C2). These deposits would extend to depths of more than 230 (southeastern property boundary) to 245 (north central property boundary) feet beneath the Site. Within the pre-Fraser coarse-grained deposits (C2), there is a relatively thin (approximately 10-foot thick) confining unit of pre-Fraser fine-grained deposits (C2F). This unit is found at an elevation of approximately 160 feet NGVD 29, or between depths of approximately 140 (southeastern property boundary) to 155 (north central property boundary) feet beneath the Site. Depending on the thickness of this unit, it may cause the upper and lower portions of the pre-Fraser coarse-grained deposits (C2) is a relatively thick (greater than 140 feet thick, based on Figure 6) unit of pre-Fraser fine-grained deposits (F3).

# Hydrogeologic Setting

The following sections provide a brief description of groundwater occurrence in the various aquifers expected to be present beneath the Site and the potential for groundwater interactions between the aquifers.

# Groundwater Occurrence

The initial and supplemental groundwater investigations completed by ENSR|AECOM (2008a and 2008b), reveal that the recessional outwash (C0) is the uppermost aquifer at the Site, with groundwater levels of between 6 to 20 feet bgs, and groundwater elevations between 275 to 293 feet NGVD 29. A groundwater elevation contour map for the Site (Appendix A), completed by ENSR|AECOM (2008a and 2008b), indicates that groundwater flow within the recessional outwash (C0) is to the southeast, towards Lora Lake. However, it is important to note that the lower boundary of this aquifer has not been defined at the Site, and it is currently uncertain whether groundwater in the recessional outwash (C0) is in hydraulic continuity with the underlying advance outwash (C1) aquifer.

The Phase I Groundwater Study (Aspect and S.S. Papadopulos, 2008) indicates groundwater levels in the advance outwash (C1) aquifer appear to be very similar (within about 5 feet) to groundwater levels observed in the overlying recessional outwash (C0). If the till (F1) is absent beneath the Site, and the recessional outwash (C0) and advance outwash (C1) aquifers are in hydraulic continuity, it would be expected that the groundwater levels would be about the same elevation. Figure 8 presents a groundwater elevation contour map for the advance outwash (C1) aquifer. Based on Figure 8, groundwater flow in the advance outwash (C1) aquifer is also to the southeast in the vicinity of the Site, towards Lora Lake. However, Figure 8 demonstrates that regional groundwater flow in the advance outwash (C1) aquifer is to the southwest, converging and flowing sub-parallel to Miller Creek towards Puget Sound.

A groundwater elevation contour map for the uppermost pre-Fraser coarse-grained deposits (C2) was also developed as part of the Phase I Groundwater Study (Aspect and S.S. Papadopulos, 2008) and is presented as Figure 9. The pre-Fraser coarse-grained deposits (C2) aquifer is the uppermost aquifer in which the nearest municipal water supply wells (City of Seattle Highline Well Field) are completed. Based on Figure 9, groundwater flow in the pre-Fraser coarse-grained deposits (C2) in

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the vicinity of the Site appears to be to the southwest, towards Puget Sound, which is consistent with the regional groundwater flow direction.

### Vertical Gradients

Because all of the Site monitoring wells are completed in the recessional outwash (C0), it is not possible to determine vertical gradients beneath the Site at this time. However, several multi-completion piezometers were installed as part of a Predesign Study for the Renton Effluent Transfer System (URS Engineers, et al., 1983). Two of these piezometers (B-9 23N4E20C1 and B-8 23N4E20A1) are located to the northwest and northeast of the Site, respectively (Figure 4 and Figure 6). Table 2 provides a summary of the vertical gradients for the various hydrostratigraphic units of completion, based on the available data.

In the center of the recessional outwash channel (vicinity of piezometer B-8 23N4E20A1), there appears to be an upward vertical gradient (-0.03) from the underlying pre-Fraser coarse-grained deposits (C2) towards the overlying advance outwash (C1) and recessional outwash (C0). This is consistent with the groundwater elevation contour map for the area in which the advance outwash (C1) appears to be discharging to Miller Creek (Figure 8). This upward gradient would thus likely limit the downward migration of dissolved DNAPL constituents and could provide a natural hydraulic barrier to contaminant transport into deeper aquifer systems.

However, there also appears to be a downward vertical gradient (0.190) from the pre-Fraser coarsegrained deposits (C2) towards the pre-Fraser fine-grained deposits (F3). This downward gradient is likely associated with recharge which occurs on the upland at topographic elevations that are higher than those of the Site. Further to the west and closer to the edge of the recessional outwash channel (vicinity of piezometer B-9 23N4E20C1), there appears to be a downward gradient (0.140) from the recessional outwash (C0) towards the underlying advance outwash (C1) and pre-Fraser coarsegrained deposits (C2). There is also a downward gradient (0.186) from the pre-Fraser coarse-grained deposits (C2) towards the underlying pre-Fraser fine-grained deposits (F3).

To better understand the vertical component of groundwater flow at the Site, two of the proposed deep monitoring wells will be completed adjacent to existing monitoring wells MW-4 and MW-5. The deep and shallow well pairs will allow for water level measurements in the recessional outwash (C0) and deeper units necessary to calculate a vertical gradient for the Site.

# Recommendations

Based on review of the available geologic and hydrostratigraphic information in the vicinity of the Site, as summarized in the previous sections, the following recommendations are provided for the locations and completion depths of the additional deep monitoring wells to be installed at the Site for the purpose of DNAPL monitoring. The deep monitoring wells will be completed to the first confining unit observed beneath the Site. The proposed target depths to the confining units and the locations for the deep wells are described below.

### Proposed Monitoring Well Completion Intervals

Based on comments provided in the letter from Ecology to the Port of Seattle (Ecology, 2010), dated May 26, 2010, Ecology is requiring that additional deep monitoring wells installed at the Site extend to the first confining unit beneath the sand and gravel water table aquifer, defined as the recessional outwash (C0) aquifer.

Based on review of the nearby existing geologic and hydrostratigraphic information, the uppermost confining unit likely to be encountered beneath the Site is the till unit (F1). This confining unit is expected to be encountered at a depth of approximately 15 feet bgs in the northwestern region of the property (290 feet NGVD 29); at a depth of approximately 30 feet bgs in the central portion of the property (270 feet NGVD 29); and at a depth of approximately 45 feet bgs in the southeastern region of the property (255 feet NGVD 29).

If the till (F1) is not encountered, due to being eroded during the deposition of the recessional outwash (C0), then the next confining unit likely to be encountered beneath the Site is the transition beds (F2). This confining unit, if present, is expected to be encountered at an elevation of approximately 220 feet NGVD 29. Therefore, it would be expected to be encountered at a depth of 95 feet bgs in the north-central region of the property; and at approximately 80 feet bgs in both the central and southeastern regions of the property.

It is important to note that all of the depths provided in this section are extrapolated from off-site investigations and are therefore only approximates. Actual depths could likely vary by as much as 20 feet; therefore, these depths should be considered minimum depths and exploration wells should be drilled at least an additional 20 feet in order confirm whether the respective units are present/absent.

The till (F1) and transition beds (F2), if present, are the most likely confining units beneath the Site based on an analysis of existing data. However, it is also possible that less extensive confining units may exist within either the recessional outwash (C0) or advance outwash (C1) aquifers. If confining units are encountered within these aquifers exhibiting a thickness of greater than 5 feet, the deep monitoring wells may instead be completed on top of these aquitards. Care will be taken during drilling not to penetrate the uppermost confining unit and sample cores will be collected continuously to evaluate the thickness of the aquitards.

As previously discussed, the deep monitoring wells will be completed on top of the uppermost confining unit that is at least 5 feet in thickness. If the first potential confining unit (F1) is not encountered, then the monitoring well will drilled to the depth of the next expected confining unit (F2). Once a confining unit with a thickness of more than 5 feet is encountered, the monitoring well will be completed on top of the respective confining unit.

# Proposed Monitoring Well Locations

Ecology is also requiring that one additional deep well be installed in the vicinity of the potential drum cleanout pond and another deep well be installed in the vicinity of MW-5, at the downgradient property boundary of the recessional outwash (C0) aquifer. A third deep well may be installed to further refine the downslope extent of any DNAPL migration along the uppermost confining unit present beneath the Site. The third deep well would also provide an additional location for determining groundwater flow directions in the aquifer above the confining unit. Due to the potential

for penetrating an aquitard beneath the Site that is currently preventing the downward migration of contaminants, all of the additional deep monitoring wells will be installed immediately downgradient or downslope of the extents of previously detected source areas and soil and groundwater contamination.

Based on the Site cross section (Figure 4), the till (F1) confining unit generally appears to be locally sloping to the south-southeast. However, based on the structural contour map of the transition beds (F2) confining unit (Figure 7), the regional slope of the transition beds (F2) and possibly the till (F1) is likely more towards the south-southwest. Therefore, the downslope direction along any perspective confining unit is considered to be south-southwest.

Based on this information, the additional deep monitoring wells will be located as follows:

- MW-15 will be located less than 20 feet south-southwest of MW-5. This well will be downgradient of previous detections of PCPs in the soil and groundwater. The well pair (MW-5 and MW-15) will also provide a better estimate of vertical gradients beneath the Site.
- MW-16 will be located approximately 40 feet south of the potential drum cleanout pond. The well will be located downslope of previous detections of TPH-Dx and lead in the soil; and previous detections of TPH-Dx, arsenic, cPAHs, dioxin, and PCPs in the groundwater.
- MW-17 will be located less than 20 feet to the south-southwest of MW-4. This well will be downslope of previous detections of lead, cPAHs and PCPs in the soil. The well pair (MW-4 and MW-17) will also provide a better estimate of vertical gradients beneath the Site.

Figure 10 presents the proposed locations of the additional deep monitoring wells discussed above.

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# Attachments

- Table 1 Geologic and Hydrogeologic Stratigraphy
- Table 2 Vertical Groundwater Gradients in the Vicinity of the Site
- Figure 1 Site Map
- Figure 2 Geologic Map
- Figure 3 Lora Lake Cross Section Location Map
- Figure 4 Lora Lake Cross Section D-D'
- Figure 5 Hydrostratigraphic Contour Map, Top of C1 Unit
- Figure 6 Cross Section A-A'
- Figure 7 Hydrostratigraphic Contour Map, Top of F2 Unit
- Figure 8 Groundwater Elevation Contour Map, C1 Aquifer
- Figure 9 Groundwater Elevation Contour Map, C2 Aquifer
- Figure 10 Proposed Deep Monitoring Well Locations
- Appendix A Site Groundwater Flow Direction Map, Monitoring Well Logs and Cross Sections

# Limitations

Work for this project was performed and this memorandum prepared in accordance with generally accepted professional practices for the nature and conditions of work completed in the same or similar localities, at the time the work was performed. It is intended for the exclusive use of Floyd|Snider for specific application to the referenced property. This memorandum does not represent a legal opinion. No other warranty, expressed or implied, is made.

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# Table 1 - Geologic and Hydrogeologic Stratigraphy Lora Lake Apartment Parcel

Geologic Group	Geologic Unit Name	Geologic Unit Abbr. <sup>1</sup>			Hydro- Stratigraphic Unit		South King Co. Groundwater Mgmt. Plan Unit	USGS <sup>2</sup>
sits	Fill	Fill	All man-placed fill and extensively graded areas.	C0/F0				m
Post-Glacial Deposits	Topsoil	Qts	Topsoil.	C0				
Glacia	Recent Alluvium	Qal	Sand and gravel deposited by streams and rivers.	CO	le			
Post-(	Recent Lacustrine Deposits	QI	Clay, silt, and peat deposited in lakes, ponds and wetlands.	F0 Sear		Perched Water Bearing Zones and Aquitards <sup>3</sup>	Qal and Qvr -	Qyal
	Recessional Lacustrine Deposits	Qvrl	Silt and clay deposited in quiet water recessional glacial environments including lakes, ponds, floodplains, and kettles.	F0				Qvrl
sits	Recessional Outwash	Qvr	Sandy to gravelly recessional glacial stream and river deposits.	CO				
Depo Drift)	All deposits ab	ove are normally	I consolidated. All deposits below are glaci -	ally overconsol	idated.		· · · · · · · · · · · · · · · · · · ·	
er Glacial Deposits (Vashon Drift)	Weathered Vashon Glacial Till	Qvtw	Weathered till.					
Fraser ( (Vå	Vashon Glacial Till	Qvt	Glacial lodgement till composed of poorly- sorted clay, silt, sand and gravel. Includes interbeds of outwash and glaciolacustrine sediments.	F1 A		Aquitard <sup>3</sup>	Qvt	Qvt
	Vashon Advance Outwash	Qva	Pro-glacial outwash composed primarily of silty sand, sand, and sand-gravel mixtures. (Known in the Seattle area as Esperance Sand.)	C1		Aquifer	Qva "Shallow Aquifer"	Qva
	Transition Beds	Qtb	Includes both Vashon glaciolacustrine silt and clay deposits (Lawton Clay) and pre- Fraser fine-grained non-glacial deposits.	E2	F2		d QvI and Qf(1)	Qpfí
	Pre-Fraser Fine- Grained Deposits	Qpff	Undifferentiated pre-Fraser fine-grained deposits.	. 2		Aquitard		
	Pre-Fraser Coarse- Grained Deposits Qpfc		Uppermost extensive coarse-grained deposit below Vashon Drift. Chiefly sand	C2 C2F				<b>Qpf</b> c
		Qpfc	and gravel. Likely includes Olympia non- glacial beds, Possession glacial outwash, and Whidbey non-glacial deposits. Includes				Qc(3) "Intermediate Aquifer"	Qpff
			a discontinuous fine-grained interbed (C2F).	C2				<b>Qpf</b> c
osits	Pre-Fraser Fine- Grained Deposits	Qpff	Older undifferentiated fine-grained deposits.	F3		Aquitard	Qf(3)	Qpff
Pre-Fraser Glacial & Non-Glacial Deposits	Pre-Fraser Coarse- Grained Deposits	Qpfc	Older undifferentiated coarse-grained deposits.	C3		Aquifer	Qc(4) "Deep Aquifer"	<b>Qpf</b> c
	Pre-Fraser Fine- Grained Deposits	Qpff	Older undifferentiated fine-grained deposits.	F4		Aquitard	Qf(4)	Qpf
	Pre-Fraser Coarse- Grained Deposits	Qpfc	Older undifferentiated coarse-grained deposits.	C4		Aquifer		<b>Qpf</b> c
	Pre-Fraser Fine- Grained Deposits	Qpff	Older undifferentiated fine-grained deposits.	F5		Aquitard		Qpff
	Pre-Fraser Coarse- Grained Deposits	Qpfc	Older undifferentiated coarse-grained deposits.	C5		Aquifer		Qpfc
	Pre-Fraser Fine- Grained Deposits	Qpff	Older undifferentiated fine-grained deposits.	F6		Aquitard		Qpf <del>í</del>
				1				

	Pre-Fraser Coarse- Grained Deposits	Qpfc	Older undifferentiated coarse-grained deposits.	C6	Aquifer		Qpfc
Bedrock	Tertiary Bedrock		Siltstone, sandstone and shale with minor coal seams. Includes basaltic and andesitic intrusives.	Br	Aquitard	Tbr	Ti, Tpr, Tpt, Tpta, and Tptm

Notes:

The Geologic Unit Abbreviations are those used in this report.
 USGS designations are those used in the *Geologic Map of the Des Moines 7.5' Quadrangle, Washington* (Booth and Waldron, 2002).
 The hydrostratigraphic units C0, F0, and F1 were combined into a single layer for the groundwater model.

Aspect Consulting 7/21/10 W:\090134 POS On-Call Environmental\Deliverables\Lora Lake RI FS\FINAL\Table 1 - Geologic and Hydrogeologic Stratigraphy .xls



# Table 2 - Vertical Groundwater Gradients in the Vicinity of the Site

Lora Lake Appartment Parcel

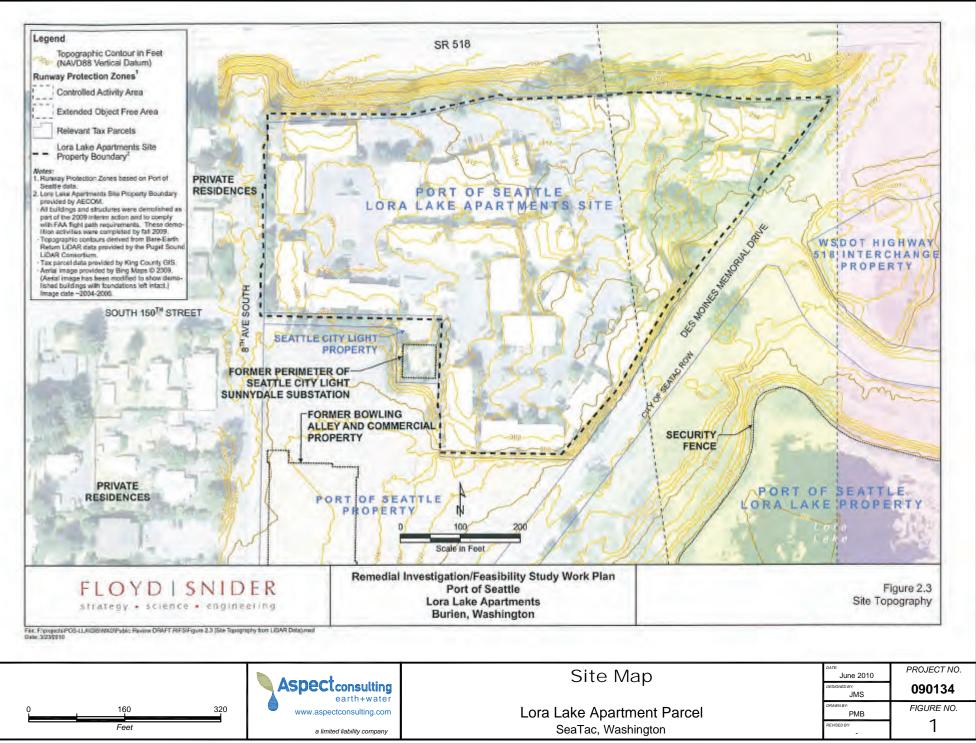
	Completion	Unit					
Well ID	Information	C0	C1	C2	F3		
	Top of Screen Elevation (ft NGVD 29)	280.4	-	140.4	5.4	-49.6	
0A1	Bottom of Screen Elevation (ft NGVD 29)	275.4	-	110.4	-4.6	-59.6	
B-8 23N4E20A1	Average Screen Elevation (ft NGVD 29)	277.9	-	125.4	0.4	-54.6	
B-8	Static Water Level Elevation (ft NGVD 29)	279.1	-	283.6	-	249.4	
	Vertical Gradient <sup>1</sup>	-0.030	-	0.190	-	-	
	Top of Screen Elevation (ft NGVD 29)	319.8	-	120.3	38.8	-55.2	
=20C1	Bottom of Screen Elevation (ft NGVD 29)	304.8	-	80.3	28.8	-60.2	
B-9 23N4E20C1	Average Screen Elevation (ft NGVD 29)	312.3	-	100.3	33.8	-57.7	
Ξ	Static Water Level Elevation (ft NGVD 29)	310.3	-	280.7	268.3	266.2	
	Vertical Gradient <sup>1</sup>		-	0.186	0.023	-	

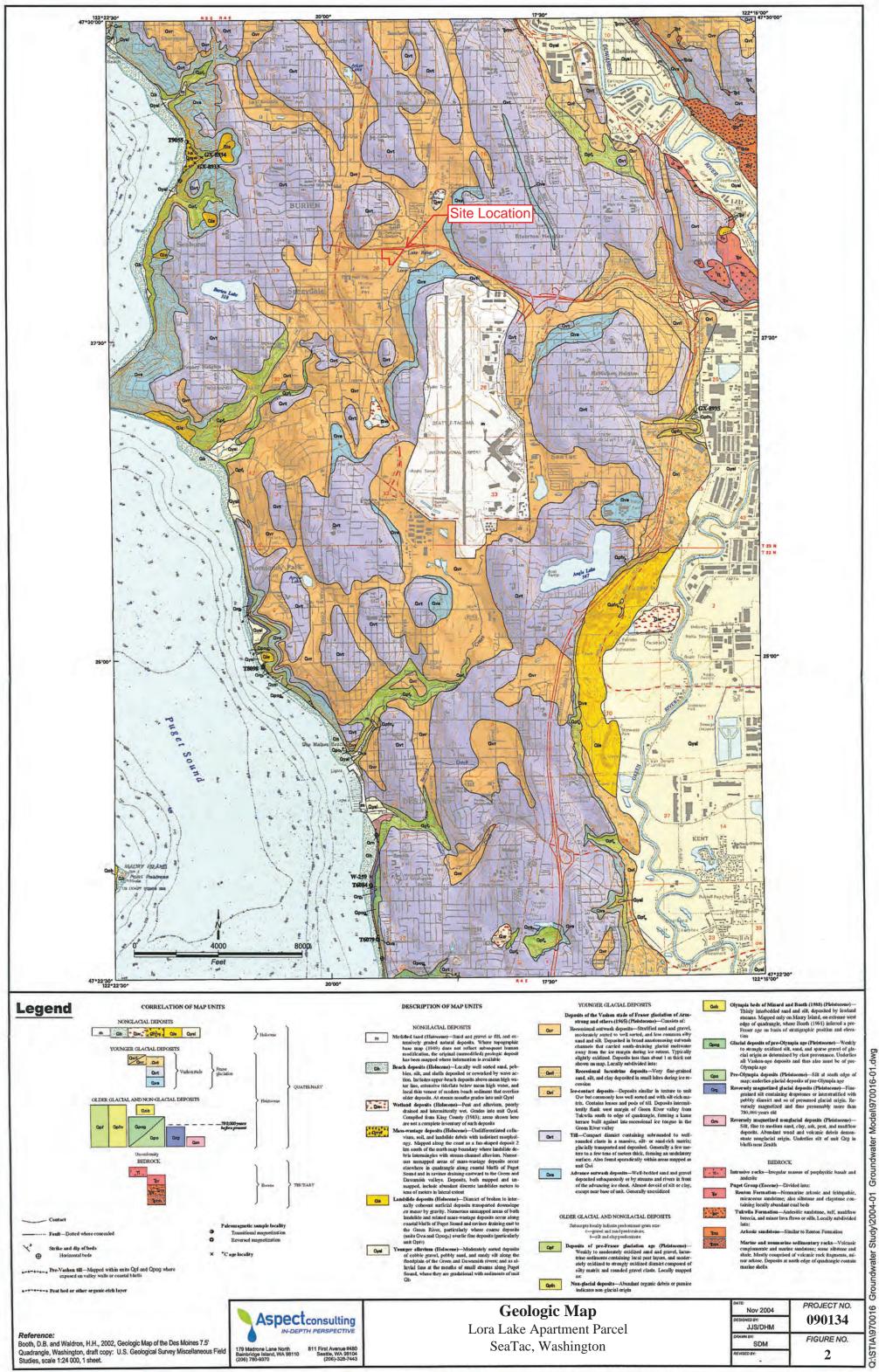
#### Notes:

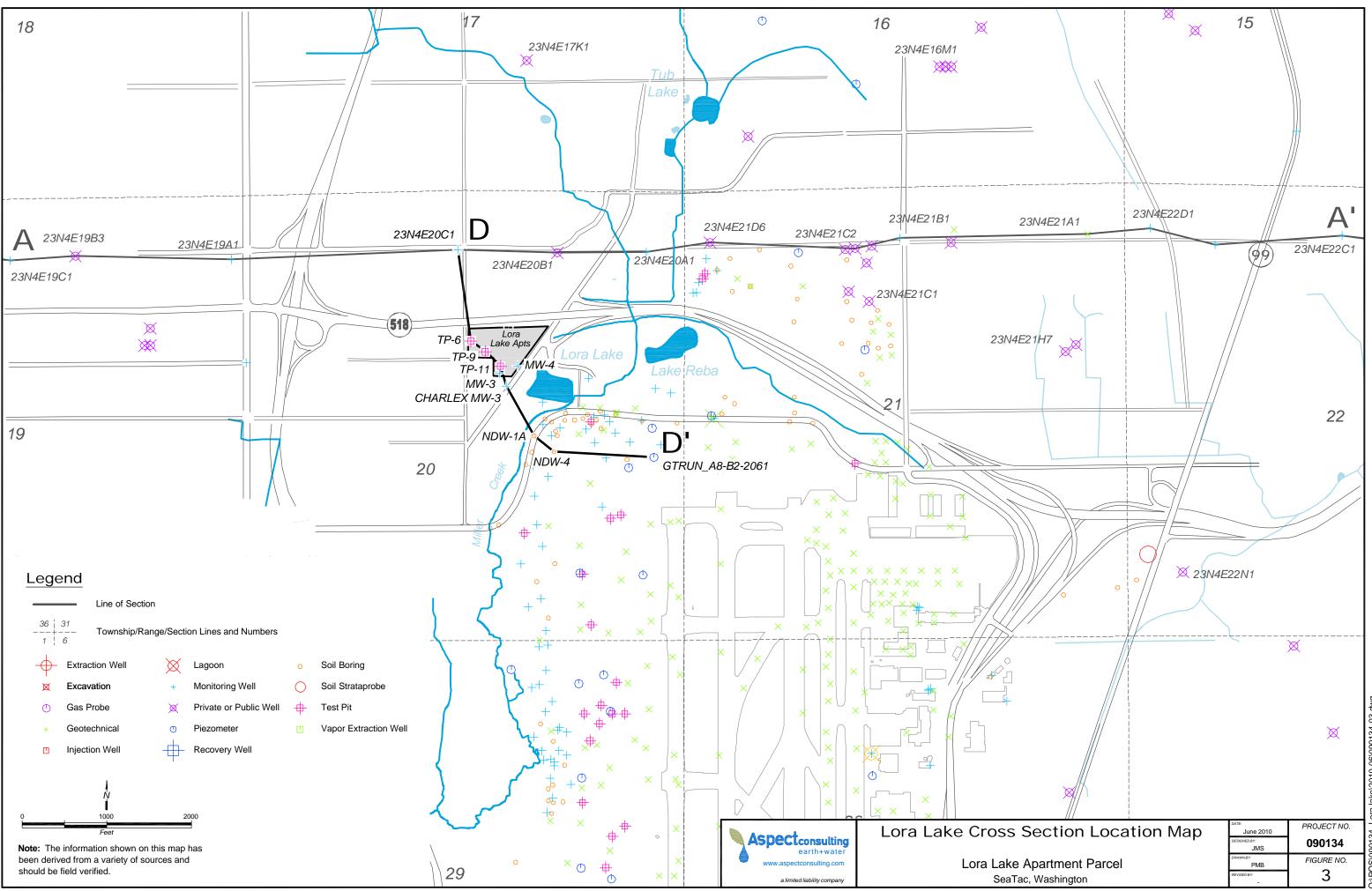
• The vertical gradient was calculated between the hydrostratigraphic unit in which the vertical

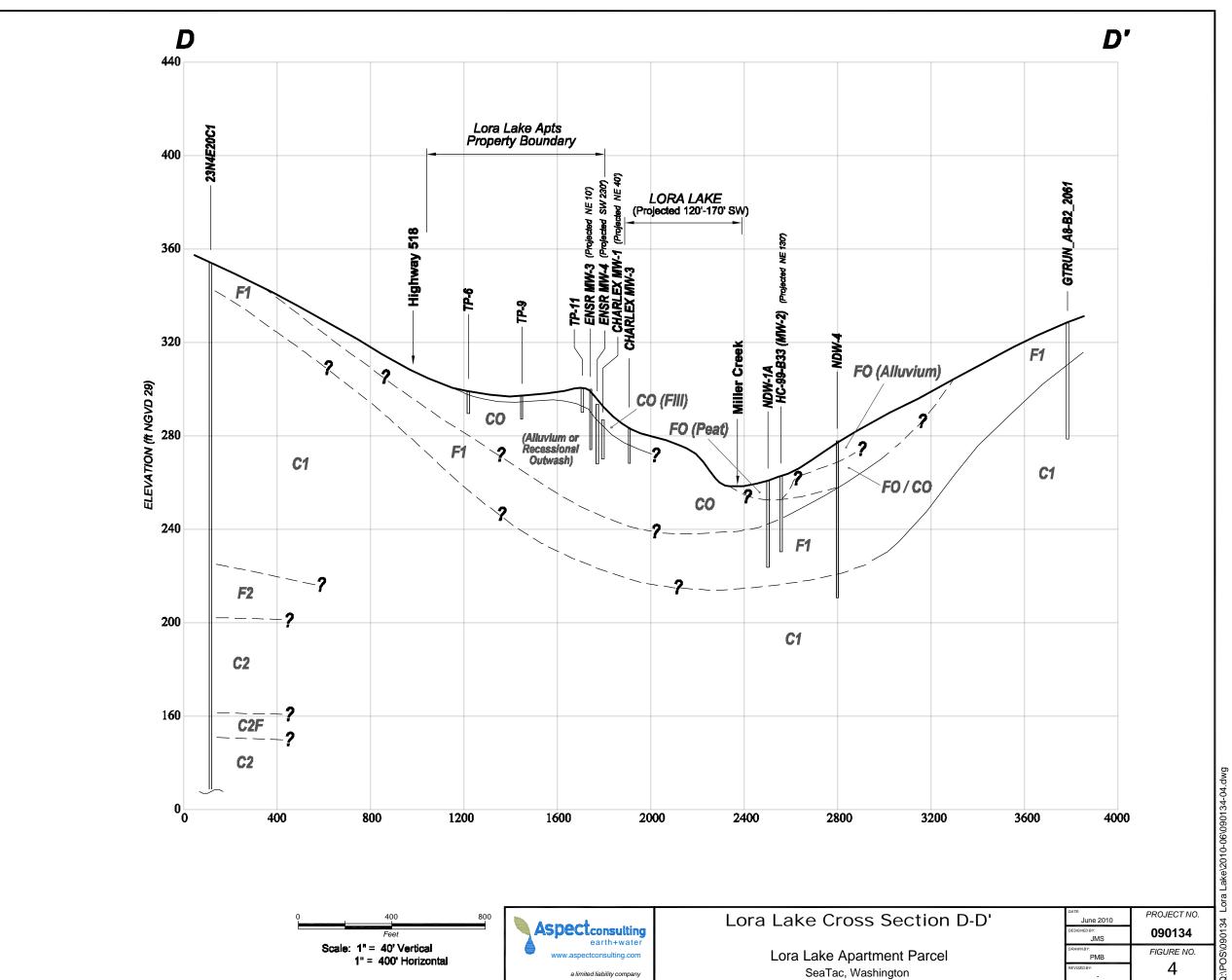
gradient value is listed and the next underlying hydrostratigraphic unit with a valid static water level.

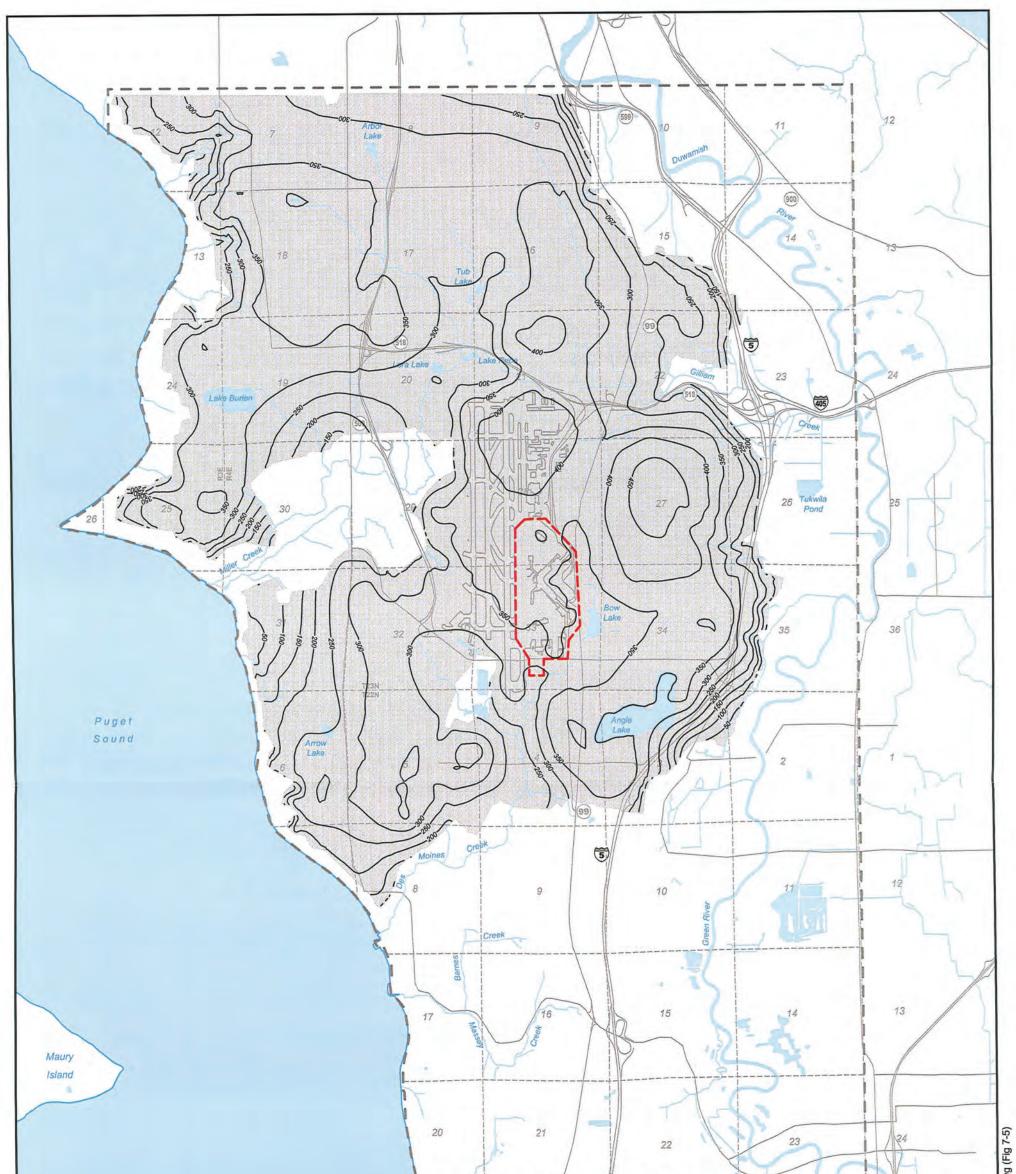
• Negative vertical gradients are upwards and positive vertical gradients are downwards.



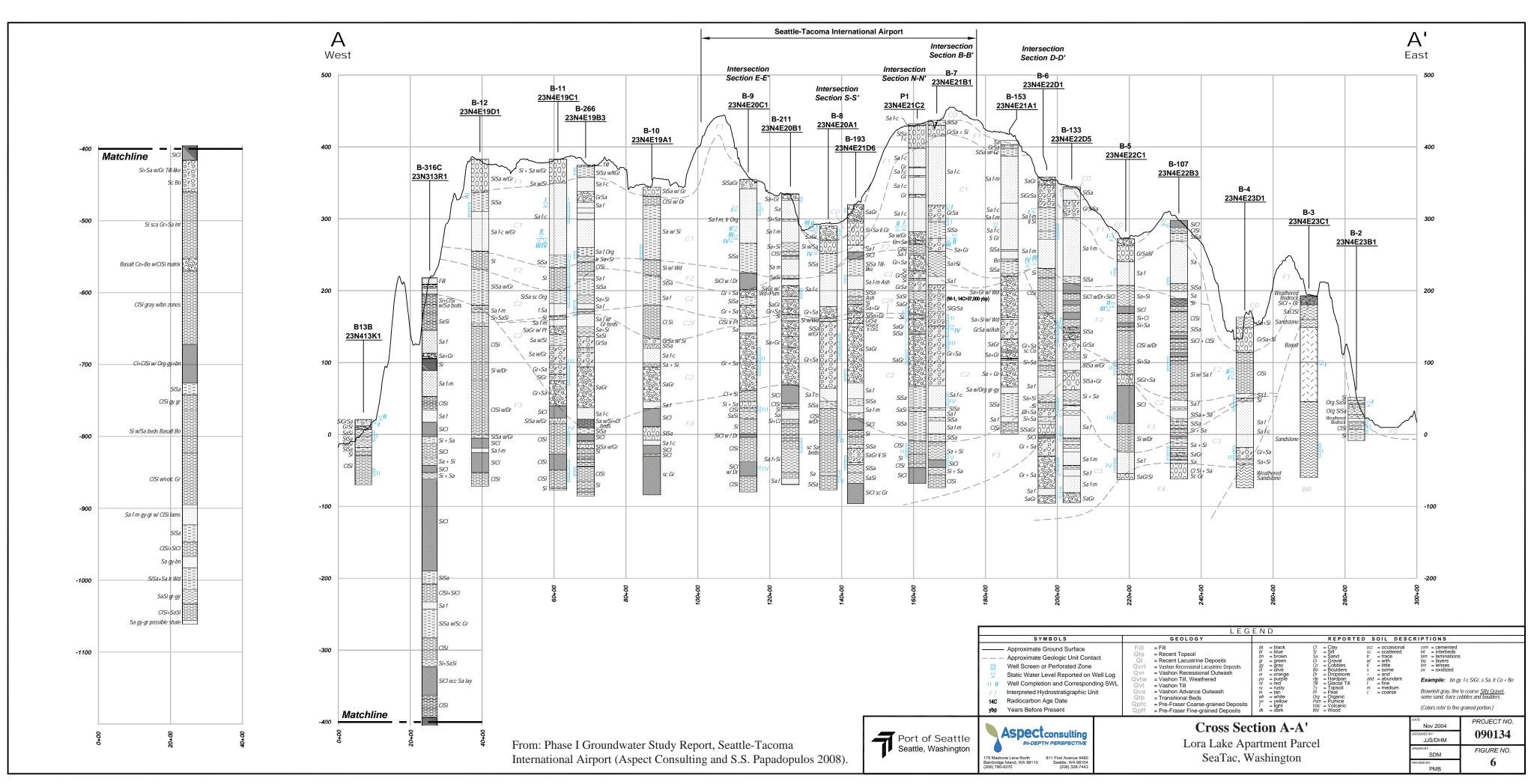


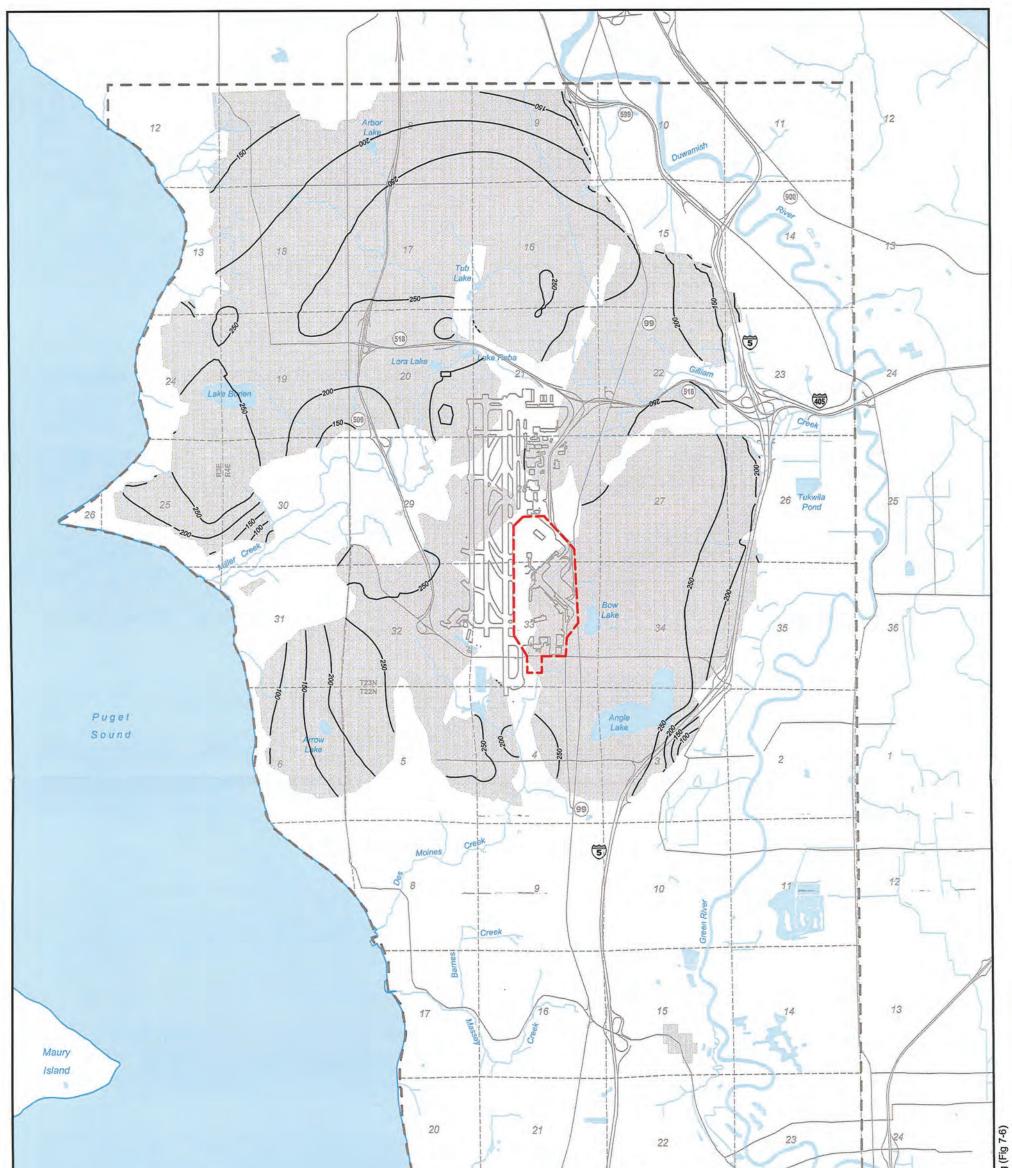




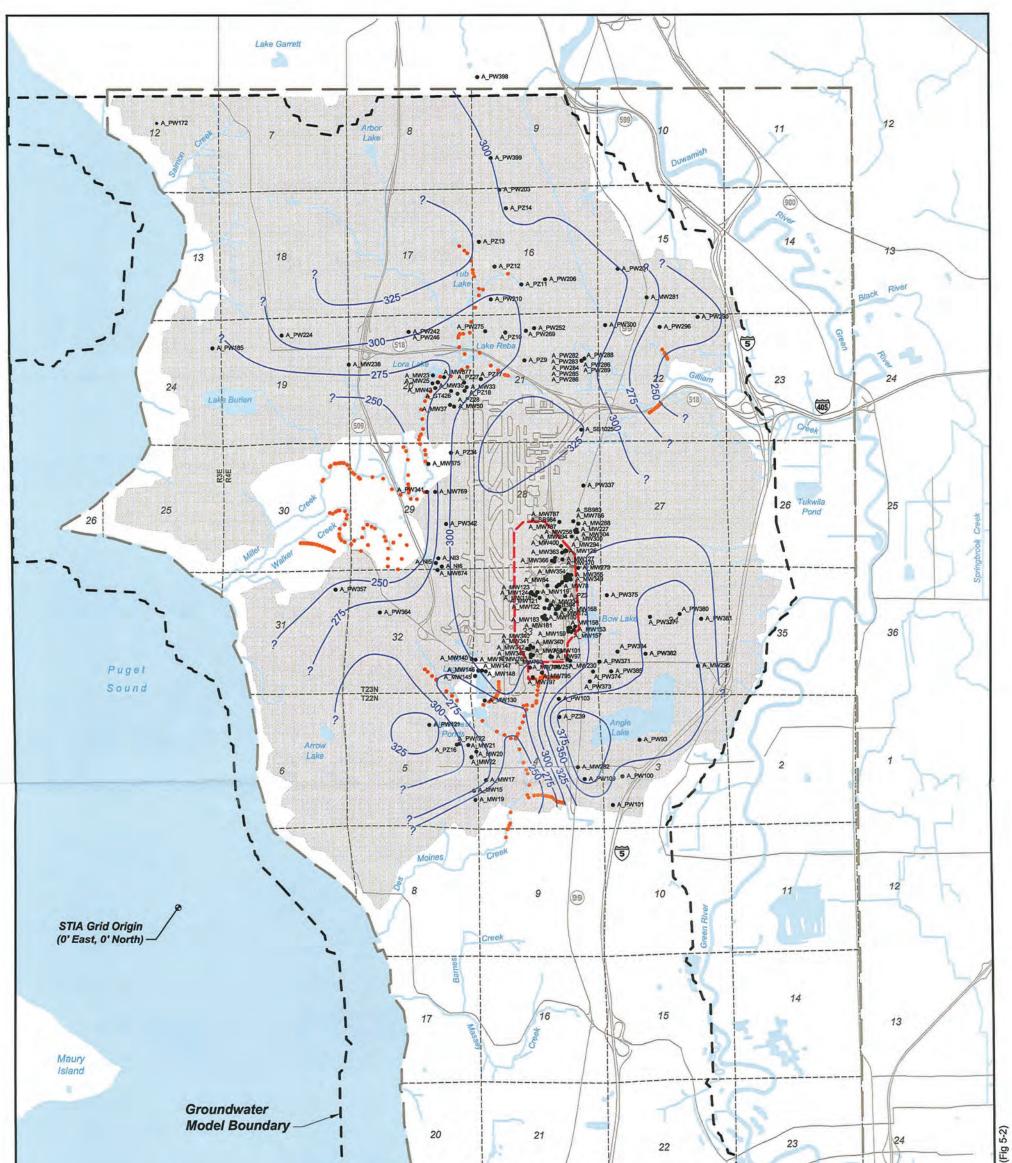


0 4000 8000 Feet		From: Phase I Groundwater Study Report, Seath International Airport (Aspect Consulting and S.	
Elevation Contours of Model Layer Area of Model Layer Aircraft Operations and Maintena Data Collection Boundary Township/Section/Range with Sec	ance Area (AOMA)		
Note: The information shown on this map has been derived from a variety of sources and should be field verified.	Aspect consulting IN-DEPTH PERSPECTIVE 179 Madrone Lane North Bainbridge Island, WA 88110 (206) 780-9307 (206) 328-744	Hydrostratigraphic Contour Map, Top of C1 Unit Lora Lake Apartment Parcel SeaTac, Washington	DATE Dec 2004 DESIGNED BY: JJS/DHM DMMIN BY: SDM REVISED BY: Rev1 - PMB 5

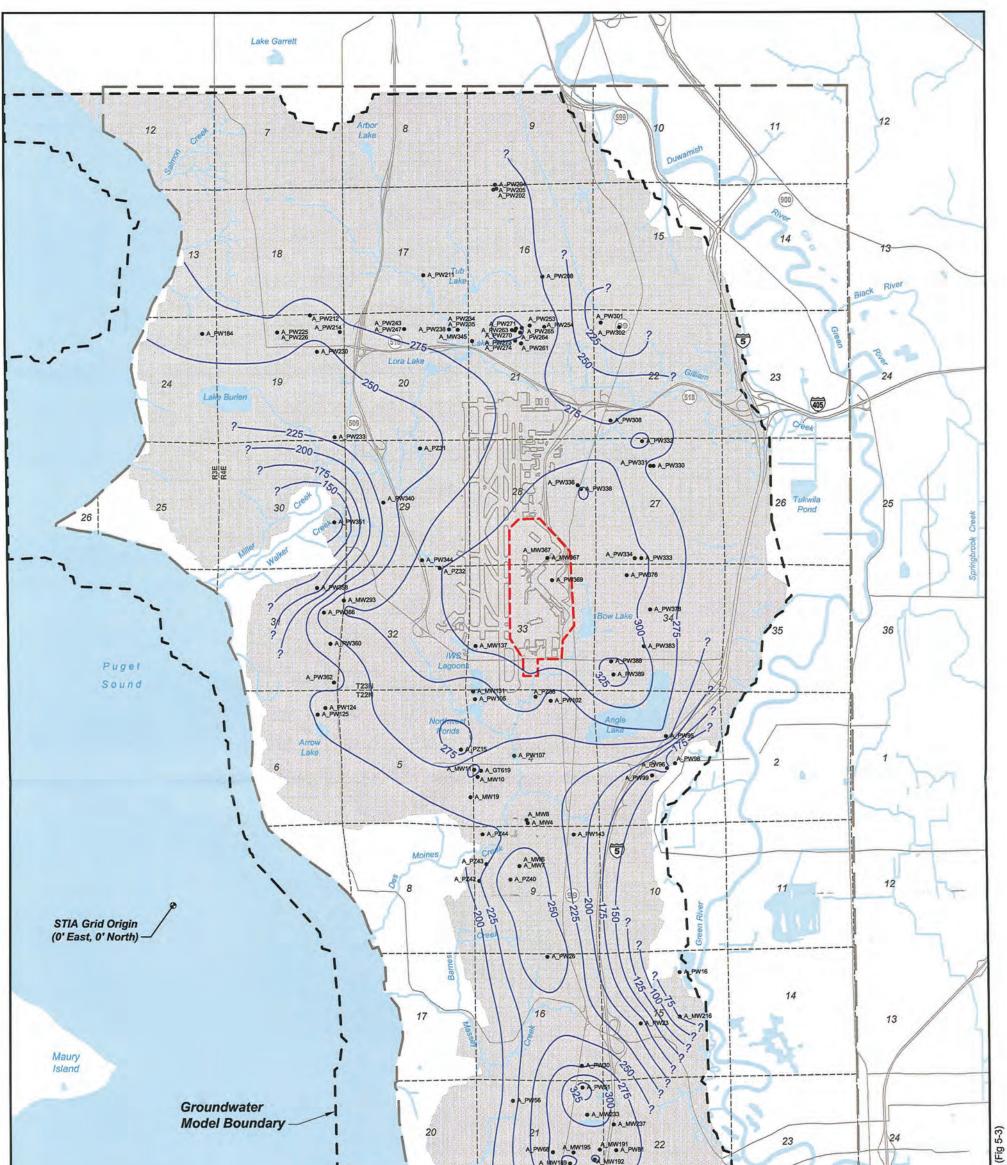




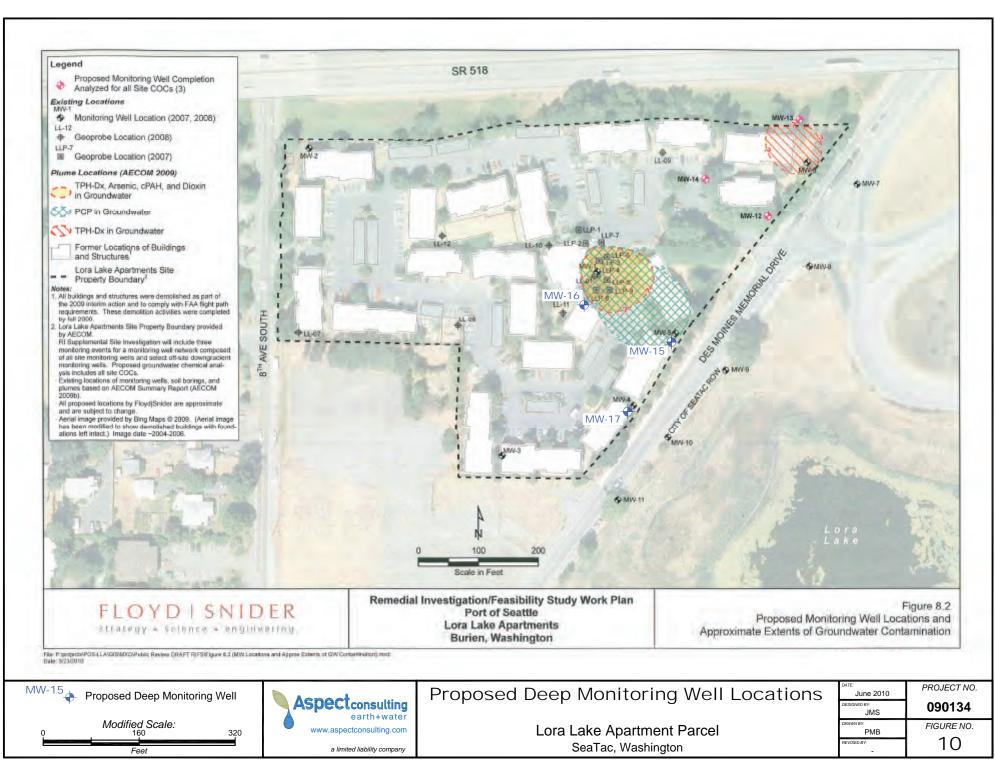
0	4000 8000 Feet		From: Phase I Groundwater Study Report, Sea International Airport (Aspect Consulting and S		
	Elevation Contours of Model Lay Area of Model Layer Aircraft Operations and Maintena Data Collection Boundary Township/Section/Range with Se	ance Area (AOMA)			
Note: The inform been derived from should be field verticed	nation shown on this map has m a variety of sources and erified.	Aspectconsulting IN-DEPTH PERSPECTIVE 179 Madrone Lane North Bainbridge Island, WA 98110 (206) 780-9370 Bainbridge Island, WA 98110 Seatte, WA 98104 (206)-328-7443	Hydrostratigraphic Contour Map, Top of F2 Unit Lora Lake Apartment Parcel SeaTac, Washington	DATE: Dec 2004 DESIGNED OF JJS/DHM DRUNN 87: SDM REVISED 87: Rev1 - PMB	PROJECT NO. 090134 FIGURE NO. 7



0	N 4000 8000	Data Collection Boundary	From: Phase I Groundwater Study Report, Seattle-Tacoma International Airport (Aspect Consulting and S.S. Papadopulos 2008)
<b>egend</b> 36   31 1   6	Feet Extent of Modeled Aquifer Unit Aircraft Operations and Maintena Data Collection Boundary Township/Range/Section Lines a		A_PW100 Well      Groundwater Contour      Control Point
	mation shown on this map has om a variety of sources and verified.	Aspect consulting IN-DEPTH PERSPECTIVE 179 Madrone Lane North Bainbridge laind, WA 98100 (206) 780-9370	Groundwater Elevation Contour Map, C1 Aquifer       Dec 2004       PROJECT NO.         Dec 2004       Dec 2004       090134         Dec 2004       JSL       090134         Dec 2004       PROJECT NO.       090134         Dec 2004       PROJECT NO.       090134         Dec 2004       PROJECT NO.       090134         Dec 2004       PBB       FIGURE NO.         Rev1 - PMB       8

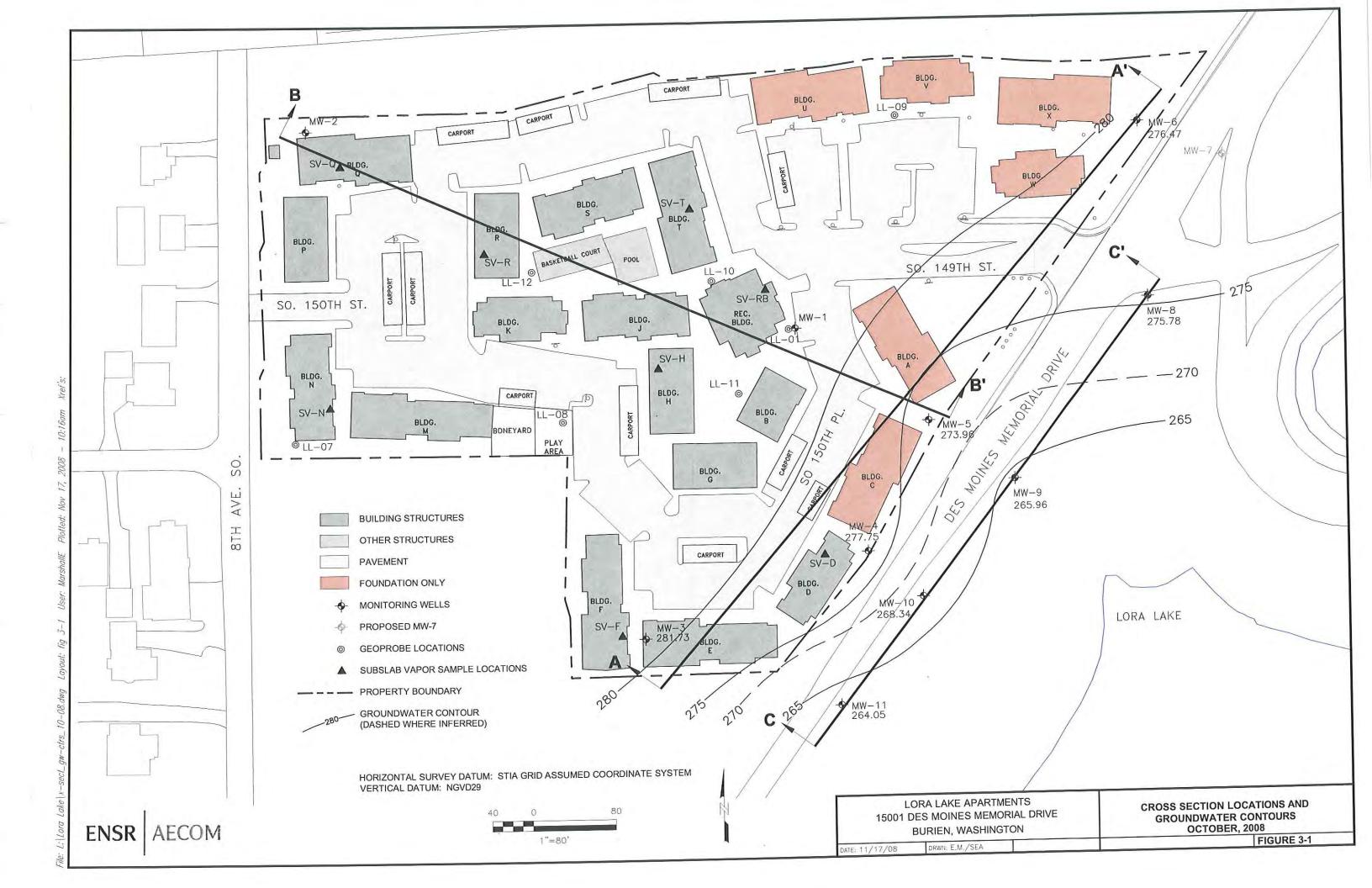


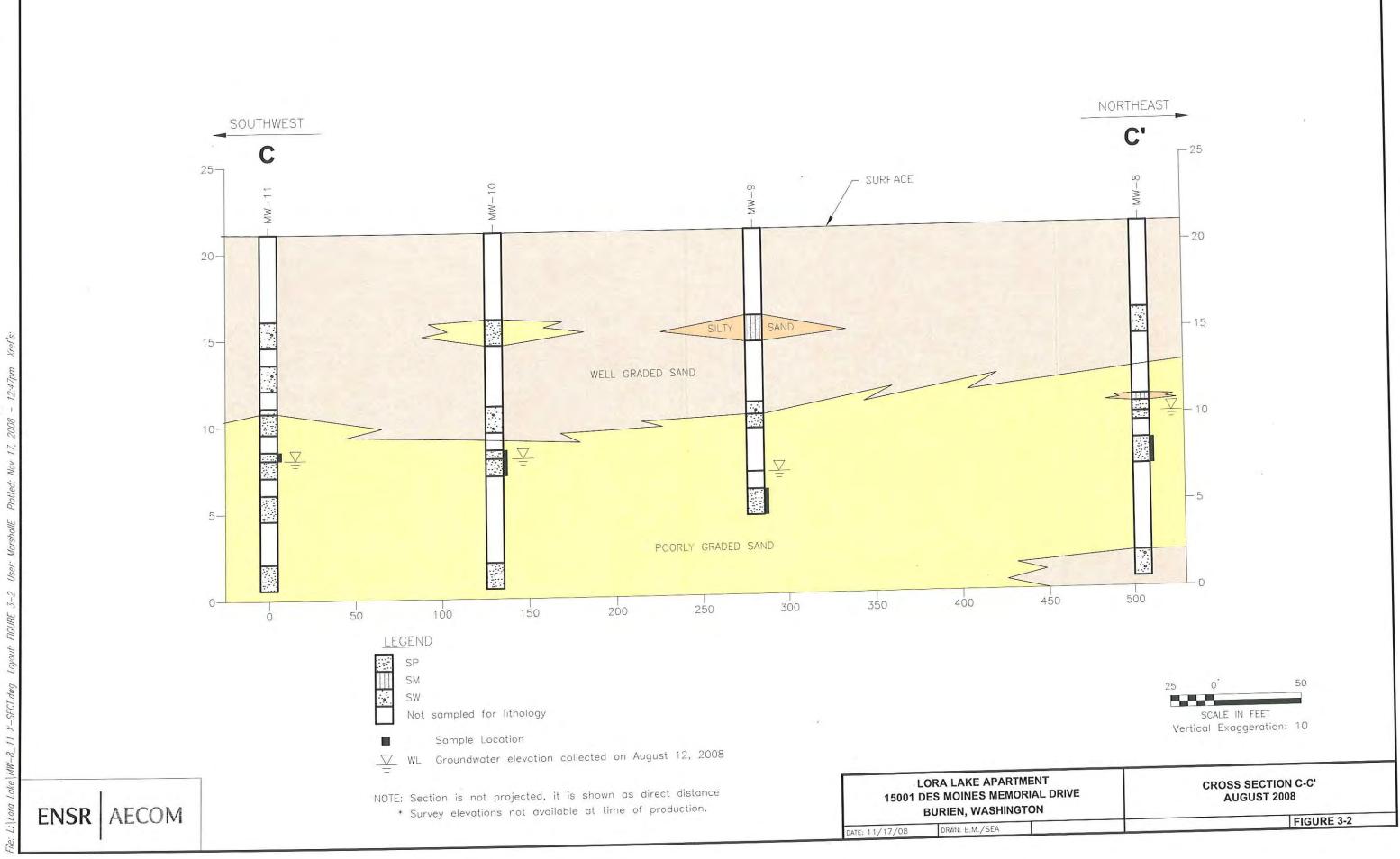
N 0 4000 8000	Data Collection Boundary	McSorley From: Phase I Groundwater Study Report, Seatt International Airport (Aspect Consulting and S.S.	
Extent of Modeled Aquifer Unit Aircraft Operations and Maintena Data Collection Boundary 36   31 Township/Range/Section Lines a		• <sup>A_PW68</sup> Well — 150 — Groundwater Contour	
1 ¦ 6		<b>Groundwater Elevation Contour Map, C2 Aquifer</b> Lora Lake Apartment Parcel SeaTac, Washington	Dec 2004 Dec 2004 Descond Dec JSL DRAWN BT PMB FIGURE NO.

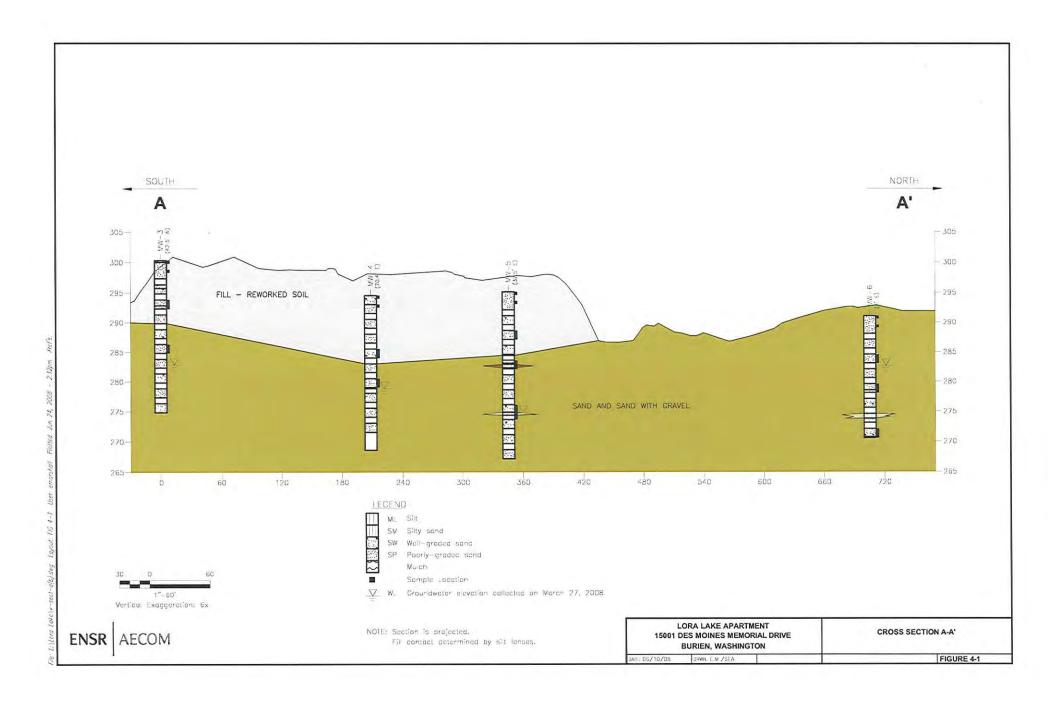


## **APPENDIX A**

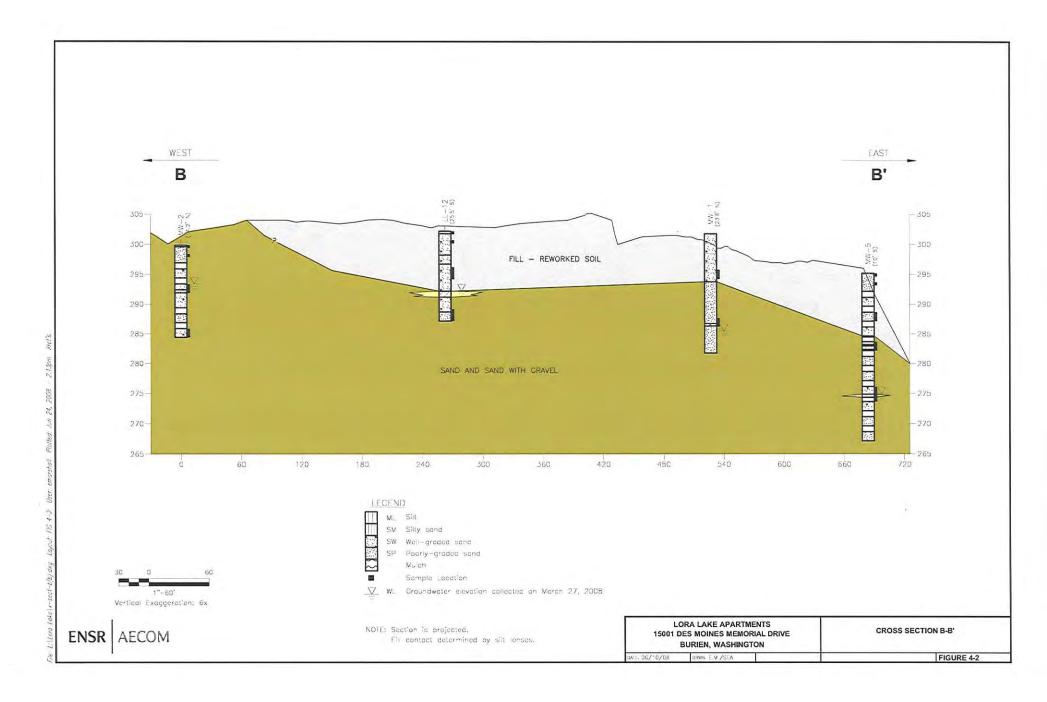
Site Groundwater Flow Direction Map, Monitoring Well Logs and Cross Sections







-



## BORING & WELL CONSTRUCTION LOG

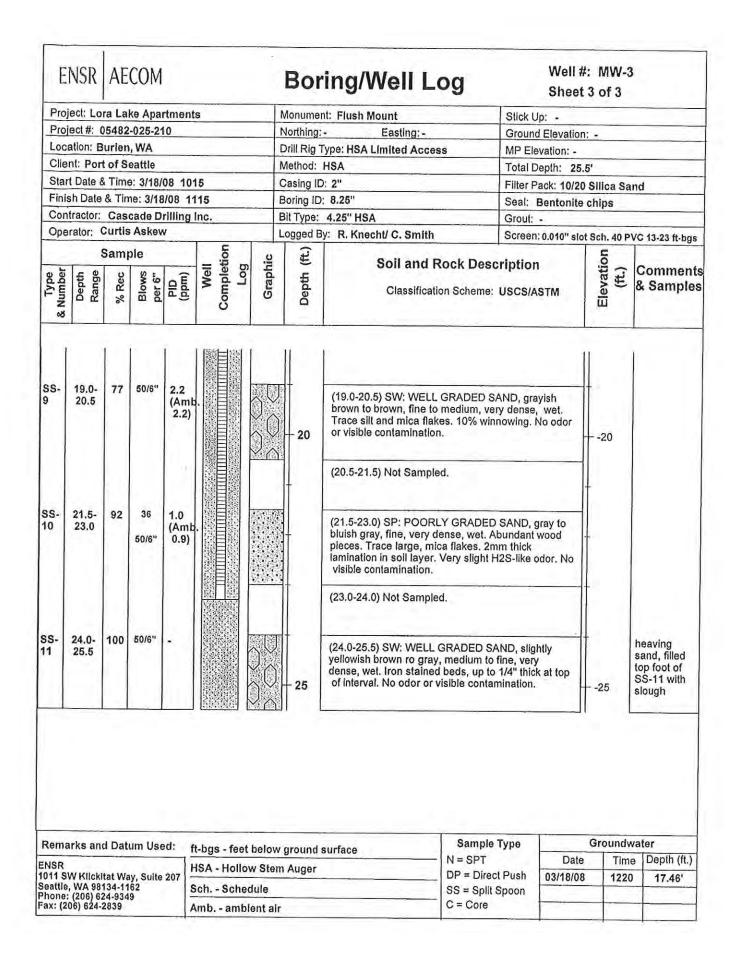
## WELL MW-1 (BORING LLP-4 LOCATION)

GeoScience Man Charcmenta, con 803 1587H S APLINDTON, W	WLING RRET N	services E	O DEPTH (FEET)	SAMPLE ID	BLOW COUNT (PER 6 INCHES)	SAMPLE INTERVAL AND RECOVERY	PID READINGS (PPM)	USCS	PROJECT       DRILLING METHOD         Lora Lake Apartments       Hollow-stem Auger (4.25 ID x 9 0D)         CLIENT       SAMPLING METHOD         Port of Seattle       3-in, O.D. Split-Spoon Sampler         DRILLING COMPANY       SURFACE COMPLETION         Hollow-stem Auger       Flush-mount steel monument         GEOLOGIST       Elevation Ground: Not Measured         H. W. Small, L.HG.       Elevation TOC: Not Measured         START DATE       END DATE         10/25/2007       10/25/2007
Concrete Gurface Seal and Steel Monument ocking, Gasketted PVC Hug Cap			1.0 Feet					FILL	Planter soil over: Gray, brown and black, damp, slightly silty, gravelly, medium to fine SAND (Fill). Occasional fragments of wood, debris, roots and organic matter to approximately 6 feet bgs.
Bentonite Seal — Medium chips			5.5 to 6.0 6.0 to 6.5	MW-1- 5.5 MW-1- 6'	50/6 50/6	1	10		Drove sample at 5.5 feet, but did not encounter target zone of substantial organics, as observed in boring LLP-4. Drove sample at 6.0 feet, but did not encounter target zone of substantial organics. Gravel in shoe.
			7.0 to 8.0 — 8 Feet	6 MW-1- 7'	10 20 22		5		Drilled to 7 feet and drove sample again, but did not encounter target zone of substantial organics. Brown, damp, trace to slightly silty, gravelly, medium to fine SAND (Native soil).
Sand Pack 2/12 Colorado Silica Sand		11111	10					SP	
		1 1 1 1 1 1	14.016	MW-1	4	0	AT	D	Water level approximately 14.0 feet below ground surface at time of drilling.
Well Screen 10 feet; 2-inch Diameter 20- Slot PVC		111111	15.0	14'	3	-/	80	GP	Gray, wet, sandy gravel zone approximately 3-inch thick. Strong hydrocarbon-like odor, sheen on gravel. Gray, wet, trace to slightly silty, gravelly, medium to fine SAND.
		11111111						SP	Total depth = 20 feet.

E	ENSR	AE	СОМ					Bor	ring/Well L	og		Well #: Sheet	: MW-2 1 of 2	
Pro	ject: Lo	ra La	ke Apar	tment	s		1	Monume	nt: Flush Mount		Stick U	D: -		
	ject #: C							Northing	ST. F. DELLANDING STATE			Elevation:		
Loc	cation: B	urlen	, WA				1	Drill Rig	Type: HSA Limited Acces	s		vation: -		
Clie	ent: Port	ofS	eattle					Method:			Total D	epth: 15.8	5'	
Sta	rt Date 8	Time	e: 3/18/	08 080	00		(	Casing I	): 2"			ack: 10/20		nd
Fini	ish Date	& Tin	ne: 3/18	/08 08	50		1	Boring ID	: 8.25"		Seal:	Bentonite	chips	
Cor	ntractor:	Cas	cade D	rilling	nc.	~	8	Bit Type:	4.25" HSA		Grout:	•		
Ope	erator: (	Curtis	Askev	٧			1	Logged E	By: R. Knecht/ C. Smith		Screen	: 0.010" slot	Sch. 40 P\	VC 5-15 ft-bgs
be Der		Sam		- F	Well	Log	Graphic	th (ft.)	Soil and F	Rock Desci	ription			Comment
Type & Number	Depth Range	% Rec	Blows per 6"	(uudd) Clid	Com		Gra	Depth	Classificati	on Scheme; l	JSCS/A	STM	Elev ()	& Sample
SS-	0.0-1.5	66	4	0.7			V	10	(0.0-0.3) MULCH				L <sub>0</sub>	Flush Mount
			5 9	(Ami 0.3)					(0.3-1.5) SP: POORLY yellowish brown, fine, r rootlets, and fine, round contamination.	medium dense	, moist.	Trace	-	Monument 2-inch Sch. 40 PVC risel from 0-5 ft- bgs
SS- 2	1.5-3.0	72	11 13 14	1.2 (Amt 0.3)				-	(1.5-3.0) SP: POORLY yellowish gray, fine, me large, long root, 1/8" in sand, and rounded, find diameter. No odor or vi	edium dense, diameter. Tra e gravel, up to	moist. C ce coar 0.5" in	)ne -	-	0.0-0.5' Sampled for analytical 1.5-2.0'
									(3.0-4.0) Not Sampled.					Sampled for analytical Bentonite
85- 3	4.0-5.5	66	13 23 25	1.1 (Amb 0.3)	- 11111			5	(4.0-5.5) SW: WELL Gi gray grading to dark ye dense, molst to wet. Tr diameter. No odor or vi	llowish gray, f	ine to m	edium,	5	seal from 2 to 4 ft-bgs
								7	(5.5-6.5) Not Sampled.				-	
SS- 4	6.5-8.0	75	20 50/6"	1.4 (Amb 0.6)			0		(6.5-8.0) SW: WELL Gi brown, fine to coarse, v rounded, fine gravel, up or visible contaminatio	very dense, we to 3/4" in dia	et. Trace		-	6.5-8.0' Sampled for analytical
									(8.0-9.0) Not Sampled.				-	
-	-				-	1.4.1				Samula	Tune	1	Groundy	wator
Rem	arks an	d Dat	um Use	d: t	t-bgs -	feet be	low	ground	surface	Sample 1 N = SPT	ype	Date		
ENSR	SW Klick	fat W.	w Sulfa	207 1	ISA - H	ollow	Ster	n Auger		DP = Direct	Push	03/18/08		
Seaftl	e. WA 98	134-11	62		Sch S	chedu	le			SS = Split S	1000000		5045	
none	206) 624-	4-934	9		mb a	mbior	it ale			C = Core	-	+		

	ENSR	AE	СОМ	<u>[</u> ]				Bo	ring/Well L	og		Well # Sheet	: MW-3 1 of 3	3
Pr	oject: Lo	ra La	ke Apa	rtment	s		T	Monum	ent: Flush Mount		Stick L	Jp: -		
	oject#:			10				Northin	g:- Easting:-			d Elevation:	10	
Lo	cation: E	lurier	, WA	_				Drill Rig	Type: HSA Limited Acce	SS	MP Ele	evation: -		
	ient: Por	5						Method	HSA		Total D	Depth: 25.5	5'	
	art Date a							Casing	ID: 2"		Filter F	ack: 10/20	Silica Sa	ind
-	nish Date	_		All and the state				Boring I	D: 8.25"		Seal:	Bentonite	chips	
	ontractor:				Inc.		_		: 4.25" HSA		Grout:	•		
OF	perator:			N			_		By: R. Knecht/ C. Smith	12	Screen	: 0.010" slot	Sch. 40 P	VC 13-23 ft-bgs
Type	Depth Range	Sam	Blows ald	(Indq)	Well	Log	Graphic	Depth (ft.)		Rock Desci	677		Elevation (ft.)	Comment & Sample
L		%	b B	a a	Cor		G	De	Classificat	ion Scheme: 1	USCSIA	SIM	Ele	
SS-	0.0-1.5	66	5	0.7 (Amt			V	0	(0.0-0.3) MULCH				L <sub>0</sub>	Flush Mount
	1		8	0.5)					(0.3-1.5) SP: POORL	VODADEDO			11	Monument 2-inch
			23		Constant Cont			]	yellowish brown to yel moist. 10% medium to	lowish brown,	fine, de	nse.	÷.,	diameter, Sch. 40
SS-	1.5-3.0	72	13	1.2	Al and Sec lines to				fine to coarse gravel, u No odor or visible con	up to 1.5" long.	Trace	rootlets.		PVC riser from 0-13 ft-
2	1		17	(Amb 0.5)				1-	(1.5-3.0) SP: POORLY		ND. bro	/	-	bgs
			20						slightly dark brown, fin to coarse sand. Trace gravel, up to 1.5" long. contamination.	e, dense, mois , elongated, fir	st. 10% ne to co	medium		0.0-0.5' Sampled for analytical;
									(3.0-4.0) Not Sampled					mulch not included in sample
SS- 3	4.0-5.5	66	13	1.1 (Amb					(4.0-4.6) SP: POORLY	GRADED SA	ND, bro	own to		1.5-2.0' Sampled for
			17	0.6)			$\overline{()}$		dark brown, fine, dens rounded, elongate, coa to 0.5" long. No odor o	arse sand and	fine gra	vel up		analytical
			16				$\sim$	-5	1			/	5	Bentonite seal from 2
									(4.6-5.5) SW: WELL G brown, fine to medium, rounded, coarse sand diameter. No odor or v	dense, moist. and fine grave	Trace	1		to 11 ft-bgs
SS-	6.5-8.0	75	20	1.4 (Amb					(5.5-6.5) Not Sampled.				I.	
			23	0.6)					(6.5-7.5) SP: POORLY dark brown, fine, very of	GRADED SA	ND, bro	wn to	-	6.5-8.0'
			27				$\hat{\cap}$		coarse sand. 10% rou in diameter. No odor or	nded, fine grav	el. up t	0 1/4"		Sampled for analytical
ss-	9.0-	66	14	1.3			- 12	+	(7.5-8.0) SW: WELL G brown, fine to medium. and fine gravel, up to 1 contamination.	Trace rounded	d coars	e sand		
Rem	arks an	d Date	ım İ lee	· · be			1			Sample 1	Vpe		Groundy	vater
ENSF					Labor Ton		-		l surface	- N = SPT	11.2	Date	Tim	
1011	SW Klicki	tat Wa	y, Suite	207 -	1000			n Auger		DP = Direct		03/18/08	1220	
seatt	e, WA 98	W Klickitat Way, Suite 207 , WA 98134-1162 ; (206) 624-9349 Sch Schedule				chedul	e			SS = Split S	poon			

E	ENSR	AE	сом				Boi	ring/Well Lo	g	Well # Sheet	: MW-3 2 of 3	
Pro	oject: Lo	ra La	ke Apa	rtmen	ts		Monume	nt: Flush Mount	SI	lick Up: -		
Pro	oject #:	05482	2-025-21	10			Northing	- Easting:-		round Elevation	: •	
Loc	cation: E	lurier	, WA				Drill Rig	Type: HSA Limited Access	M	P Elevation: -		
Clie	ent: Por	t of S	eattle			1	Method:	HSA	To	otal Depth: 25.	5'	
	irt Date a						Casing II	D: 2"	FI	lter Pack: 10/20	Silica Sa	nd
	ish Date			A Contract of the local sector			Boring IC	): 8.25"	Se	eal: Bentonite	chips	
	ntractor:				Inc.	-		4.25" HSA	G	rout: -	_	
Op	erator:	Curtis	s Askev	v	1		1	By: R. Knecht/ C. Smith	Sc	creen: 0.010" slo	t Sch. 40 P	VC 13-23 ft-bgs
er Der		Sam		Ê	Well Completion Log	Graphic	th (ft.)	Soil and Ro	ock Descrip	otion	Elevation (ft.)	Comment
Type & Number	Depth Range	% Rec	Blows per 6"	(mqq) Cliq	Com	Gra	Depth	Classification	Scheme: US	CS/ASTM	Elev (1	& Samples
5	10.5		50/6"	(Am 0.7)	b.		3	(8.0-9.0) Not Sampled.		'/	ſ	
							- 10	(9.0-10.5) SP: POORLY ( yellowish gray, fine, very coarse sand. Trace round long. No odor or visible co	dense. 20% m ded. fine grave	redium to	-10	2-inch diameter, 0.010-inch
						Q	1	(10.5-11.5) Not Sampled.			t	slot, Sch. 40 PVC screen from 13 to
SS- 6	11.5- 13.0	83	50/6"	1.7 (Ami 0.7)				(11.5-13.0) SW: WELL G grayish brown to brown, f 15% coarse sand. 10-15 fine to coarse gravel, up and iron staining. No odd	fine to medium % sub rounde to 1" in diame	, very dense. d to rounded, ter. Trace silt	-	23 ft-bgs
						979999-9 875. •		(13.0-14.0) Not Sampled.	6		+	
SS- 7	14.0- 15.5		23 50/6"	0.5 (Amt 0.5)			- 	(14.0-15.5) SP: POORLY yellowish brown, fine, ven Trace medium sand and s contamination.	v dense, moist	to wet.	15	14-15.5' Sampled for analytical 10/20 silica sand pack
						<u></u>		(15.5-16.5) Not Sampled.		0		from 11 to 23.5 ft-bgs
5S- 3	16.5- 18	94	50/6"	2.2 (Amb 0.9)				(16.5-18.0) SP: POORLY brownish gray, medium, v 10-15% fine sand. Trace s contamination.	ery dense, mo	ist to wet.	-	
Į							11	(18.0-19.0) Not Sampled.				2 - 1 - 1
Rem	arks and	1 Dat	um Ha-	d.		72		6	Sample Typ	e	Groundy	vater
		a Dat	un Use		t-bgs - feet			surface	N = SPT	Date		
NSR 011 S	W Klicki	tat Wa	y, Sulte	207	HSA - Hollow	w Ster	m Auger		DP = Direct Pu			
eattle	, WA 98 : (206) 62	34-11	62		Sch Sched	dule			SS = Split Spo			
av. 12	206) 624	2839	r,		Amb ambi	ant al			C = Core			



E	ENSR	AE	СОМ				Во	ring/Well L	og		Well # Sheet	: MW-4 1 of 3	
Pro	oject: Lo	a La	ke Apa	tmer	its		Monum	ent: Flush Mount		Stick Up	): -		
Pro	oject#: 0	5482	-025-21	0			Northin	g:- Easting:-			Elevation		
Loc	cation: B	urlen	, WA				Drill Rig	Type: HSA Limited Acces	SS	MP Elev	vation: -		
Clie	ent: Port	ofS	eattle				Method			Total De	epth: 26'		
Sta	art Date 8	Time	e: 3/17/	08 14	100		Casing	ID: 2"		Filter Pa	ack: 10/20	Silica Sa	nd
Fin	ish Date	& Tin	ne: 3/17	/08	1515		Boring	D: 8.25"		Seal: E	Bentonite	chips	
Co	ntractor:	Cas	cade D	rilling	lnc.		Bit Type	: 4.25" HSA		Grout:	•		
Op	erator: C	Curtis	Askev	V			Logged	By: R. Knecht/ C. Smith		Screen:	0.010" slot	Sch. 40 P	VC 11-25.75 ft-
		Sam			di etion	g hic	(ft.)	Soil and F	Rock Desc	ription		tion .)	Comment
& Number	Depth Range	% Rec	Blows per 6"	(mqq)	Well	Log Graphic	Depth	Classificat	ion Scheme:	USCS/A	бтм	Elevation (ft.)	& Sample
SS- 1 SS-	0.0-1.5		11 10 10 17	0.0	in formation to the service state each			(0.0-1.5) SW: SAND, t medium dense, moist. fine gravel. Trace silt up to 1" long. Abunda 0.2', Moderate organic contamination.	20% rounded, and rounded, int grass and r	, coarse coarse g ootlets fr	ravel,	0	Flush Moun Monument 2-inch diameter, Sch. 40 PVC riser from 0 to 11
2			10 10				-	(1.5-2.0) SP: POORLY fine, medium dense, m fine, gravel. Organic-li contamination.	noist. 15% silt.	Trace ro			ft-bgs
SS-	4.0-5.5	66	12	0.0			<u>986</u> _	(2.0-3.0) SP: POORLY brown with pockets of dense, moist. 20% me rounded, fine gravel. N contamination.	gray from 2.5- edium sand fro	3', fine, r	nedium	-	0.0-0.5' Sampled for analytical 1.5-2.0'
3	4.0-5.5	00	10	0.0				(3.0-4.0) Not Sampled					Sampled for analytical
			10				-5	(4.0-5.5) SP: POORLY brown grading to light medium dense, moist. Trace rounded, fine gra downhole. Trace rootle contamination.	yellowish brov 20% medium avel, content o	vn at 5.0' sand fro decrease	, fine, m 4-5'. s	5	Bentonite seal from 2 to 9 ft-bgs
								(5.5-6.5) Not Sampled.					
88- 4	6.5-8.0	75	10 50/6"	0.0				(6,5-8.0) SP: POORLY brown, medium, very d fine to coarse gravel. T visible contamination.	GRADED SA	20% rour	ided,		
								(8.0-9.0) Not Sampled.					
Rem	narks an	d Dat	um Us	ed:	ft-bas - fe	eet belo	ow groun	d surface	Sample	Туре		Ground	
ENSE	2			-	HSA - Ho				- N = SPT		Date		Call In Column Color
1011 Seatt	SW Klicki le, WA 98 e: (206) 6: (206) 624-	134-1	162	207	Sch Sc				DP = Direct SS = Split C = Core		03/17/08	164	4 15.70'

E	NSR	AE	СОМ				Boi	ing/Well Log			: MW-4 2 of 3	
Pro	ect: Lo	ra La	ke Apai	tmen	ts	1	Monume	nt: Flush Mount	Stick	Up: -		
1.5			-025-21				Northing			nd Elevation		
1.00	ation: B				-1-1-1			Type: HSA Limited Access		levation: -		
	nt: Por		time				Method:			Depth: 26'	100	
Star	t Date 8	Time	e: 3/17/	08 14	00		Casing II	D: 2"		Pack: 10/20		nd
Finis	sh Date	& Tin	ne: 3/17	/08 1	515		Boring ID	and the second sec	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Bentonite		
Con	tractor:	Cas	cade D	rilling	Inc.		Bit Type:	4.25" HSA	Grou		****	
Ope	rator: (	Curtis	s Askev	v			Logged E	By: R. Knecht/ C. Smith	Scree	en: 0.010" slo	t Sch. 40 PV	VC 11-25.75 ft-b
-		Sam			etion	hic	(ft.)	Soil and Rock				Comment
Type & Number	Depth Range	% Rec	Blows per 6"	(uudd) QId	Well Completion Log	Graphic	Depth	Classification Sci				& Samples
SS- 5	9.0- 10.5	66	19 50/6"	0.0			10	(9.0-10.5) SW: WELL GRAD GRAVEL, brown to yellowish very dense, moist. 30% round flat, elongate, fine to coarse of Slight sweet odor, no visible of (10.5-11.5) Not Sampled.	brown, fine to ded to sub rou gravel, up to 1	o coarse, unded, " long.	-10	9.5-10.5' Sampled for analytical
SS- 6	11.5- 13.0	83	19 21 24	0.0				(11.5-13.0) SP: POORLY GR yellowish brown grading to ye dense, moist. Few 0.5" thick I sand. Trace coarse sand. No contamination.	ellowish gray, lenses of very	fine, fine		2-inch diameter, 0.010-inch slot, Sch. 40 PVC screen from 11 to 25.75 ft-bgs
SS- 7	14.0- 15.5		23 50/6"	0.0			- 15	(13.0-14.0) Not Sampled. (14.0-15.5) SP: POORLY GR brownish gray, fine, very dens Trace mica. No odor or visible	se, moist. 10-	15% silt.	15	14-15.5' Sampled for analytical 10/20 silica
							4	(15.5-16.5) Not Sampled.				sand pack from 9 to 26 ft-bgs
SS- 8	16.5- 18	94	19 50/6''	0.0				(16.5-18.0) SP: POORLY GR. grayish brown, fine, very dens sand at 16.75-17'. Little iron 17.9', 4mm thick black and iro odor or visible contamination.	se, moist. 20% staining at 17 on stained be	5-18'. At		
Rema	arks and Datum Used: ft-bos - feet I						N APALINA	surface Si	ample Type		Groundy	vater
ENSR					ft-bgs - feet			Surface N =	SPT	Date	Tim	e Depth (ft.)
1011 S	R SW Klickltat Way, Suite 207 le, WA 98134-1162				17.5 × 17.4		m Auger	UP -	= Direct Push = Split Spoon	03/17/08	3 1644	15.70'

EN	<b>VSR</b>	AE	COM				Во	ring/Well L	og	Well # Sheet	: MW-4 3 of 3	
Proje	cl: Lo	ra Lak	e Apar	tment	s ·		Monume	ent: Flush Mount	St	ck Up: -		
			025-21				Northing			ound Elevation		
Loca	tion: B	urlen	WA				Drill Rig	Type: HSA Limited Acces	ss MF	Elevation: -		
Clien	it: Por	t of Se	attle				Method:	HSA	То	tal Depth: 26'		
Start	Date 8	k Time	: 3/17/	08 14	00		Casing I	D: 2"	Fill	ter Pack: 10/20	Silica Sa	nd
Finis	h Date	& Tim	e: 3/17	/08 1	515		Boring I	D: 8.25"	Se	al: Bentonite	chips	
			ade D		Inc.			: 4.25" HSA	Gr	out: -		
Oper	ator: (	Curtis	Askev	V	r	-	1 1 22	By: R. Knecht/ C. Smith	Sc	reen: 0.010" slo	t Sch. 40 P	VC 11-25.75 ft-t
9 er		Samp		Ê	Well	Graphic	th (ft.)	Soil and I	Rock Descrip	tion		Comment 8 Somela
& Number	Depth Range	% Rec	Blows per 6"	(mqq) DIA	Com	0	Depth	Classificat	ion Scheme: USC	CS/ASTM	Elev ()	& Samples
1		Ì		1		<u>ate</u>		(18.0-19.0) Not Sampl	ed		+	
SS- 9	19.0- 20.5	77	22 50/6"	0.0				(19.0-20.0) SP: POOR fine, very dense, wet. pocket with one white visible contamination.	2.5" long, gray, fri	able, clay		
							20	(20.0-20.5) SP: POOR fine, very dense, wet. No odor or visible cont	Trace rounded, co	ND, brown, barse sand.	+ -20	
							T	(20.5-21.5) Not Sampl	ed.		<b>†</b>	
	21.5- 23.0	92	19 50/6"	0.0			-	(21.5-23.0) SP: POOR slightly yellowish brow Wood pieces and gray odor or visible contam	vn, fine, very dens sand at top of int	se, wet.	÷	
								(23.0-24.0) Not Sampl	ed.			Slight heaving sand at bottom of
	24.0- 25.5	100	50/5"	0.0				(24.0-25.5) SP: POOR yellowish brown to bro 25% winnowing. No o	wn, medium, very	dense, wet.	25	borehole
				-				(25.5-26.0) Not Sample	ed.			
									1.2000			
Rema	rks an	d Dat	um Us	ed:	ft-bgs - fe	et belo	ow groun	d surface	Sample Typ		Ground	
INSR					HSA - Hol	low St	em Auge	r	- N = SPT DP = Direct Pt	Date 15h 03/17/0		
Seattle,	WA 98	134-11	ay, Sulte 62		Sch Sch	edule			SS = Split Spo		0 104	4 15.70
-none:	(206) 6	24.934	3				200		C = Core		1	

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E	ENSR	AE	СОМ				Boi	ring/Well L	og		#: MV t 1 of		
Pro	ject: Lo	ra La	ke Apa	tmer	its		Monume	ent: Flush Mount	Stic	(Up: -			
Pro	oject #: (	5482	-025-21	0			Northing	:- Easting:-		und Elevatio	n: •		
Lo	cation: B	urien	, WA				Drill Rig	Type: HSA Limited Acces	s MP	Elevation: -			
Cli	ent: Por	ofS	eattle	_			Method:	HSA	Tota	Depth: 28	3'		
Sta	rt Date &	Time	e: 3/17/	08 10	)25	- 1	Casing II	D: 2"	Filte	r Pack: 10/2	0 Silica	Sar	d
Fin	ish Date	& Tin	ne: 3/17	/08 *	210		Boring IC	): 8,25"	Sea	Bentonit	e chips	•	
	ntractor:				Inc.		Bit Type:	4.25" HSA	Grou	ıt; -			
Ор	erator: (	Curtis	Askev	/			Logged E	By: R. Knecht/ C. Smith	Scre	en: 0.010" sl	ot Sch. 4	10 PV	C 13-28 ft-bg
	-	Samp	-		Well Ipletion Log	ohic	(ft.)	Soil and R	ock Descripti	on	tion	•	Commen
Type & Number	Depth Range	% Rec	Blows per 6"	(mqq)	Well Completion Log	Graphic	Depth	Classificatio	on Scheme: USCS	ASTM	Elevation		& Sample
SS- 1 SS-	0.0-1.5	66	15 14 20 50/5"	0.0			0	(0.0-1.5) SP: POORLY dark brown, fine, loose, gravel, 0.25-0.5" long. ( diameter. Trace straw. contamination.	moist. 15% round One rounded grave	ed, fine	<b>1</b> 0		Flush Moun Monument 2-inch diameter Sch. 40 PVC riser
2	1.5-3.0	100	50/5	0.0				(1.5-3.0) SP: POORLY brown, fine, dense, moi gravel, rounded, up to 0 contamination.	st. Trace coarse s	and to fine			from 0 to 13 ft-bgs 0.0-0.5' Sampled for
								(3.0-4.0) Not Sampled.			][		analytical 1.5-2.0'
SS- 3	4.0-5.5	91	24 50/6"	0.0		$)_{0}^{0}$		(4.0-5.5) SW: WELL GF brown, fine to medium, rounded, gravel, up to 1 content increases to 30 visible contamination.	very dense, moist. /2" in diameter. Gi	20% sub avel			Sampled for analytical Bentonite seal from 2
							]	(5.5-6.5) Not Sampled.					to 11 ft-bgs
SS- 4	6.5-8.0	100	50/6"	0.0				(6.5-8.0) SM: SILTY SA gray, fine, very dense, rounded, sand and fine long. No odor or visible	moist. 20% silt. 10 gravel. One grave	%	+	18	6.5-8.0' Sampled for analytical
								(8.0-9.0) Not Sampled.					
SS- 5	9.0- 10.5	75	30 50/6"	0.0			+ + 10	(9.0-10.5) SW: WELL G brownish gray, fine to m wet. Trace coarse sand, pockets of silt and very hydrocarbon-like odor in No visible contaminatio	edium, very dense fine gravel, and 1 fine sand. Trace 0.5" thick silt lens	, moist to " thick			
Rem	arks an	d Dat	um Use	d:	ft has fast !	alar			Sample Type		Grou	ndw	ater
ENSR		a val	ani Ust	1	ft-bgs - feet l			surrace	N = SPT	Dat		Time	12-12-12-12-12
1011	SW Klick	tat Wa	ay, Suite	207	HSA - Hollow	Ster	m Auger		DP = Direct Pust			1332	
Seattl	e, WA 98	134-11	62		Sch Sched	ule			SS = Split Spoor			-	1

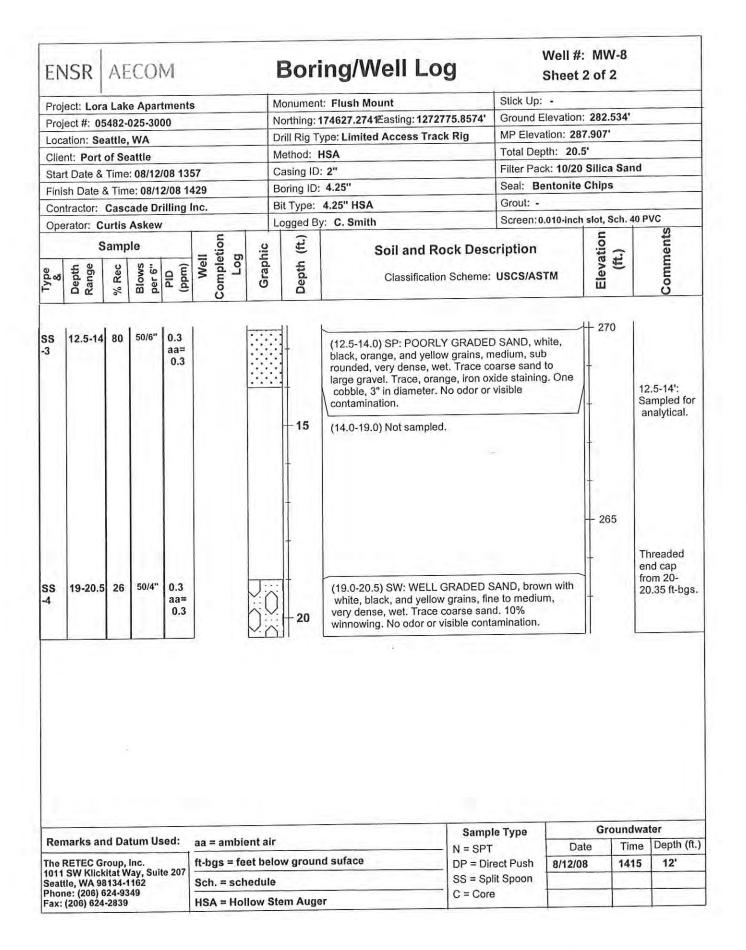
Project Project Locatic Client: Start D Finish Contra Operat	on: Bu Port	a Lak					Bor	ing/Well Log		#: MW-5 t3 of 3	
Project Locatio Client: Start D Finish I Contra	on: Bu Port	-	e Anar	tment	s	-	Monumer	nt: Flush Mount	Stick Up: -	1. N. M.	
Locatio Client: Start D Finish I Contra	on: Bi Port	5482-					Northing:		Ground Elevatio	n' •	
Client: Start D Finish Contra	Port	A		-				Type: HSA Limited Access	MP Elevation: -		
Finish I Contra	S ate C						Method:		Total Depth: 28		
Contra	Jaio u	Time	: 3/17/	08 10:	25		Casing ID	): 2"	Filter Pack: 10/2	1. 12 Jac - 1. 1.	nd
	Date	& Tim	e: 3/17	/08 12	210		Boring ID	: 8.25"	Seal: Bentonite	chips	
Operat	actor:	Caso	ade Di	rilling	Inc.		Bit Type:	4.25" HSA	Grout: -		
	tor: C	urtis	Askew	1			Logged B	y: R. Knecht/ C. Smith	Screen: 0.010" sl	ot Sch. 40 P	VC 13-28 ft-bgs
	S	Samp	le		uo	0	(ft.)	Orlined Deals Dea		5	
& Number	Depth Range	% Rec	Blows per 6"	(mqq) QIq	Well Completion Log	Graphic	Depth (	Soil and Rock Des Classification Scheme		Elevation (ft.)	Comment & Samples
	21.5- 23.0	66	32 50/6''	0.0			-	(21.5-23.0) SW: WELL GRADED a brown grading to yellowish brown, sub rounded, very dense, wet. 209 rounded, elongated, fine to coarse or visible contamination. (23.0-24.0) Not Sampled.	sub angular to % sub angular to		
	24.0- 25.5	100	25 50/6''	0.0			- 25	(24.0-25.5) SP: POORLY GRADE brown, fine, very dense, wet. Trac- and rounded, fine gravel. No odor contamination.	e medium sand		
	6.5-	100	50/6''	0.0		旅程	-	(25.5-26.5) Not Sampled. (26.5-28.0) SP: POORLY GRADE	D SAND, gray		Slight heaving sand at
2 2	28			2190				from 26.5-27.0', sharp contact to y 27.0', fine, very dense, wet. 15% g coarse sand. Trace, soft, silt. 50% odor or visible contamination.	ray, medium to		boltom of borehole

El	NSR	AE	СОМ				Bo	ring/Well Lo	og		Well #	: MW-6 1 of 2	
Proj	ect: Lor	a Lal	ke Apa	rtment	S		Monume	nt: Flush Mount	s	Stick Up			
Proj	ect #: 0	5482	-025-21	0			Northing	:- Easting:-			Elevation:		
Loca	ation: B	urien	, WA				Drill Rig	Type: HSA Limited Acces	s N	AP Elev	ation: -		
	nt: Port						Method:	HSA	T	otal De	pth: 20.8	5'	
	t Date &				the second second		Casing II	D: 2"	F	iller Pa	ck: 10/20	Silica Sa	nd
	sh Date						Boring ID		S	eal: B	entonite	chips	
	tractor:				nc.	_		4.25" HSA		Frout: -			
Ope	rator: C			V	-			By: R. Knecht/ C. Smith	S	creen: (	0.010" slot		VC 5-15 ft-bgs
177	S	amp	ole		tion	ic	(ft.)	Soil and R	ock Descrip	otion		u	
Type & Number	Depth Range	% Rec	Blows per 6"	(mqq) Olq	Well Completion Log	Graphic	Depth		on Scheme: US		тм	Elevation (ft.)	Comments & Samples
1	0.0-1.5 1.5-3.0	83 66	3 5 14 20	0.9 (Amb 0.3) 1.2 (Amb	אראי עוד וריינשי בקבורבר איני אראי עוד וריינשי בקבור איני		0	(0.0-1.5) SP: POORLY fine, medium dense, mo medium to coarse sand in diameter. Abundant top. Moderate organic o contamination.	bist. 20-25% sill Trace fine gra rootlets through	t. 10% ivel, up nout, gr	to 1/4"	0	Flush Mount Monument 2-inch diameter Sch. 40 PVC riser from 0 to 5
			25 25	0.3)				(1.5-3.0) SP: POORLY slightly dark brown, fine Trace rounded, coarse rootlets. Friable. Modera contamination.	, very dense, m sand to fine gr	noist. 20 avel. Lil	% silt. Ile	_	ft-bgs 0.0-0.5' Sampled for analytical
SS-	4.0-5.5	66	3	1.3				(3.0-4.0) Not Sampled.					
5			4 5	(Amb 0.4)			-5	(4.0-5.5) SP: POORLY ( reddish brown, fine, loos to coarse sand. Trace ro 3/4" in diameter. Trace r or visible contamination.	se, moist. 10-18 ounded, fine gra rootlets in catch	5% mec avel, up	lium to	5	1.5-2.0' Sampled for analytical
							I	(5.5-6.5) Not Sampled.					6.5-8.0'
SS- 6 4	6.5-8.0	72	11 9 13	1.4 (Amb 0.6)				(6.5-8.0) SP: POORLY ( brown grading to light re dense, moist. Trace fine diameter. No odor or vis	ddish brown, fi gravel, up to 1	ne, med /2" in	sh dium		Sampled for analytical
								(8.0-9.0) Not Sampled.					
SS- 5	9.0- 10.5	94	20 20	1.5 (Amb 0.6)			- 10	(9.0-10.5) SP: POORLY iron staining, fine, dense matter and silt. No odor	e, moist. Trace	organic		10	
			20					(10.5-11.5) Not Sampled	J.				Bentonite seal from 2 to 4 ft-bgs
Der		1.0.1							Sample Ty	ne		Groundy	water
	arks and	1 Dat	um Use		t-bgs - feet				N = SPT	ha	Date		
ENSR 1011 S	W Klicki	tat Wa	v. Suite	207	ISA - Hollow	w Ste	em Auger		DP = Direct P	ush	03/18/08		
Seattle	, WA 98	34-11	62		ch Sched	lule			SS = Split Sp	1 1 1 1 1 1 L	<u></u>		
av: 12	06) 624-2	839	•		mb ambl	ent a	ir		C = Core	t			-

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EN	VSR	AE(	COM				Bor	ring/Well Lo	og	Well #: Sheet :		
Proie	ect: Lor	a Lak	e Apar	tment	s	1	Monume	nt: Flush Mount	Stick	Up: -		
	ect #: 0						Northing			d Elevation:		
	tion: B					-		Type: HSA Limited Acces	s MPE	evation: -		
	t: Port						Method:			Depth: 20.5		
Start	Date &	Time	: 3/18/0	08 13	44		Casing I	D: 2"	Filter	Pack: 10/20	Silica Sa	nd
Finis	h Date	& Tim	e: 3/18	/08 1	445		Boring ID	: 8.25"	Seal:	Bentonite d	chips	
Cont	ractor:	Case	ade Di	rilling	Inc.		Bit Type:	4.25" HSA	Grout			
Oper	rator: C	Curtis	Askew	/			Logged E	By: R. Knecht/ C. Smith	Scree	n: 0.010" slot	Sch. 40 P	VC 5-15 ft-bgs
		Samp		*	etion etion	hic	(ft.)	Soil and R	ock Descriptio	n	ttion .)	Comment
Type & Number	Depth Range	% Rec	Blows per 6"	(mqq) Cliq	Well Completion Log	Graphic	Depth	Classificatio	on Scheme: USCS/	ASTM	Elevation (ft.)	& Sample
SS- 6	11.5- 13.0	92	25 50/6"	1.4 (Am 0.6)				(11.5-13.0) SP: POORL fine, very dense, wet. 2 coarse gravel. 10% coa soapy-like to hydrocarb contamination.	5% sub angular, fir rse sand. Slight to	ne to moderate		11.5-13.0' Sampled for analytical
				1-1		11		(13.0-14.0) Not Sample	d.			2-inch diameter,
SS- 7	14.0- 15.5	100	50/6"	2.4 (Am) 0.7)			) 	(14.0-15.5) SW: WELL brownish gray, fine to c 15% rounded, fine to cc diameter. Trace to little like to hydrocarbon-like contamination.	oarse, very dense, parse gravel, up to 2 iron mottles. Slight	wet. 10-	15	0.010-inch slot, Sch. 40 PVC screen from 5 to 15 ft-bgs
				ł			1	(15.5-16.5) Not Sample	d.		_	
SS- 8	16.5- 18	61	19 50/6"	2.1 (Am) 0.8)	b,			(16.5-17.25) ML: SILT, plasticity, hard, wet. 15 no visible contaminatio	% clay. Slight soap-	ay, high like odor,	-	10/20 silica sand pack from 4 to 16 ft-bgs
				0.07				(17.25-18.0) SW: WELL to coarse, very dense, v gravel, up to 1/2" in diar no visible contamination	vet. 25% rounded, 1 meter. Slight soap-l	ine		Bentonite from 16 to
ss-	19.0-	55	21	2.2		100		(18.0-19.0) Not Sample	d.			20.5 ft-bgs
9	20.5	55	36 50/4"	(Aml 1.0)	b.		) 	(19.0-20.3) SW: WELL to medium, very dense, fine gravel, up to 1/4" in contamination.	GRADED SAND, gi wet. 10% silt and r	ounded,	20	19.0-20.5' Sampled for analytical
								(20.3-20.5) SP: POORL fine, wet. No odor or vis				
									1			
Rema	arks an	d Dat	um Us	ed:	ft-bgs - fee	t belo	w ground	l surface	Sample Type		Ground	
ENSR					HSA - Holl				N = SPT	Date		ne Depth (ft.)
1011 S	W Klick			e 207					DP = Direct Push SS = Split Spoon		155	3 12.51'
ovallie	le, WA 98134-1162 e: (206) 624-9349								100 - 0011 00000			

	NSR		ECO				Во	oring/Well Lo	g			4: MW-8 1 of 2	3
	oject: Lo				ts		Monum	nent: Flush Mount		Stick L	Jp: -		
	oject #: 1			3000			Northin	ig: 174627.2741Easting: 12727	775.8574'	Ground	d Elevation	: 282.534	\$'
	cation: S						Drill Rig	g Type: Limited Access Trac	k Rig	MP Ele	evation: 28	7.907'	
	ent: Por	_						: HSA		Total D	epth: 20.	5'	
	rt Date						Casing			Filter F	ack: 10/20	Silica Sa	and
	ntractor:	_		C. SHILL DEPT. E				ID: 4.25"		Seal:	Bentonite	Chips	
	erator: (				inc.		1.000	e: 4.25" HSA		Grout:			
		Sam			E	1		By: C. Smith		Screen	:0.010-inch		
Type	-	% Rec	Swo	(mqq)	Well	Granhic	Depth (ft.)	Soil and Ro Classification		1993		Elevation (ft.)	Comments
							0	(0.0-5,0) Not sampled.					Flush mour
							+				-	- 280	monument Bentonite chip plug from 1.5-8 ft-bgs.
SS -1	5-6.5	80	7 8 17	0.4 aa= 0.4		2.0	5	(5.0-6.5) SW: WELL GRAI coarse, sub angular to sul dense, dry. Trace rootlets. contamination.	b rounded, r	nedium	14		2-inch schedule 40 riser pipe from 0-10 ft- bgs.
					÷1		+	(6.5-10.0) Not sampled.	pist. 40% silf	Trace			10/20 silica sand pack from 8-20.5 ft-bgs.
SS 1	10-11.5	46	50/6"	0.3 aa= 0.3			10	angular, coarse sand. Trac visible contamination. (10.4-11.5) SP: POORLY C fine, very dense, moist. Tra rounded peice of large grav contamination. (11.5-12.5) Not sampled.	GRADED SA	ND, br	own,	5	0.010-inch slot, 2-inch schedule 40 PVC screen from 10-20 ft-bgs.
Remai	rks and	Datu	um Use	d: aa	i ≈ ambie	ntair			Sample Ty	pe		270 Groundwa	
he RET	TEC Gro	up. In	C.				H GROUND	N	= SPT	-	Date	Time	1
011 SV eattle.	V Klickita WA 981:	at Way	y, Suite	201 -	bgs = fee ch. = sche		w ground	D	P = Direct P		8/12/08	1415	12'
hone:	(206) 624	1-9349	9			1999 B	m Auger		S = Split Spo = Core	oon			



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EN	VSR	A	ECO	М			Bor	ring/Well Lo	g		Sheet	: MW-9 1 of 2	
Pro	ect: Lor	aLak	e Ana	tmen	ts		Monume	ent: Flush Mount		Stick U	D: -		
	ect #: 0							: 174474.2134 Easting: 1272	627.3356'	1	Elevation:	283.698	· · · · · ·
	ation: Se						•	Type: Limited Access Trac			vation: 283		
	nt: Port					_	Method:				epth: 20.5		
	t Date &			/08 12	17	1	Casing II	11935			ack: 10/20		nd
Fini	sh Date	& Tin	ne: 08/1	2/08	240	-	Boring IC			Seal:	Bentonite	Chips	
	tractor:							4.25" HSA		Grout:			
	erator: C							By: C. Smith		Screen	0.010-inch	slot, Sch.	40 PVC
	5	Samp	ole		etion	hic	(ft.)	Soil and Ro	ock Desc	ription		ation (.)	lents
Type	Depth Range	% Rec	Blows per 6"	(maa)	Well Completion Log	Graphic	Depth	Classification	n Scheme:	USCS/A	STM	Elevation (ft.)	Comments
							-	(0.0-5.0) Not sampled.				- 280	Flush mou monumer Bentonite chip plug from 1.5-8 ft-bgs.
SS -1	5-6.5	53	24 50/6"	0.4 aa= 0.4	-	11111111111111111111111111111111111111	5	(5.0-6.5) SM: SILTY SAN very dense, moist. At 5-5 rootlets, large 3", rounde grades to gray. No odor (6.5-10.0) Not sampled.	5.3', trace c d, cobble.	oarse san At 5.3-5.6	d and	-	2-inch schedule 4 riser pipe from 0-10 bgs.
SS -2	10-11.5		21 25	0.3 aa=			10	(10.0-10.7) SW: WELL G to coarse, sub rounded, crushed, dark gray bould visible contamination.	very dense	, moist. A	Angular, V	- 275	10/20 silica sand pack from 8-20.5 ft-bgs.
			30	0.3				(10.7-11.5) SP: POORLY fine, very dense, moist. F at 11.1' and 11.3'. No odd	ew 1/4" thi	ck gray le	enses	-	0.010-inch slot, 2-inch schedule 4 PVC scree from 10-20
Rem	arks and	d Dat	um Us	ed:	aa = ambie	nt air	11	(11.5-15.0) Not sampled.	Sample	Туре		Ground	ft-bgs. water
The R	ETEC Gr	oup, li	10.		ft-bgs = fee	et belo	ow groun	d suface	N = SPT DP = Dire	of Duch	Date	0.110	
1011 5	W Klicki e, WA 98	tat Wa	y, Suite	207	Sch. = sch				SS = Split		8/12/08	123	0 14'
	: (206) 62					Junio		er	C = Core	Shoon			-

EN	ISR	A	ECO	М		Во	ring/Well Log		Well #: Sheet 2		
Proi	ect: Lor	alak	e Ana	rtment	s	Monume	ent: Flush Mount	Stick Up			
	ect #: 0					10000000	: 174474.2134 Easting: 1272627.3356'		Elevation:	283.698	
	ation: Se						Type: Limited Access Track Rig		ation: 283	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
	nt: Port		-			Method:		Total De	oth: 20.5	5'	
1.000	t Date &			/08 12	17	Casing I			ck: 10/20	1000000	nd
	sh Date a					Boring IC			entonite		
	tractor:						4.25" HSA	Grout: -	a to the to be to be		
1000	erator: C						By: C. Smith		0.010-inch	slot, Sch.	40 PVC
		amp			E .	1 3					1
Type	Depth Range	% Rec		(mqq)	Well Completion Log	Depth (ft	Soil and Rock Desc Classification Scheme:		тм	Elevation (ft.)	Comments
SS -3	15-16.5	00	50/6"	0.3 aa= 0.3			(15.0-16.5) SP: POORLY GRADED with white, black and yellow grains, wet. Trace silt. 15% winnowing. No contamination. (16.5-19) Not sampled.	fine, very	dense,	-	15-16.5 ft- bgs: Sampled for analytical.
8S 4	19-20.5		50/6"	0.3 aa= 0.3			(19.0-20.5) SP: POORLY GRADED with white, black and yellow grains, wet. Trace silt. 15% winnowing. No contamination.	fine, very	dense,	- 265	Threaded end cap from 20- 20.35 ft-bgs.

EN	ISR	A	ECO	М			Bor	ing/Well Log	Ċ.		Well #: Sheet	: MW-1 1 of 2	0	
Proi	ect: Lor	alal	e Ana	rtmen	ts	1	Monumer	nt: Flush Mount		Stick Up	i: -			
	ect #: 0			Simola -		-		174386.6154Easting: 1272561			Elevation:	284.397		_
	ation: Se					-		Type: Limited Access Track R			ation: 284			
1	nt: Port					-	Method:			Total De	opth: 20.5	5'		
Star	t Date &	Time	e: 08/12	/08 10	)41	(	Casing ID	): <b>2"</b>		Filter Pa	ck: 10/20	Silica Sa	nd	
Finis	sh Date	& Tin	ne: 08/1	2/08 1	115	E	Boring ID	): 4.25"		Seal: B	entonite	Chips		
Con	tractor:	Case	cade D	rilling	Inc.	E	Bit Type:	4.25" HSA		Grout: -				
Ope	erator: C	urtis	Askev	V	1 1	L	ogged E	By: C. Smith		Screen:	0.010-inch		40 PV	
- 1		amp		1	Well	hic	(ft.)	Soil and Rock	k Descr	iption		Elevation (ft.)		nents
Type	Depth Range	% Rec	Blows per 6"	(uudd) QId	Well Completion Log	Graphic	Depth	Classification So	cheme: L	JSCS/AS	бтм	Eleva (fi		Comments
							0	(0.0-5.0) Not sampled.				ł		sh moun
							-					- 280	chip	ntonite o plug n 1.5-8 gs.
SS -1	5-6.5	46	24 50/6"	0.3 aa= 0.3			5	(5.0-6.5) SP: POORLY GRA brown, medium, very dense coarse sand to small gravel. silt. No odor or visible contai (6.5-10.0) Not sampled.	, dry. Tra . Some fir	ce, angune sand	lar,	+	rise	edule 40 er pipe n 0-10 ft-
							-					- 275	san	20 silica d pack n 8-20.5 gs.
SS -2	10-11.5	44	21 25 30	0.3 aa= 0.3		0	++ 10  -	(10.0-11.5) SW: WELL GRA to medium, very dense, moi sand. No odor or visible cor	ist. Some	silt and		-	slot, sch	10-inch , 2-inch edule 40
							1	(11.5-12.5) Not sampled.					1.5.5	C screer m 10-20 gs.
1	10.00		7-7			-			Samela "	Turo		Ground	wate	r
Rem	arks an	d Dat	um Us	ed:	aa = ambient	air		the second se	Sample 1 = SPT	type	Date			Depth (ft.)
The R	ETEC Gr	oup, I	nc.		ft-bgs = feet l	belo	w groun	d autom	= SPT P = Direct	Push	8/12/08		-	13'
Seattl	SW Klicki e, WA 98	134-1	162	e 207	Sch. = sched	ule		1 1/12	S = Split S				-	
Phone	e: (206) 6:	24-93- 2839	49		HSA = Hollow	-			= Core				-	

	SR	AE	ECO	M			Bor	ing/Well Log		Well # Sheet	: MW-1 2 of 2	D
Proje	ct: Lora	Lak	e Apar	tmen	ts		Monumer	nt: Flush Mount	Stick Up:		- D 30	
	ct #: 05			_			Northing:	174386.6154 Easting: 1272561.6472'	Ground E	levation	: 284.397	
-	tion: Se			5.8 ····		1		Type: Limited Access Track Rig	MP Eleva	tion: 28	4.149'	
	t: Port						Method:	Contraction of the second s	Total Dep	oth: 20.5	5'	
	Date &	-		/08 10	41		Casing ID				Silica Sa	nd
	h Date 8					-	Boring ID	And All	Seal: Be	ntonite	Chips	
110*	actor:			<u> </u>			Ser out the set	4.25" HSA	Grout: •		e nipe	
	ator: Ci				ino,			By: C. Smith		010-inch	slot, Sch.	40 PVC
opere				-	5		1		1001001110			
ß	Depth Range	amp % Kec	Blows G	(mqq)	Well Completion Log	Graphic	Depth (ft.)	Soil and Rock Desc Classification Scheme:		тм	Elevation (ft.)	Comments
3				aa= 0.3			- 15	(12.5-14.0) SP: POORLY GRADED medium, sub rounded, very dense, 13'. Trace, fine sand and silt. No od contamination. (14.0-18.5) Not sampled.	moist to we	et at	- 270	12.5-14 ft- bgs: Sampled fo analytical.
S 1 4	19-20.5	80	50/6"	0.3 aa= 0.3				(19.0-20.5) SP: POORLY GRADED with black, white, yellow, and orang medium, sub rounded, very dense, No odor or visible contamination.	e grains,		- 265	Threaded end cap from 20- 20.35 ft-bgs

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EN	ISR	A	ECO	М				Bor	ing/Well Lo	og		Well #: Sheet 1	1993	1
Proj	ect: Lor	a La	ke Apa	rtmen	ts			Monume	nt: Flush Mount	1	Stick U	p: •		
1	ect #: 0						- 1	Northing:	174287.7124Easting: 127	2485.4391'	Con March	Elevation:	284.948	
Loca	ation: S	eattle	, WA					Drill Rig	Type: Limited Access Tra	ack Rig	MP Ele	vation: 284	.36'	
1.000	nt: Port	1.1.1			_			Method:			Total D	epth: 20.5		
Star	t Date 8	Time	e: 08/12	2/08 0	930			Casing ID	): 2"		Filter P	ack: 10/20 :	Silica Sa	nd
Finis	sh Date	& Tin	ne: 08/*	2/08	1005			Boring ID	: 4.25"		Seal: I	Bentonite (	Chips	
Con	tractor:	Cas	cade D	rilling	Inc.			Bit Type:	4.25" HSA		Grout:			
Ope	rator: C	urtis	Askev	v		-		Logged E	y: C. Smith		Screen	0.010-inch	slot, Sch.	10 PVC
		Sam	ale	1	5		~	(ft.)					E	ts
Type &	Depth Range	% Rec	Blows per 6"	(maa)	Well	Log	Graphic	Depth (f	Soil and R Classification	COCK Desci		100	Elevation (ft.)	Comments
					]			<b>∏</b> ⁰	(0.0-5.0) Not sampled.	1		-		Flush mou
								-				-		monument Bentonite chip plug from 1.5-8 ft-bgs.
SS -1	5-6.5	100	50/2"	0.0				5	(5.0-6.5) SW: WELL GF fine to medium, very de sand. Few, angular gra grass and rootlets. No o	ense, moist. To vel up to 1" in	race coa diamete	er. Few	- 280	2-inch schedule 4 riser pipe from 0-10 f bgs.
			1					]	(6.5-7.5) Not sampled.			-	-	595.
SS -2	7.5-9	100	50/6"	0.0					(7.5-9.0) SW: WELL GF fine to medium, very de coarse sand and small odor or visible contamir	nse, moist. Tr gravel. Some	ace, and	gular,		10/20 silica sand pack from 8-20.5
								1	(9.0-10.0) Not sampled.					ft-bgs.
SS -3	10-11.5	100	50/6"	0.0			<u>~ ^</u>	10	(10.0-10.3) SW: WELL to medium, very dense No odor or visible conta	GRADED SA			275	0.010-inch
									(10.3-11.5) SP: POORL medium, very dense, m contamination. (11.5-12.5) Not sampled	oist. No odor				slot, 2-inch schedule 40 PVC screet from 10-20 ft-bgs.
Rema	arks and	Dat	um He	ed.	6 har-	- fa-1	hala		l aufass	Sample 1	Гуре		Groundw	vater
		_	1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 - 1944 -					w ground	l suface	N = SPT	1	Date	Time	Depth (ft.
011 S	TEC Gro W Klicki	tat Wa	y, Suite	207	Sch. =	sche	dule			DP = Direct		8/12/08	0950	12.8'
·	, WA 981	134-11	62		UCA -	Halla	u Cto	m Auger		SS = Split S	haan	1		

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Project: Lora Lake Apartments       Monument: Flush Mount       Stick Up: -         Project #: 05482-025-3000       Northing: 174287.7124Easting: 1272485.4391'       Ground Eleval         Location: Seattle, WA       Drill Rig Type: Limited Access Track Rig       MP Elevation:         Client: Port of Seattle       Method: HSA       Total Depth:         Start Date & Time: 08/12/08 0930       Casing ID: 2"       Filter Pack: 10         Finish Date & Time: 08/12/08 1005       Boring ID: 4.25"       Seal: Bentor         Contractor: Cascade Drilling Inc.       Bit Type: 4.25" HSA       Grout: -         Operator: Curtis Askew       Logged By: C. Smith       Screen:0.010-i         Sample       Image: Signal of Si	284.36' 20.5' /20 Silica Sa ite Chips	ind
Project #: 05482-025-3000       Northing: 174287.7124Easting: 1272485.4391'       Ground Eleval         Location: Seattle, WA       Drill Rig Type: Limited Access Track Rig       MP Elevation:         Client: Port of Seattle       Method: HSA       Total Depth:         Start Date & Time: 08/12/08 0930       Casing ID: 2"       Filter Pack: 10         Finish Date & Time: 08/12/08 1005       Boring ID: 4.25"       Seal: Bentor         Contractor: Cascade Drilling Inc.       Bit Type: 4.25" HSA       Grout: -         Operator: Curtis Askew       Logged By: C. Smith       Screen:0.010-i         Sample       Image: Seal:	284.36' 20.5' /20 Silica Sa ite Chips nch slot, Sch.	ind 40 PVC
Location: Seattle, WA       Drill Rig Type: Limited Access Track Rig       MP Elevation:         Client: Port of Seattle       Method: HSA       Total Depth:         Start Date & Time: 08/12/08 0930       Casing ID: 2"       Filter Pack: 10         Finish Date & Time: 08/12/08 1005       Boring ID: 4.25"       Seal: Bentor         Contractor: Cascade Drilling Inc.       Bit Type: 4.25" HSA       Grout: -         Operator: Curtis Askew       Logged By: C. Smith       Screen: 0.010-1         Sample       Image: Seal: Sea	284.36' 20.5' /20 Silica Sa ite Chips nch slot, Sch.	ind 40 PVC
Client: Port of Seattle       Method: HSA       Total Depth:         Start Date & Time: 08/12/08 0930       Casing ID: 2"       Filter Pack: 10         Finish Date & Time: 08/12/08 1005       Boring ID: 4.25"       Seal: Bentor         Contractor: Cascade Drilling Inc.       Bit Type: 4.25" HSA       Grout: -         Operator: Curtis Askew       Logged By: C. Smith       Screen: 0.010-1         Sample       Image: Soil and Rock Description       Classification Scheme: USCS/ASTM	20.5' /20 Silica Sa ite Chips nch slot, Sch.	40 PVC
Start Date & Time: 08/12/08 0930       Casing ID: 2"       Filter Pack: 10         Finish Date & Time: 08/12/08 1005       Boring ID: 4.25"       Seal: Bentor         Contractor: Cascade Drilling Inc.       Bit Type: 4.25" HSA       Grout: -         Operator: Curtis Askew       Logged By: C. Smith       Screen: 0.010-it         Sample       Image: Soil and Rock Description       Image: Soil and Rock Description         Soil and Rock Description       Classification Scheme: USCS/ASTM	/20 Silica Sa ite Chips 1ch slot, Sch.	40 PVC
Finish Date & Time: 08/12/08 1005       Boring ID: 4.25"       Seal: Bentor         Contractor:       Cascade Drilling Inc.       Bit Type: 4.25" HSA       Grout: -         Operator:       Curtis Askew       Logged By: C. Smith       Screen: 0.010-i         Sample       Operator:       Operator:       Operator:       Operator:       Operator:       Operator:       Curtis Askew       Logged By: C. Smith       Screen: 0.010-i         Sample       Operator:       Operator:       Operator:       Operator:       Operator:       Operator:       Operator:       Operator:       Operator:       Classification Scheme:       USCS/ASTM	ite Chips nch slot, Sch.	40 PVC
Contractor:       Cascade Drilling Inc.       Bit Type:       4.25" HSA       Grout: -         Operator:       Curtis Askew       Logged By:       C. Smith       Screen: 0.010-1         Sample       Image: Signal and Sign	nch slot, Sch.	and the second se
Operator:     Curtis Askew     Logged By:     C. Smith     Screen: 0.010-it       Sample     io     <		and the second se
Sample     And Control     Source     Sour		and the second se
	evatio (ft.)	nen
	Π	Comr
SS       12.5-14       100       60/5"       0.0         -4       (12.5-14.0) SP: POORLY GRADED SAND, brown, medium, moist to wet at 12.8'. Trace coarse sand. No odor or visible contamination.		
SS       15-16.5       100       50/5"       0.0       15       (14.0-15.0) Not sampled.         .5       15-16.5       100       50/5"       0.0       15       (15.0-16.5) SP: POORLY GRADED SAND, brown, medium, very dense, wet. Trace silt at 16'. No odor or visible contamination.         .5       (16.5-19.0) Not sampled.	270	12.5-13 ft- bgs: Sampled fr analytical.
SS 19-20.5 100 50/6" 0.0 (19.0-20.5) SP: POORLY GRADED SAND, brown with white, black, orange and yellow grains, medium, sub angular, very dense, wet. Trace silt. 10% winnowing. No odor or visible contamination.		Threaded end cap from 20- 20.35 ft-bg

