

Remedial Investigation and Feasibility Study Ken's Auto Wash 1013 East University Way Ellensburg, Washington

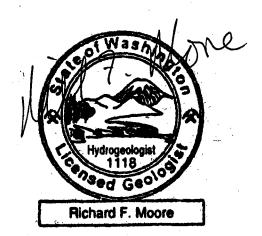
Prepared for Ken's Auto Wash

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CC	DNTENTS	Page
1.0	INTRODUCTION	•
2.0	FACILITY BACKGROUND	
	Site Description and Use	•
	Surrounding Properties Potential Sources of Environmental Contaminants	
	Regulatory History	2
3.0	ENVIRONMENTAL INVESTIGATIONS AND REMEDIAL ACTIONS	3
3.1	1995 to 2000—Site Characterization and Groundwater Monitoring (Sage 1998a and 1998b; Hart Crowser 1999a, 2000a, 2000b, 2000c, and 2000e)	•
3.2	2000—Hot Spot Soil Excavation and In Situ Bioremediation (Hart Crowser 2001)	2
3.3	Strataprobe Explorations and In Situ Bioremediation (Hart Crowser 2005a)	
3.4	UST Closure (Hart Crowser 2005b)	ļ
3.5	5 2001 to Present—Groundwater Monitoring (Hart Crowser 2001, 2002a, 2002b, 2003, 2004a, 2004b, 2004c, 2005b, 2005c, 2006, and Data Provided in Draft RI/FS Report)	1
4.0	SITE PHYSICAL CHARACTERISTICS	6
4.1	Geology	ϵ
4.2	? Hydrogeology	6
4.3	Underground Utilities	;
5.0	NATURE AND EXTENT OF CONTAMINATION	
	Identifying Potential Chemicals of Concern	8
	Soil Quality	}
	Groundwater Quality Free Product	11
	Site Conceptual Model	12
	CLEANUP OBJECTIVES AND CRITERIA	14
	Chemicals of Concern	14
	Potential Exposure Pathways	14
6.3	Remedial Action Objectives (RAOs)	15

CONTENTS (Continued)	<u>Page</u>
6.4 ARARs and Applicable Regulations 6.5 Cleanup Levels 6.6 Terrestrial Ecological Evaluation	15 16 16
7.0 REMEDIAL ALTERNATIVE DEVELOPMENT AND EVALUATI	ON 17
7.1 Technology Screening 7.2 Remedial Alternative Descriptions 7.3 Evaluation of Alternatives 7.4 Preferred Remedial Alternative Identification	17 18 22 23
8.0 REFERENCES	23
TABLES	
Summary of Site Investigation, Remediation, and Monitoring Activities Groundwater Elevation Data Summary of Soil Chemistry Data Summary of Groundwater Chemistry Data – TPH-G, BTEX, and Lead Summary of Groundwater Chemistry Data – Other Detected Constite Summary of Groundwater Chemistry Data – Groundwater Grab Sam Measured Free Product Thickness in Well MW-1/MW-14 Summary of Cleanup Levels for Chemicals of Concern Remedial Alternative Evaluation – Compliance with WAC 173-340-36 Monitoring Schedule for Preferred Alternative	uents nples
FIGURES	
 Vicinity Map Site and Well Location Plan Generalized Geologic Cross Section A-A' Groundwater Elevation Contour Map – June 2006 TPH-G Occurrences in Soil UST Verification Samples Locations and Detected Analytical Results "Pothole" Test Pit Sample Locations and Detected Analytical Results TPH-G Occurrences in Groundwater Benzene Occurrences in Groundwater Long-Term Trends in TPH-G Concentration in Groundwater 	
 Long-Term Trends in Benzene Concentration in Groundwater Dissolved Oxygen in Groundwater 	

APPENDIX A
REQUEST FOR SITE HAZARD ASSESSMENT RERANKING

APPENDIX B
BORING LOGS AND WELL CONSTRUCTION DETAILS
BY HART CROWSER AND OTHERS

APPENDIX C
COST ESTIMATES FOR ALTERNATIVES

REMEDIAL INVESTIGATION AND FEASIBILITY STUDY KEN'S AUTO WASH 1013 EAST UNIVERSITY WAY ELLENSBURG, WASHINGTON

1.0 INTRODUCTION

This report presents the Remedial Investigation (RI) and Feasibility Study (FS) for the Ken's Auto Wash site in Ellensburg, Washington (Figure 1). Site soils and groundwater are affected by a previous release of petroleum hydrocarbons associated with a gasoline underground storage tank (UST) at the site. The petroleum hydrocarbon release was discovered during tank tightness testing in 1995.

The RI/FS incorporates information from several previous site investigations, initial soil removal/treatment efforts in 2000, UST removal in 2005, and on-going groundwater monitoring. These efforts were conducted by Hart Crowser and others on behalf of the site owner, Ken Peterson. Mr. Peterson has entered into an Agreed Order with the Washington State Department of Ecology (Ecology) pursuant to the Model Toxics Control Act (MTCA – RCW 70.105D.040(5)).

This RI/FS compiles and summarizes results of previous site investigations, soil cleanup, and monitoring efforts from 1995 to present. The RI/FS presents a chronology of the work completed and a conceptual model of current site conditions. Accordingly, this RI/FS has been formatted for consistency with elements listed in WAC 173-340-350 Remedial Investigation and Feasibility Study, WAC 173-340-360 Selection of Cleanup Actions, and other pertinent sections of MTCA. Note that laboratory analytical data for testing results summarized in this RI/FS report is provided with the source data reports referenced herein.

2.0 FACILITY BACKGROUND

2.1 Site Description and Use

The Ken's Auto Wash property is located at 1013 East University Way in Ellensburg, Washington (Figure 1), at the northwest corner of East University Way and Alder Street. The property covers a total area of approximately 15,000 square feet (0.35 acre). The site is a former gasoline service station. The current business (Ken's Auto Wash) is an active car wash. Site structures include a three-stall car wash and convenience store building, which is currently inactive.

The site is paved with concrete beneath the car wash and on the southern half of the site and with asphalt to the north and east of the car wash/convenience store.

2.2 Surrounding Properties

The Ken's Auto Wash property is bound to the west by a vacant lot (unpaved gravel), to the north by an alley, to the south by East University Way, and to the east by Alder Street. The property is zoned commercial/highway. In the surrounding area, there are a mixture of commercial, office, and residential properties.

The site is located within city limits, and surrounding businesses and residences receive their drinking water from the municipal water supply. Well inventories by Ecology (1996) and Hart Crowser (1999a) identified three municipal water supply wells located in the surrounding area: two wells located 1,200 and 1,600 feet south of the site, and one well located 4,000 feet northwest of the site. Locations and logs of these supply wells are included in Attachment A in Appendix A. As discussed in Section 4.2 and Appendix A, there is no identified connection between affected groundwater at the site and the hydrogeologic units tapped by the water supply wells.

2.3 Potential Sources of Environmental Contaminants

Gasoline USTs and associated piping and delivery systems have been located on the southern half of the site. Former UST and pump island locations associated with Ken's Auto Wash operations are shown on Figure 2. Four unleaded and regular (leaded) gasoline USTs were replaced in 1988 with three new USTs: one 6,000-gallon super unleaded UST (UST No. 1), one 10,000-gallon unleaded gasoline UST (UST No. 2), and one 10,000-gallon regular (unleaded) gasoline UST (UST No. 3). The new USTs were replaced in the same UST "nest" that held the previous USTs. In 1996, a leak was discovered during a tank tightness test in a product line from the super unleaded gasoline UST No. 1. After discovery and correction of the leak, gasoline odors were noted in two observation wells located adjacent to the USTs.

2.4 Regulatory History

Following the reported UST leak, a Site Hazard Assessment was conducted in 1996 (Ecology 1996). The site was ranked a "2." In 1999, the site was entered into Ecology's Voluntary Cleanup Program (VCP) (Hart Crowser 1999b). Groundwater monitoring, hot spot excavation of petroleum hydrocarbon-

affected soils in 2000, UST removal, and related site investigations since 1999 were conducted under the VCP.

In January 2003, reranking of the site was requested because the original site ranking was based on an assumed connection between petroleum-affected groundwater at the site and the City of Ellensburg supply wells (see Appendix A). Ecology denied the request on the basis that reevaluating the site would not result in a significant change in rank (i.e., a new Site Hazard Assessment would likely change the rank from a "2" to a "3"). In August 2003, Mr. Peterson and Ecology entered into an Agreed Order to prepare an RI/FS for the site.

3.0 ENVIRONMENTAL INVESTIGATIONS AND REMEDIAL ACTIONS

Following the 1996 leak, Hart Crowser and others have conducted several environmental field investigations, remedial actions, and groundwater monitoring at the site. These activities are summarized below in chronological order. Exploration locations are shown on Figure 5 and Figure 6. A summary of investigation results is presented in Section 4. A list of available reports regarding environmental conditions at the site is provided in Section 8.0. Table 1 presents a summary of the site investigation, remediation, and monitoring activities, as discussed below.

3.1 1995 to 2000—Site Characterization and Groundwater Monitoring (Sage 1998a and 1998b; Hart Crowser 1999a, 2000a, 2000b, 2000c, and 2000e)

Between 1996 and 1998, Sage Earth Sciences (Sage) installed six monitoring wells (MW-1 through MW-6) at the site, measured groundwater elevations, and sampled and analyzed soil and groundwater at each location for gasoline-range petroleum hydrocarbons (TPH-G) and benzene, toluene, ethylbenzene, and xylenes (BTEX). Selected soil samples were analyzed for diesel- and oil-range petroleum hydrocarbons (TPH-D and TPH-O), and selected groundwater samples were analyzed for total and dissolved lead. In 1998, Sage continued monthly elevation measurements and quarterly monitoring of TPH-G, BTEX, and total lead in groundwater. In 1999, Hart Crowser completed a receptor analysis, a utility survey, analyzed soil and groundwater samples from five borings (HP-7 through HP-11) for TPH-G and BTEX, analyzed groundwater samples collected from the borings via hydropunch methods, and installed monitoring wells MW-12 and MW-13. Between December 1999 and September 2000, Hart Crowser analyzed groundwater samples from the eight wells for TPH-G and BTEX on a quarterly basis. In September 2000, Hart Crowser advanced five soil borings (HP-12 through HP-16) and collected and analyzed soil and

groundwater samples for TPH-G and BTEX. Selected soil samples were analyzed for lead.

3.2 2000—Hot Spot Soil Excavation and In Situ Bioremediation (Hart Crowser 2001)

In October and November 2000, Hart Crowser conducted a hot spot removal of accessible petroleum-impacted materials at the location shown on Figure 5. Approximately 520 tons of TPH-affected soil and 5,500 gallons of TPH-affected groundwater, containing an estimated 2,700 pounds of TPH, were removed from the area between the existing USTs and East University Way. Excavation extent was spatially limited by the presence of the USTs to the north and the East University Way sidewalk to the south. Verification samples were collected from the side walls of the excavation and chemically analyzed. Verification sample results indicated soil containing TPH concentrations above applicable MTCA cleanup levels remained in place to the north (next to the UST area) and the south (beneath the sidewalk). Approximately 600 pounds of Oxygen-Releasing Compound (ORC) were added to the excavation backfill (below the seasonal high water table elevation) to promote biodegradation of petroleum hydrocarbons remaining in place.

3.3 Strataprobe Explorations and In Situ Bioremediation (Hart Crowser 2005a)

In February 2005, strataprobe (push-probe) explorations were advanced in eight locations at the Ken's Auto Wash site to collect soil and grab groundwater samples. These locations are identified as HCSP-04-01 through HCSP-04-08 on Figure 5. Soil and groundwater samples were submitted for laboratory analysis of TPH-G and BTEX. Soil samples were also analyzed for total lead, while groundwater samples were analyzed for methyl tertiary-butyl ether (MTBE).

While on site, approximately 120 pounds of ORC were injected at the six strataprobe locations as a relatively low cost, opportunistic measure to benefit the additional remediation alternatives considered. Five injection locations were in the westbound lane of East University Way near the sidewalk, which is near the apparent leading edge of the TPH source area in soil (Figure 5). Locations included strataprobes HCSP-04-01, HCSP-04-02, HCSP-04-03, and two (unnamed) locations for areal coverage between HCSP-04-01, HCSP-04-02, and HCSP-04-03. The sixth injection location was completed near the centerline of the westbound lanes where the highest MultiRae photoionization detector (PID) reading in soil was recorded during advancement of strataprobe HCSP-04-07. The depth of the injections was below and across the inferred water table, taking into account seasonal fluctuations in water table elevation.

3.4 UST Closure (Hart Crowser 2005b)

In April 2005 Clearcreek Contractors, with observation by Hart Crowser, completed closure activities to remove the three site USTs and the associated piping. Once the USTs were removed, Hart Crowser collected and analyzed verification soil samples of the excavation side walls and bottom. Laboratory analytical results were below applicable Washington State MTCA Method A cleanup levels, with the exception of the south UST side wall sample, which exceeded the applicable criteria for TPH-G and BTEX. The south side wall sample is located near the edge of the TPH "hot spot" excavation completed in 2000. During UST closure, approximately 8 cubic yards (cy) of TPH-affected soil were removed from the south side wall for off-site disposal. Excavation of additional TPH-affected soil was not possible because of stability concerns related to utility lines in the adjacent East University Way right of way: Soil sampling data and observations from the various site exploration, UST removal, and remediation activities indicate that soils remaining in this area with elevated TPH concentrations are limited to a localized area identified on Figure 6 and Figure 7.

Well MW-4 was removed during UST closure activities and was replaced with MW-4R in October 2005. In conjunction with removing the USTs, six "pothole" test pits were completed to delineate the extent of TPH-affected soils to the east and west of the UST excavation. Soil sample analytical results contained gasoline-range hydrocarbons and benzene at concentrations exceeding Method A cleanup levels. TPH concentrations are expected to be amenable to natural attenuation and these soils were left in place following discussions with Ecology.

3.5 2001 to Present—Groundwater Monitoring (Hart Crowser 2001, 2002a, 2002b, 2003, 2004a, 2004b, 2004c, 2005b, 2005c, 2006, and Data Provided in Draft RI/FS Report)

In January 2001, following the hot spot removal effort, Hart Crowser installed two additional monitoring wells at the site: MW-14 to replace MW-1, and MW-15 to define the southeastern plume boundary. The nine wells on site were monitored for TPH-G and BTEX on a quarterly basis in 2001. These wells, with the exception of MW-13, were monitored for TPH-G and BTEX in November 2002. They were monitored again in May 2003 for TPH-G, BTEX, total lead, and gasoline additives including MTBE, 1,2-dibromoethane (EDB), and 1,2-dichloroethane (EDC). Wells MW-2 through MW-6 and MW-12 through MW-15 were monitored on a quarterly basis from May 2003 to December 2004. Beginning in September 2003, the conventional parameters—nitrate, nitrite, and sulfate—were analyzed. One year later, in September 2004, the analysis of MTBE, EDB, and EDC was discontinued. In accordance with the schedule

shown in Table 9, monitoring events were completed on a biannual basis in 2005. The most recent round of groundwater samples was collected in June 2006.

4.0 SITE PHYSICAL CHARACTERISTICS

The site is generally flat, with a slight downward slope (less than 5 percent) from the convenience store to the south toward East University Way. The entire Ken's Auto Wash facility is covered with concrete and asphalt pavement, but the neighboring property to the west is unpaved. A description of subsurface geology and hydrogeology is provided below.

4.1 Geology

A cross section showing generalized subsurface conditions at the site is provided on Figure 3. Soils typically encountered at the site are related to alluvial deposition, and consist primarily of silty, sandy gravel with occasional cobbles. In several borings and during the 2000 hot spot excavation, a surface layer of sandy silt, typically 4 to 6 feet thick, was encountered. The former UST area is backfilled with pea gravel to an approximate depth of 13 feet. A compendium of site boring logs by Hart Crowser and others is provided in Appendix C.

A description of regional geology, including regional cross sections, is included in Appendix A.

4.2 Hydrogeology

Groundwater is present beneath the site within the shallow silty, sandy gravel unit at depths between 6 and 10 feet below ground surface. Depth to water measurements and elevations obtained between 1996 and 2006 are summarized in Table 2.

A description of regional hydrogeology is included in Appendix A. Groundwater at the site appears to be a shallow perched zone above a clay aquitard that is typically encountered in the surrounding area at depths of less than 30 feet. Drilling logs of the municipal supply wells indicate that several aquitards separate shallow site groundwater from deeper water-bearing units, including zones used for water supply beyond the vicinity of the site.

Groundwater elevation contours for the latest monitoring event in June 2006 are illustrated on Figure 4. The groundwater flow direction is toward the southwest, consistent with historical data. Calculated gradients are typically between 0.015

and 0.025 and do not change significantly with season. Groundwater elevations at the site typically fluctuate 1 to 2 feet seasonally, reaching their peak in late spring and low point in late fall. Utilities on and downgradient of the site are located at depths above the seasonal high water table elevation, and thus do not appear to affect groundwater flow. A more detailed discussion concerning utilities is presented below.

Physical groundwater characteristics collected during groundwater monitoring events include temperature, pH, and conductivity. These characteristics are relatively consistent in groundwater on and downgradient of the site, with normal ranges of each parameter as follows:

- Temperature. Measured groundwater temperatures are typically between 10 and 18 degrees Celsius, with the slightly lower temperatures most often measured during the winter months.
- **pH.** The pH of shallow groundwater at the site is typically between 5.9 and 7.1.
- Conductivity. The electrical conductivity (which is an indicator of total dissolved solids) is typically between 180 and 360 uS in site groundwater. The higher measured conductivity values are generally measured in wells MW-5, MW-6, and MW-14, which are also affected by petroleum contamination (see Section 5.3).

4.3 Underground Utilities

A potential concern for contaminant migration at a typical site is the tendency for trench backfill to act as a preferential pathway for groundwater. However, utilities at the Ken's Auto site are not identified as a significant concern for the following reasons:

- Existing utilities within the Ken's Auto Wash property are located either upgradient of the source area.
- Off-property utilities include a City of Ellensburg storm sewer in the East University Way right of way with invert depths of approximately 4 feet below grade. Based on historical groundwater monitoring, the invert depths are above the depth to groundwater typically encountered on the site.
- The City's sanitary sewer is located along Alder Street to the east of the site, upgradient of the source area.

Historical groundwater monitoring data indicate that depth to groundwater is typically greater than 6 feet below ground surface along the East University Way right of way; therefore, migration of contaminants down the utility corridor is unlikely. Furthermore, any future utility maintenance is not expected to encounter contaminated soil and groundwater. Figure 5 illustrates the location of the City's storm sewer along East University Way.

5.0 NATURE AND EXTENT OF CONTAMINATION

This section summarizes the environmental conditions in soil and groundwater that currently exist at the site. Our understanding of site conditions is based on environmental data from the investigations and remedial actions summarized in Section 3.0, and described in greater detail in reports referenced herein.

5.1 Identifying Potential Chemicals of Concern

Based on historical site use as a gasoline fueling station, the chemicals of potential concern at the Ken's Auto Wash site are TPH-G, BTEX, lead, and related gasoline additives including MTBE, EDB, and EDC. Based on the timing of the leak and the history of gasoline additive use in Washington, it was determined to be unlikely that gasoline additives would be present. However, in accordance with recent MTCA guidance, these constituents were added to the groundwater monitoring program in 2003, and were analyzed for through 2004.

Occurrences of chemicals of potential concern are described below.

5.2 Soil Quality

Since 1996, twenty-three soil borings have been advanced, six "pothole" test pits were completed, and sixteen excavation verification soil samples have been collected and analyzed during site investigation and remediation activities. Soil samples were screened for the presence of volatile organics (including TPH-G and BTEX) with a photoionization detector (PID). Selected soil samples were analyzed for TPH-G, BTEX, and lead. Analytical results are provided in Table 3. Chemical concentrations detected in soil were screened against MTCA Method A cleanup levels for unrestricted use. Detected soil concentrations of the following constituents exceed screening levels:

■ TPH-G. Site maps showing locations of soil samples where chemical concentrations exceed Method A cleanup levels are provided on Figures 5, 6, and 7. Soil containing elevated concentrations of TPH-G is present primarily on the south side of the UST area and underneath the East

University Way sidewalk. The occurrence of gasoline-range hydrocarbons is generally limited to soils at depths between 6 and 10 feet, in the groundwater "smear zone." The highest concentration of TPH-G detected was 11,000 mg/kg, in the northeast wall verification sample from the October 2000 hot spot excavation. A second sample was collected in that area during the April 2005 UST removal. Although the second sample was collected within 5 feet of the first, the concentration detected was only 2,400 mg/kg, indicating that TPH-G concentrations have been decreasing, or that higher concentration areas are very limited at the site.

- BTEX. Soil containing elevated concentrations of BTEX is collocated with those with elevated concentrations of gasoline-range petroleum hydrocarbons. The highest concentrations of BTEX detected in soil were also present in the October 2000 hot spot excavation northeast wall verification sample, with 10.6 mg/kg benzene, 73.9 mg/kg ethylbenzene, and 588 mg/kg xylenes. These concentrations were significantly lower in a second sample collected from that area during the UST removal in 2005, with 0.935 mg/kg benzene, 24.8 mg/kg ethylbenzene, and 43.0 mg/kg toluene. The highest detected concentration of toluene in soil at the site was 10 mg/kg at a depth of 7 to 9 feet at well MW-5. No subsequent samples have been collected near this location.
- Additional Areas of TPH and BTEX in Soil. Additional areas of relatively low concentration TPH and BTEX were encountered during attempts to excavate soils between the UST area and 2000 hot spot excavation to the south. Additional pothole test pit explorations were then completed at the locations shown on Figure 7 to evaluate the nature and extent of TPH-affected soils elsewhere. Gasoline-range TPH was detected up to concentrations of 705 mg/kg, and benzene was detected at concentrations up to 1.42 mg/kg at these locations. The TPH-affected soils are present between about 7 to 12 feet below ground surface, coincident with the approximate depth range of the shallow water table. TPH and benzene concentrations decreased rapidly with depth based on sample analytical results and/or field screening PID readings. As noted above, these soils were left in place following discussions with the Ecology and are expected to be amendable to natural attenuation.

Lead has not been detected in site soils above the MTCA Method A cleanup level of 250 mg/kg.

5.3 Groundwater Quality

Nine groundwater monitoring wells (Figure 2) have been installed on or downgradient of the site to characterize groundwater quality. Ten groundwater

grab samples were collected using hydropunch sampling methods before the October 2000 hot spot excavation to define the potential extent of the groundwater plume. Seven additional samples were collected during the February 2005 strataprobe investigation. Both sets of samples were collected as assumed "worse case" indicators of groundwater quality conditions, and contained high concentrations of solids. For this reason sample analytical results are not representative of actual groundwater quality than samples collected from the site monitoring wells. Chemical concentrations in groundwater samples collected from monitoring wells are provided in Table 4 for TPH-G, BTEX, and lead, and in Table 5 for other detected compounds. Groundwater grab sample results are provided in Table 6. Below, we describe chemical occurrences in groundwater, identify contaminants of concern, and discuss long-term trends in groundwater quality at the site.

Occurrence of Contaminants of Potential Concern

TPH-G and benzene in groundwater are identified in an area south of the UST area to near MW-6 (Figures 8 and 9). This area is bound to the south by MW-13; to the southwest by MW-12; to the southeast by MW-15; to the east by MW-4R; and to the west by MW-5.

Chemical concentrations detected in groundwater were screened against MTCA Method A cleanup levels for unrestricted use. Concentrations of the following constituents exceed screening levels:

■ TPH-G. A site map showing the extent of TPH-G occurrences in groundwater during the most recent monitoring event in June 2006 is provided on Figure 8. Groundwater containing elevated concentrations of TPH-G is present on the southwest corner of the Ken's Auto Wash property and extends underneath East University Way to well MW-6. During the most recent groundwater monitoring event in June 2006, after the completion of the remedial activities described above (2000 hot spot excavation, 2005 ORC injection, and 2005 UST removal), TPH-G was detected in well MW-14 at a concentration of 0.53 mg/L, below the MTCA Method A cleanup level of 0.8 mg/L and significantly decreased from previous sampling events. Although well MW-6 was not sampled during the June 2006 event, a TPH-G concentration of 1.38 mg/L (estimated) was detected at this location in October 2005. This well will be included with future sampling events.

TPH-G was not detected in well MW-2 between April 1996 through December 2003. However, monitoring during 2004 identified TPH-G concentrations of 13.0, 1.48, and 1.29 mg/L, respectively. During sampling

it was noted that the MW-2 well monument contained water with a notable TPH-like odor. Although the water was removed before opening of the well casing, we suspect that surface water previously entered the casing. Supported by the substantial TPH concentration decreases in more recent sampling events, we believe that these detections were not representative of site groundwater conditions, but resulted from introduction of minor amounts of TPH constituents from surface runoff through the well surface seal. We will continue to monitor MW-2 closely and make any necessary future recommendations.

- Benzene. A site map showing the extent of benzene occurrences in groundwater during the most recent monitoring event in June 2006 is provided on Figure 9. Groundwater containing elevated concentrations of benzene is generally collocated with elevated concentrations of TPH-G. In October 2005, after the completion of the remedial activities described above, benzene was detected in MW-6 at a concentration of 8.10 ug/L, marginally above the MTCA Method A cleanup level of 5 ug/L. This well was not sampled during the most recent groundwater monitoring event in June 2006, but is planned for inclusion in future sampling events.
- Lead. In early groundwater monitoring at the site, lead was detected above the MTCA Method A cleanup level of 15 ug/L at two wells during several sampling events—at a maximum concentration of 220 ug/L at MW-1 and 34 ug/L at MW-2. These samples were collected with a bailer, and the results may have been biased high due to suspended solids in the sample. Lead has not been detected in site groundwater above cleanup levels since March 2000.

Gasoline additives, including MTBE, EDB, and EDC, have not been detected above MTCA Method A cleanup levels at the site except for one anomalous hit in MW-4 during the March 2004 groundwater monitoring event. The exceedance is most likely associated with the detection of TPH-G at that location as discussed above.

Trends in Groundwater Quality for Contaminants of Potential Concern

Concentrations of TPH-G and benzene at wells where exceedances of cleanup levels have been recorded are shown on Figures 8 and 9, respectively. Concentrations of both of these chemicals have generally declined at well locations since monitoring began in 1996, indicating that the affected area of groundwater has decreased in terms of TPH-G concentrations. There is some variability in the data, with several spikes; however, long-term trends indicate an

overall concentration decline site-wide. TPH-G concentrations exceeding the 0.8 mg/L MTCA Method A cleanup level are currently limited to well MW-6 (1.38J mg/L). During the latest sampling round TPH-G concentrations were non-detect in well MW-14 for the first time. The trends in xylene and lead concentrations mirror trends in TPH-G and benzene concentrations. Overall the results indicate marked decrease in TPH and benzene concentrations near the central source area of affected soil following ORC injection and UST removal in 2005.

5.4 Free Product

Free product has been observed intermittently at well MW-1/MW-14. Product thicknesses measured before and after the October 2000 hot spot excavation are summarized in Table 7. Free product has not been observed at other site wells. Note that the actual free product thickness in the formation is typically one-half to one-sixth the free product thickness measured in the well, due to capillary forces.

An overall decline in free product thickness has been observed since 1998. After the hot spot excavation, product was only been observed during the winter monitoring events, when the water table was low and free product was most likely to accumulate. No free product has been observed since the UST removal in 2005, indicating that ORC injection efforts and soil removal activities during UST closure have been effective in further reducing the mass of TPH.

5.5 Site Conceptual Model

Subsurface contamination at the Ken's Auto Wash site appears to have been caused by a fuel line leak reported and repaired in 1995. This leak occurred in the pea gravel backfill around the UST. Free product likely migrated down through the UST backfill to the top of the water table (within the pea gravel) and from there along the top of the water table to the downgradient edge of UST backfill, at the southwest corner of the property. Finer grained soils outside the UST area limit the extent of migration of free product farther downgradient.

Seasonally fluctuating water tables smear the gasoline product (which is less dense than water, and thus floats on top of the water table) in the soil at depths between 6 and 10 feet below ground surface. The highest dissolved phase contaminant concentrations are typically observed in the spring, at high water levels when groundwater is in contact with the greatest amount of soils containing TPH-impacted material.

After repair of the fuel leak, product occurrences and contaminant concentrations peaked between 1996 and 1998 and have since declined as the source was removed and natural attenuation processes continue to remove and degrade petroleum constituents. The hot spot excavation in 2000 removed approximately half the estimated mass of TPH-affected soil from the subsurface. ORC injection and additional soils excavation during UST removal also aided in further reducing this mass. As a result of these actions and continued natural attenuation of petroleum hydrocarbons, dissolved phase concentrations and free product occurrences continue to decrease.

The product released was unleaded gasoline and does not appear to have contained gasoline additives, based on groundwater chemistry data. Exceedances of the lead cleanup level in groundwater samples previously collected by Sage are likely due to suspended solids entrained in the samples as a result of collecting samples with bailers.

Dissolved oxygen concentrations have varied significantly in site wells since 2000 (Figure 12). Although distinct trends are difficult to discern from data, detected concentrations of dissolved oxygen (DO) may be attributable to a number of factors:

- The lowest DO concentrations have commonly been observed within the petroleum-affected area, as observed in wells MW-1/MW-14, MW-4/MW-4R, and MW-6. Concentrations in these wells rarely exceed 1 mg/L. Depressed DO concentrations are typical of groundwater containing petroleum hydrocarbons and may be indicative of aerobic petroleum-degrading microbial activity, although DO concentrations at the site have continued to vary significantly between sampling events.
- The highest DO concentrations have generally been observed outside the petroleum-affected area, as noted in wells MW-12, MW-13, and to lesser extent in MW-2, MW-3, and MW-15. Concentrations up to 6.05 mg/L were detected (well MW-2 during the December 2004 sampling event, the most recent event with complete DO data), as compared with a maximum concentration of 0.42 mg/L from wells located inside the plume (well MW-6) during the December 2004 sampling event. High DO concentrations are indicative of well oxygenated water beyond the petroleum-affected area.
- Infiltration events and seasonal recharge of relative oxygen-rich precipitation may promote sporadic increases in DO concentrations. Many of the wells, both inside and outside of the plume, exhibited increased concentrations during the December 2004 monitoring event that may be attributable to this effect.

 Instrument drift and accuracy may also contribute to observed variability in DO concentrations.

6.0 CLEANUP OBJECTIVES AND CRITERIA

6.1 Chemicals of Concern

We compared chemical occurrences to MTCA Method A cleanup levels to identify chemicals of concern for each medium at the site. These chemicals of concern are as follows:

- Soil. TPH-G, BTEX, and lead.
- **Groundwater.** TPH-G, BTEX, and lead.

6.2 Potential Exposure Pathways

Direct Contact with Soil

TPH-G and benzene occurrences in soil are in relatively deep 'smear zone' soils below depths of 6 feet. The potential for direct contact exposures is minimal due to the presence of concrete pavement above the affected area (beneath the subject property and adjacent sidewalk).

Direct Contact with Groundwater

The receptor analysis performed by Hart Crowser in 1999 (Hart Crowser 1999a) identified the following hypothetical receptors of contaminated groundwater from the site for evaluation:

- The west branch of Wilson Creek, located approximately 600 feet in the cross-gradient direction of the site; and
- Two City of Ellensburg water supply wells, located 1,200 and 1,600 feet in the downgradient direction of the site.

Site groundwater data indicate that the TPH-affected zone is limited to an area within about 100 feet of the site and is shrinking. Furthermore, an analysis of regional hydrogeology shows that the shallow water-bearing zone affected at the site is not hydraulically connected to the aquifers tapped by the water supply wells (see Appendix A). The upper water-bearing zone is isolated by several underlying aquitards between the shallow water-bearing zone and the water supply aquifers located at depths between 200 and 700 feet. In addition, the

Hart Crowser Page 14

receptor analysis and well survey by Hart Crowser have not identified any consumptive use of shallow groundwater in the vicinity of the site.

Although a completed groundwater exposure pathway does not exist, affected groundwater has been detected both on and off the site, and represents a potential exposure risk in the event that excavations are performed in the affected area. Utilities along East University Way are located above the water table, so the likelihood of encountering affected groundwater in utility repair work is low.

6.3 Remedial Action Objectives (RAOs)

Cleanup actions to be implemented at the Ken's Auto Wash site are designed to address the following RAOs:

- Prevent Direct Contact with Contaminated Soil. Prevent direct contact with petroleum-impacted soils exhibiting concentrations above MTCA unrestricted cleanup levels.
- **Protect Groundwater.** Address petroleum-impacted soil and groundwater to reduce gasoline-range hydrocarbon and benzene concentrations in groundwater to concentrations below MTCA Method A criteria.
- Remove Free Product. Remove free product from the subsurface to the extent practicable.

Under current site conditions, contact with contaminated soil is prevented by a concrete cap over the affected area.

Achieving Method A cleanup levels in soil and groundwater on site may not be practicable because of the heterogeneous site soils containing both coarse- and fine-grained materials. Under this scenario, the downgradient property boundary (near well MW-14) could be used as a conditional point of compliance. Residual petroleum hydrocarbons exceeding MTCA Method A cleanup levels in soil and groundwater may be addressed by appropriate institutional controls, such as a deed restriction or maintenance of pavement areas.

6.4 ARARs and Applicable Regulations

Potential remedial technologies are evaluated in Section 7.0 based on their ability to meet Applicable or Relevant and Appropriate Requirements (ARARs) associated with federal, state, and regional regulations. The following ARARs have been identified:

- Model Toxics Control Act (MTCA 70.105D RCW, Chapter 173-340 WAC). MTCA contains detailed requirements and Washington State's expectations for cleanup of contaminated sites.
- State Environmental Policy Act (SEPA 43.21 RCW, Chapter 197-11 WAC). An environmental checklist is necessary as part of any permitting activity within the City of Ellensburg and pursuant to MTCA.
- Minimum Standards for Construction and Maintenance of Wells (Chapter 173-160 WAC). This regulation contains requirements for abandonment and construction of resource protection wells.
- Dangerous Waste Regulations (Chapter 173-303 WAC). This regulation addresses requirements for identification and proper management of dangerous wastes. It is unlikely that petroleum-impacted soils or groundwater on the Ken's Auto Wash property would be designated as Dangerous or Extremely Hazardous Wastes.
- State Clean Air Act (RCW 70.94), General Regulations for Air Pollution Sources (Chapter 173-403 WAC), and Toxic Air Contaminant New Source Review Guidelines. Emissions during any on-site treatment operations may be subject to these regulations and may require a Notice of Construction Permit.

6.5 Cleanup Levels

The Ken's Auto Wash site is a routine cleanup action, as defined in WAC 173-340-200, and involves relatively few constituents. Therefore, in accordance with WAC 173-340-700(5)(a), MTCA Method A cleanup levels may be used. (i.e., for unrestricted site uses). Cleanup levels for chemicals of concern are summarized in Table 8.

6.6 Terrestrial Ecological Evaluation

Ecology's policy for protection of terrestrial ecological receptors (Terrestrial Ecological Evaluation Procedures) is described in WAC 173-340-7490 of MTCA. The site also qualifies for an exclusion from a terrestrial ecological evaluation, as described in WAC 173-340-7491(c). A portion of the site located immediately west of the Ken's Auto Wash property is an unpaved vacant lot, qualifying this area as contiguous undeveloped land based on the definition presented in WAC 173-340-7491(1)(c)(iii). The area of contiguous undeveloped land is less than 1.5 acres, and no further terrestrial ecological evaluation is therefore required based on the criteria listed in WAC 173-340-7491 9(c).

Hart Crowser 7168-04 November 14, 2006 Further, there is no potential exposure pathway to terrestrial wildlife at the site:

- The site is entirely paved where TPH constituents in soil exceed applicable MTCA ecological indicator concentrations;
- Where present, depth to soils contamination is more than 7 feet below ground surface; and
- Where present, depth to groundwater is more than 5 feet below ground surface.

The depth and location of contamination, therefore, is beyond the range of reasonable exposure scenarios.

7.0 REMEDIAL ALTERNATIVE DEVELOPMENT AND EVALUATION

In the Focused Feasibility Study and Remedial Design (Hart Crowser 2000d), we screened potential remediation technologies and proposed two groundwater remediation technologies as potential supplements to the October 2000 excavation—oxygen infusion and ORC injection. These were identified based on the assumption that the majority of contaminated soil and free phase product would be removed by excavation, and that residual hydrocarbons left in-place could be removed using *in situ* bioremediation methods. Current site conditions require a broader evaluation of remedial alternatives. This section describes the development and screening of remedial alternatives.

7.1 Technology Screening

We identified the following remediation technologies to be potentially applicable for addressing petroleum contamination remaining in place:

- **Natural Attenuation.** Natural processes, including biodegradation by native bacteria, would remove petroleum hydrocarbons remaining in-place.
- Enhanced In Situ Bioremediation. Oxygen would be added to groundwater using biosparging, oxygen infusion, or ORC injection. The added oxygen helps to stimulate biodegradation of petroleum hydrocarbons by native bacteria.
- Soil Vapor Extraction. Soil vapor would be removed from the subsurface. Volatile contaminants in soil would evaporate into the vapor, and the vapor would be treated above ground. Increased flow of oxygen to the subsurface

would stimulate biodegradation of petroleum hydrocarbons. Because most of the contamination occurs in soil near the water table, this technology would only be implemented in conjunction with air sparging for effective treatment.

■ Air Sparging. Air would be bubbled into the groundwater. Volatile contaminants in groundwater would evaporate into the air, which would then be collected and treated by a soil vapor extraction system. Oxygen in air would dissolve into the groundwater and stimulate biodegradation of petroleum hydrocarbons remaining in-place.

In this section we describe remedial action objectives and compare estimated project costs and preliminary remediation time frames for four remedial alternatives that could achieve these objectives:

- Alternative 1 Monitored Natural Attenuation;
- Alternative 2 Monitored Natural Attenuation with Passive Product Recovery;
- Alternative 3 Enhanced Biodegradation by ORC Injection; and
- Alternative 4 Air Sparging and Soil Vapor Extraction.

7.2 Remedial Alternative Descriptions

MTCA requires at a minimum that cleanup actions protect human health and the environment, comply with cleanup standards, comply with applicable state and federal laws, and provide for compliance monitoring. Using the technologies identified in Section 7.1, we developed four remedial alternatives that meet the above requirements. These alternatives are described below and compared in Table 9.

Cost estimate details are provided in Appendix D. The level of accuracy of these estimated costs is "order of magnitude," as defined by the American Association of Cost Engineers. The target accuracy of an order of magnitude estimate is plus 50 percent and minus 30 percent. Construction cost estimates at this level may be used to compare alternatives, but should not be used to plan, finance, or develop projects. Estimated alternative costs were calculated using a present worth analysis assuming a discount rate of 2.6 percent for 5-year returns or less, 2.8 percent for returns between 5 and 10 years, and 3.0 percent for returns greater than 10 years. These discount rates are based on rates from January 2006 listed in the Office of Management and Budget Circular A-94. Nominal estimated Ecology oversight costs are included for each alternative. In response to Ecology review comments on the previous draft version of the document,

costs include a contingency for replacing up to three monitoring wells over the lifetime of each alternative.

Note that estimated costs are for comparing alternatives and do not include costs for preparation and review of deliverables associated with a second Agreed Order, if issued by Ecology to complete remediation. Tasks may include preparation of a Cleanup Action Plan, interaction with Ecology, and related project management. Costs are expected to be comparable for Alternatives 1 and 2 and are estimated in the \$15,000 to \$25,000 range. Costs for Alternatives 3 and 4 may also be comparable and are estimated to be in the \$25,000 to \$40,000. Costs exclude additional field work (other than on-going monitoring currently being conducted at the site), and we caution that significant uncertainty is associated with these estimates. Additional input from Ecology will be necessary to refine these preliminary estimates more accurately.

Alternative 1 - Monitored Natural Attenuation

Monitored natural attenuation consists of allowing naturally occurring processes, such as dilution, dispersion, adsorption, and biodegradation, to remove contaminants and reduce concentrations. This approach is potentially effective at the site based on the following observations:

- Chemical data indicate that the contaminant plume in groundwater is stable or shrinking.
- Depressed concentrations of DO in groundwater are typical of petroleum hydrocarbon plumes where significant biological activity is occurring. The depressed concentrations result from utilization of the petroleum contaminants as a growth substrate. As discussed above, low DO concentrations in many of the samples collected from the site plume area is one weight-of-evidence factor suggesting that biological degradation is continuing to occur.
- Conversely, relatively higher DO concentrations in groundwater samples outside of the plume area indicate that oxygen continues to be available to promote natural attenuation.

Continued periodic groundwater monitoring would be required to verify the removal of contaminants and that the contaminant plume in groundwater does not expand. Additional limited monitoring for constituents indicative of biodegradation (e.g., dissolved iron, nitrate, and sulfate) would be prudent. This approach provides minimal site or area impacts. Note that for comparative purposes with Alternative 2, Alternative 1 does not include removal of residual

free product near the source area. As an MTCA requirement, removal of residual free product was included as part of Alternative 2, as discussed below. A free product monitoring program would ensure free product occurrences at well MW-14 diminish and that product does not migrate to downgradient wells. Free product was not detected in well MW-14 during the last three sampling rounds, which immediately followed the UST removal and ORC injection in 2005. There is no indication that free product is currently present or migrating in the subsurface.

If free product remains at the site and is detected during future sampling events, the projected remediation time frame could be more than 20 years. Therefore, the estimated cost of this alternative is based on a nominal monitoring period between 15 to 30 years. Estimated costs range from about \$325,000 to \$491,000. Cost estimate details are provided in Table D-1 of Appendix D.

Alternative 2 - Monitored Natural Attenuation with Passive Free Product Recovery

This alternative is the same as Alternative 1 except a sorbent sock or similar passive recovery device would be placed in well MW-14 if free product was observed. Although not recently observed in MW-14, free product may still be present near the southern border of the UST excavation, as described in Section 3.4. Active free product recovery is not viable, nor expected to be necessary because only a small amount of product has intermittently been observed in this well. Passive free product recovery would minimize the potential for free product migration and increase the speed of site cleanup. Based on current decreasing trends in constituent concentrations, the projected remediation time frame could be 5 to 10 years, but for costing purposes a time period of up to 30 years was used for reaching target groundwater cleanup levels at monitoring well MW-14. The longer time frame was also used for comparative purposes with Alternative 1. The estimated costs of this alternative range from about \$333,000 to \$505,000. Cost estimate details are provided in Table D-2 of Appendix D.

Alternative 3 - Enhanced Biodegradation

An enhanced biodegradation approach uses the same processes involved in natural attenuation but speeds up remediation by stimulating the biodegradation component. Because biodegradation of contaminants is oxygen-limited, oxygen can be added by several methods to increase the rate of biodegradation. In the Focused Feasibility Study (Hart Crowser 2000d), we evaluated two methods—oxygen infusion (e.g., via passive diffusion) and ORC injection—of introducing oxygen to groundwater in the subsurface that would incur limited site and area

impacts. Implementing oxygen infusion would require installing two wells, storing a secured oxygen cylinder on site, and replacing the oxygen cylinder every month. ORC injection would require injection of ORC slurry into groundwater beneath the source area and downgradient plume twice a year. Injection would be performed with a direct-push drill rig and require limited concrete coring and temporary partial street closure for one day. This method was used to complete ORC injection at the locations previously noted along East University Way in 2005.

The Focused Feasibility Study considered implementing oxygen infusion to remove contaminants left after excavation but recognized that this technology had not yet been proven and that re-evaluation of this selection following excavation and groundwater monitoring was warranted. Since that time the oxygen infusion technology has not been clearly demonstrated to be more effective than other oxygen introduction technologies. Therefore, in this RI/FS, we discard the oxygen infusion alternative and further evaluate enhanced biodegradation using ORC injection as the most viable biodegradation alternative. Because of the uncertainties associated with the time frame needed for this alternative, enhanced biodegradation may need to be implemented on a relatively long-term basis to be effective and meet remedial action objectives. The estimated cost of this alternative, based on a nominal 10- to 15-year operating lifetime ranges from about \$485,000 to \$653,000. The operating lifetime is based on our preliminary projection, and assuming that additional free product may be present, as discussed above for Alternative 2. Cost estimate details are provided in Table D-3 of Appendix D.

Alternative 4 - Air Sparging and Soil Vapor Extraction

Two aggressive *in situ* technologies were identified in the Focused Feasibility Study as potentially applicable to the site—sparging and sparging combined with soil vapor extraction (SVE). Based on the estimated contaminated mass remaining in-place, sparging combined with SVE would likely be necessary to collect and treat hydrocarbon emissions.

In sparging with SVE, air would be injected into groundwater, stripping contaminants from the water and from soil. Air containing contaminant vapors would be collected using SVE and treated to remove contaminants and meet air discharge requirements. During the October 2000 soil removal, porous horizontal pipes for sparging were laid at the base of the excavation to facilitate implementation of these technologies, if necessary. Implementing this alternative would require installing four vapor extraction wells, five sparging wells, piping, and a secure equipment compound containing a sparging blower, SVE blower, knockout drum, 500-gallon condensate collection tank, and control

panel. Sound enclosures would be placed around the blowers, but the blowers will still be audible when running.

A typical time to achieve site closure using sparging/SVE at sites with similar conditions (e.g., some free product and relatively heterogeneous soil) is approximately 5 years of system operation followed by 1 year of confirmation monitoring. The actual duration of remediation would be determined by evaluating system performance over time. The estimated cost for this alternative, based on an operating lifetime of 5 to 7 years for comparative purposes, including 1 year of monitoring, ranges between about \$395,000 to \$464,000. The estimated operating lifetime is based on our experience at similar sites with comparable conditions, and is intended for cost comparison and planning purposes only. Cost estimate details are provided in Table D-4 of Appendix D.

7.3 Evaluation of Alternatives

These four proposed alternatives would meet the threshold requirements for cleanup actions outlined in WAC 173-340-360 (2)(a): they protect human health and the environment, comply with cleanup standards, comply with applicable state and federal laws, and provide for compliance monitoring. In Table 9, we evaluate each of the four alternatives described in Section 7.2 based on their use of permanent solutions to the maximum extent practicable and on the ability of each alternative to provide for restoration in a reasonable timeframe following the criteria described in WAC 173-340-360.

Alternative 1 meets the criteria described in WAC 173-340-360, except if free product continues to be present in the future. Alternative 2 provides additional control and removal of free product for a relatively small increase in cost for comparable project lifetimes. Costs could conceivably be less for Alternative 2 if product removal further accelerates the natural attenuation process. Alternative 3 and 4 potentially provide faster source removal than Alternative 2, but have disproportionately higher costs and resource utilization. Further, even the most aggressive alternative (Alternative 4) will not provide for complete source removal during active remediation, as complete removal of petroleum hydrocarbons from fine-grained soils in the short term is unlikely. The capital costs for Alternative 4 (estimated \$186,100) are also substantially greater than the other alternatives. Conversely, it should be noted that if Alternatives 3 or 4 achieved cleanup goals faster than the estimated time projections, they could be less expensive than Alternative 2. However, there is no assurance of this outcome, given uncertainties associated with duration for the in situ technologies considered. Therefore, Alternative 2-Monitored Natural Attenuation with Free Product Recovery was identified as the preferred remedial

alternative. This alternative provides for a reasonable restoration time frame in accordance with WAC 173-340-360(4). Implementation of this alternative is described below.

7.4 Preferred Remedial Alternative Identification

Monitoring data indicate that previous remediation actions and ongoing natural attenuation of contaminants have been effective in removing contamination from the subsurface, and that the contaminant plume in groundwater is shrinking. It is our opinion that occurrences of residual petroleum hydrocarbons in soil and groundwater at this site do not represent a risk to human health or the environment.

Because some free product is still present at the site, we recommend adopting Alternative 2—Monitored Natural Attenuation with Passive Product Recovery as the remediation strategy. This alternative meets site RAOs: direct contact with contaminated soils on site is prevented by maintaining the existing concrete surface; removes free product to the extent practicable using passive recovery devices; and in the long term reduces soil and groundwater concentrations below cleanup levels by natural degradation of contaminants.

Alternative 2 would be sufficiently protective of human health and the environment and is the most cost-effective alternative. Passive product recovery would mitigate potential free product migration while adding a relatively small cost. Although this alternative has a long remediation time frame, contaminants will be completely destroyed *in situ* while using a minimum of energy and natural resources. Monitoring would be conducted to ensure that this alternative remains protective of human health and the environment.

A preliminary monitoring schedule is included in Table 10. This schedule includes the continuation of periodic monitoring for natural attenuation parameters to demonstrate contaminants are degraded *in situ*. Monitoring frequency will continue on a biannual basis and will be conducted during wet and dry season conditions. Every 5 years, in accordance with Ecology policy, we assume that the site data would be reviewed by Ecology to ensure the alternative is still protective of human health and the environment, that the contaminant plume is still contained, and that long-term trends show constituent concentrations are decreasing.

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Hart Crowser 716804/Kens Auto Wash RIFS Tables - Table 1

		Activity	,		
Dates	Soil and Groundwater Investigation	Soil Sampling and	Groundwater	Remediation	Consultant
		Analysis	Monitoring		
1995 to 1998	Monitoring Well Installation	×	×		Coac Forth Colombos
1998			×		
1999	Recepter Analysis	×	×		
	Utility Survey Soil Boring Evoloration				
	Monitoring Well Installation				-
2000			×		
September 2000	Additional Soil Borings	×	×		
	Hydropunch Groundwater Samples from Borings				
					Hart Crowser
October and	Soil Sampling from Hot Spot Excavation	×		Hot Spot Excavation	
November 2000				ORC Application to Excavation	
January 2001	Install MW-14 Replacement Well and MW-15		×		
2000 to 2005			×		
February 2005	Strataprobe (Push-Probe) Explorations	×	×	ORC Injection via Strataprobes	
April 2005	Soil Sampling from UST Explorations	×		UST Removal	
	"Pothole" Test Pit Explorations				
October 2005	Install MW-4R Replacement Well		×		
2005-On-Going			×		

Table 1 - Summary of Site Investigation, Remediation, and Monitoring Activities

Measured Depth to Groundwater in Feet

Table 2 - Groundwater Elevation Data Measured Depth to Groundwater in Feet Measured Depth to Groundwater in Feet

								,					;							
Well No.	80	8-Apr-96	5-Jan-98	5-Feb-98	5-Mar-98	6-Apr-98	5-May-98	5-Jun-98	6-Jul-98	5-Aug-98	4-Sep-98	5-0ct-98	5-Nov-98	29-Dec-99	21-Mar-00 14-Jun-00		12-Sep-00	30-Jan-01	26-Apr-01	29-Jul-01
MW-1		6.85	БП	7.67	8.01	8:38	6.88	6.94	7.50	7.69	7.82	7.85	8.33	9.65	8.51	7.08	7.85	1	1	
MW-14 (b)		ı	ı	I	ı	1	I	i	ı	I	1		I	. I,	i	1	l	8.55	8.35	7.01
MW-2		6.70	7.53	6.50	6.88	7.18	5.69	5.79	6.19	6.55	6.58	7.70	90'2	7.23	7.18	6.10	6.70	7.54	7.11	6.23
MW-3		8.08	8.42	7.65	8.01	8.17	6.71	7.50	7.42	7.51	7.66	7.80	8.28	8.41	8.29	7.42	7.92	8.70	79.7	7.28
MW-4		ŀ	7.84	7.17	7.43	7.67	6.42	6.57	6.90	7.01	7.14	7.21	7.62	2.68	7.60	6.80	7.23	8.08	7.85	6.93
MW-4R (c)		1	ŀ	ı	į	1	i	. 1	1	1	I	ı	ı	i	1	ı	i	i	1	ı
MW-5			8.23	7.15	7.45	7.96	6.24	6.34	6.65	7.16	7.29	7.41	7.94	7.52	7.32	6.25	6.87	ē	7.98	6.29
MW-6		I	9.70	8.67	9.13	9.46	8.14	8.21	8.66	8.87	9.01	9.05	9.51	8.60	8.36	7.70	8.07	멸	9.28	8.09
MW-12		ı	i	1	i	i	ı	ı	ļ	I	1	. 1	J.	6.91	6.64	6.05	6.36	2	7.30	6.38
MW-13		ı	ı	ı	1	I.	Ī	ı	ı	ı	1	1	Í	5.42	5.33	4.70	4.98	2	5.74	4.67
MW-15		ı	I	ı	ï	i	ı	ı	ı	ı	1	l	1	ŀ	ı	1	I	9.23	8.83	7.59
	Ď	oundwate	Groundwater Elevation in Feet	in Feet										Groundwater Elevation in Feet	r Elevation	in Feet				
Well No. TC	TOC Elev. (a) 8-	8-Apr-96	5-Jan-98	5-Feb-98	5-Mar-98	6-Apr-98	5-May-98	5-Jun-98	6-Jul-98	5-Aug-98	4-Sep-98	5-0ct-98	5-Nov-98	29-Dec-99	21-Mar-00 14-Jun-00 12-Sep-00	14-Jun-00		30-Jan-01	26-Apr-01	29-Jul-01
MW-1	1588.38 1	1581.53	2	1580.71	1580.37	1580.00 - 1581.50	1581.50	1581.44	1580.88	1580.69	1580.56	1580.53	1580.05	1578.73	1579.87	1581.30	1580.53	ı	1	ı
MW-14 (b)	1588.4	ı	1	ŀ	ı	I	ı	i	ļ	1	ı	ı	ı	1	ı	ı	ı	1579.85	1580.05	1581.39
MW-2	1588.92 1	1582.22	1581.39	1582.42	1582.04	1581.74	1583.23	1583.13	1582.73	1582.37	1582.34	1581.22	1581.86	1581.69	1581.74	1582.82	1582.22	1581.38	1581.81	1582.69
MW-3	1591.43	1583.35	1583.01	1583.78	1583.42	1583.26	1584.72	1583.93	1584.01	1583.92	1583.77	1583.63	1583.15	1583.02	1583.14	1584.01	1583.51	1582.73	1583.76	1584.15
MW4	1589.50	ļ	1581.66	1582.33	1582.07	1581.83	1583.08	1582.93	1582.60	1582.49	1582.36	1582.29	1581.88	1581.82	1581.90	1582.70	1582.27	1581.42	1581.65	1582.57
MW-4R (c)	1591.43	i	ı	j	i	i	ļ	ŀ	i	1	i	ı	ı	ı	I	ŀ	ı	i	1	1
MW-5	1587.75	ı	1579.52	1580.60	1580.30	1579.79	1581.51	1581.41	1581.10	1580.59	1580.46	1580.34	1579.81	1580,23	1580.43	1581.50	1580.88	멸	1579.77	1581.46
MW-6	1587.72	ı	1578.02	1579.05	1578.59	1578.26	1579.58	1579.51	1579.06	1578.85	1578.71	1578.67	1578.21	1579.12	1579.36	1580.02	1579.65	2	1578.44	1579.63
MW-12	1585.41	ı	ļ	I	ı	1	1	1	ı	ı	ł	,1	ı	1578.50	1578.77	1579.36	1579.05	e E	1578.11	1579.03
MW-13	1582.45	ı	ı	ı	l	I	ı	I	I	ł	ı	i	I	1577.03	1577.12	1577.75	1577.47	ē	1576.71	1577.78
MW-15	1588.39	I.	i	١	Ι	1	ı	ı	١	ı	١	I	I	i	ŀ	ı	ı	1579.16	1579.56	1580.80

Table 2 - Groundwater Elevation Data Measured Depth to Groundwater in Feet

Feet
water in
Ground
pt to
Measured Deg
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Well No.	27-0ct-01	15-Nov-02	9-May-03	27-Oct-01 15-Nov-02 9-May-03 30-Sep-03 11-Dec-03 31-Mar-04 2-Jun-04 30-Sep-04 14-Dec-04 4-Apr-05	11-Dec-03	31-Mar-04	2-Jun-04	30-Sep-04	14-Dec-04	4-Apr-05	6-0ct-05	28-Jun-06
MW-1	1	1	1	ŀ	i	. 1	ı	1	1	ı	,	ı
MW-14 (b)	9.02	8.90	6.23	8.05	8.58	8.32	6.28	62.7	8.45	8.63	7.83	6.15
MW-2	7.64	7.61	5.95	6.81	7.03	7.05	5.94	69.9	70.7	75.7	7.21	Ę
MW-3	8.66	8.63	6.89	8.06	8.48	8.30	6.98	7.92	8. 26.	8.80	8.37	E
MW-4	8.09	8.04	6.71	7.65	7.81	7.70	6.62	7.44	7.86	8.02	ı	1
MW-4R (c)	ı	ı	1	i	i	!	1	1	I	i	7.78	6.01
MW-5	7.97	8.05	6.19	7.55	7.83	7.59	6.14	ı	9.21	8.32	7.73	6.38
MW-6	9.44	9.37	7.91	8:30	9.19	9.00	7.82	8.88	9.49	9.78	9.14	Ē
MW-12	7.13	7.52	6.50	7.25	7.38	7.18	6.40	7.31	7.81	7.89	7.51	6.90
MW-13	5.78	i	1	5.32	5.73	5.49	4.63	5.18	5.81	5.16	5.56	Ē
MW-15	9.30	9.08	7.38	8.55	8.67	8.85	7.31	8.33	9.20	9.40	8.02	Ē
							to the second	Contraduction Classical in East	ii Pool			

Well No.	TOC Elev. (a	TOC Elev. (a) 27-Oct-01 15-Nov-02 9-May-03 30-Sep-03 11-Dec-03 31-Mar-04 2-Jun-04 30-Sep-04 14-Dec-04 4-Apr-05	15-Nov-02	9-May-03	30-Sep-03	11-Dec-03	31-Mar-04	2-Jun-04	30-Sep-04	14-Dec-04	4-Apr-05	6-Oct-05	6-Oct-05 28-Jun-06
MW-1	1588.38	i	1	1	ł	1	i	i	ı	ı	ı	ı	
MW-14 (b)	1588.4	1579.38	1579.50	1582.17	1580.35	1579.82	1580.08	1582.12	1580.61	1579.95	1579.77	1580.57	1582.25
MW-2	1588.92	1581.28	1581.31	1582.97	1582.11	1581.89	1581.87	1582.98	1582.23	1581.85	1581.35	1581.71	Ē
MW-3	1591.43	1582.77	1582.80	1584.54	1583.37	1582,95	1583.13	1584.45	1583.51	1582.79	1582.63	1583.06	Ē
MW-4	1589.50	1581.41	1581.46	1582.79	1581.85	1581.69	1581.80	1582.88	1582.06	1581.64	1581.48	i	i
MW-4R (c)	1591.43	I	ı	ı	ļ	!	ı	ļ	ı	ı	I	1583.65	1585.42
MW-5	1587.75	1579.78	1579.70	1581.56	1580.20	1579.92	1580.16	1581.61	ı	1578.54	1579.43	1580.02	1581.37
MW-6	1587.72	1578.28	1578.35	1579.81	1578.82	1578.53	1578.72	1579.90	1578.84	1578.23	1577.94	1578.58	E
MW-12	1585.41	1578.28	1577.89	1578.91	1578.16	1578.03	1578.23	1579.01	1578.10	1577.60	1577.52	1577.90	1578.51
MW-13	1582.45	1576.67	I	1	1577.13	1576.72	1576.96	1577.82	1577.27	1576.64	1577.29	1576.89	E
MW-15	1588.39	1579.09	1579.31	1581.01	1579.84	1579.72	1579.54	1581.08	1580.06	1579.19	1578.99	1580.37	Ē

Notes:

(a) TOC Elevation = top of casing elevations are surveyed relative to Mean Sea Level by Sage Environmental.

MW-12 and MW-13 were surveyed relative to existing well MW-1, and existing wells MW-5 and MW-5 were re-surveyed and corrected slightly.

(b) Well MW-1 replaced as well MW-14 by Hart Crowser following remediation work in November 2000.

(c) Well MW-4 was replaced as well MW-4R by Hart Crowser in October 2005, following removal of the well during UST removal activities in April 2005.

— Well not installed or not available as of date indicated mm Indicates well was not monitored

Table 3 - Summary of Soil Chemistry Data

		Sample					Concenti	ation in mg/kg			
Location	Sample ID	Depth in Feet	Date Sampled	TPH-G	TPH-D	TPP-O	Benzene	Toluene	Ethylbenzene	Xylenes	Lead
Soil Boringe											
MW-1	BP-0296-S1	7.5	4/5/1996	4,800	_	_	3	45	37	270	
MW-2	BP-0296-S5	7.5	4/5/1996	4	_	_	0.02 U	0.02 U	0.02 U	0.03	
MW-3	BP-0296-S8	7.5	4/5/1996	20 U	50 U	100 U	_	_	-	_	_
MW-4	BP-0397-S1	cuttings	12/9/1997	3	·_		0.02 U	0.02	0.02 U	0.03	_
MW-5	BP-0397-S2	7 to 9	12/9/1997	870	-	-	0.2 U	10	0,2 U	12	
MW-5	BP-0397-S3	12.5 to 13	12/9/1997	110	· - -	_	0.02 U	1.6	0.27	1.6	_
MW-6	BP-0397-S4	12.5	12/9/1997	1 U	-	_	0.02 U	0.02 U	0.02 U	0.02 U	-
HP-8	HP-8	8.5	7/14/1999	8.12	_	_	0.05 U	0.06 U	0.05 U	0.1 U	_
HP-9	HP-9	10	7/14/1999	20.7	_	_	0.05 U	0.05 U	0.05 U	0,12 U	_
HP-12	HP-12 S-3	10 to 11.5	9/12/2000	11.3	_	_	0.05 U	0.05 U	0.05 U	0.1 U	2.37
HP-13	HP-13 S-3	10 to 11.5	9/12/2000	24.3	_	_	0.05 U	0.05 U	0.05 U	0.206	· -
HP-14	HP-14 S-2	8 to 9,5	9/12/2000	68.1	-	_	0.0686	0.0701	0.062	0.474	_
HP-15	HP-15 S-2	8 to 9.5	9/12/2000	5 U	-	_	0.05 U	0.05 U	0.05 U	0.1 U	-
HP-16	HP-16 S-2	8 to 9.5	9/12/2000	247	_	_	0.273	0.408	1.41	3.27	3.76
MW-14	MW-14 S-1	8 to 9.5	1/26/2001	7,15		_	0.05 U	0.06 U	0.05 U	0.1 U	_
MW-14	MW-14 S-2	15 to 16.5	1/26/2001	5.47	_	_	0.05 U	0.06 U	0.05 U	0.103	
MW-15	MW-15 S-1	8 to 9.5	1/26/2001	5 U	_	_	0.05 U	0.06 U	0.05 U	0.1 U	
				,							
Hot Spot Excev	ation Verification Sam 11/3-E Wall	7 to 8	11/3/2000	32.8	_	_	0.05 U	0.05 U	0.05 U	0.1 U	
Northeast Wall	11/3-NE Wall	7 to 8	11/3/2000	11,000	_	_	10.6	13.5 U	73.9	588	_
			11/3/2000	7,130	_		2.5 U	7.75 U	72.3	423	
Southeast Wall	11/3-SE Wall	7 to 8			-	-			72.3 0.05 U	0.1 U	
West Wall	11/10-W Wall	7 to 8	11/10/2000	5 U	-	-	0.05 U	0.05 U 1.7 U	0.05 U	29.6	
Northwest Wall Southwest Wall	11/10-NW Wall 11/10-SW Wall	7 to 8	11/10/2000	1,250 128		_	1 U 0.134	0.195 U	0.05 U	0.945 U	=
Stratenrohe Inv	estigation Samples							•			
HCSP-04-01	HCSP-04-01 S-4	6.0 to 8.0	2/24/2005	21.6		_	0.030 U	0.050 U	0.050 U	0.100 U	4.15
				108		_	0.030 U	0.136 J	0.158 J	0.305 J	5.09
HCSP-04-01	HCSP-04-01 S-5	8.0 to 10.0			-				0.050 U	0.100 U	3.71
HCSP-04-02	HCSP-04-02 S-2	4.0 to 8.0	2/24/2005	23.1	-	-	0.030 U	0.050 U			
HCSP-04-02	HCSP-04-02 S-3	8.0 to 12.0		9.4	-		0.030 U	0.050 U	0.050 U	0.100 U	5.32
HCSP-04-03	HCSP-04-03 S-4	8.0 to 10.0		19.1	_	-	0.030 U	0.050 U	0.0717 J	0.132 J	10.5
HCSP-04-03	HCSP-04-03 S-5	10.0 to 12.	2/24/2005	5.0	-	-	0.030 U	0.050 U	0.050 U	0.100 U	3.54
HCSP-04-04	HCSP-04-04 S-4	8.0 to 10.0	2/24/2005	5.0 U	_	-	0.030 U	0.050 U	0.050 U	0.100 U	16.8
HCSP-04-04	HCSP-04-04 S-5	10.0 to 12.	2/24/2005	5.0 U	_	_	0.030 U	0.050 U	0.050 U	0.100 U	2.91
HCSP-04-05	HCSP-04-05 S-4	6.0 to 8.0	2/24/2005	108 J	-	_	0.030 U	0.109 J	0.432 J	0.400 J	5.30
HCSP-04-05	HCSP-04-05 S-5	8.0 to 10.0		116 J	_	_	0.030 U	0.134 J	0.563 J	0.522 J	5.77
HCSP-04-06	HCSP-04-06 S-4	6.0 to 8.0	2/24/2005	5 U	_	_	0.030 U	0,050 U	0.050 U	0.100 U	5.07
		8.0 to 10.0		64.7	_	_	0.030 U	0,123 J	0.482 J	0.548 J	7.92
HCSP-04-06	HCSP-04-06 S-5									0.100	8.76
HCSP-04-07	HCSP-04-07 S-3	4.0 to 7.0	2/24/2005	5 U	_	-	0.030 U	0.050 U	0.050 U		
HCSP-04-07	HCSP-04-07 S-4	8.0 to 11.0		49.0	-	_	0.030 U	0.050 U	0.109 J	0.163	2.95
HCSP-04-08	HCSP-04-08 S-3	6.0 to 8.0	2/24/2005	5.0 U	_	-	0.030 U	0.050 U	0.050 U	0.100 U	2.85
HCSP-04-08	HCSP-04-08 S-4	8.0 to 10.0	2/24/2005	16.4		-	0.030 U	0.050 U	0.0696 J	0.102 J	2.91
	Verification Sample	es							*		
East Pump											
Island		2.5 to 3.0	4/6/2005	5.0 U		-	0.030 U	0.050 U	0.050 U	0.100 U	-
West Pump											-
Island		3.0	4/6/2005	9.51 U	_		0.057 U	0.0951 U	0.0951 U	0.190 U	-
Fuel Line 1		3.0	4/6/2005	3.74 U		_	0.0225 U	0.0374 U	0.0374 U	0.0749 U	-
Fuel Line 2		3.0	4/6/2005	5.0 U	_	_	0.030 U	0.050 U	0.050 U	0.100 U	_
Fuel Line 3		14	4/6/2005	3.84 U	_	٠	0.023 U	0.0384 U	0.0384 U	0.0768 U	_
Bottom Tank 1		12	4/6/2005	5.0 U	_	_	0.030 U	0.050 U	0.050 U	0.100 U	_
					_	_	0.030 U	0.030 U	0.0437 U	0.0874 U	-
Bottom Tank 2		12	4/6/2005	16.5	-	-	0.0262 U				
Bottom Tank 3		12	4/6/2005	20.2	-	-	0.0264 U	0.0439 U	0.0439 U	0.148	
West Wall		4						0.000	0.000 1:	0.0770 **	
center		8 to 10	4/6/2005	6.43	_	-	0.0337	0.039 U	0.039 U	0.0779 U	_
South Wall		8	4/7/2005	2400	-	-	0.935	0.436 U	24.8	43.0	-
Pothole A		9	4/7/2005	162	_		0.180	0.0433 U	0.423	1.00	-
Pothole B		7	4/7/2005	490	-	_	0.697	0.0952	1.77	3.28	·
Pothole D		10	4/7/2005	16	_	_	0.030 U	0.050 U	0.050 U	0.100 U	-
Pothole E-6'		6	4/8/2005	705	_		1.42	0.0435 U	1.21	1.59	_
					-	-	0.555	0.0433	0.948	1.76	_
Pothole E-10		10	4/8/2005	346	_	-					
Pothole E-12		12	4/8/2005	65	-	-	0.0685	0.050 U	0.158	0.326	-
Pothole F-6'		6	4/8/2005	144	-	-	0.0358	0.050 U	0.0961	1.86	-
Pothole F-10		10	4/8/2005	36.7	-	-	0.030 U	0.050 U	0.050 U	0.242	-
	Cleanup Levels			100/30 °	2000	2000	0.03	7	. 6	9	250

TPH analyzed by EPA Method 8015 or WTPH-HCID for 1996 and 1997 samples and NWTPH-G for post-1997 samples. BTEX (Benzene, Toluene, Ethylbenzene, Xylenes) analyzed by EPA Method 8021B.

U Not detected above specified reporting limit.

Not analyzed.

*Cleanup level with/without benzene detected Bolded concentrations exceed MTCA Method A cleanup levels.

Table 4 - Summary of Groundwater Chemistry Data - TPH-G, BTEX, and Lead

!		SOLICE INCOME.		1				3	CELIDADO					
Well ID	Sampled	Cxygen	-errous tron	-	-	Benzene	Toluene	Eth	Ethylbenzene	ž	sues	Total	Lead	ss. Lead
MW-1	9661/8/9	1	1		160,000	2,500	19,000		3,000	7	90,		92	ŀ
	1/5/1998	ı	1		1	•	'						ı	ı
	4/6/1998	i	1		100.000	180	260		8		0.800		180	
	7/6/1998	` 1	,		93.000	7	2 5		92		200		3 5	l
	10/5/1998		1		1	: 1	}		3				}	}
	12/29/1999				21 600	87.4	47.7		1 2		2 00 5		ı	5
	3/21/2000	90	1		19.800	7	29.5		5		2740			1 4
	6/14/2000	-	ı		18,800	9	26.4	•	174		2,870			2 00
	9/12/2000	4.0	1		21,400	11	35.1		496		2 930		1	5.5
MW-14	1/30/2001	2.4		H	7,450	19.3	4.		424		673		,	,
(Replaces MW-1)	4/26/2001	ı	1		26.100	37.2	29.7	٠	280		2.680		ı	ı
	7/29/2001	2.3	ı		14,200	10,3	14.2		318		1.480		, ,1	ı
	10/27/2001	80	,		9.970	797	4.55		187		10.			
	11/15/2002	¦ 1	1		8,380	£	2.5	_	2		į		1	
	5/9/2003	1.2	ı		4.520	262	0.5	, 5	0.775		12		5.33	
	9/30/2003	5	4		F 230	11.7	181	, -	15.	_	9 9		8 4	1
	12/11/2003	3.5	} ₹		2 28 4	12.6	2	• =	2 5	, =	5 5	,	5.5	1
	37317004	5	. 5		270	9 5	3 4		3	,	7 2		* 70 7	ı
ē	50000	2 6	2.5		9,270	9.71		.	9.9		4.00		 G	ı
	007770	20.0	7 0		2,730	0.4	היי	> :	6.5	,		,	4.12	1
	#007/00/s	= 1	o (n/6	20.0	C.2	5	87.		8		4.29	ı
	12/14/2004	0.07	6.3		5,500	4.36	0.643		96.1		138		ı	ı
	4/4/2005	ı	4.82	_	8,100	6.89	0.746		75.8		ä		ı	ı
	10/6/2005	ı	9.74		4,070	7.85	0.5	Þ	43.1		62.8		3.7	ı
	6/28/2006	9.0		_	533	0.545	0.5	Þ	0.593		5,34		3.41	1
MW-2	4/8/1996			ŀ	OS	-	0		-	ļ	-	١	5 0	\
	1/5/1998	ı	ı	•	얆	-	- -	-	-	_	-	_	15	5 0
	4/6/1998	1	ı		98	-		-	-	_	-	_	5	. 1
	7/6/1998	1	ı		95	-		_	-	_	-	_	7	1
	10/5/1998	ı	1		200	-	-	Þ	-		_		ä	1
	12/29/1999	1	1		0.05	0.5	0 0 0	=	5.0	-	-	. =	; 1	1
	3/21/2000	26	1		205	0.5	0.0	=	0.5			· =	1	
	6/14/2000	2.8	•			50	0.5	=	35	,	341	,	۱ ۱	
	9/12/2000	8.0	1	_	200	0.5	U 0.5	- =	0.5	-	-	=	ı	
	1/30/2001	1.5		-	200	0.5	U 0.5		0.5	Ļ	-	_	,	-
	4/26/2001	4.5	ı	-	98	0.5	U 0.5	_	0.5	_	-	_		
	7/29/2001	3.3	ı		유	0.5	U 0.5	>	0.5	-	-	_	ı	ı
	10/27/2001	7	ı		얆	0.5	U 0.5	>	0.5	7	-	-	1	ı
	11/15/2002	1.5	1	_	유	0.5	U 0.5	>	0.5	_	-	_	ı	1
	5/9/2003	2.3	ı	_	⊃	0.5	U 0.5	>	0.5	_	-	5	ا د	. 1
	9/30/2003	1.51	1,2		⊃ S	0.5	U 0.5	>	0.5	_	-	_	2.61	ı
	12/11/2003	3.90	0.0		⊃ 83	0.5	U 0.5	>	0.5	_	-	_	∵	ı
	3/31/2004	0.82	0.0	_	13,000	5	U 119		竪		2,541	_	- -	ı
	6/2/2004	3.	0.0		1,480	2.10	0.5	>	0.5	_	1.0		- -	ı
	9/30/2004	0.52	0.2	_	1,290	2.40	0.5	>	0.859		5.11		- -	ı
	12/14/2004	6.05		_	요	0.5	U 0.5	_	0.5		-	5	ı	ı
	4/4/2005	ı		œ	Ē	0.5	U 0.5	-	0.5	_	-	_	ı	ı
	10/6/2005	1		_	160	0.741	0.5	>	0.5		-	_	-	,
	6/28/2006	ı	-	1	,				1				,	-
MTCA Method A Groundwater Cleanup Leve	indwater Clear	nup Level		4	1,000/800 -	2	1000		700	9	8	J	5	15

MIVE Sampled MIVE A 105/1968 115/1988 115/1988 115/1988 115/1988 115/1988 115/1988 115/1988 115/1988 115/1988 115/1988 115/1988 115/1988	0.59gen	Perrous Iron 1		Berizene 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	>>>>>>>>>>>>>>	1 U 1 U 1 U 1 U 1 U 1 U 1 U 1 U 1 U 1 U	2222222222	Total Lead Dis	28. Lead
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	2.1 2.1 2.1 2.1 2.1 2.1 3.2 3.2 4.4 3.2 3.2 4.4 3.2 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4			2	2000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	25 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			1111
	2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1			5 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	28.50 28.50 28.50 20.50	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			111
	1. 1. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2.			8 8 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	98 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9) S & &	11
	2.1. 2.1. 1.8 2.1. 2.1. 2.1. 3.20 1.58 0.54 2.10			2	2 2 3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			1
	2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 1.5 2.1 1.5 2.1 1.5 2.1 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1			95 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			
	2.2 1.4 1.8 2.3 2.3 3.2 2.1 0.38 0.38 0.38			95 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0.5 U			
	2.1 2.7 1.8 1.8 2.1 2.1 2.10 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5 1.5			0.5 C C C C C C C C C C C C C C C C C C C	285 20 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0.55 U 0.	22222		
	1.4 2.7 2.7 2.7 2.7 2.7 3.20 0.88 0.54 2.10 2.10			0.55 U 0.		0.55 U 0.55 U 0.55 U 0.55 U 0.55 U 0.55 U 0.55 U	2222	1 1 1	-
<u> </u>	2.7 2.3 2.4 2.4 3.20 0.88 0.88 0.88 0.89			0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2000 000 000 000 000 000 000 000 000 00	0.5 U			11111
	1.8 2.7 2.7 2.7 3.20 0.88 0.89 0.34 1.59			9.5 U U U U U U U U U U U U U U U U U U U		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	 	1 1	11111
	2.1 2.1 3.20 44,0 1.59 0.54 1.59 1.59 1.50			0.5 U		0 50 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-	1	1 1 1 1
<u> </u>	2.3 2.7 2.7 3.20 3.20 0.54 0.54 1.59			0.55 U 0.	0.55 U U U U U U U U U U U U U U U U U U	20 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			1 1 1
<u> </u>	2.1 2.7 2.7 3.20 0.89 0.54 0.54 1.0			0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5	-	1	1 1
<u> </u>	2.7 3.20 1.58 0.54 2.10			0.5 0.5 0.2 0.2 0.5 0.5 0.5 0.5	0.55 0.55 0.55 0.55 0.55 0.55 0.55 0.55	0.5 0.5 0.5 0.5 0.5 0.5	1 C	1	1
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	0.44 3.20 1.58 0.54 2.10			0.5 0.5 0.2 0.2 0.5 0.5	0.5 0.5 U	0.5 U 0.5 U 0.2 U	-	-	
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3/31/2004	0.11	5.4	267	29.0	1.43	-	2.94	2	1
6/2/2004	0.03	5.2	140	46.4	4.2	0.5 U	-	-	i
9/30/2004	90.0	3.8	88.7 J	0.5 U	0.5 U	1.83	→	-	1
-	0.12	2.0	⊃ 85	0.5 U	0.5 U	0.5 U	-	ı	ı
	ı	3.47	112	1.93	0.5 U	0.5 U	- ! - !	1 ;	ı
(Kepiaces MW-4) 10/6/2005	1 6	1.39	4 5	0.929	2.0	9.31	3.57	£ .	1
9/28/2006 U.B	9.0	0.25	0 06	0.5 0	0 6.0	0.5	-	-	'

Table 4 - Summary of Groundwater Chemistry Data - TPH-G, BTEX, and Lead
Concentration in mg/L
Well ID Sampled Oxygen Ferrus fron 1PH Benzene

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	Acceptance and Total	D. all bondone	OF Property AC.	14E -cine to 1000 .	-A	VETTOR O' DATE:	A Annah - and L. P.	A 14-14-1	•			

Gasoline-range TPH analyzed by EPA Method 8015 prior to 1999-after that, analyzed by NWTPH-G. BTEX Analyzed by EPA Method 8021B BTEX analyzed by EPA Method 8250B in March 2004. Total and Dissolved Lead analyzed by EPA Method 6010 or 6020.

Not analyzed.
 U = Not detected at specified reporting limit.

R = Rejected concentration.
Bolded concentration.
Bolded concentration.
Access to well MWH: 3 obstructed in November 2002 and May 2003.
Access to well MWH: 3 obstructed in September 2004.
Data from 1996 and 1998 collected by Sage Environmental.
Well MWH: I was removed during the October 2000 excavation. Well MWH: I wan Immany 2001 after the excavation.
Well MWH: was replaced as well MWHR by Hart Crowser in October 2005, following removal of the well during UST removal activities in April 2005.

Dashed line indicates date of excavation.

a) Cleanup level for withwithout detectable benzane
b) Values shown are the average of the results for the sample and its field duplicate.
c) The value is the result for the field duplicate. The result for the sample was ND (not detected at the detection limit of 1.0 µg/L).

Table 5 - Summary of Groundwater Chemistry Data - Other Detected Compounds

		Cor	centration in ug/	L/	Co	ncentration in mg	/L
Exploration	Date Sampled	MTBE	EDB	EDC	Nitrate	Nitrite	Sulfate
MW-1/MW-14	5/9/2003	2 U			~	-	-
	9/30/2003	20 U	10 U	10 U	0.349	0.200 U	0.400 U
	12/11/2003	3.65	1.00 U	1.00 U	0.200 U	0.200 U	1.14
	3/31/2004 6/2/2004	6.80 0.500 U	5.00 U 0.500 U	5.00 U 0.500 U	0.200 U 0.200 U	0.200 U 0.200 U	1.08 4.24
	9/30/2004	1 U .	1 U	1 U	0.200 U	0.200 U	0,635
	12/14/2004	-	-		0.200 U	0.200 U	0.400 U
	4/4/2005	-	-		0.200 U 0.200 U	0,200 U 0,200 U	0.484 0.400 U
MW-2	10/8/2005 5/9/2003	1 U			0.200 0	0.200 0	0.400 0
MVV-2	9/30/2003	0.200 U	0,200 U	0.200 U	0.489	0.200 U	3.38
	12/11/2003	0.500 U	0.200 U	0.200 U	1.08	0.200 U	3.79
	3/31/2004	5.00 U	10.00 U	10.00 U	0.912	0.200 U	4.60
	6/2/2004	0.500 U	0.500 U	0.500 U	0.487	0.200 U	3.23
	9/30/2004 12/14/2004	1 U	1 U 	1 U	0.443 0.922	0.200 U 0.200 U	2,93 3,05
	4/4/2005	-	-	-	0.719	0.200 U	3.52
I	10/6/2005	-	-	-	0.219	0.200 U	3.75
MW-3	5/9/2003	1 U .	-	-		-	
1	9/30/2003	0.200 U	0.200 U	0.200 U	0.228	0.200 U	4.39
	12/11/2003 3/31/2004	0.500 U 0,500 U	0.200 U 0.200 U	0,200 U 0,200 U	0.200 U 0.812	0.200 U 0.200 U	4.79 5.53
	6/2/2004	0,500 U	0.200 U	0.200 U	0.816	0.200 U	5.61
1	9/30/2004	1 U	1 U	1 U	0.253	0.200 U	4.43
1	12/14/2004	••	-	-	0.206	0.200 U	4.69
	4/4/2005 10/8/2005	-	-		0.358 0.200 U	0.200 U 0.200 U	4.23 3.67
MW-4	5/9/2003	5.89 J			0.200 U	0.200 0	5.07
MAA	9/30/2003	5.10	1.00 U	1.00 U	0.200 U	0.200 U	4.57
:	12/11/2003	1.80	0.200 U	0.200 U	1.05	0.200 U	15.3
	3/31/2004	31.60	1.00 U	1.00 U	0.200 U	0.200 U	7.41
	6/2/2004 9/30/2004	0.500 U 1 U	0.500 U 1 U	0.500 U 1 U	0.200 U 0.200 U	0.200 U 0.200 U	8.32 4.91
	12/14/2004	-		-	0.200 U	0.200 U	5.13
	4/4/2005	-			0.200 U	0.200 U	5.79
MW-4R	10/8/2005	· .	-	-	0.200 U	0.200 U	8.07
MW-5	5/9/2003	1.47 J				-	
	9/30/2003 12/11/2003	0.200 U 0.500 U	0.200 U 0.200 U	0,200 U 0,200 U	0.200 U 0.200 U	0.200 U 0.200 U	8.61 6.85
	3/31/2004	0.500 U	0.200 U	0.200 U	1.32	0.200 U	16.1
	8/2/2004	0.500 U	0.200 U	0.200 U	1.36	0.200 U	11.7
	12/14/2004	•-			0,200 U	0,200 U	7.57
	4/4/2005 10/6/2005	-		-	0.200 U 0.200 U	0.200 U 0.200 U	9.92 9.50
MW-6	5/9/2003	10.6 J					
	9/30/2003	13.2	1.00 U	1.00 U	0.200 U	0.200 U	0.400 U
	12/11/2003	0.500 U	0.200 U	0.200 U	0.200 U	0.200 U	0.685
	3/31/2004 6/2/2004	1.88 1.15	0.200 U 0.500 U	0.200 じ 0.500 U	0.200 U 0.200 U	0.200 U 0.200 U	3.02 0.557
	9/30/2004	1.10	1 U	1 U	0.200 U	0.200 U	0.400 U
	12/14/2004			1	0.200 U	0.200 U	0.400 ∪
	4/4/2005 b	-		-	0.200 U	0.200 U	3.19
	10/6/2005				0.200 U	0.200 U	0.400 U
MW-12	5/9/2003 9/30/2003	1 U 0.200 U	0.200 U	0,200 U	0.452	0,200 U	5.32
	12/11/2003	0.500 U	0.200 U	0.200 U	0.492 0.200 U	0.200 U	2.77
	3/31/2004	0.500 U	0.200 U	0.200 U	3.88	0.200 U	8.45
	8/2/2004	0.500 U	0.200 U	0.200 U	3.64	0.200 U	11.7
	9/30/2004 12/14/2004	1 U	1 U 	1 U	0.573 0.200 ∪	0.200 U 0,200 U	5.66 2.95
:	4/4/2005	_	-		0.200 U	0.200 U	3.32
i	10/12/2005			-	0.200 U	0.200 U	3.37
MW-13	9/30/2003	0,200 U	0.200 U	0.200 U	0.455	0.200 U	4.91
ı	12/11/2003	0.500 U	0.200 U	0.200 U	0.477	0.200 U	5.58
l	3/31/2004 6/2/2004	0.500 U 0.500 U	0.200 U 0.200 U	0.200 U 0.200 U	1,60 1.05	0.200 U 0.200 U	8.04 6,52
	9/30/2004	0.500 U	0.200 U	1 U	0.496	0.200 U	4.49
	12/14/2004			"	0.412	0.200 U	5.10
	4/4/2005	-		• ••	0.582	0.200 U	4.99
	10/6/2005	-			0.348	0.200 U	3.68
MW-15	5/9/2003	1 U	0.200.11	0.200 U	0.282	0.200 U	 5.02
	9/30/2003 12/11/2003	0,200 U . 0,500 U	0.200 U 0.200 U	0.200 U	0.282 0.415	0.200 U	5.02 8.52
l	3/31/2004	0.500 U	0.200 U	0.200 U	0.200 U	0.200 U	8.42
	6/2/2004	0.500 U	0.200 U	0.200 U	1.67	0.200 U	8.32
	9/30/2004	1 U	1 U	1 U	0.429	0.200 U	4.58
•	12/14/2004 4/4/2005	_	-		0.200 U 0.200 U	0.200 U 0.200 U	6.68 7.45
	10/6/2005	-	-		0.340	0.200 U	4.14
					1		
MTCA Method A							

MTBE, EDB, and EDC analyzed by EPA Method 8280B.
Nitrate, nitrite, and sulfate analyzed by EPA Method 300.0.

Not analyzed.
U = Not detected above specified reporting limit.
J = Estimated concentration.
Bolded concentrations exceed MTCA Method A cleanup levels.
a) MTCA Method B Cleanup Level. No Method A value available.
b) Values shown are the average of the results for the sample and its field duplicate.
na No MTCA Method A or B value available.

Table 6 - Summary of Groundwater Chemistry Data - Groundwater Grab Samples

			Con	Concentration in ug/L		
Exploration	Date Sampled	TPH-G	Benzene	Toluene	Ethylbenzene	Xylenes
		-	-			
Soil Borings						
HP-7	7/14/1999	167	0.5 U	49.7	2.73	11.7
HP-8	7/14/1999	14,100	27.9	50.8	458	804
HP-9	7/14/1999	920	1.28	91.8	16.1	40.9
HP-10	7/14/1999	232	0.968	9.48	0.5 U	2.54
HP-11	7/14/1999	420	1.06	71.6	10.6	31.6
HP-12	9/12/2000	13,600	95.1	12.2	404	1150
HP-13	9/12/2000	541	34.2	1.7	1.71	7.06
HP-14	9/12/2000	1,770	22.6	2.52	1.13	4.2
HP-15	9/12/2000	20 U	0.811	0.5 U	0.5 U	1 U
HP-16	9/12/2000	31,100	255	48.6	1,170	4,830
Strataprobe Explorations	plorations					
HCSP-04-01	2/24/05	12500	25.6	35.2	126	78.0
HCSP-04-02	2/24/05	814 J	1.38	0.50 U	2.00	3.40 J
HCSP-04-03	2/24/05	1220	4.76	0.559	6.91	8.10
HCSP-04-04	2/24/05	257	1.81	0.50 U	0.924	6.15
HCSP-04-05	2/24/05	1930 J	3.22	5.33 J	8.85	10.9 յ
HCSP-04-06	2/24/05	1630 յ	5.37	0.618	15.9	22.4
HCSP-04-07	2/24/05	1340 J	7.05	0.974	42.5	27.1
Trip Blank	2/24/05	20 U	0.50 U	0.50 U	0.50 U	1.00 U
MTCA Method	MTCA Method A Cleanup Levels	800	2	1000	200	1000

TPH analyzed by Ecology Method NWTPH-G.

BTEX (Benzene, Toluene, Ethylbenzene, Xylenes) analyzed by EPA Method 8021B.

U Not detected above specified reporting limit. Bolded values exceed MTCA Method A Cleanup Levels.

Table 7 - Measured Free Product Thickness in Well MW-1/MW-14

	Product Thickness in]
Date Measured	Well in Inches]
4/8/1996	0	
4/6/1998	6	1
10/5/1998	6	
12/29/1999	0.2	·
3/21/2000	5	
6/14/2000	1	
9/12/2000	11	- Hot Spot Excavation
1/30/2001	0	- Tiot oper Excavation
4/26/2001	0	
7/29/2001	0	
10/27/2001	4	
11/15/2002	3	·
5/9/2003	0	
9/30/2003	0	
12/12/2003	1	·
3/31/2004	1.80	
6/2/2004	0 -	•
9/30/2004	0	
12/14/2004	0.18	ORC Injection and UST Removal
4/4/2005	0	Torto injection and our removal
10/6/2005	0	
6/28/2006	0	

Table 8 - Summary of Cleanup Levels for Chemicals of Concern

	MTCA Method A Clean	up Level for Unrestricted Use
Constituent	Soil Cleanup Level in mg/kg	Groundwater Cleanup Level in ug/L
Gasoline-range Petroleum		
Hydrocarbons (TPH-G)	100/30 ^a	1,000/800 ^a
Benzene	0.03	5
Toluene	7	1,000
Ethylbenzene	6 ·	700
Xylenes	9	1,000
Lead	250	15

^a Upper concentration represents Method A cleanup level when benzene is not present, and total ethylbenzene, toluene, and xylenes are less than 1 percent of the total gasoline mixture.

Table 9 - Remedial Alternative Evaluation - Compliance with WAC 173-340-360

Evaluation Criterion	Alternative 1 Monitored Natural Attenuation	Alternative 2 Monitored Natural Attenuation and Passive Free Product Recovery	Alternative 3 Enhanced Bioremediation	Alternative 4 Air Sparging and Soli Vapor Extraction
Meets Definition of Permanent Cleanup Action	Yes	Yes	Yes	Yes
Protectiveness	Eliminates exposure pathways. Reduces soil and groundwater toxicity in the long term.	Eliminates exposure pathways. Reduces soil and groundwater toxicity In the long term.	Eliminates exposure pathways. Reduces soil and groundwater toxicity In the long term.	Eliminates exposure pathways. Reduces soil and groundwater toxicity in the long term.
Permanence	Natural attenuation will result in reduced soil and groundwater toxicity over the very long term.	Mobility and toxicity of contaminants will be reduced by collecting and properly disposing of free product. Natural attenuation will result in reduced soil and groundwater toxicity over the very long term.	Enhanced natural attenuation will result in reduced soil and groundwater toxicity over the long term.	Air sparging and soil vapor extraction will reduce contaminant mobility by removing and collecting or destroying contaminants from the subsurface. Natural attenuation will result in reduced soil and groundwater toxicity over the very long term for contaminants not removed by soil vapor extraction.
Estimated Cost ^a	\$325,000 to \$491,000	\$333,000 to \$505,000	\$485,000 to \$653,000	\$365,000 to \$420,000
Effectiveness over the Long Term and Restoration Time Frame	Will effectively remove contaminants over the long term. Estimated restoration time frame for groundwater, based on current trends, up to 25 years.	Will effectively remove contaminants over the long term. Estimated restoration time frame for groundwater, based on current trends, is 10 to 30 years.	Will effectively remove contaminants over the long term. Estimated restoration time frame for groundwater, based on current trends, is 10 to 15 years.	Will effectively remove contaminants over the long term. Estimated restoration time frame for groundwater, based on professional experience, is 5 to 7 years.
Management of Short-Term Risks	Protection monitoring will confirm protection of human health and the environment during site activities that may encounter contaminated materials.	Protection monitoring will confirm protection of human health and the environment during site activities that may encounter contaminated materials, such as free product removal.	Protection monitoring will confirm protection of human health and the environment during site activities that may encounter contaminated materials.	Protection monitoring will confirm protection of human health and the environment during site activities that may encounter contaminated materials such as construction of wells. Air monitoring will be performed during soi vapor extraction.
Technical and Administrative Implementability	Easily implemented.	Easily implemented.	Moderately easily implemented; however, injection of ORC beneath sidewalk will require street use permit; may not be able to gain access to East 10th Avenue.	Moderately easy implemented if adjacent property is available to stage equipment and treatment compound.

^a Cost estimate details provided in Appendix D.

Table 10 - Monitoring Schedule for Preferred Alternative

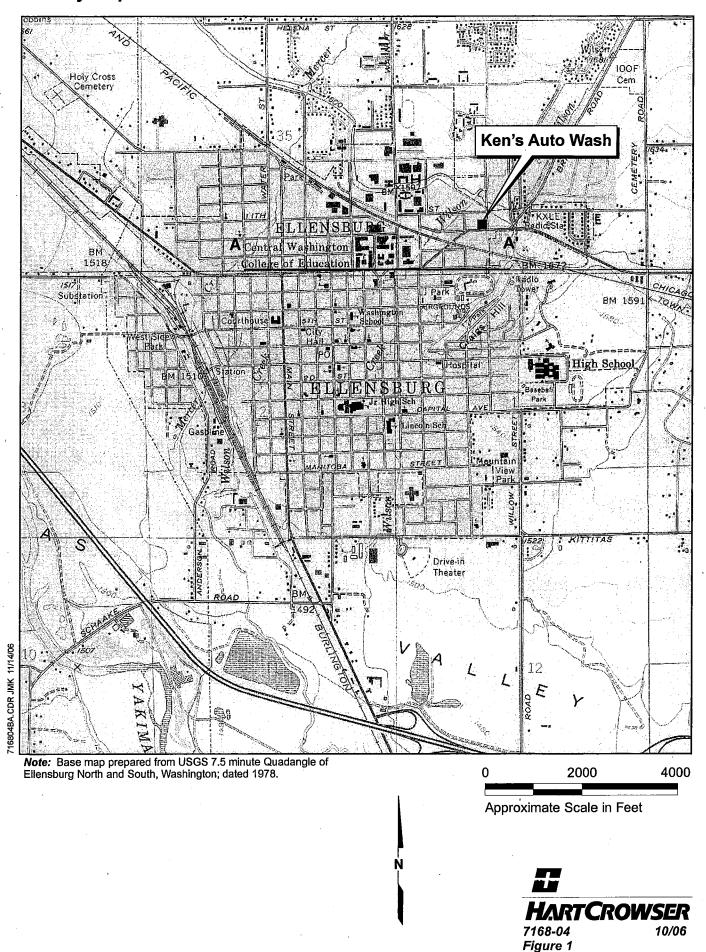
Well	Purpose	2003	2004	2005	2006	2007	2008 2009	2009	2010	2011	2012
	-									-	
MW-2	Bound Plume - East	Quarterly NA	Quarterly	Biannual		Quarterly NA					Quarterly NA
MW-3	Background	Quarterly NA	Quarterly	Biannual		Quarterly NA					Quarterly NA
MW-4	Source Area (Upgradient Edge)	Quarterly NA Quarterly	Quarterly	Biannual	Biannual	Biannual Quarterly NA Annual	Annual	Annual	Annual	Annual	Quarterly NA
MW-5	Bound Plume - West	Quarterly NA	Quarterly	Biannual	Biannual	Quarterly NA	Annual	Annual	Annual	Annual	Quarterly NA
MW-6	Plume Extent	. Quarterly NA	Quarterly	Biannual		Quarterly NA					Quarterly NA
MW-12	Bound Plume - Southwest	Quarterly NA	Quarterly	Biannual	Biannual	Quarterly NA Annual	Annual	Annual	Annual	Annual	Quarterly NA
MW-13	Bound Plume - South	Quarterly NA	Quarterly	Biannual		Quarterly NA					Quarterly NA
MW-14	Source Area	Quarterly NA	Quarterly	Biannual	Biannual	Quarterly NA Annual	Annual	Annual	Annual	Annual	Quarterly NA
MW-15	Bound Plume - Southeast	Quarterly NA	Quarterly	Biannual		Quarterly NA					Quarterly NA

Notes:

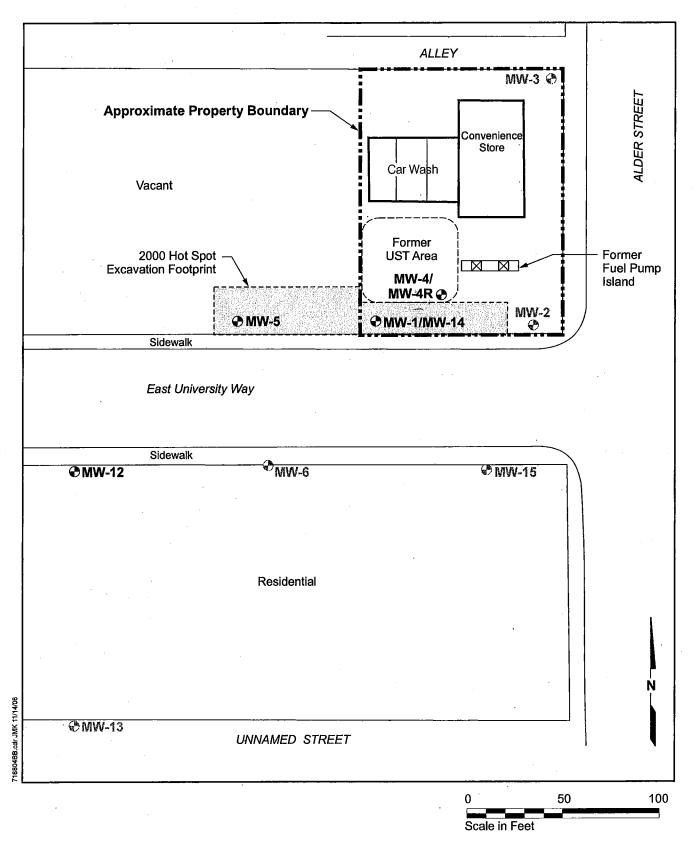
Monitoring will include measurement of groundwater elevation and dissolved oxygen and collection of a groundwater sample for analysis by NWTPH-Gx/BTEX. Biannual refers to twice per year. Biannual and annual monitoring schedules will be based on the dates of highest seasonal concentrations. First year of monitoring will include analysis for MTBE, EDB, and EDC by EPA Method 8260B and total Lead by EPA Method 6020.

Monitoring will also include collection of a groundwater sample for analysis for nitrate/nitrite, sulfate, and ferrous iron. Monitoring schedule after 2012, if necessary, will be based on review of previous data. Schedule assumes 5-year review by Ecology following 2007 sampling round. Schedule after 2007 is tentative pending Ecology 5-year review. Blank entries indicate no monitoring planned in specific wells. ž

Vicinity Map



Site and Well Location Plan

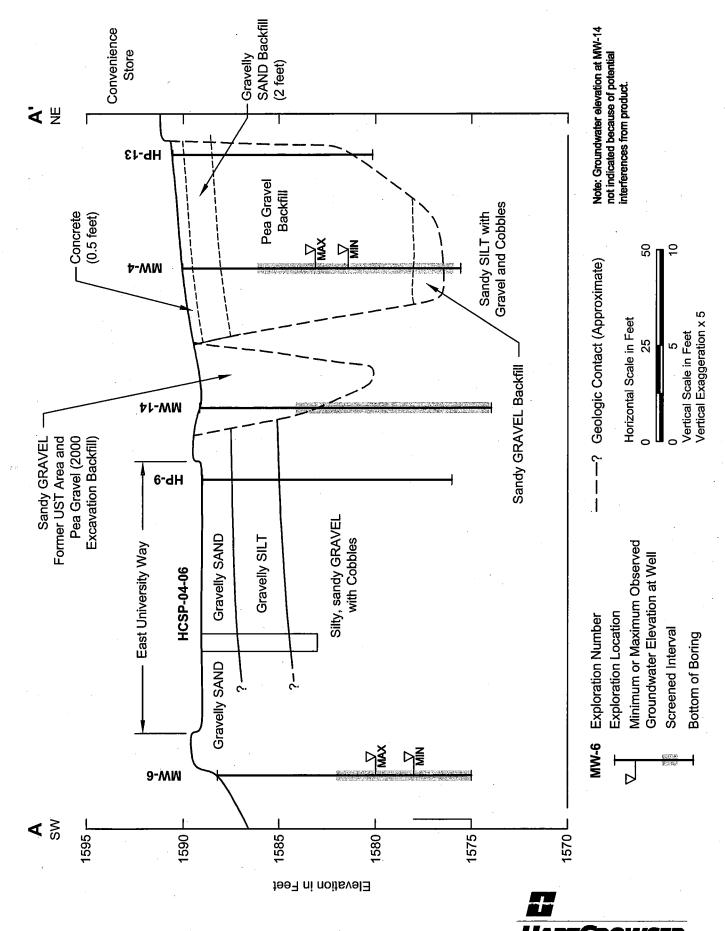


MW-3 Monitoring Well Location and Number
Not Sampled in this Study

Note: Base map prepared from drawing provided by Sage Earth Sciences titled "Proposed Additional Monitoring Well and ORC Injection Locations", dated January 1998.



Generalized Geologic Cross Section A-A'

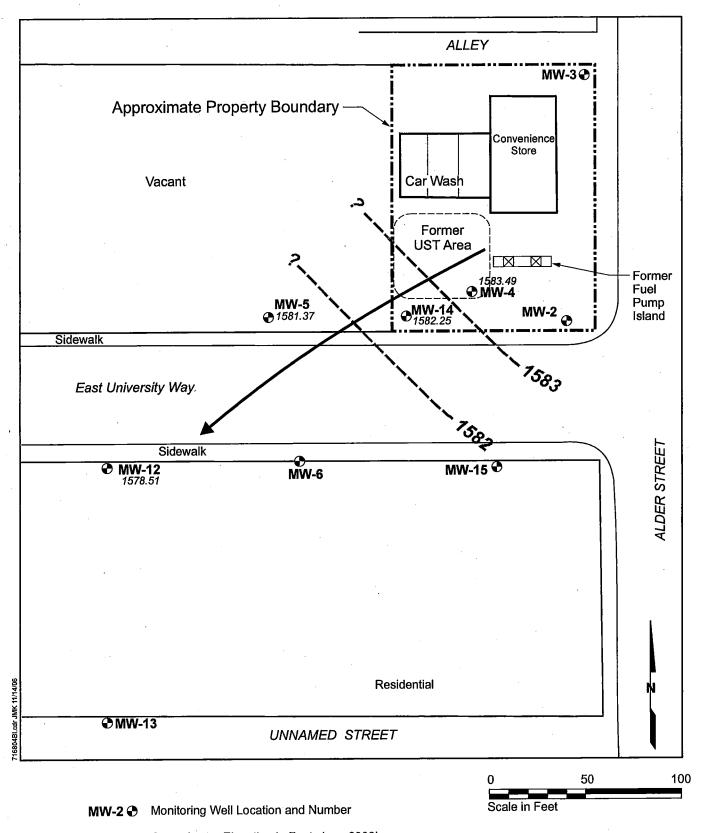


7163-04

10/06

Figure 3

Groundwater Elevation Contour Map June 2006



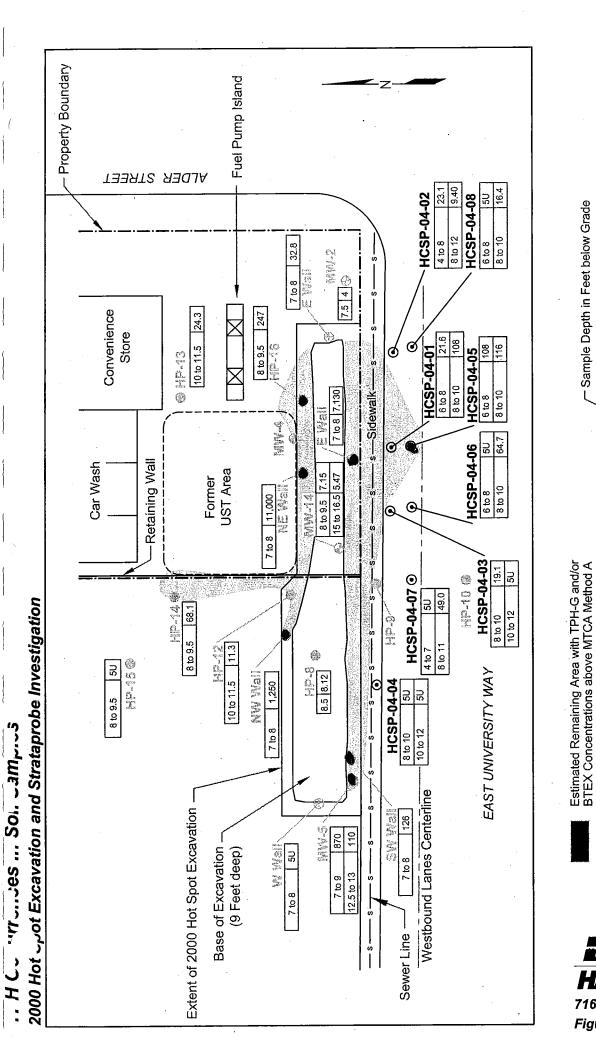
1578.28 Groundwater Elevation in Feet (June 2006)

1582 — Groundwater Elevation Contour in Feet

Inferred Groundwater Flow Direction

Note: Elevations shown are in feet above Mean Sea Level.





Sample Depth in Feet below Grade

7 to 9 | 8700 |

U Indicates Not Detected at Indicated Detection Limit

Soil TPH-G Concentration in mg/kg

8

Scale in Feet

Temporary Hydropunch Exploration Location and Number (Hart Crowser, July 1999 and September 2000)

Permanent Monitoring Well Location and Number (Sage, 1996 and 1998)

MW-4 @

Cleanup Levels

Figure 5

SW Wall ⊕ Excavation Side Wall Sample

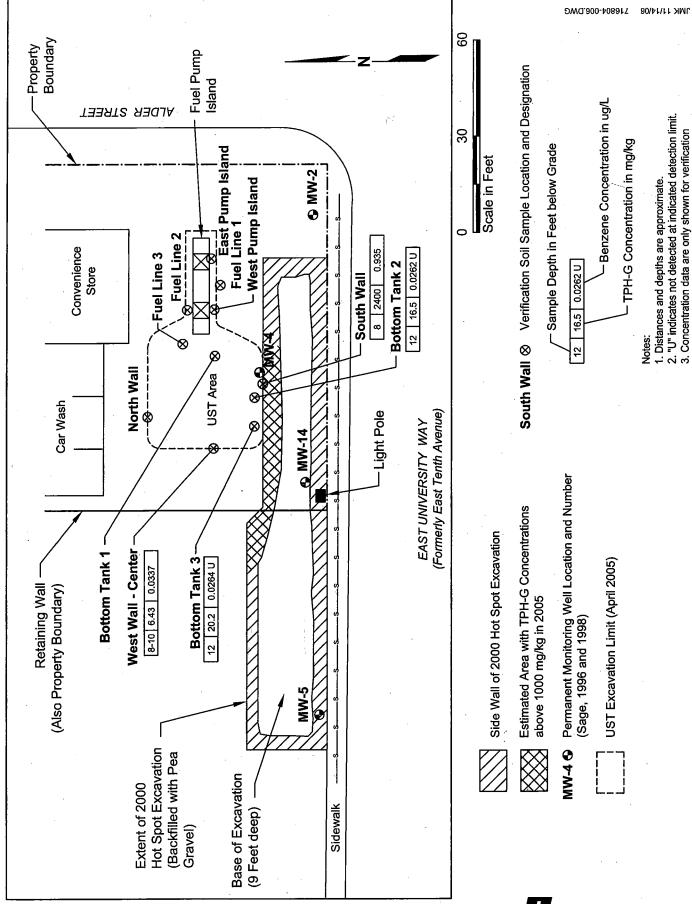
HCSP-04-01 © Strataprobe Location and Number (Hart Crowser 2005)

HARTCROWSER
7168-04 10/06

HP-14

samples with detected analytes.

UST Excavation Verification Sample Locations and Detected Analytical Results



WSER 10/06 7168-04

Figure 6

9

JMK 11/13/06 716804-007.dwg

3. Extent of remaining TPH-G-affected soil is based on results of current sample analyses (see Figures 3 and 4) and previous

investigations (samples not shown for clarity).

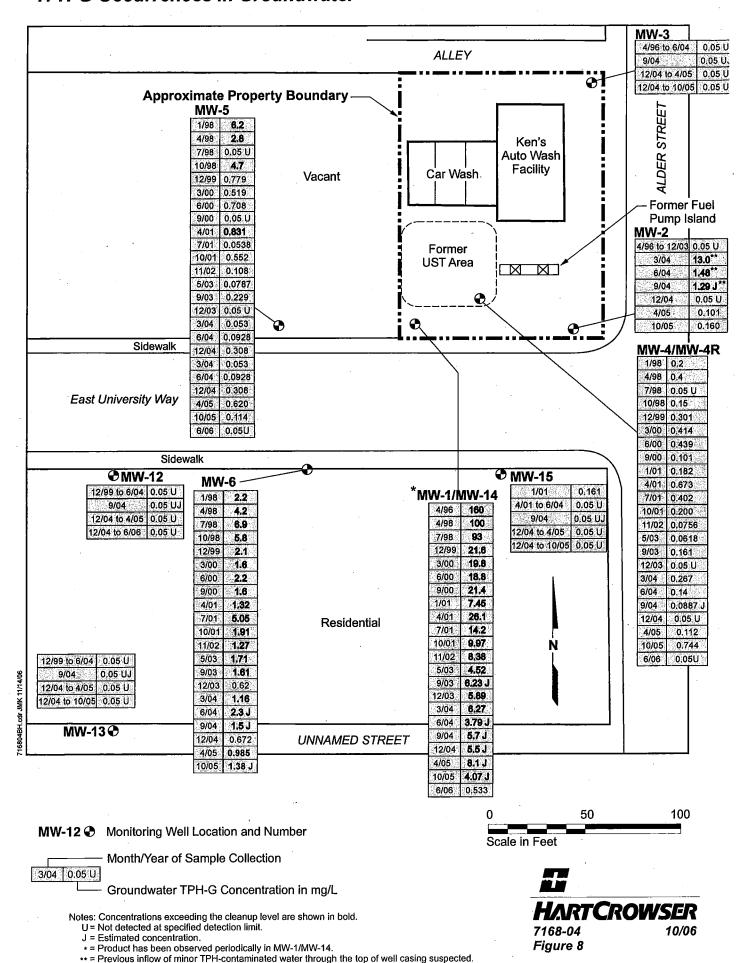
"U" indicates not detected at indicated detection limit.

UST Excavation Limit (April 2005)

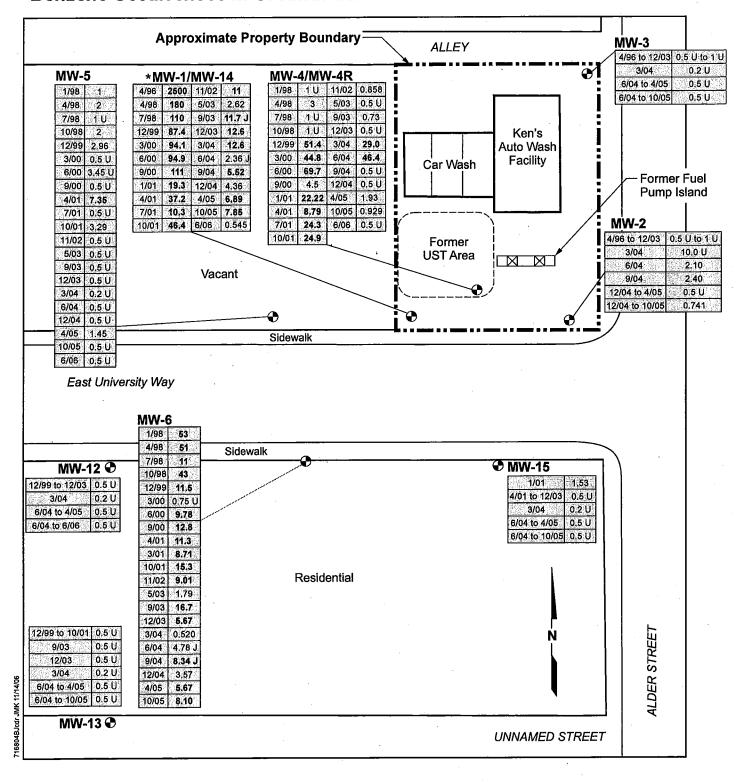
10/06

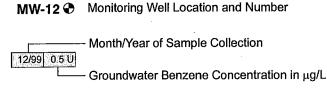
7168-04 Figure 7

TPH-G Occurrences in Groundwater



Benzene Occurrences in Groundwater





Notes: Concentrations exceeding the cleanup level are shown in bold.

U = Not detected at specified detection limit.

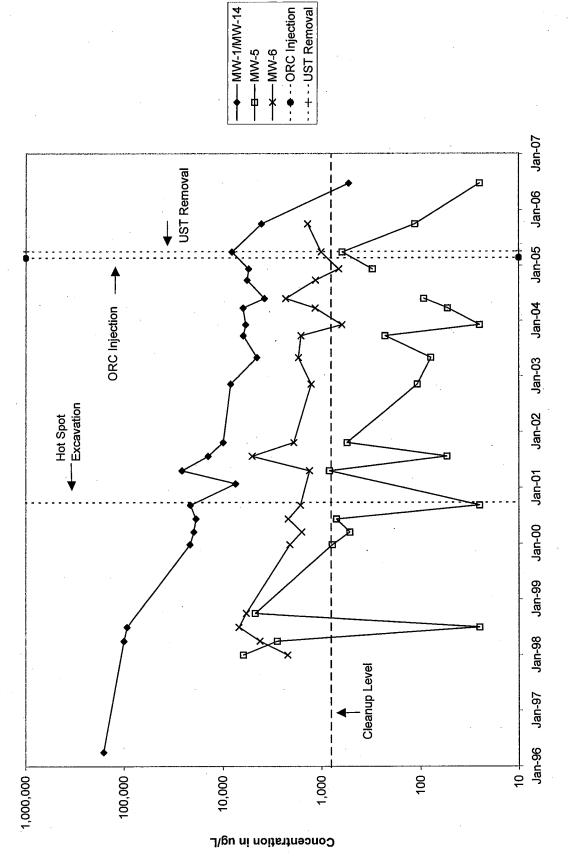
J = Estimated concentration.

* = Product has been observed periodically in MW-1/MW-14.

0 50 100 Scale in Feet

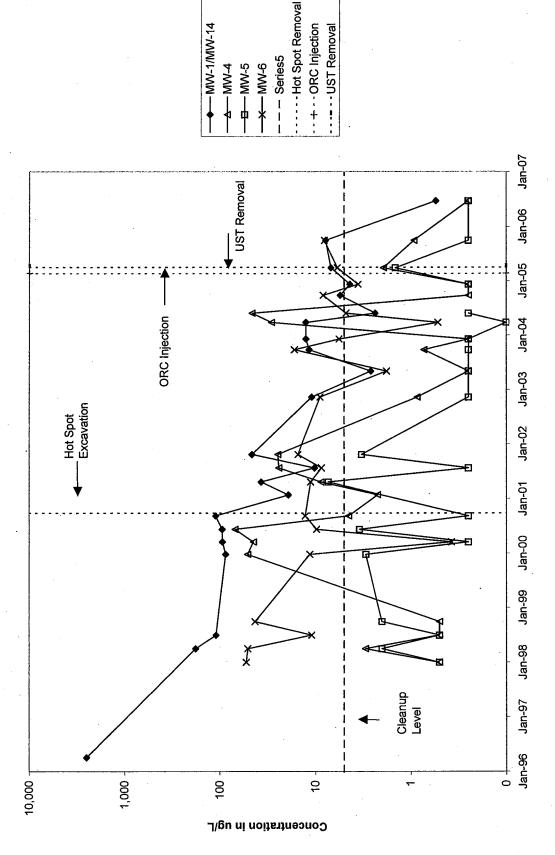


Long-Term Trends in TPH-G Concentration in Groundwater Ken's Auto Wash

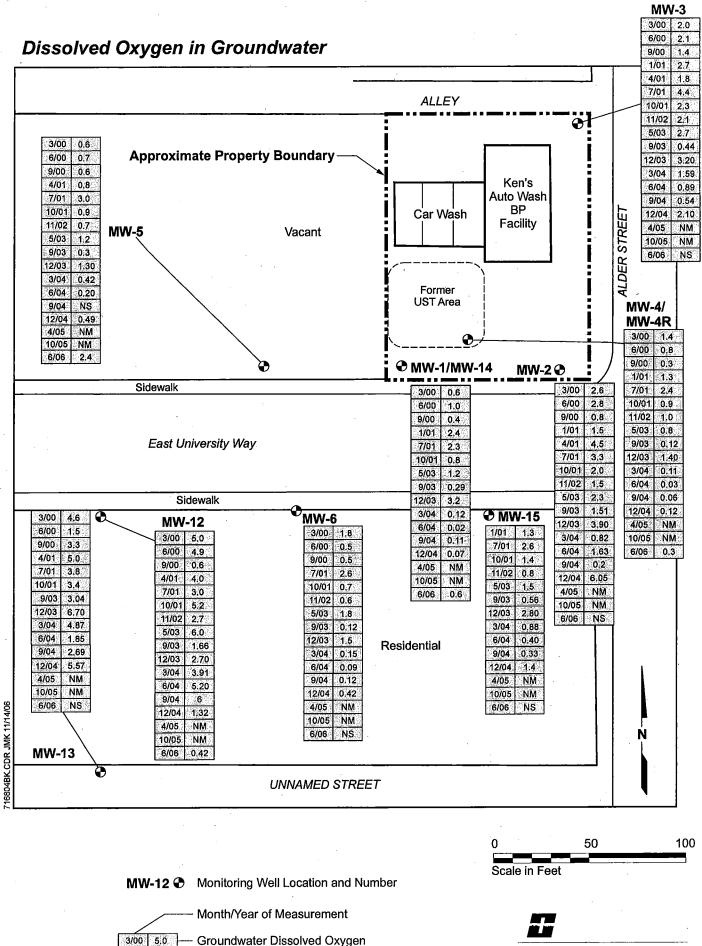




Long-Term Trends in Benzene Concentration in Groundwater Ken's Auto Wash







Concentration in mg/L

Well Not Sampled

Not Monitored

NM =

HART CROWSER
7168-04 10/06
Figure 12





February 24, 2003

Ancherage

Ms. Krystal Rodriguez
Washington State Department of Ecology
15 West Yakima Ave, Suite 200
Yakima, Washington 98902

Baston

Denver

Re: Request for Site Hazard Assessment Reranking

Ken's Auto Wash

Ellensburg, Washington

7168-03

Dear Krystal:

Frimonds

On the behalf of our client, Ken Peterson, we are requesting that the Washington State Department of Ecology (Ecology) rerank the Ken's Auto Wash site located at 1013 10th Avenue East in Ellensburg, Washington. This request is based on conversations with Michael Spencer of Ecology and data obtained by Hart Crowser since the original Site Hazard Assessment was performed in 1996. The collected information indicates that the original Site Hazard Ranking of "2" was based on an erroneous assumption (described below). We ask that the site be reranked using updated information in accordance with WAC 173-340-330.

Jarsey City

Eureka

Shallow groundwater at the Ken's Auto Wash site is affected by petroleum hydrocarbons related to a historical release from an underground storage tank, as described in previous reports (Sage 1998; Hart Crowser 1999 and 2002). According to Mr. Spencer, the original Site Hazard Assessment assumed that shallow site groundwater was hydraulically connected to the regional aquifer serving the City of Ellensburg drinking water supply. Two City of Ellensburg water supply wells are located approximately 1,200 and 1,600 feet south of the site. However, these wells are screened at depths greater than 230 and 480 feet below ground surface, respectively, whereas site wells exhibit contaminated groundwater at depths of 15 feet. Our research indicates that the shallow water-bearing zone at the Ken's Auto Wash site is not directly connected with the deeper water-bearing zones that supply the City of Ellensburg. Below we discuss the regional geology setting and hydrogeology, and the location of municipal wells relative to the Ken's Auto Wash site.

Juneau

Long Beach

Portland



Washington State Department of Ecology February 24, 2003

REGIONAL GEOLOGIC SETTING

Ellensburg is located at the northwestern extent of the Columbia Basin physiographic region just east of the Cascade Range. The upper 1,000 feet of stratigraphy in the area consist of fluvial deposits grading upward from coarser deposits at depth to shallower finer-grained deposits. We obtained well logs from Ecology for the Ellensburg area to provide a more focused look at the regional geology units in the vicinity of the site and the city wells. Based on these logs, we identified two primary stratigraphic units as follows:

- The Ellensburg Formation. The deepest unit recorded in the well logs is the Ellensburg Formation consisting mainly of sands and sandstones with interbedded gravel and clay (Bentley and Campbell 1985). These deposits, derived from the Cascade Range, are best characterized as alluvial fan deposits built out eastward from the Cascades and stream deposits (Orr and Orr 1996). On the west side of town, a cemented layer is recorded at approximately 300 feet below ground surface. Similar strata are deeper on the east side of town and are referred to as sandstones in the drillers' logs. We have interpreted these strata to be the Ellensburg Formation.
- Yakima River Deposits. Deposits overlying the Ellensburg Formation are typically finer-grained, consisting of clays and clay-bound gravels with interbedded sand and gravel layers. These deposits represent alluvial deposits from the Yakima River and associated side streams (Bentley and Campbell 1985).

LOCAL HYDROGEOLOGY

Water-bearing units occur in both the Ellensburg Formation and Yakima River Deposits discussed above. The sandstones and gravels in the Ellensburg Formation have been developed as a water resource for the City of Ellensburg. In the overlying Yakima River Deposits, the interbedded gravel and sand layers have some potential for water development but on a very localized scale. A more detailed discussion of the hydrogeology of Ellensburg is provided below with focus on the Ellensburg water supply wells and the Ken's Auto Wash property.

Ellensburg Water Supply Wells

The municipal water supply wells for the City of Ellensburg are deep wells ranging in total depth from 700 to 1,200 feet. Wells locations are illustrated on Figure 1, and well logs are provided in Attachment A. Two of the wells are located approximately 1,200 and 1,600



Washington State Department of Ecology February 24, 2003

feet south of Ken's Auto Wash. These wells were installed in 1946 and 1988. An east-west cross section along 8th Street is presented on Figure 2 to illustrate the municipal water levels relative to the site. A third municipal well installed in 1987 is located 4,000 feet west northwest of the site. Note that this well does not affect the Site Hazard Ranking because of its distance from the site, but it has been included in this discussion to provide a more complete understanding of the hydrogeology of the area.

The 1988 City well, located 1,200 feet south of the site, is screened from a depth of 488 to 757 feet, in the upper part of the Ellensburg Formation in intervals of sand and sandstone with some clay and gravel. Layers of clay and clay-bound gravels make up over 200 feet of the upper 400 feet overlying the screened interval. These fine-grained layers would act as a barrier to downward migration of contaminants. The final standing water level just after drilling was encountered at a depth of 29 feet.

The 1946 City well, located 1,600 feet to the south of the site, is screened at various intervals with the shallowest being at depths of 232 to 250 feet. Three clay units, 9, 25, and 10 feet in thickness, as well as more than 100 feet of clay-bound gravel, overlie this first screened interval. During drilling, the depth to water decreased with increasing depth, and flowing water was observed at a depth of 232 feet when the shallowest screen section was placed. Similar depth-to-water patterns were observed in deeper strata during drilling. This indicates that the water-bearing units served by the 1946 City well are confined and that a strong upward hydraulic gradient was present at the time of drilling.

The 1987 City well, located 4,000 feet to the northwest of the site, is also screened in the Ellensburg Formation from a depth of 410 to 595 feet. Layers of clay and clay-bound gravel make up over 280 feet of the upper 400 feet overlying the screened interval. The water level after drilling was at a depth of 39 feet.

GEOLOGY AND HYDROGEOLOGY OF THE KEN'S AUTO WASH SITE

Site soils are typically alluvially deposited silty, sandy gravels with cobbles (Hart Crowser 1999). These soils are consistent with those observed in the upper 32 feet of the 1988 City well. In addition, areas of silty sand and sandy silt fill have been observed at shallow depths typically less than 6 feet. The greatest depth at which soil affected by the petroleum release has been observed is 13 feet.

Site monitoring wells are screened above a depth of 16 feet with associated water levels ranging between depths of 6.5 and 9.5 feet. These wells are screened in a shallow



Washington State Department of Ecology February 24, 2003

7168-03 Page 4

unconfined sand and gravel unit. Groundwater elevations at the site have been shown to fluctuate nearly 2 feet seasonally (Hart Crowser 2002). The fact that these minor fluctuations correlate with seasonal precipitation, combined with knowledge of the local stratigraphy, suggests that the shallow groundwater unit is a perched on the underlying clay and clay-bound gravel unit. The depth to the uppermost confining unit at the site is not known but has been observed at a depth of 11 feet at the 1947 City well and a depth of 32 feet at the 1988 City well.

CONCLUSIONS

Evaluation of the water level data and the geology demonstrates that shallow groundwater at the Ken's Auto Wash site is not in direct connection with the water-bearing zones tapped by the City of Ellensburg for the following reasons:

- Substantial confining layers of clay and clay-bound gravels exist between the confined, deep water-bearing zones in the City wells, and the shallow, unconfined groundwater unit affected by releases at the site; and
- Upward hydraulic gradients prevent transport of contaminants from the shallower zone to the deeper zones.



Washington State Department of Ecology February 24, 2003

7168-03 Page 5

Based on these data, the assumption made during the 1997 hazard assessment that affected groundwater at the site could potentially migrate or otherwise affect the City of Ellensburg drinking water supply is erroneous. Therefore, we request that Ecology update the Site Hazard Ranking to reflect this information.

Sincerely,

HART CROWSER, INC.

Dara Cannon

DANA CANNON

Senior Staff Geologist

JEREMY PORTER

Remediation Engineer

Juny Porter

RICHARD F. MOORE, L. HYD.

Senior Associate Environmental Specialist

Attachments:

Figure 1 - Vicinity Map

Figure 2 - Generalized Geologic Cross Section A-A'

Attachment A - Well Logs

City of Ellensburg and Ken's Auto Wash

cc: Ken Peterson, Ken's Auto Wash

Steven Lathrop, Lathrop, Winbauer, Harrel, Slothower & Denison, LLP

Larry Daniels, AIG

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REFERENCES

Bentley, R.D. and N.D. Campbell 1983. Geologic Map of the Ellensburg Quadrangle, Washington: Geologic Map GM-28. State of Washington, Department of Natural Resources.

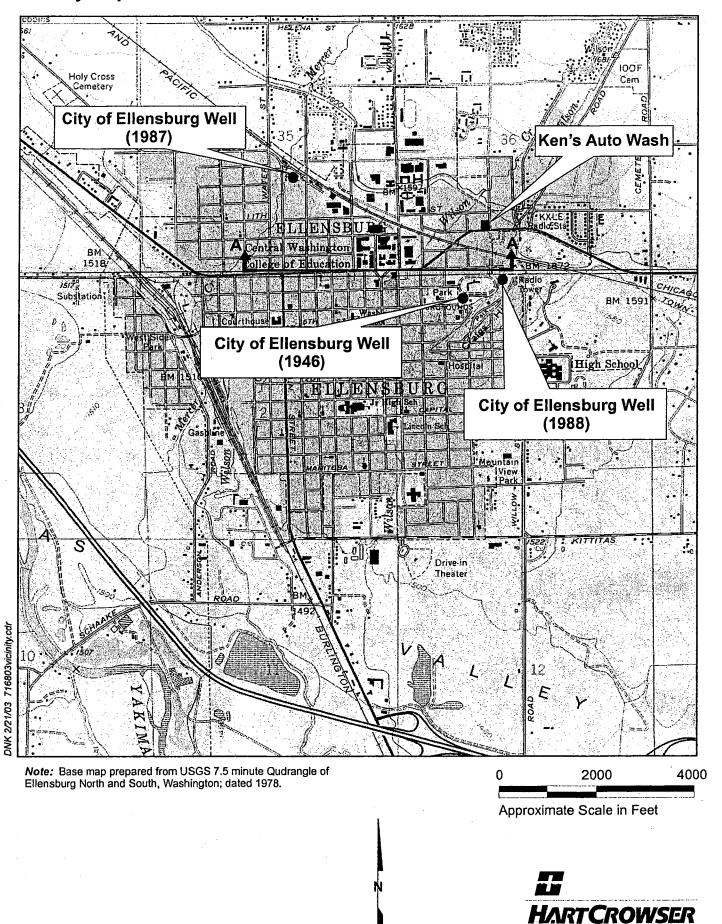
Hart Crowser 1999. Limited Phase II Environmental Assessment, Ken's Auto Wash Facility, Ellensburg, Washington. Prepared by Hart Crowser, Inc., November 29, 1999.

Hart Crowser 2002. Groundwater Monitoring Report and Evaluation of Remedial Alternatives. Ken's Auto Wash Facility, Ellensburg, Washington. Prepared by Hart Crowser, Inc., December 16, 2002.

Orr, E.L. and W.N. Orr 1996. Geology of the Pacific Northwest. McGraw - Hill Companies, Inc.

Sage 1998. Limited Site Characterization Report, Ken's Auto Wash Facility, Ellensburg, Washington. Prepared by Sage Earth Sciences, Inc., March 1998.

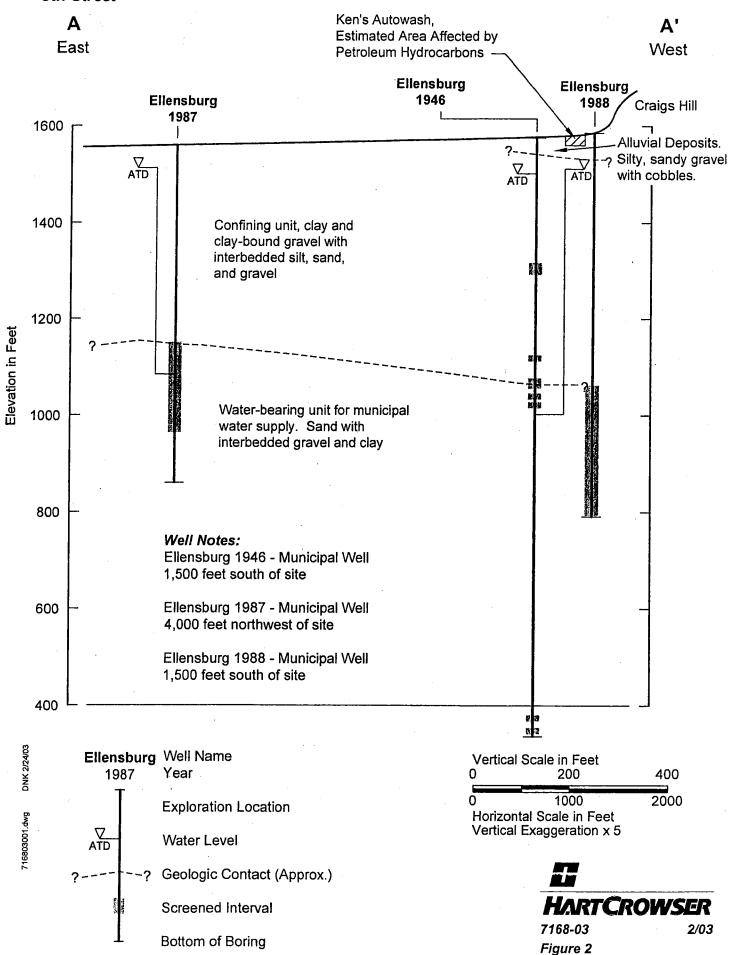
Vicinity Map



2/03

7168-03 Figure 1

Generalized Geologic Cross Section A-A' 8th Street



ATTACHMENT A
WELL LOGS
CITY OF ELLENSBURG AND KEN'S AUTO WASH

word!	TATE OF WASH	NSERVATION '	. (
•	AND DEVELOP		603
WELL LO	og , <u>19</u> 46	No.Appli/. # Cert. #8	16A
Date	Gerald C. Hoff		
Source			
		=	
	State of WASHINGTON		
-	y Kittitas	-	
	4 NW 4 sec. 1 T. 17 N., R. 18	E. DIAGRAM O	SECTION
-	0		·
	s		
	d of Drilling City of Ellensburg W		19
Owner	s 420 N. Pearl St.;	Ellenshurg	
	sce, datumft. above_below	BITCHOORIE	
Lana surja	ice, uatumtt. below		
CORRE- LATION	Material	THICKNESS (feet)	DEPTH (feet)
(Transcr material water surface datur ang log of ma	nbe duller's terminology literally but paraphra er-bearing, so state and record static level if repo m unless otherwise indicated. Correlate with stra iterials, list all casings, perforations, screens, etc.	se as necessary, in parted Give depths in le tugraphic column, if fea	rentheses. If et below land- sible Pollow-
	See attached sheets	·	
		· · · · ·	
		i	
Turn up		Sheetof	sheets

1 12 J

LOG OF ELLENSBURG WELL #4

Depth	Description	Comme	nts
0 - 2' 8 - 10' 1 - 11' 9 - 20' 7 - 27' 41 - 68' 25 - 93'	Loam Gravel and round rock Coarse sand and gravel Yellow clay Fine gravel Clay, sand and fine gravel Clay	S.W.L.	12 7'6"
5 - 5! 22 - 120! 50 - 170!	Clay and fine gravel Gravel, rock and clay Sand, gravel, rocks and clay		
13 - 183' 39 - 222'	Clay and some sand Gravel and clay Clay	11	41 -
10 - 232' 35 - 267'	Gravel, sand, and cement gravel	н	3016" - 941
24 - 291' 7 - 298'	Clay and gravel Gravel	11	601
26 - 324	Sand and clay		201
13 - 337'	Sand, gravel and clay	11	30'
10 - 347'	Clay and gravel	11	261
1 - 348'	Fine gravel and sand		
21 - 369'	Fine gravel, course sand, and clay		
	Clay, fine gravel, and sand stone	77	15'-43'
	Sandy clay, pea gravel, gravel & sand		•
10 - 403	Sandy Clay, pea graver, graver w bund	11	901
21 - 424	Clay and sand stone	77	401-431
7 - 431'	Gravel and fine sand	11	481-851
34 - 465'	Clay, fine gravel, sand		
17 - 4821	Sand, gravel and clay	11	651
21 - 503'	Clay		
8 - 511'	Clay, fine sand, and gravel	77	4019"-601
13 - 524'	Clay and sand	#1	431
	Gravel, sand, and clay	#1	521-331
26 - 550'	Clar and cond	11	401
44 - 594	Clay and sand		
6 - 600'	Gravel	· 11	851-811
45 - 645!	Clay and sandy clay	¥T	721-801
29 - 674 1	Sand and fine sand	77	801
16 - 690'	Sand, gravel, and clay		
24 - 714!	Sandy clay	17	841
2 - 716'	Sand		
38 - 754'	Sandy clay	71	831
3 - 7571	Gravel and sand	11	851-1001
12 - 7691	Sand and clay		
3 2001	Sand and gravel		
	Sand and heaving sand	. 11	107'-100'
10 - 780'		71	881-941
14 - 794!	Sand and gravel	71	83'-105'
55 - 849! 6 - 855!	Clay, sand and gravel		0) -10)
6 - 8551	Heaving sand		100100
6 - 861'	Sandy clay	T1	105'9"
4 - 8651	Red sand	Ħ	105'
71 - 9361	Clay and sand and gravel clay mix	*1	981
5 - 941'	Clay		•

7"1- 942' Quick sand heaving badly 27 - 969' Clay, sand, and gumbo 3 - 976' Sand heaving 41 - 1017' Clay, blue mud, and gumbo 5.W.L. 90'-130' 7 - 1024' Fine sand, D.O.I. " 100' 16 - 1040' Blue clay " 97' 6 - 1046' Coarse gravel 18 - 1064' Fine sand 32 - 1100' Blue clay and blue mud " 110'-98' 53 - 1153' Clay " 98'-102' 3 - 1156' Sand 14 - 1170' Clay and blue clay " 96', Shut down from Aug. 14th to Sept. 15th. Showed S.W.L. 35'8" on Sept. 15th. 3 - 1173' Sand and gravel S.W.L. 31'5" 4 - 1177' Sand 1 - 1178' Blue shale 17 - 1195' Green and greenish gray mud 10 - 1205' Sand and gravel 57-1209'	<u>Depth</u>	Description	Comments
## 1017' Clay, blue mud, and gumbo 7 - 1024' Fine sand, D.O.I. 16 - 1040' Blue clay 6 - 1046' Coarse gravel 18 - 1064' Blue clay 4 - 1068' Fine sand 32 - 1100' Blue clay and blue mud 53 - 1153' Clay 3 - 1156' Sand 14 - 1170' Clay and blue clay ## 98' - 102' ## 98' - 102' ## 98' - 102' ## 98' - 102' ## 96' Shut ## down from Aug. 14th ## to Sept. 15th. Showed ## S.W.L. 35'8" on Sept. ## 1177' Sand ## 31'5" - 28' ## 1178' Blue shale ## 17 - 1195' Green and greenish gray mud ## 25' - 35' ## 41' - 45' On Oct. 10th, after fishing out tools S.W.L. ## 29'6"	27 - 9691	Clay, sand, and gumbo	
7 - 1024' Fine sand, D.O.I. 16 - 1040' Blue clay 6 - 1046' Coarse gravel 18 - 1064' Blue clay 4 - 1068' Fine sand 32 - 1100' Blue clay and blue mud 32 - 1156' Sand 14 - 1170' Clay and blue clay 3 - 1156' Sand 14 - 1170' Clay and blue clay 3 - 1173' Sand and gravel 4 - 1177' Sand 1 - 1178' Blue shale 17 - 1195' Green and greenish gray mud 10 - 1205' Sand and gravel 57-1209'			S-W-L. 90'-130'
16 - 1040! Blue clay " 97! 6 - 1046! Coarse gravel 18 - 1064! Blue clay " 105!-112! 4 - 1068! Fine sand 32 - 1100! Blue clay and blue mud " 110!-98! 53 - 1153! Clay " 98!-102! 3 - 1156! Sand " 98! 14 - 1170! Clay and blue clay " 96'. Shut down from Aug. 14th to Sept. 15th. Showed S.W.L. 35'8" on Sept. 15th. 3 - 1173! Sand and gravel S.W.L. 31'5" 4 - 1177! Sand " 31'5"-28! 1 - 1178! Blue shale 17 - 1195! Green and greenish gray mud " 25!-35! " 41'-45'. On Oct. 10th, after fishing out tools S.W.L. 5"-1209!		Fine sand, D.O.T.	
# 1064' # 1068' Fine sand 32 - 1100'	16 - 1040!	Blue clay	
32 - 1100' Blue clay and blue mud " 110'-98' 53 - 1153' Clay " 98'-102' 3 - 1156' Sand " 98' 14 - 1170' Clay and blue clay " 96'. Shut down from Aug. 14th to Sept. 15th. Showed S.W.L. 35'8" on Sept. 15th. 3 - 1173' Sand and gravel S.W.L. 31'5" 4 - 1177' Sand 1 - 1178' Blue shale 17 - 1195' Green and greenish gray mud 10 - 1205' Sand and gravel " 25'-35' Sand and gravel " 41'-45'. On Oct. 10th, after fish- ing out tools S.W.L. 29'6"	18 - 1064'	Blue clay	" 105'-112'
53 - 1153' 3 - 1156' 14 - 1170' Clay and blue clay Clay and blue clay " 98' " 96'. Shut down from Aug. 14th to Sept. 15th. Showed S.W.L. 35'8" on Sept. 15th. 3 - 1173' 4 - 1177' Sand 1 - 1178' Blue shale 17 - 1195' Green and greenish gray mud 10 - 1205' Sand and gravel " 25'-35' " 41'-45'. On Oct. 10th, after fishing out tools S.W.L. 29'6"			" 110'-98'
3 - 1156' 14 - 1170' Clay and blue clay " 96'. Shut down from Aug. 14th to Sept. 15th. Showed S.W.L. 35'8" on Sept. 15th. 3 - 1173' Sand and gravel S.W.L. 31'5" 1 - 1178' Blue shale 17 - 1195' Green and greenish gray mud To - 1205' Sand and gravel " 25'-35' " 41'-45'. On Oct. 10th, after fishing out tools S.W.L. 29'6"			
# 96'. Shut down from Aug. 14th to Sept. 15th. Showed S.W.L. 35'8" on Sept. 15th. 3 - 1173' 4 - 1177' 5 and 1 - 1178' Blue shale 17 - 1195' Green and greenish gray mud 10 - 1205' Sand and gravel 5"-1209'			
3 - 1173' Sand and gravel S.W.L. 31'5" 4 - 1177' Sand 1 - 1178' Blue shale 17 - 1195' Green and greenish gray mud 10 - 1205' Sand and gravel 5"-1209' S.W.L. 31'5" " 31'5"-28' " 25'-35' " 41'-45'. On Oct. 10th, after fishing out tools S.W.L. 29'6"			<pre>" 96'. Shut down from Aug. 14th to Sept. 15th. Showed S.W.L. 35'8" on Sept.</pre>
4 - 1177' 1 - 1178' 1 - 1195' 10 - 1205' Sand and gravel 5"-1209' Sand " 31'5"-28' " 25'-35' " 41'-45'. On Oct. 10th, after fishing out tools S.W.L. 29'6"	3 - 11731	Sand and gravel	
10 = 1205' Sand and gravel " 41'-45'. On Oct. 10th, after fishing out tools S.W.L. 29'6"	4 - 1177'	Sand	
Oct. 10th, after fishing out tools S.W.L. 29'6"			
		Sand and gravel	Oct. 10th, after fish- ing out tools S.W.L.
Shale and greenish gray mud S.W.L. 29'6"-31'	, 6m	Shale and greenish gray mud	S.W.L. 29'6"-31'

ELLENSBURG WELL NO. 4

PERFORATIONS

4 rows of $3/8^n \times 1-1/8^n$ perforations

Depth	Depth	REMARKS
* 232	250	Very good water flow
424	431	Fair water flow
470	475	Poor to fair
477	482	Poor to fair
507	509	No good
524	529	Fair
1170	1177	Light flow
1195	1205	Very little water

*Well flowed when perforated here but quit flowing when perforating was continued.

37' of 20" conductor pipe left in hole.

15 yards of gravel were fed between the 16" and 20" casing Level of gravel was left at 175'.

Pumped 1000 g.p.m. for 24 hrs. Would break suction at 1100 g.p.m. with 195' pump setting. Pumped 3 to 4 yds. of sand. Temperature of water 542 degrees.

Gerald C. Hoff

λò	The Original and Pirst Copy with
~	Department of Ecology
,	Pagend Conv. Opening Conv.
	Second Copy - Owner's Copy
	Third Copy - Driller's Copy

WATER WELL REPORT STATE OF WASHINGTON

Application No

Permit !	No	 	 	
		 	 _	_

1) OWNER: Name CITY OF EllENSBURG	Address Ellens Burg WA.		
(2) LOCATION OF WELL: County KITITAS	- NE & NW & Sec / T/	7n., R.	18 WM
ring and distance from section or subdivision corner			
(a) PROPOSED USE: Domestic Industrial Municipal	(10) WELL LOG:		
Irrigation Test Well Other	Formation: Describe by color, character, size of material show thickness of squifers and the kind and nature of stratum penetrated, with at least one entry for each c	il and stru	icture, and
(4) TYPE OF WORK: Owner's number of well	stratum peneirated, with at least one entry for each c	hange of	formation.
New well Method: Dug Bored	MATERIAL	PROM	70
Deepened 🖸 Cable 🕱 Driven 🖸	SAND, GRAVEL, AND BOWLACKS	16	32
Reconditioned Rotary Jetted	SAND, GRAVEL, AND COMPLES	32	42
(5) DIMENSIONS: Diameter of well 20 inches.	GRAVEL TO 172" WATER BEANING	42	47
Drilled 76/. It. Depth of completed well 76/	BROWN CLAY ROUND GRAVEL	47	8.9
(6) CONSTRUCTION DETAILS:	AROUN SILTY SAND	89	9/
	6 RAVEL	9/	93.
Casing installed: 20 plam. from 1/5 ft. to 5.7.3. ft. Threaded 1 1/2" Diam. from 1/68. ft. to 2.5.7. ft.	BROWN CLAY-BOUND GRAVEL	93	127
Welded W	BROWN SAMA AND GRAVER WITH CLAY	122	120
Desferations >/	BROWN CLAY AND GRAVEL	720	243
Perforations: Yes No TOR CH.	CREAM BROWN CLAY	228	247
SIZE of perforations in. by in.	BROWN SAND AND GAZULL WITH CLAY	247	3/4
5/4. perforations from 488. ft. to2.5.2 ft.	BROWN CLAY	314	316
perforations from	BROWN CLAY-ROUND GRAVEL		367
	BROWN CLAY	362	329
Screens: Yes 🗆 No 🖸	GRAVEL WITH SOME CLAY AND SAND	329	385
Manufacturer's Name	BROWN CLAY	385	387
Diam. Slot site from ft. to ft.	AROUN CLAY BOLWA GRAVEL	382	1475
Diam. Slot size from ft. to ft.	BROWN SAND WITH SOME GRAYEL	475	727
Gravel packed: Yes No D Size of gravel. 74 Minus	BROWN CLAY WITH SOME SAND BROWN SAND AND BRAVEL TO I"	445	445
Gravel placed from 47.0	BROWN CLAY-BOUND BRAVEL	449	487
	BROWN SILVY SANCE	482	480
Surface seal: Yes No To what depth? ~ 20 ft. Material used in seal CONCARTE	BROWN CLAY WITH SAND SOMEGUE		520
Did any strata contain unusable water? Yes No	BROWN CLAY	520	585
Type of water? SURFACE Depth of strata	BROWN CLAY-BOWN GRAVEL	545	523
Method of sealing strate offCONCHETE	WEATHERED GRANTE AND CLAY	23.3	202
(7) PUMP: Manufacturer's Name. NA	BROWN SILTY SANDSTONE	187	636
Туре: НЪ-	GRAY SWAY CLAY WITH ANK OKNIK		663
(8) WATER LEVELS: Land-surface elevation 1550	BADWA SIETY SANDSTONE WITH CLAY + GALL BROWN SANSTONE WITH YOU CAN'T ASH	663	665
Static level 28.75 . ft. below top of well Date 4-26-38.	GROWN SILTY SANOSTONE SOME GYL	665	233
Artesian pressure libe per square inch Date	AROUNS:LTY SMOSTONE AND GENEL	A	242
Artesian water is controlled by (Cap, valve, etc.)	GRAY SILTY SAND WITH GRAVEL	147	259
(6) WELL TECTS. Drawdown is amount water level is	BINK BROWN SANDY CLAY	1257	26.L+
(5) WEAR IEDID. , lowered below matic level _	Work started 19436 1987 Completed 5/	// f	1982
Was a pump test made? Yes No If yes, hy whom? BACH Yield: 1200 gal/min. with 116 ft. drawdown after 10 Air 2015.	WELL DRILLER'S STATEMENT:	•	
/200 /49 / /8	This well was drilled under my jurisdiction		
1200 165	true to the best of my knowledge and belief	TE	REI
Recovery data (time taken as zero when pump turned off) (water level measured from well top to water level)	DACH LIEU DAYLA	16	
Time Water Level Time Water Level Time Water Level	(Person, firm, or corporation)	Type or p	orint)
SM:N 71 104 35	0 0		_
10Ain 65	Address Rt. 5 Box 1010, Ell	يعويم	
7 Aid 50	一点原图	1 3	n II
Date of teet	[Signed] Buck SA THE (Well Driller)		
rest gai /min. with ft. drawdown after		92 11	مم الا
Temperature of water 62 f. Was a chemical analysis made? Yes No []	License No. 4.5.5.5 Date	× +	, JO. KA
			!
USE ADDITIONAL #	DEPARTMENT OF E	OFFICE.	2
A A COMPANY OF THE PROPERTY OF	CEMILIAN OF COLOR		

WATER WELL REPORT STATE OF WASHINGTON

Application No. G4-29206

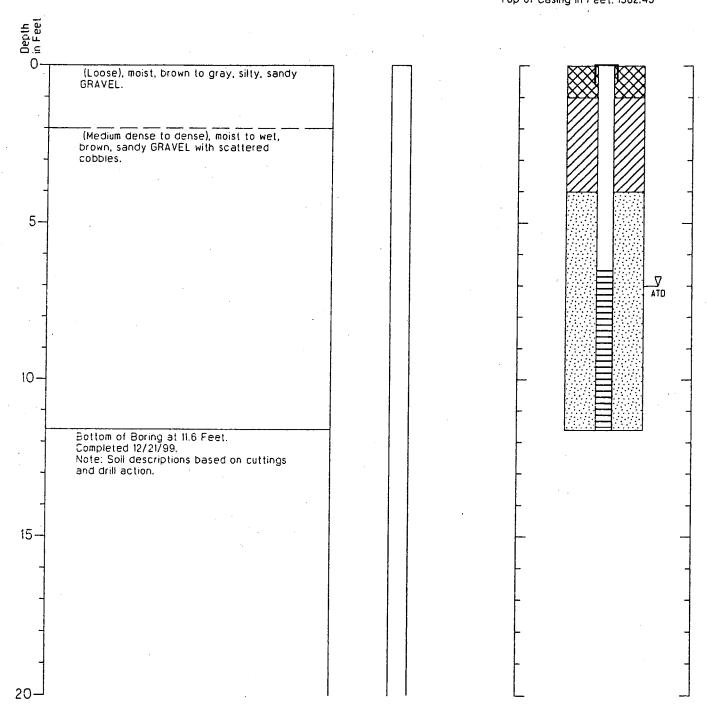
Permit No .

(1) OWNER: Name City OF Ellens BURG	Addres Ellens BURG, WA.	<u> 98</u>	726
OCATION OF WELL: County KITTITAS		8 N. R.	18.E W.M.
Braiding and distance from section or subdivision corner			• •
(3) PEOPOSED USE: Domestic Industrial Municipal	(10) WELL LOG:		
Irrigation Test Well Other	Formation: Describe by color, character, size of materia show thickness of aquifers and the kind and nature of a stratum penetrated, with at least one entry for each c	i and stru he materi hange of	cture, and ial in each formation.
(4) TYPE OF WORK: Charles's number of well it more than one;	MATERIAL	FROM	TO
New well Method: Dug Bored	S. /ty GRAY CLAY	0	13
Deepened	TAN CLAYBOUND GRAVEL.	/3	126
	TAN CLAY	126	128
(5) DIMENSIONS: Diameter of well 20 inches.	TAN SANDY CLAY WITH SMALL CAME	128	148
Drilled 700 It Depth of completed well 6/7	BROWN SAND AND GRAVEL WITH CAY	148	Þ
(6) CONSTRUCTION DETAILS:	LAYENS	<	164
Casing installed: 20 " Diam. from +2. n. to 575 n.	BROWN CLAY WITH HEXCANIC ASIL,	164	Z
Threaded	GRANULES + PUTICE		184
Welded " Diam. from . ft to ft	BROWN SAMO LLYTH GRAMICES	184	174.
Parforations	BIOWN CLAY BOUND GULAVEL	174	7/2
Perforations: Yeal No TORCH	DENSE BROWN CLAYBOUND CHALL	42	=77
SIZE of perforations	BROWN FLAY MOUND GRAYEL WITH LAYERS	3()	2.31
Spen Foot perforations from 410. It to 5.75 ft.	BROWN CLAY AND GRAYEL TO YH"	232	237
perforations from ft. to ,	BROWN CLAY AND GRAYEL TO YY	237 -	-3.7
perforations from It to ft.	CERY LAYERS	2	247
Screens: Yes NO D	PINK DIZOWN MED SAND AND DAKE GUL	247	260
Manufacturer's Name UDP JOWN SON	BROWN (LAY AND GRAYEL	260	280
Type SQT Model No	TAN CLAY	280	182
Diam Slot size from ft to	TAN CLAY BOUND GRAYEL	182	288
	TAN CLAY	288	297
Gravel packed: Yes No Size of gravel /4"	BROWN CEMENTEN SAND	297	307
Gravet placed from 610 It to 420 ft	BROWN CEMENTED SAND WITH GRAVEL	307	2
Surface seal: Yes X No To what depth? / 8 ft	70 34 " AND SOME CLAY		323
Material used in seal CONCURTE	BROWN CLAY	323	340
()id any strata contain unusable water? Yes 🗋 No 💢	BROWN (EMENTED GRAVEL	340	P
Type of water? Depth of strata	- AND SAND		1.357
Method of sealing strata of	BROWN CEMENTEN GRAVEL	327	1-33%
(7) PUMP: Minufacturer's Name NA	CHOCCLATE BROWN CLAY	358 380	395
Тург Н.Р.	BROWN SILTY SAND WITH WHE GAME	395	464
(8) WATER LEVELS: Land-surface elevation / 5 80 m.	BROWN MED SAND GRAVEL WITH CLAY	464	467
Static level 39 ft below top of well Date 10/1/87	GROWN SANDSTONE WITH CXCASIONAL GVE	489	560
Artesian spessure - lbs per square inch Date	BROWN SANDSTONE	560	683
Artesian water is controlled by (Cap. valve, etc.)	BROWN SAND	683	686
	BROWN CLAYBOND GRAVEL	686	1 700
(9) WELL TESTS: Drawdown is amount water level is lowered below static level	Work started Co 2 1987 Completed 10	29	. 19.87
Was a pump test name? Yes & No If yes, by whom? REN, BWD Yield /200 gal. name with /39 it drawdown after 24 hrs.	WELL DRILLER'S STATEMENT:		
	This well was drilled under my jurisdiction	and this	report
	true to the best of my knowledge and belief."		
Recovery data time taken as zero when unip turned on thater measured from well top to water levels	NAME 13ACH DR. 11 NO CO.		. .
Time Water Level Time Water Level Time Water Level	(Person, firm, or corporation) (Type or p	rint)
5AN 69.	Address RT. 5 BOX 1010, EllENSA	BURG	98926
60 Min 59	Address		
1 110)	01 448 -10 5	#	
Aller test gal min. with ft drawdown after	[Signed] DATTEL H. Buck' Sm	1.1.M	
Artesian flow 6 gpm Date	1.555	0	0 11
Temperature of water 61 F Was a chemical analysis made? Yes No []	License No. 1555 Date ///	r	, 19 .X Z

Boring Log and Construction Data for Monitoring Well MW-13

Geologic Log

Monitoring Well Design Top of Casing in Feet: 1582.45



and actual changes may be gradual.

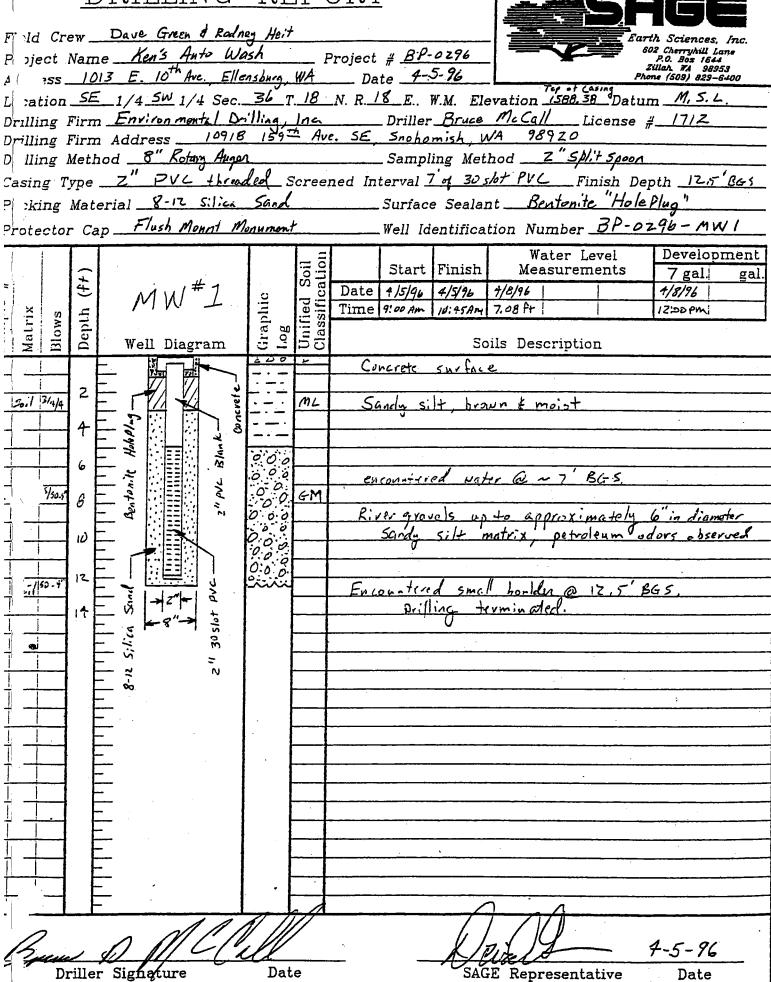
Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



^{1.} Refer to Figure C-1 for explanation of descriptions and symbols.

^{2.} Soil descriptions and stratum lines are interpretive

DRILLING REPORT



DRILLING REPORT R : Id Crew Dave Green & Rodney Heit 602 Cherryhill Lane P.O. Box 1644 Zillah WA 98953 Phone (509) 829-6400 Poject Name Ken's Auto Wash Project # BP-0296 255 1013 E. 10th Ave. Ellensburg, WA Date 4-5-96 L cation SE 1/4 SW 1/4 Sec. 36 T. 18 N. R. 18 E., W.M. Elevation Top of (asing Datum M.S. L. Drilling Firm Environmental Drilling, Inc., Driller Bruce McCall License # 1712 Drilling Firm Address 10918 159 4 Avenue S.E. Snehomish, WA 98920 Sampling Method Z" Split Spoon D illing Method 8" Rotary Augu Casing Type 2" Threaded PVC Screened Interval 8' of 30 Slot PVC Finish Depth 13 Bas P cking Material 8-17 Silica Sand Surface Sealant Bentonite "Hole Plug" Protector Cap Flush mant Monnment w/ J Plug Well Identification Number BP-0296-MWZ Development Water Level Unified Soil Classification Start Finish Measurements /Z gal. MW #2 Date 4-5-96 4-5-96 4-8-96 4-8-96 Time 11: wAm 12:15 m 2:10 PM Well Diagram Soils Description Concrete Surface Sundy Silt brown & moist. A little water @ 2' BGJ. + |Suil |2/2/1 150.5 GM No petroleum odors observed. 50-6"

Driller Signature Date

SAGE Representative

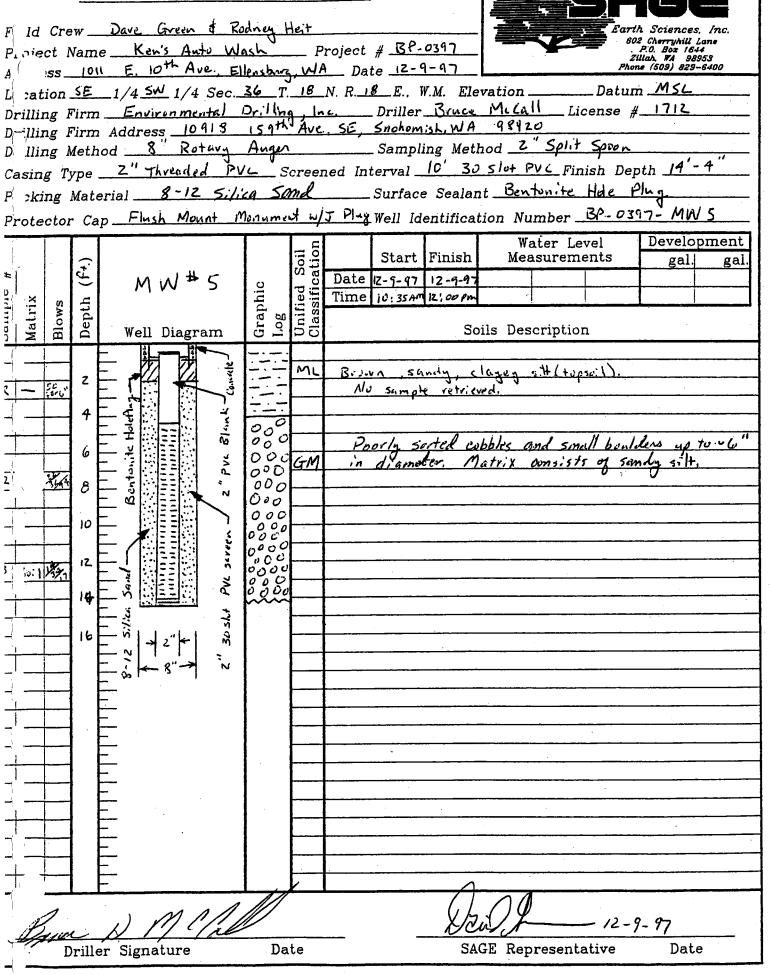
DRILLING REPORT

I sld Crew Dave Green & Rodney Heit Earth Sciences. Inc. 602 Cherryhill Lane P.O. Boz 1644 Zillah WA 98953 Phone (509) 829-6400 Loject Name Kon's Anto Wash Project # BP-0296 ess 1013 E. 10th Ave. Ellenshing, WA Date 4-5-96 cation SE 1/4 SW 1/4 Sec. 36 T. 18 N. R. 18 E., W.M. Elevation 1591.43 Datum MSL Drilling Firm <u>Environmental Drilling</u>, Inc. Driller Bruce McCall License # 1712
Prilling Firm Address 10918 159th Ave. SE, Snohomish, WA 98920 I illing Method 8" Rotary Augen Sampling Method 2" Split Spoon Finish Depth 15 BGS Casing Type Z"Threaded PYC Screened Interval 10' 30 56t PYC A cking Material 8-12 5.1: Ca Sand Surface Sealant Bentonite Hole Plug Protector Cap Flush Monat Monament w/ J-Plug Well Identification Number BP-0296 - MW3 Water Level Development Start Finish Measurements *15* gali MW #3 Date 4-5-96 4-5-96 4-8-96 4-8-96 Graphic Time 1:30 Pm 3715 Pm 8.08 ft. 3:15-pm Well Diagram Soils Description Asphalt Surface upper Z.". 501 11/16/7 Pourly Sorted cobbles & Small boulders up to 6" in diameter, Matrix consists of GM 000 0.0 0.00 No petroleum odors observed 12 4-5-96 Date Driller Signature Date SAGE Representative

DRILLING REPORT

rield	rield Crew Dave Grown & Rodney Heit Earth Sciences. Inc.									
Proj	Project Name Ken's Ante Wash Project # BP-0397 P.O. Boz 1644 Project Name Ken's Ante Wash Project # BP-0397 P.O. Boz 1644 Zillah, WA 98953 Phone (509) 829-6400									
({z	ess_	101	<u>13</u> 1	<u> </u>	1022	Ave t	:llensbi	119.1	WA Date 12 - 9 - 47 Phone (509) 829-5400	
,uca	tion	_5 <u>c</u>	1/	4.30	<u>씨</u> 1/4	! Sec	36 T	18	8 N. R. 18 E., W.M. Elevation Datum MSC	
Drill.	ing I	Firm	1	Env	ironn	nemtal	1 Phi	ling.	g, Inc. Driller Bruce McCall License # 1712 Vc. SE, Snohomish, WA 98920	
Driii.	ng r	iirm Mati	i Ado	ires	is 15.	stava	Ange	<u> </u>	Sampling Wather 7" SAL't Some	
Cani	Orilling Method 8" Rotary Anger Sampling Method Z" Split Spoon Casing Type Z"Threaded PVC Screened Interval 38'30 slot PVC Finish Depth 13.5'845									
'ack	ig ij ing	ype Mat	erial	8	3-12	Zilic	50	ind	Surface Sealant Bentonite Hole Plug	
rot	ectoi	c Ca	ip Eli	तरपृ	Mount	. Monu	ment	<u>w/J</u>	Well Identification Number BP-0397 - MW 4-	
	'	_	1					Soil ation	Start Finish Water Level Development Measurements	
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e x	"	ا ج	1	141	γv	ļ	Graphic Log	ed jiji	Time 8:45am 10:00Am	
Se Matrix	Blows	Depth	1			•	Grap Log	Unified Classific	0	
X S	m	ă	W	ell	Diagr	am	<u>5 4</u>	อ์อ		
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- 	+		_ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~			J. Brun		 	Collected from slutt of previous annulus	
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		6	<u>_</u>		EE	+				
		1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		語為	PYC Exis		GW	V 1/2" minus pen gravel used for UST backfill.	
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Acres 100	Di	rille	r Sig	gnat	ure	<u> </u>	Dat	.e	SAGE Representative Date	

DRILLING REPORT



Key to Exploration Logs

Sample Description

Classification of soils in this report is based on visual field and laboratory observations which include density/consistency, moisture condition, grain size, and plasticity estimates and should not be construed to imply field nor laboratory testing unless presented herein. Visual—manual classification methods of ASTM D 2488 were used as an identification guide.

Soil descriptions consist of the following:

Density/consistency, moisture, color, minor constituents, MAJOR CONSTITUENT, additional remarks.

Density	y/Consi	istency
---------	---------	---------

Soil density/consistency in borings is related primarily to the Standard Penetration Resistance.

Soil density/consistency in test pits is estimated based on visual observation and is presented parenthetically on the test pit logs.

SAND or GRAVEL	Stondard Penetration Resistance (N)	SILT or CLAY	Standard Penetration Resistance (N)	Approximate Shear Strength
Density	in Blows/Foot	Consistency	in Blows/Foot	in TSF
Very loose	0 - 4	Very soft	0 - 2	<0.125
Loose	4 - 10	Soft	2 - 4.	0.125 - 0.25
Medium dense	10 - 30	Medium stiff	4 - 8	0.25 - 0.5
Dense	30 - 50	Stiff	8 - 15	0.5 - 1.0
Very dense	>50	Very stiff	15 - 30	1.0 - 2.0
	•	Hard	>30	>2.0

Moisture

Dry Little perceptible moisture

Damp Some perceptible moisture, probably below optimum

Moist Probably near optimum moisture content

Wet Much perceptible moisture, probably above optimum

Minor Constituents	Estimated Percentage
Not identified in description	0 - 5
Slightly (clayey, silty, etc.)	5 - 12
Clayey, silty, sandy, gravelly	12 – 30
Very (clayey, silty, etc.)	30 - 50

Legends

Sampling Test Symbols

BORING SAMPLES

Split Spoon

Shelby Tube

Cuttings

Core Run

* No Sample Recovery

P Tube Pushed, Not Driven

TEST PIT SAMPLES

Grab (Jar)

Bog

Shelby Tube

Groundwater Observations

V

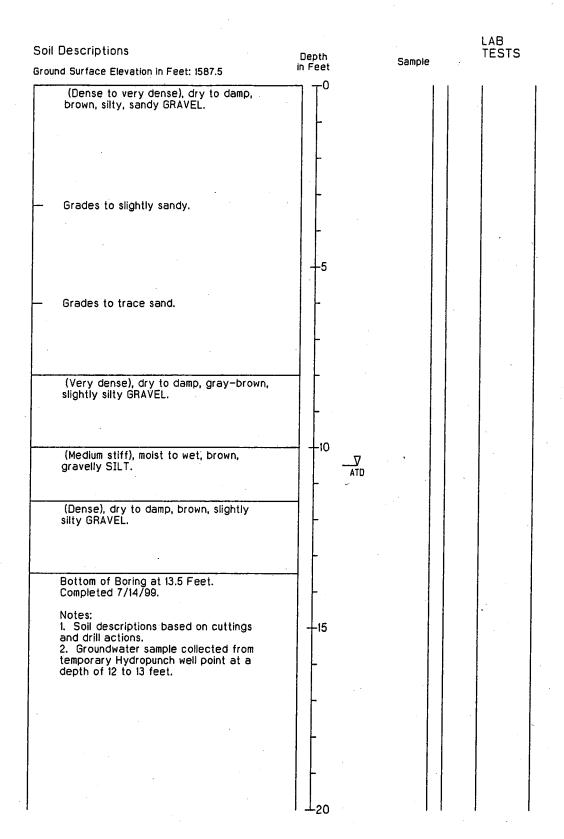
Groundwater Level on Date (ATD) At Time of Drilling

Test Symbols GS Grain Size Classification CN Consolidation TUU Triaxial Unconsolidated Undrained TCU Trioxial Consolidated Undrained TCD Triaxial Consolidated Drained QU Unconfined Compression DS Direct Shear Permeability Pocket Penetrometer Approximate Compressive Strength in TSF PP TV Torvane Approximate Shear Strength in TSF CBR California Bearing Ratio MD Moisture Density Relationship AL Atterberg Limits Water Content in Percent Liquid Limit Notural Plastic Limit Photoionization Detector Reading PID CA Chemical Analysis

HARTCROWSERJ-7168 11/99

Figure A-1

=1



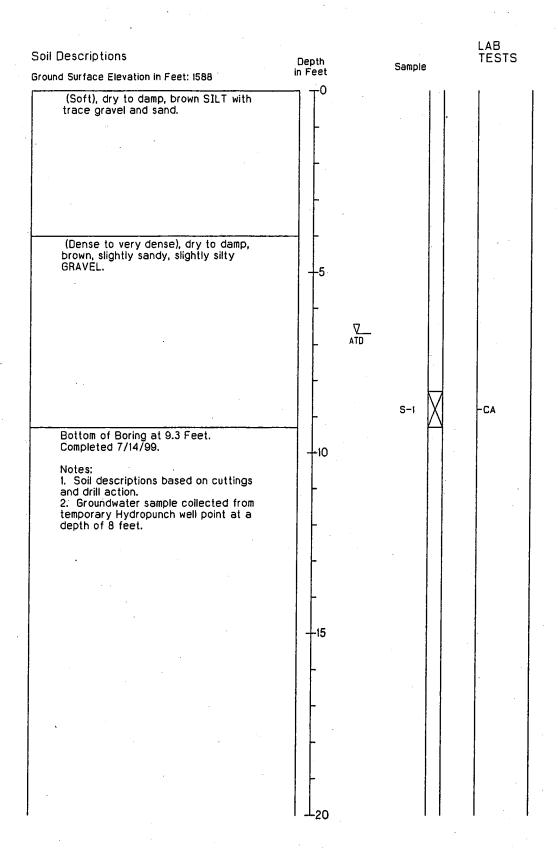
^{1.} Refer to Figure A-1 for explanation of descriptions and symbols.



^{2.} Soil descriptions and stratum lines are interpretive

and actual changes may be gradual.

3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

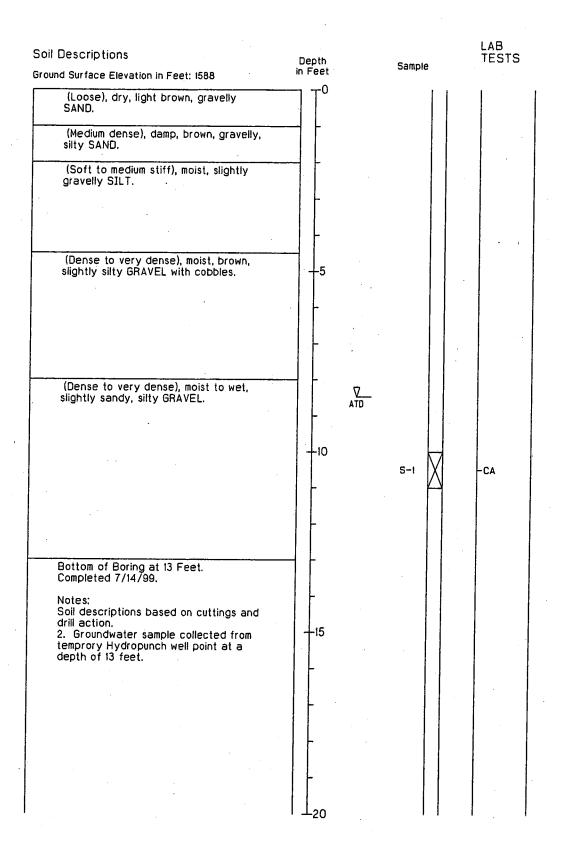


1. Refer to Figure A-1 for explanation of descriptions and symbols.

Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Ground water level, if indicated, is at time of drilling

(ATD) or for date specified. Level may vary with time.

8/99 J-7188 Figure A-3



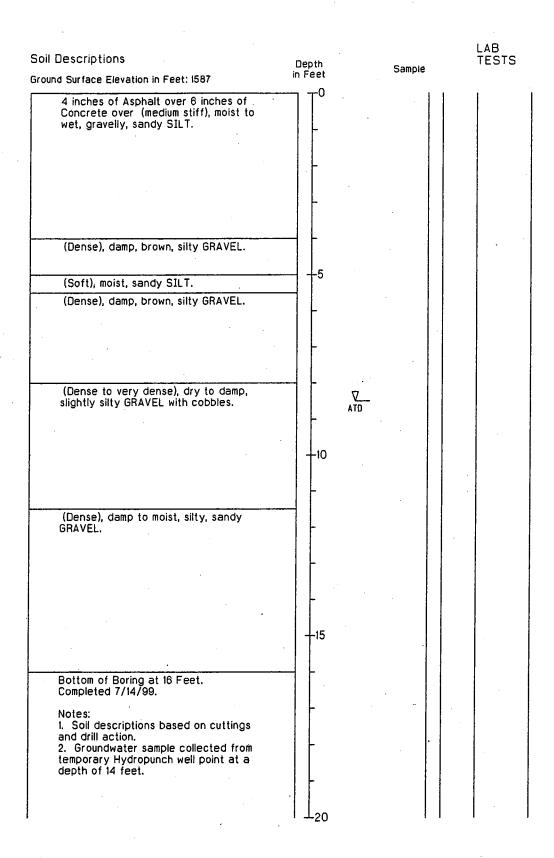
1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive

and actual changes may be gradual.

3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



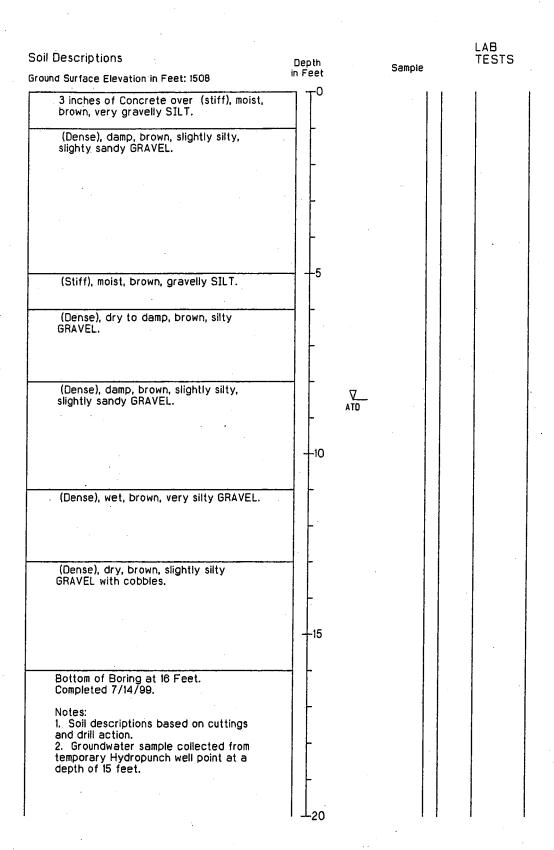


1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual. 3. Ground water level, if indicated, is at time of drilling

(ATD) or for date specified. Level may vary with time.

7/99 Figure A-5



1. Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Ground water level, if indicated, is at time of drilling

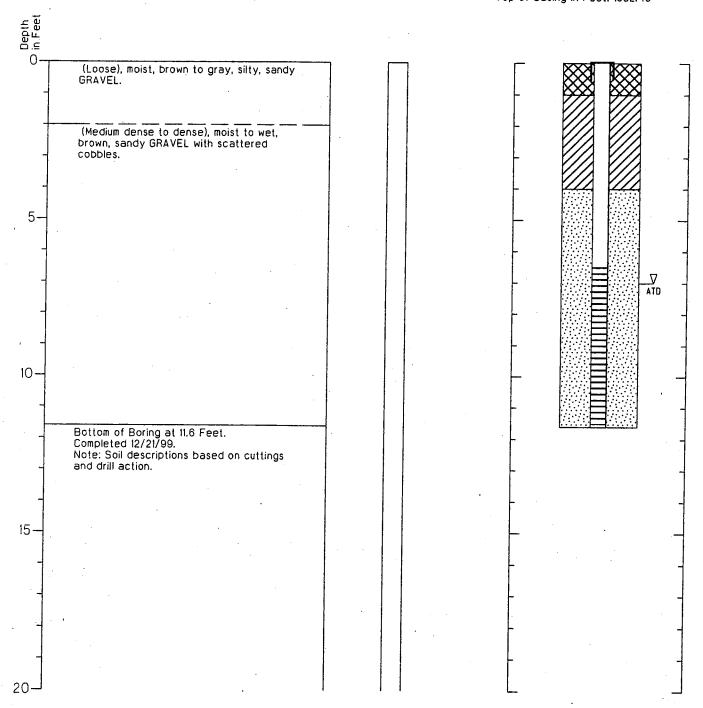
(ATD) or for date specified. Level may vary with time.



Boring Log and Construction Data for Monitoring Well MW-13

Geologic Log

Monitoring Well Design Top of Casing in Feet: 1582.45



and actual changes may be gradual.

3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



^{1.} Refer to Figure C-1 for explanation of descriptions and symbols.

^{2.} Soil descriptions and stratum lines are interpretive

Key to Exploration Logs

Sample Description

Classification of soils in this report is based on visual field and laboratory observations which include density/consistency, moisture condition, grain size, and plasticity estimates and should not be construed to imply field nor laboratory testing unless presented herein. Visual—manual classification methods of ASTM D 2488 were used as an identification guide.

Soil descriptions consist of the following:

Density/consistency, moisture, color, minor constituents, MAJOR CONSTITUENT, additional remarks.

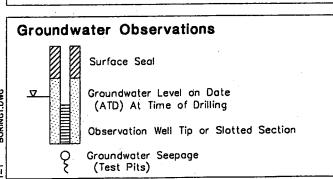
Density/Consister Soil density/consister	icy in borings is related	primarily to the Standar	d Penetration Resistance	
•	Standard		Standard	enthetically on the test pit log Approximate
SAND or GRAVEL	Penetration Resistance (N)	SILT or CLAY Consistency	Penetration Resistance (N) in Blows/Foot	Shear Strength in TSF
Density	in Blows/Foot	•	•	
Very loose	0 - 4	Very soft	0 - 2	<0.125
Loose	4 - 10	Soft	2 - 4	0.125 - 0.25
Medium dense	10 - 30	Medium stiff	4 - 8	0.25 - 0.5
Dense	30 - 50	Stiff	8 - 15	0.5 - 1.0
Very dense	>50	Very stiff	15 - 30	1.0 - 2.0
		Hard	>30	>2.0

Moisture					
Dry	Little perceptible moisture				
Damp	Some perceptible moisture, probably below optimum				
Moist	Probably near optimum moisture content				
Wet	Much perceptible moisture, probably above optimum				

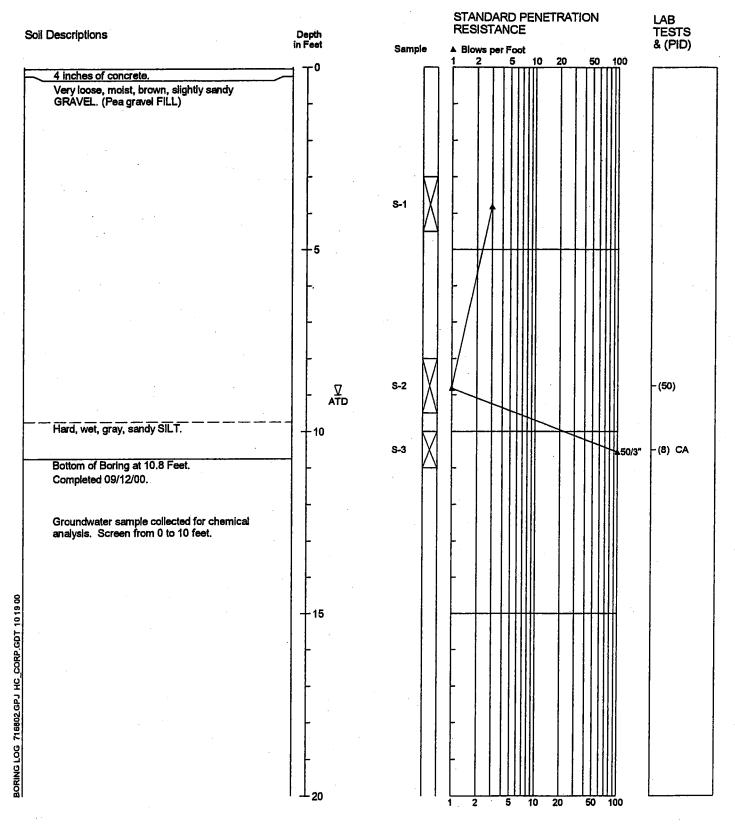
Minor Constituents	Estimated Percentage
Not identified in description	0 - 5
Slightly (clayey, silty, etc.)	5 - 12
Clayey, silty, sandy, gravelly	12 - 30
Very (clayey, silty, etc.)	30 – 50

Legends

Sam	Sampling Test Symbols						
BORIN	G SAMPLES						
\boxtimes	Split Spoon						
	Shelby Tube						
	Cuttings						
	Core Run						
*	No Sample Recovery						
Р	Tube Pushed, Not Driven						
TEST	PIT SAMPLES						
\boxtimes	Grab (Jar)						
	. Bag						
	Shelby Tube						



Tes	t Symbols
GS	Grain Size Classification
CN	Consolidation
บบ	Unconsolidated Undrained Triaxial
cu	Consolidated Undrained Triaxial
CD	Consolidated Drained Triaxial
QU	Unconfined Compression
DS	Direct Shear
к	Permeability
PP	Pocket Penetrometer Approximate Compressive Strength in TSF
TV	Torvane Approximate Shear Strength in TSF
CBR	California Bearing Ratio
MD	Moisture Density Relationship
AL	Atterberg Limits
	Water Content in Percent
	L. Liquid Limit Natural Plastic Limit
PID	Photoionization Detector Reading
CA	Chemical Analysis
TO.	In Situ Density Test



1. Refer to Figure A-1 for explanation of descriptions and symbols.

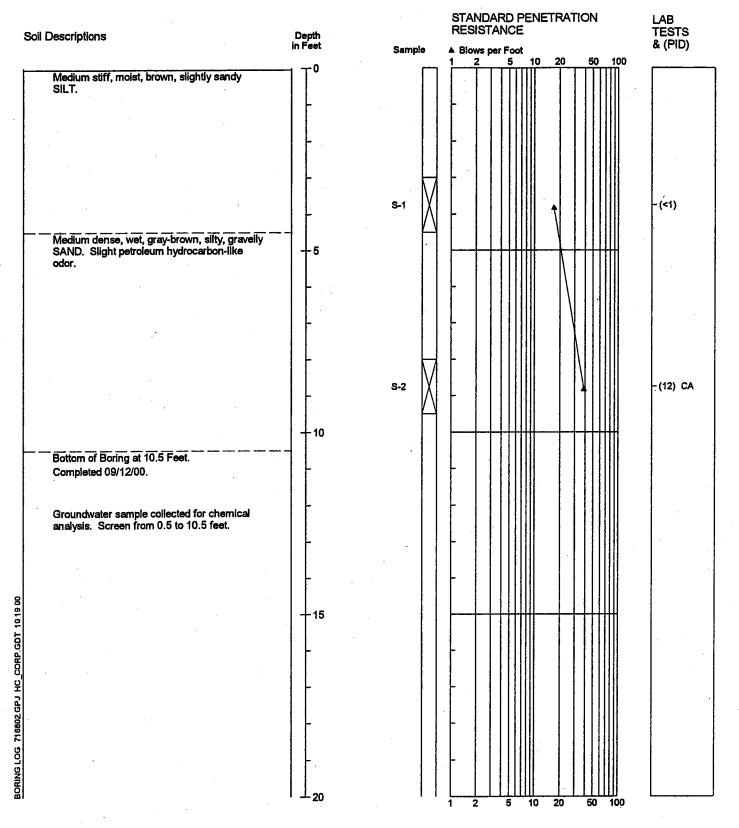
Soil descriptions and stratum lines are interpretive and actual changes may
be gradual.

Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



J-7168-02

09/00



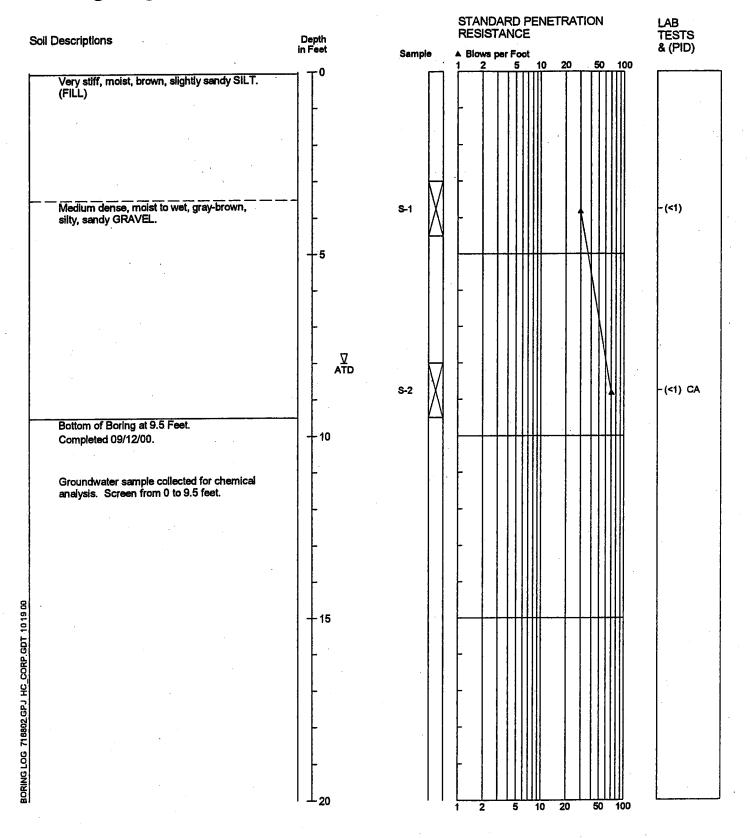
Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



J-7168-02

09/00



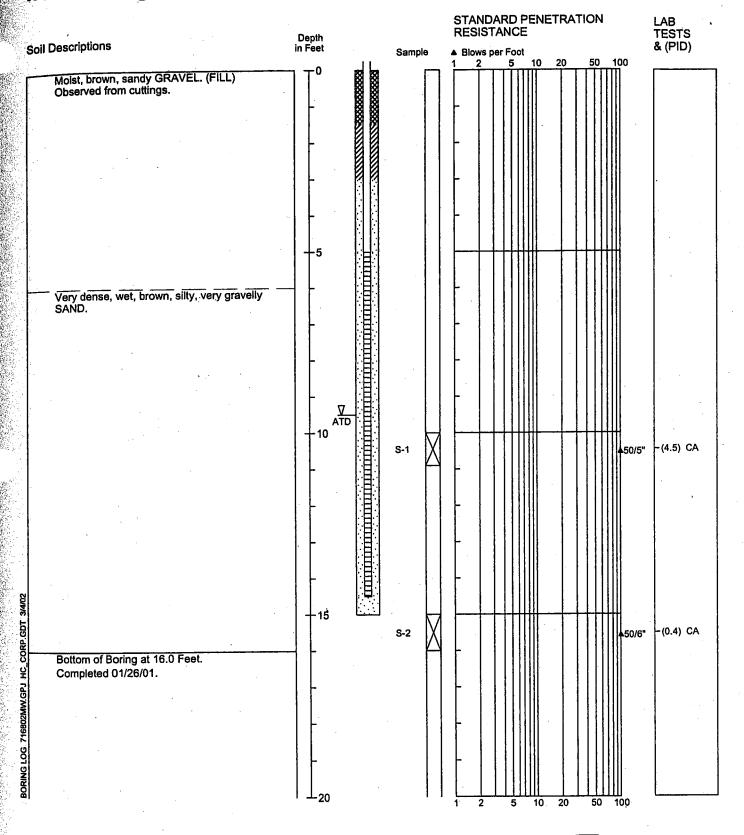
 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



J-7168-02 Figure A-5 09/00

Monitoring Well Log MW-14



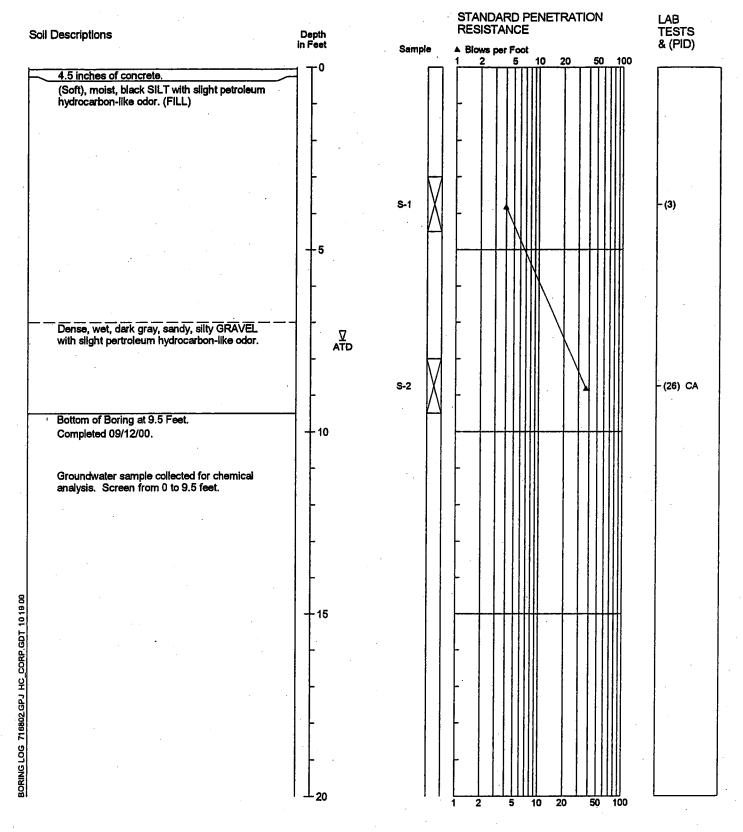
Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date

specified. Level may vary with time.



7168-02

01/01



Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may

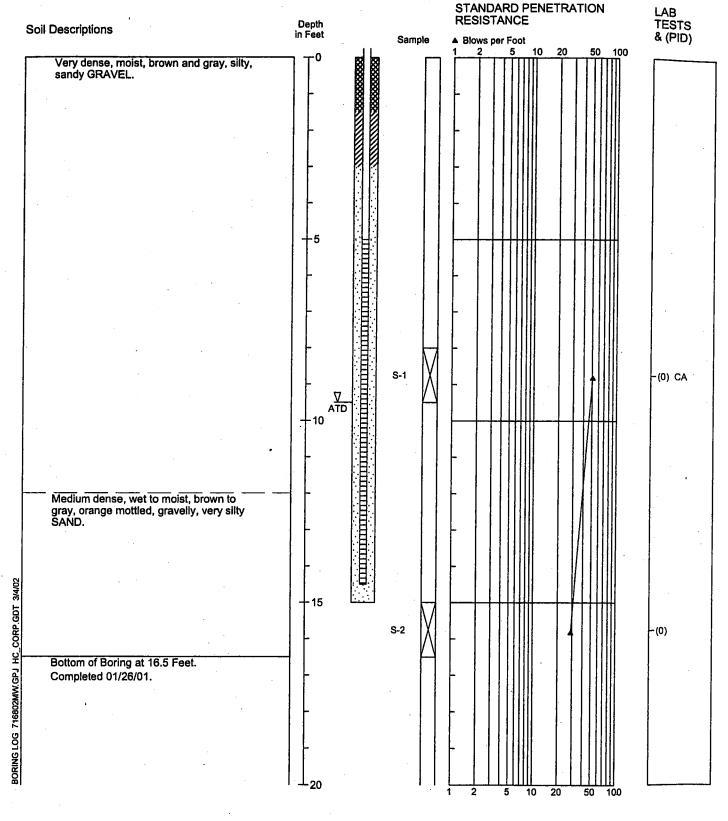
3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



J-7168-02

09/00

Monitoring Well Log MW-15



Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes.

may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

HARTCROWSER

7168-02

01/01

Key to Exploration Logs

Sample Description

Classification of soils in this report is based on visual field and laboratory observations which include density/consistency, moisture condition, grain size, and plasticity estimates and should not be construed to imply field nor laboratory testing unless presented herein. Visual-manual classification methods of ASTM D 2488 were used as an identification guide.

Soil descriptions consist of the following:

Density/consistency, moisture, color, minor constituents, MAJOR CONSTITUENT, additional remarks.

Density/Consistency

Soil density/consistency in borings is related primarily to the Standard Penetration Resistance. Soil density/consistency in test pits is estimated based on visual observation and is presented parenthetically on the test pit logs.

SAND or GRAVEL Density	Standard Penetration Resistance (N) in Blows/Foot	SILT or CLAY Consistency	Standard Penetration Resistance(N) In Biows/Foot	Approximate Shear Strength in TSF
Very loose	0 - 4	Very soft	0 - 2	<0.125
Loose	4 - 10	Soft ·	2 - 4	0.125 - 0.25
Medium dense	10 - 30	Medium stiff	4 - 8	0.25 - 0.5
Dense	30 - 50	Stiff	8 - 15	0.5 - 1.0
Very dense	>50	Very stiff	15 - 30	1.0 - 2.0
		Hard	>30	>2.0

Moist	Moisture				
Dry	Little perceptible moisture				
Damp	Some perceptible moisture, probably below optimum				
Moist	Probably near optimum moisture content				
Wet	Much perceptible moisture, probably above optimum				

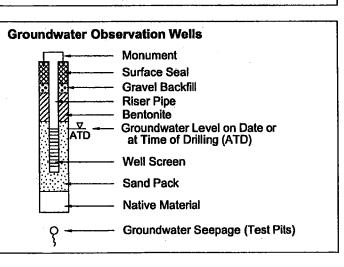
Minor Constituents	Estimated Percentage
Not identified in description	0 - 5
Slightly (clayey, silty, etc.)	5 - 12
Clayey, silty, sandy, gravelly	12 - 30
Very (clayey, silty, etc.)	30 - 50

Legends

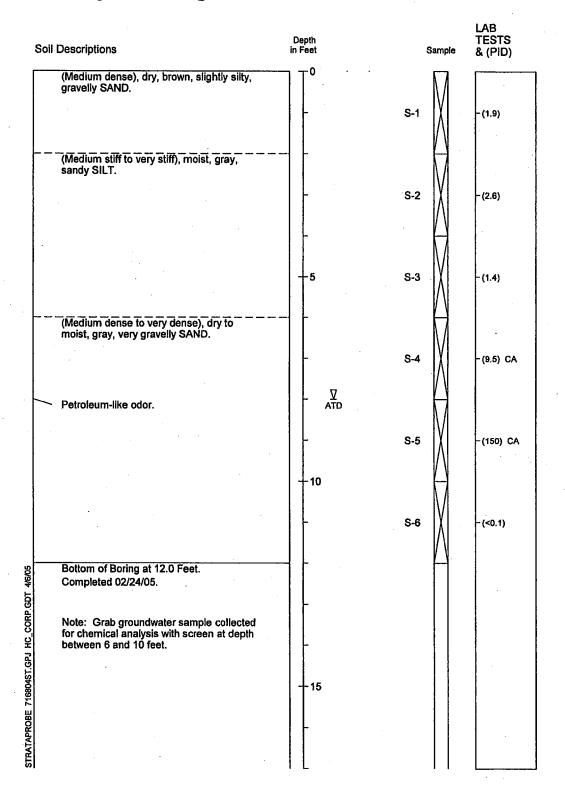
윰

716804-002.DWG (SRF A-1 STANDARD (2005).DWG)

Sampl	ing Test Symbols		
Boring	Samples	Test P	it Samples
\boxtimes	Split Spoon	\boxtimes	Grab (Jar)
	Shelby Tube		Bag
	Cuttings		Shelby Tube
	Core Run	•	•
*	No Sample Recovery		
P	Tube Pushed, Not Driven		







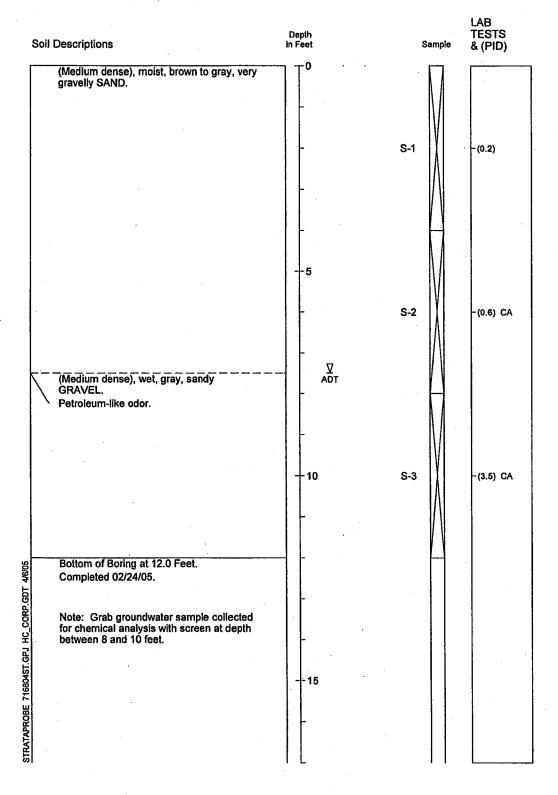
 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



7168-04

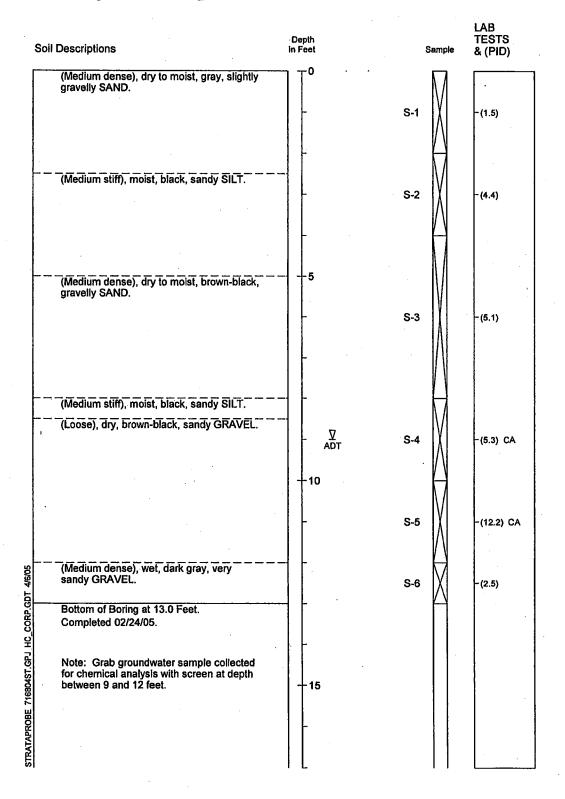
02/05



Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



7168-04 Figure A-3 02/05

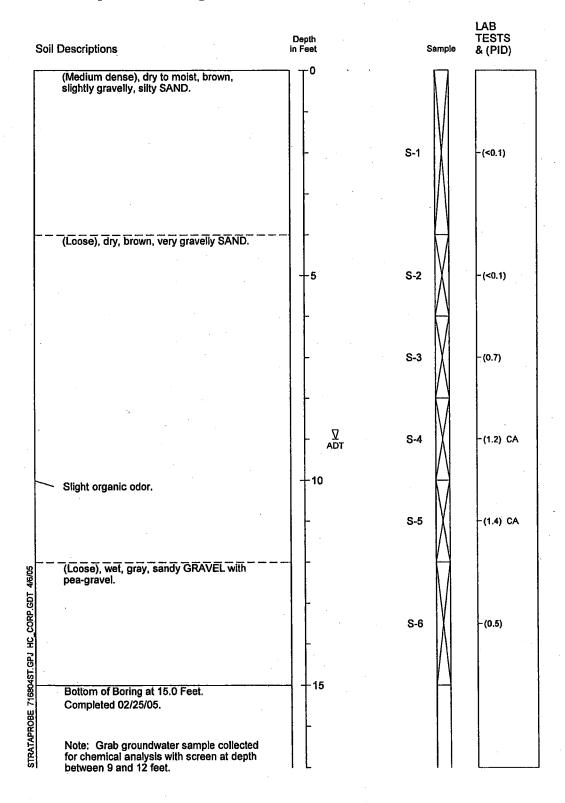


 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



7168-04 02/05 Figure A-4



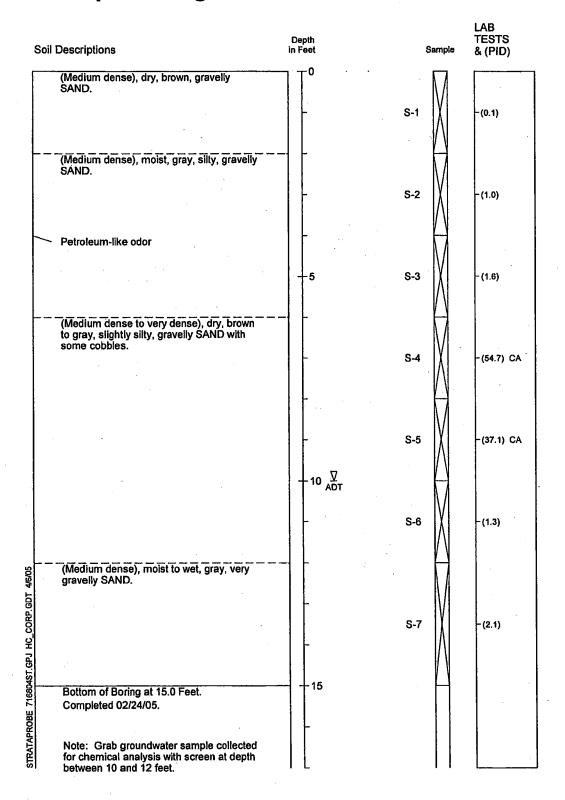
 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



7168-04

02/05



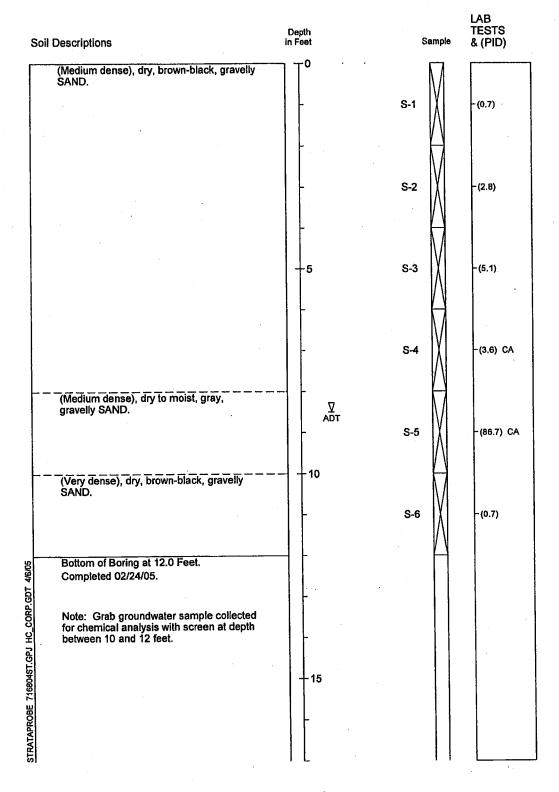
Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 Groundwater level, if indicated, is at time of drilling (ATD) or for date

specified. Level may vary with time.



7168-04

02/05

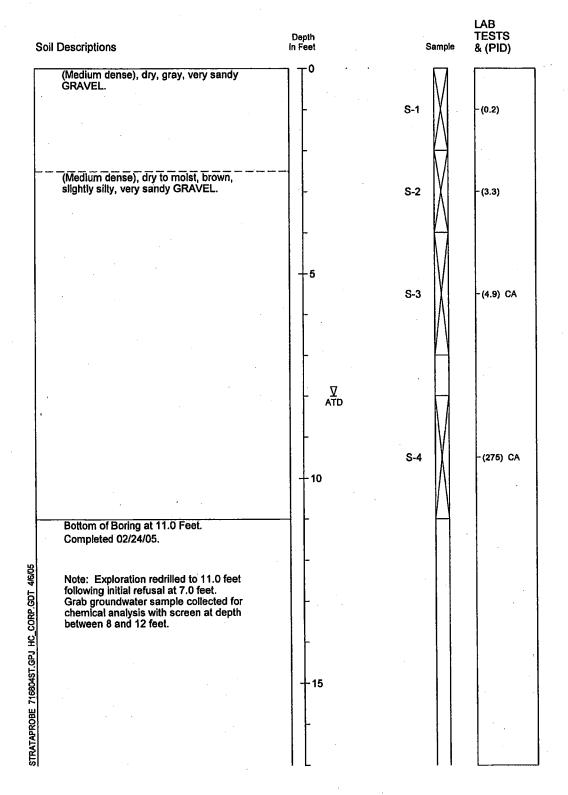


 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



7168-04 Figure A-7 02/05





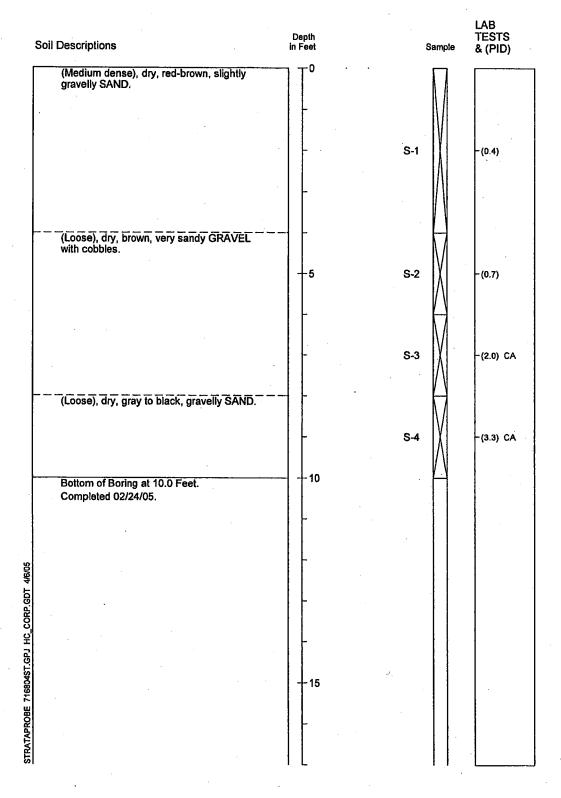
 Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

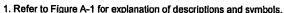
3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



7168-04

02/05





Refer to Figure A-1 for explanation of descriptions and symbols.
 Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

3. Groundwater level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.



7168-04

02/05

Table C-1 - Cost Estimate for Alternative 1 - Monitored Natural Attenuation

Groundwater monitoring Estimated Ecology Oversight Costs Additional Contingency 15% S 2,625 TAL ANNUAL MONITORING COST Estimated Costs DESCRIPTION QUANTITY UNIT UNIT COST TOTAL SOURCE NOTES Ecology interaction at 5-year review periods Confirmational groundwater monitoring A EA 4,400 \$ 17,600 Cost File Confirmational groundwater monitoring Report and Ecology Interaction for closure Estimated Ecology Oversight Costs 1 LS 3,000 \$ 9,000 Estimate Confirmational groundwater monitoring A EA 4,400 \$ 17,600 Cost File Confirmational groundwater monitoring A ES 15,000 \$ 15,000 Estimate Estimated Ecology Oversight Costs A 1 LS 3,000 \$ 3,000 Estimate Continued Cost File Confirmational Groundwater Monitoring A ES 4,490 Towns Cost File Confirmational Groundwater Monitoring A ES 4,490 Towns Cost File Confirmational Groundwater Monitoring A ES 4,490 Towns Cost File Confirmational Monitoring A ES 4,490 Towns Cost File Cost	ite: Ken's Auto Wash ccation: Ellensburg, Washingtor hase: Feasibility Study (-30% ase Year: 2007		Description: Monitoring of groundwater contaminated with gasoline-range petroleum and BTEX for up to 30-y program assumes twice yearly events (wet season and dry season) at all well locations, or a por includes monitoring of natural attenuation (NA) parameters every other year.								
Project management and reporting	NNUAL MONITORING COSTS - A	LL WELLS BIANNUA	LLY			-					
Visite disposal 1 LS 1,000 Estimate control purple water control	DESC	RIPTION	QUANTITY		UNIT	UNIT COST		TOTAL	SOURCE	NOTES	
Estimated Ecology Oversign() Cost 1 1 1 1 1 1 1 1 1	Waste disposal	ng	1	LS	•	1,000	\$	1,000	Estimate	up to 9 wells, include NA parameters every oth	
Page	Estimated Ecology Oversight Co	sts	1	LS			\$	3,000		Subject to revision by Ecology	
DESCRIPTION QUANTITY UNIT UNIT COST TOTAL SOURCE NOTES	OTAL ANNUAL MONITORING CO	ST	•				\$	23,125			
Ecology interaction at 5-year review periods 3 EA 4,00 \$ 1,700 Cost File Ecology requirement Confinentional groundwater monitoring 4 EA 4,00 \$ 17,000 Cost File Ecology requirement Confinentional productions 1 LS 3,000 \$ 1,000 \$ 1,000 Estimate Ecology requirement Confinency 15% \$ 3,000 Estimate Ecology requirement Confinency	FA/ REGULATORY CLOSURE CO	STS:	····					•			
Contingency Contingency Lis 1,500 Section Subject to revision by Ecology Contingency Lis 1,500 Section Subject to revision by Ecology Contingency Lis 1,500 Section Subject to revision by Ecology Contingency Lis 1,500 Section Subject to revision by Ecology Contingency Lis	DESCI	RIPTION	QUANTITY		UNIT	UNIT COST		TOTAL	SOURCE	NOTÉS	
State	Confirmational groundwater mor Report and Ecology Interaction for	altoring or closure	. 4	LS	•	4,400 15,000	\$	17,600 15,000	Cost File Estimate	confirmational montitoring at 9 wells	
COST TYPE		sts				3,000			Estimate		
COST TYPE	OTALCLOSURE COST						\$	49,490			
COST TYPE	RESENT VALUE ANALYSIS:										
Contingency Well Replacement	cos	TYPE	YEAR	TO1	TAL COST						
Annual Operating Cost 1 to 5 \$ 115,825 2.8% \$ 107,126 Annual Operating Cost 6 to 10 \$ 115,825 2.8% \$ \$ 92,776 Annual Operating Cost 11 to 15 \$ 115,825 2.8% \$ \$ 92,776 Annual Operating Cost 16 \$ 148,826 2.8% \$ \$ 92,776 Annual Operating Cost 16 \$ 148,826 2.8% \$ \$ 324,547 DTAL PRESENT VALUE OF ALTERNATIVE (16-Year Duration) COST TYPE YEAR TOTAL COST RATE Annual Operating Cost 1 to 5 \$ 115,825 2.8% \$ 107,126 Annual Operating Cost 1 to 20 \$ 231,250 0.30% \$ 288,267 DTAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) DISCOUNT (Closure Cost 21 \$ 49,480 0.30% \$ 146,781 0.30% \$ 388,267 DTAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) DISCOUNT (Annual Operating Cost 1 to 5 \$ 115,825 2.8% \$ 107,126 0.30% \$ 388,267 DTAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) DISCOUNT (Annual Operating Cost 1 to 5 \$ 115,825 2.8% \$ 107,126 0.30% \$ 388,267 DTAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) DISCOUNT (Annual Operating Cost 1 to 5 \$ 115,825 2.8% \$ 92,776 0.30% \$ 388,267 DTAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) DISCOUNT (Annual Operating Cost 6 to 10 \$ 115,825 2.8% \$ 92,776 0.30% \$ 205,418 0.30% \$			•				\$	15,000	Up to 3 wells o	ver project lifetime. Not Discounted.	
Annual Operating Cost 6 to 10 \$ 115,825 2.8% 92,776 Annual Operating Cost 11 to 15 \$ 115,825 3.0% \$ 78,804 Closure Cost 18 \$ 49,400 3.0% \$ 30,841 \$ 368,865 \$ 324,647 DTAL PRESENT VALUE OF ALTERNATIVE (15-Year Duration) COST TYPE YEAR TOTAL COST Annual Operating Cost 10 to 5 115,825 2.8% 15,000 Annual Operating Cost 11 to 20 \$ 23,250 2.8% \$ 92,776 Annual Operating Cost 11 to 20 \$ 23,250 3.0% \$ 26,603 \$ 511,900 \$ 388,267 DISCOUNT RATE Contingency Well Replacement \$ 15,000 \$ 115,825 2.8% \$ 92,776 Annual Operating Cost 11 to 20 \$ 23,1250 3.0% \$ 26,603 \$ 511,900 \$ 388,267 DISCOUNT RATE Contingency Well Replacement \$ 15,000 0 % \$ 16,7126 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 11 to 25 \$ 49,400 3.0% \$ 22,948 DISCOUNT RATE VALUE OF ALTERNATIVE (25-Year Duration) \$ 482,000 3.0% \$ 22,948 DISCOUNT RATE Contingency Well Replacement \$ 15,000 0 % \$ 433,300 DISCOUNT RATE Contingency Well Replacement \$ 15,000 0 % \$ 22,948 DISCOUNT RATE Contingency Well Replacement \$ 15,000 0 % \$ 107,126 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10 to 5 115,825 2.8% \$ 92,776 Annual Operating Cost 10			1 to 5	\$	115,625	2.6%	\$	107,126	•		
Annual Operating Cost 11 to 15 \$ 115,625 3.0% \$ 76,804 Closure Cost 16 \$ 49,490 3.0% \$ 30,841 \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ 324,547 \$ \$ \$ 324,547 \$ \$ \$ 324,547 \$ \$ \$ 324,547 \$ \$ \$ 324,547 \$ \$ \$ \$ 324,547 \$ \$ \$ \$ 324,547 \$ \$ \$ \$ 324,547 \$ \$ \$ \$ \$ 324,547 \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$						2.8%	\$	92,776			
Closure Cost 16			11 to 15	\$	115,625	3.0%	\$	78,804		•	
COST TYPE	Closure Cost		- 16	\$		3.0%			,	•	
COST TYPE YEAR TOTAL COST RATE CONTINGENCY Well Replacement - \$ 15,000 15,000	OTAL PRESENT VALUE OF ALTE	RNATIVE (15-Year D	uration)				\$	324,500			
Contingency Well Replacement											
Annual Operating Cost 6 to 10 \$ 115,625 2.8% \$ 107,126 Annual Operating Cost 6 to 10 \$ 115,625 2.8% \$ 92,776 Annual Operating Cost 6 to 10 \$ 231,250 3.0% \$ 146,781 Closure Cost 21 \$ 49,490 3.0% \$ 26,603 TAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) COST TYPE			YEAR				_				
Annual Operating Cost 6 to 10 \$ 115,625 2,8% \$ 92,776 Annual Operating Cost 11 to 20 \$ 231,250 3,0% \$ 146,781 Closure Cost 21 \$ 49,490 3,0% \$ 26,603 ***STAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) COST TYPE COST TYPE YEAR TOTAL COST Annual Operating Cost 1 to 5 \$ 115,625 2,8% \$ 107,126 Annual Operating Cost 6 to 10 \$ 115,625 2,8% \$ 92,776 Annual Operating Cost 1 to 5 \$ 146,875 3,0% \$ 22,948 Closure Cost 26 \$ 49,490 3,0% \$ 22,948 COST TYPE YEAR COST TYPE YEAR TOTAL COST Annual Operating Cost 6 to 10 \$ 115,625 2,8% \$ 92,776 Annual Operating Cost 6 to 10 \$ 115,625 2,8% \$ 92,776 Annual Operating Cost 1 to 5 \$ 16,605 2,8% \$ 22,948 Closure Cost 26 \$ 49,490 3,0% \$ 22,948 **STAN OPERATOR OF ALTERNATIVE (25-Year Duration) **TOTAL PRESENT VALUE OF ALTERNATIVE (25-Year Duration) **TOTAL COST TYPE YEAR TOTAL COST RATE Contingency Well Replacement - \$ 15,000 0% \$ 15,000 Up to 3 wells over project lifetime. Not Discounted. Annual Operating Cost 1 to 5 \$ 115,625 2,8% \$ 107,126 **STAN OPERATOR OF ALTERNATIVE (25-Year Duration) **TOTAL COST TYPE YEAR TOTAL COST RATE Contingency Well Replacement - \$ 15,000 0% \$ 15,000 Up to 3 wells over project lifetime. Not Discounted. Annual Operating Cost 1 to 5 \$ 115,625 2,8% \$ 107,126 Annual Operating Cost 1 to 5 \$ 115,625 2,8% \$ 92,776 Annual Operating Cost 1 to 3 \$ 462,500 3,0% \$ 255,999 Closure Cost 11 to 30 \$ 462,500 3,0% \$ 255,999 Closure Cost 13 \$ 49,490 3,0% \$ 19,795 \$ 490,697									Up to 3 wells o	ver project lifetime. Not Discounted.	
Annual Operating Cost 11 to 20 \$ 231,250 3.0% \$ 146,781 Closure Cost 21 \$ 49,490 3.0% \$ 26,603 \$ 388,300 COTAL PRESENT VALUE OF ALTERNATIVE (20-Year Duration) COST TYPE											
Closure Cost 21				-							
S S S S S S S S S S				- :							
COST TYPE	Closure Cost		21	\$		3.0%					
COST TYPE	OTAL PRESENT VALUE OF ALTE	RNATIVE (20-Year D	uration)				\$	388,300			
COST TYPE						DISCOUNT		PRESENT			
Contingency Well Replacement	COST	TYPE	YEAR	TO1	TAL COST	RATE		VALUE			
Annual Operating Cost 1 to 5 \$ 115,625 2.6% \$ 107,126 Annual Operating Cost 6 to 10 \$ 115,625 2.8% \$ 92,776 Annual Operating Cost 11 to 25 \$ 346,875 3.0% \$ 205,418 Closure Cost 26 \$ 49,490 3.0% \$ 22,948 COST TYPE YEAR TOTAL COST RATE Contingency Well Replacement	Contingency Well Repla	acement	-	\$	15,000	0%	\$	15,000	Up to 3 wells o	ver project lifetime. Not Discounted.	
Annual Operating Cost 6 to 10 \$ 115,625 2.8% \$ 92,776 Annual Operating Cost 11 to 25 \$ 346,875 3.0% \$ 205,418 Closure Cost 26 \$ 49,490 3.0% \$ 22,948 \$ 627,615 \$ \$ 443,300 DISCOUNT COST TYPE YEAR TOTAL COST RATE Contingency Well Replacement - \$ 15,000 0% \$ 15,000 Annual Operating Cost 1 to 5 \$ 115,625 2.6% \$ 107,126 Annual Operating Cost 6 to 10 \$ 115,625 2.6% \$ 92,776 Annual Operating Cost 11 to 30 \$ 482,500 3.0% \$ 255,999 Closure Cost 31 \$ 49,490 3.0% \$ 19,795 \$ 743,240 \$ 490,697			1 to 5	\$	115,625	2.6%	\$				
Closure Cost 26 \$ 49,490 \$ 3.0% \$ 22,948 \$ 443,269 \$ 27,615 \$ \$ 443,300 \$ \$ 22,948 \$ 243,300 \$ \$ 243,300 \$ \$ 243,300 \$ \$ 243,300 \$ \$ 243,300 \$ \$ 243,300 \$ \$ 243,300 \$ 243,300			6 to 10	\$							
State Stat	Annual Operating Cost		11 to 25	\$							
State Contingency Well Replacement Contingency Well Replacement Contingency Contingenc	Closure Cost		26	\$		3.0%			•		
COST TYPE YEAR TOTAL COST strained and the placement TOTAL COST strained and the placement RATE strained and the placement VALUE strained and the placement	OTAL PRESENT VALUE OF ALTE	RNATIVE (25-Year D	uration)	•	02.,010		_				
COST TYPE YEAR TOTAL COST strained and the placement of the placemen						DISCOUNT					
Annual Operating Cost 1 to 5 \$ 115,625 2.6% \$ 107,126 Annual Operating Cost 6 to 10 \$ 115,625 2.8% \$ 92,776 Annual Operating Cost 11 to 30 \$ 462,500 3.0% \$ 255,999 Closure Cost 31 \$ 49,490 3.0% \$ 19,795 \$ 743,240 \$ 490,697			YEAR	TOT							
Annual Operating Cost 6 to10 \$ 115,625 2.8% \$ 92,776 Annual Operating Cost 11 to 30 \$ 462,500 3.0% \$ 255,999 Closure Cost 31 \$ 49,490 3.0% \$ 19,795 \$ 743,240 \$ 490,697	Contingency Well Repli	acement	•	\$					Up to 3 wells o	ver project lifetime. Not Discounted.	
Annual Operating Cost 11 to 30 \$ 462,500 3.0% \$ 255,999 Closure Cost 31 \$ 49,490 3.0% \$ 19,795 \$ 743,240 \$ 490,697				\$							
Closure Cost 31 \$ 49,490 3.0% \$ 19,795 \$ 490,697			6 to 10							•	
\$ 743,240 \$ 490,697			11 to 30	\$							
	Closure Cost			<u> </u>		3.0%					
				•	•		-				

Notes

Costs are +50/-30% FS-level estimates. They do not represent a bid to do the work.

Table C-3 - Cost Estimate for Alternative 3 - Enhanced Bioremediation

ocation: Ellensburg, Washington Phase: Feasibility Study (-30% to +50%) Base Year: 2007							asoline-range petroleum and BTEX. ORC injectic eriod. Contingency cost provided for 15-year per
CAPTITAL COSTS		÷	•				
DESCRIPTION Site Preparation	QUANTITY	UNIT	UNIT COST		TOTAL	SOURCE	NOTES
Ecology interaction Permits, utility check, revised health and safety plan	1 Li 1 Li		1,500 6,000		1,500 6,000	Estimate Cost File plus	Review through Voluntary Cleanup Program Interaction with city
Core injection ports in concrete surface install monuments	1 L: 8 E		2,000 300	•		Additional Estimate Estimate	Up to 8 12-inch holes includes labor
Estimated Ecology Oversight Costs Contingency	1 L9 15%	5	3,000	\$ \$	3,000 660	Estimate	Subject to revision by Ecology 10% scope + 5% bid
OTAL CAPITAL COST			[\$	15,560		
NNUAL OPERATING AND MONITORING COSTS						 	
DESCRIPTION	QUANTITY	UNIT	UNIT COST		TOTAL	SOURCE	NOTES
Project management and reporting	1 13	•	10,000	\$	10,000	Estimate	Includes monthly monitoring performed by site worker
Groundwater monitoring	2 E		4,500			Cost File.	performance monitoring at up to 9 well location
ORC Injections and field oversight	750 lb 2 E		10 7,000		7,500 14,000	Cost File Cost File	1 field day; includes traffic control
Estimated Ecology Oversight Costs Contingency	1 LS 15%		5,000			Estimate	Subject to revision by Ecology 10% scope + 5% bid
OTAL ANNUAL OPERATING AND MONITORING COST				\$	49,475		
LOSURE COSTS:							-
DESCRIPTION	QUANTITY	UNIT	UNIT COST		TOTAL	SOURCE	NOTES
Ecology interaction at 5-year review periods	1 E		3,000			Estimate	Ecology requirement
Groundwater monitoring Report and Ecology Interaction	4 E		4,400 15,000			Cost File Estimate	9 wells
Estimated Ecology Oversight Costs Contingency	1 LS 15%		5,000			Estimate	Subject to revision by Ecology 10% scope + 5% bid
OTAL CLOSURE COST			[\$	42,490		
RESENT VALUE ANALYSIS:							
		TOTAL	DISCOUNT	-	RESENT		
COST TYPE Contingency Well Replacement	YEAR	COST 15,000	RATE 0%	\$	VALUE 15.000	Up to 3 wells o	ver project lifetime. Not Discounted.
Capital Cost	0	15,560	0.0%	\$	15,560		Not discounted
Annual Operating and Monitoring Cost Annual Operating and Monitoring Cost	1 to 5 6 to 10		2.6% 2.8%	\$ \$	229,192 198,492	•	
Closure Cost	11 _3	42,490	3.0%	\$	26,968		•
	•	552,800	_	\$	485,212		
OTAL PRESENT VALUE OF ALTERNATIVE(10-Year Duration	on)		[\$	485,200	,	
COST TYPE	YEAR	TOTAL COST	DISCOUNT RATE		RESENT VALUE		
Contingency Well Replacement	- 1	15,000	0%	\$	15,000	Up to 3 wells o	ver project lifetime. Not Discounted.
Capital Cost	0		0.0%		15,560		Not discounted
Annual Operating and Monitoring Cost Annual Operating and Monitoring Cost	1 to 5	247,375 247,375		\$ \$	229,192 198,492		
Annual Operating and Monitoring Cost Closure Cost	11 to 15	247,375 42,490 800,175	3.0%	\$ \$	168,598 26,478 653,320		
DTAL PRESENT VALUE OF ALTERNATIVE(15-Year Duration		,	-	\$	653,300		

Costs are +50/-30% FS-level estimates. They do not represent a bid to do the work. Assumptions
ORC effective for 6 months following injection

Injection close to source area only

Table C-4 - Cost Estimate for Alternative 4 - Air Sparging and Soil Vapor Extraction

ite: ocation: 'hase: lase Year:	Ken's Auto Wash Ellensburg, Washington Feasibility Study (-30% to +50%) 2007	Description:	pipe already in	stalled suppleme	nted with 3 deep		soline-range petroleum and BTEX. Uses horizontal spi and 4 vapor extraction wells. Nominal 4-year remedia atlon.
APITAL COS	BT8;						
	DESCRIPTION	QUANTITY	UNIT	UNIT COST	TOTAL	SOURCE	NOTES
Site Prepa	aration						
	Permits		I LS	5000		Estimate	air permit, street use permit
	Mobilization		LS	5000		Estimate	Drill rig, SVE/sparge equipment, etc.
	Utilities Security		ILS ILS	1500 1500		Estimate Estimate	utility locate Security fencing for equipment compound
	SUBTOTAL		1 20	. ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	\$ 13,000	- Familiate	Cacally tellong for equipment compound
							· · · · · · · · · · · · · · · · · · ·
01/51:-4-	W-W-		•				
SVE Insta	Extraction well installation		I EA	2.000	\$ 8,000	Cost File	To 10-foot depth
	Deep sparging well installation		EA	1,000		Cost File	To 16 feet using direct-push
	Disposal of contaminated drill cuttings		LS	2,500		Cost File	12 drums of non-haz waste
	SVE equipment		LS	9,000		Cost File	Blower and accessories
	Sparging equipment	1	LS	8,000	\$ 8,000	Cost File	Blower and accessories
	Blower sound enclosure		LS	3,500		Cost File	Contains both blowers
	Condensate removal and collection		LS	2,000		Cost File	Knockout pot, pump, level switches, and tank
	Treatment equipment - Cat Ox Mechanical, electrical, and control work		LS LS	50,000 15,000		Cost File Estimate	150 SCFM includes control panel with autodialer installation and hookup
	SVE and air sparging piping		LS	4,000		Estimate	Piping, fittings, gauges, etc.
	Installing SVE and air sparging piping		LS	6,000		_Estimate	Below ground but already installed under slab
	SUBTOTAL				\$ 113,000	-	•
							Laboration that the season of
	anagement and design	15% 10%			\$ 18,900 \$ 12,600		Includes Health and Safety Plan Includes startup labor
	on oversight Ecology Oversight Costs		LS	5,000	\$ 12,600 \$ 5,000	Estimate	Subject to revision by Ecology
Contingen		15%		0,000	\$ 23,625		10% Scope + 5% bid
TAL CAPIT				г	\$ 186,100	7	
						•	
NUAL OPE	RATING AND MONITORING COSTS	1					
	anagement, reporting, and O&M		LS	10,000			Monthly monitoring performed by site worker
	iter monitoring		Ea	4,500			performance monitoring
Energy	Ecology Oversight Costs		: Month LS	500 5,000			150 SCFM sir flow, gas fired catox, blower electricity Subject to revision by Ecology
Contingen		15%			\$ 3,750	Louinato	10% scope + 5% bid
OTAL OPER	ATING AND MONITORING COST		•	[\$ 33,750)	
.08URE CO	STS:						
	DESCRIPTION	QUANTITY	UNIT	UNIT COST	TOTAL	SOURCE	NOTES
Groundwa	iter monitoring	4	EA	4,400	\$ 17.600	Cost File	9 wells
	anagement, Report, and Ecology Interaction		LS	15,000		Estimate	
	Ecology Oversight Costs	. 1	LS	5,000	\$ 5,000	Estimate	Subject to revision by Ecology
Contingen	cy	15%			\$ 4,890		10% scope + 5% bid
TAL CLOS	LIBE COST			-	\$ 42,490	1	
				L	- +2,480	1,	
ESENT VAI	LUE ANALYSIS:						
				DISCOUNT	PRESENT		
	COST TYPE	YEAR	TOTAL COST	RATE	VALUE		
	Contingency Well Replacement	=	\$ 15,000	0%		Up to 3 wells o	ver project lifetime. Not Discounted.
	Capital Cost Annual Operating and Monitoring Cost	0	\$ 186,100		\$ 186,100 \$ 426,664		Not Discounted
	Annual Operating and Monitoring Cost Closure Cost	1 to 4 . 5	\$ 135,000 \$ 42,490	2.6% 2.6%	\$ 126,661 \$ 37,372		•
	Citating Cost	. 3	\$ 363,590	2.070	\$ 365,134		
TAL PRESI	ENT VALUE OF ALTERNATIVE (5-Year Dun	ation)		[\$ 365,100]	
				DISCOUNT	PRESENT		
-	COST TYPE	YEAR	TOTAL COST	RATE	VALUE		
	Contingency Well Replacement	•	\$ 15,000	0%		Up to 3 wells o	ver project lifetime. Not Discounted.
	Capital Cost	0	\$ 186,100	0%	\$ 186,100	-	Not Discounted
	Annual Operating and Monitoring Cost	1 to 5	\$ 168,750	2.6%	\$ 156,346		
	Annual Operating and Monitoring Cost	6	\$ 33,750	2.8%	\$ 27,702		•
	Closure Cost	7	\$ 42,490 \$ 431,090	2.8%	\$ 35,022 \$ 420,170		
	ENT VALUE OF ALTERNATIVE (7-Year Dun	-41	4 491,080	_	\$ 420,170		

Notes

Costs are +50/-30% FS-level estimates. They do not represent a bid to do the work.

Assumptions

Ecology will require 4 quarters of confirmational monitoring

150 SCFM SVE air flow

20-foot SVE radius of influence

Gas available on site

Room for system on adjacent vacant lot