B&L Woodwaste Site Pierce County, Washington

Engineering Design Report (EDR) Addendum 3

Phase 2 Part 1 Remediation Design Report

Groundwater Recovery and Treatment System

Prepared for

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Prepared by

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June 2011

FINAL

B&L Woodwaste Site Pierce County, Washington

Engineering Design Report (EDR) Addendum 3

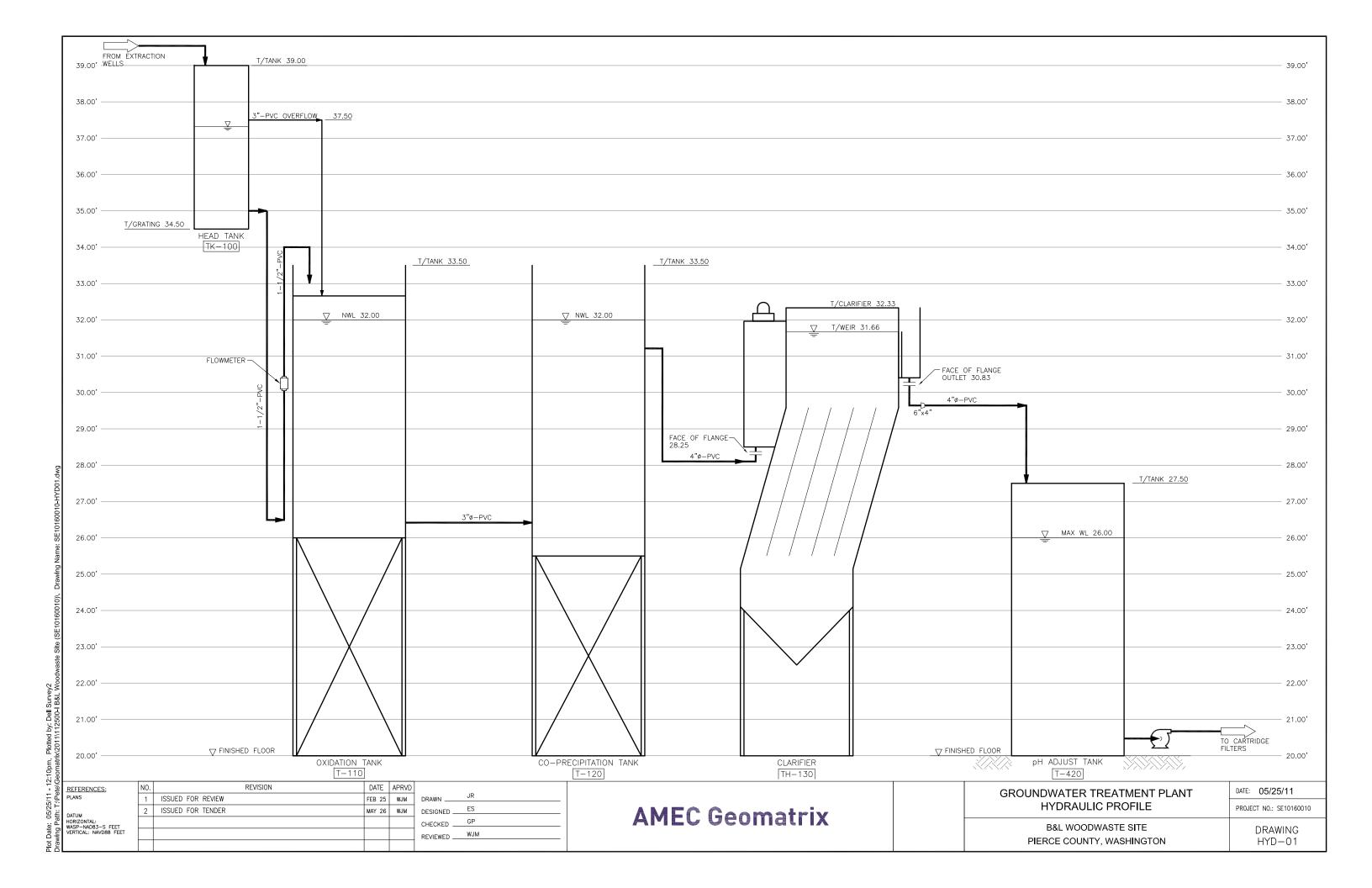
Phase 2 Part 1 Remediation Design Report

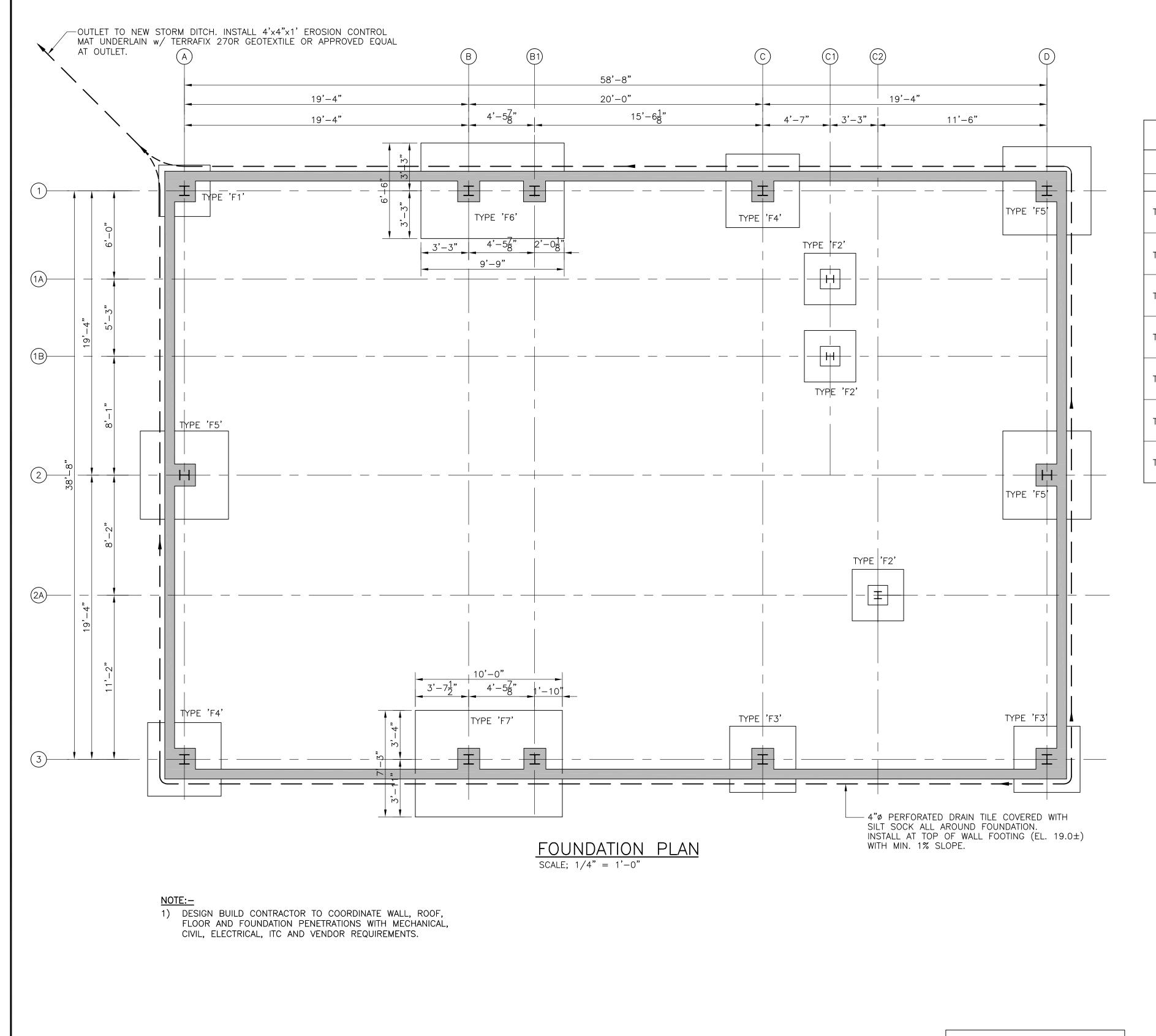
Groundwater Recovery and Treatment System

Appendix 3F Construction Drawings and Specifications

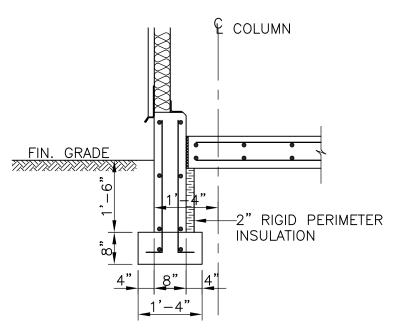
(Specifications provided on CD-ROM)

Note: Part 2 of Appendix 3F drawings - Part 1 of Appendix 3F and remainder of EDR Addendum 3 are separate files

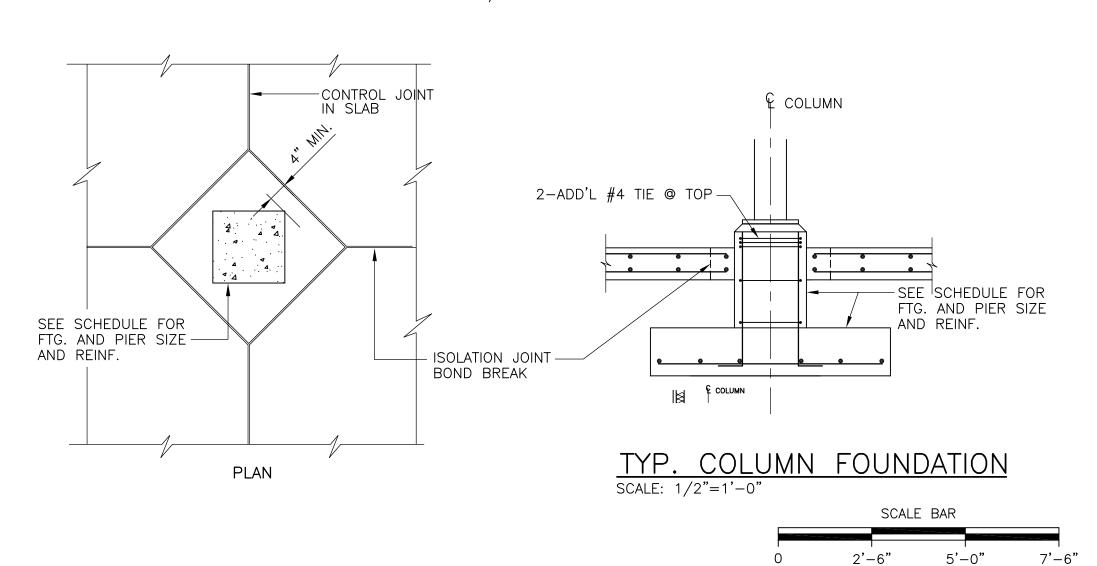




COLUMN FOOTING SCHEDULE									
	COLUMN FOC	TING		PIER					
NO.	SIZE	TOP OF FOOTING ELEV.	REINFORCING	SIZE	DOWELS	TIE	TIE TYPE		
TYPE 'F1'	3'-6"x3'-6"x12"	19'-4"	4-#5 EW	18"×18"	8-#6	#4 @ 12"	6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6		
TYPE 'F2'	3'-6"x3'-6"x12"	19'-4"	4-#5 EW	16"×16"	8-#5	#4 © 12"			
TYPE 'F3'	4'-6"x4'-6"x12"	19'-4"	5-#5 EW	18"×18"	8-#6	#4 © 12"			
TYPE 'F4'	5'-0"x5'-0"x12"	19'-4"	6-#5 EW	18"×18"	8-#6	#4 © 12"	□		
TYPE 'F5'	6'-0"x6'-0"x12"	19'-4"	7-#5 EW	18"×18"	8-#6	#4 © 12"			
TYPE 'F6'	6'-6"x9'-9"	17'-5"	11-#5 (N-S) 7-#5 (W-E)	2-18"x18"	8-#6	#4 © 12"	[
TYPE 'F7'	7'-3"×10'-0"	16'-5"	11-#5 (N-S) 8-#5 (W-E)	2-18"x18"	8-#6	#4 @ 12"			



TYP. WALL FOUNDATION SCALE: 1/2"=1'-0"



FOR INFORMATION ONLY

DESIGN/BUILD CONTRACTOR
TO PROVIDE FINAL DESIGN

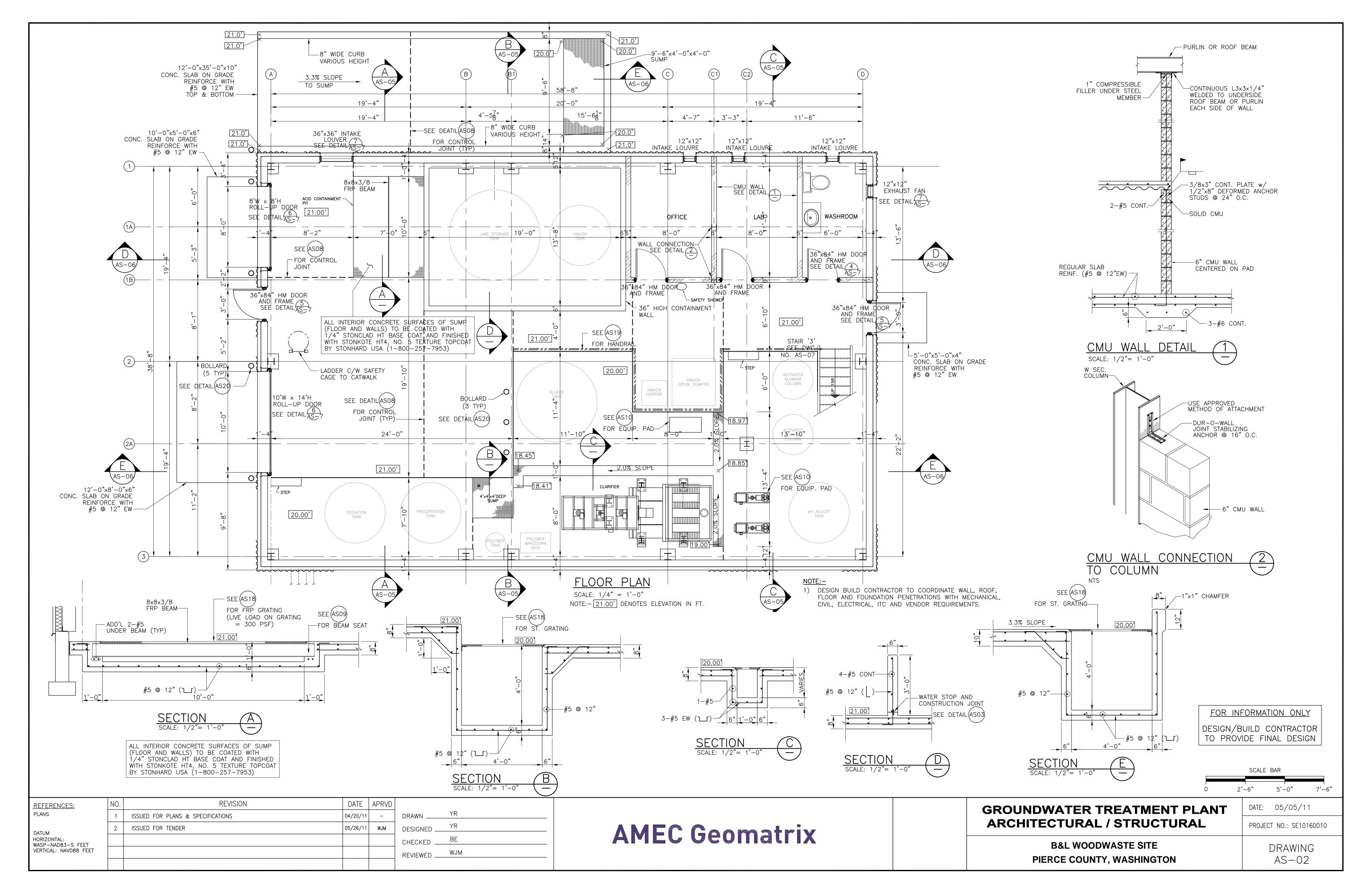
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8.7.04	2	ISSUED FOR TENDER	05/26/11	WJM	DESIGNED	YR
DATUM HORIZONTAL:					CHECKED	BE
WASP-NAD83-S FEET VERTICAL: NAVD88 FEET					REVIEWED	WJM

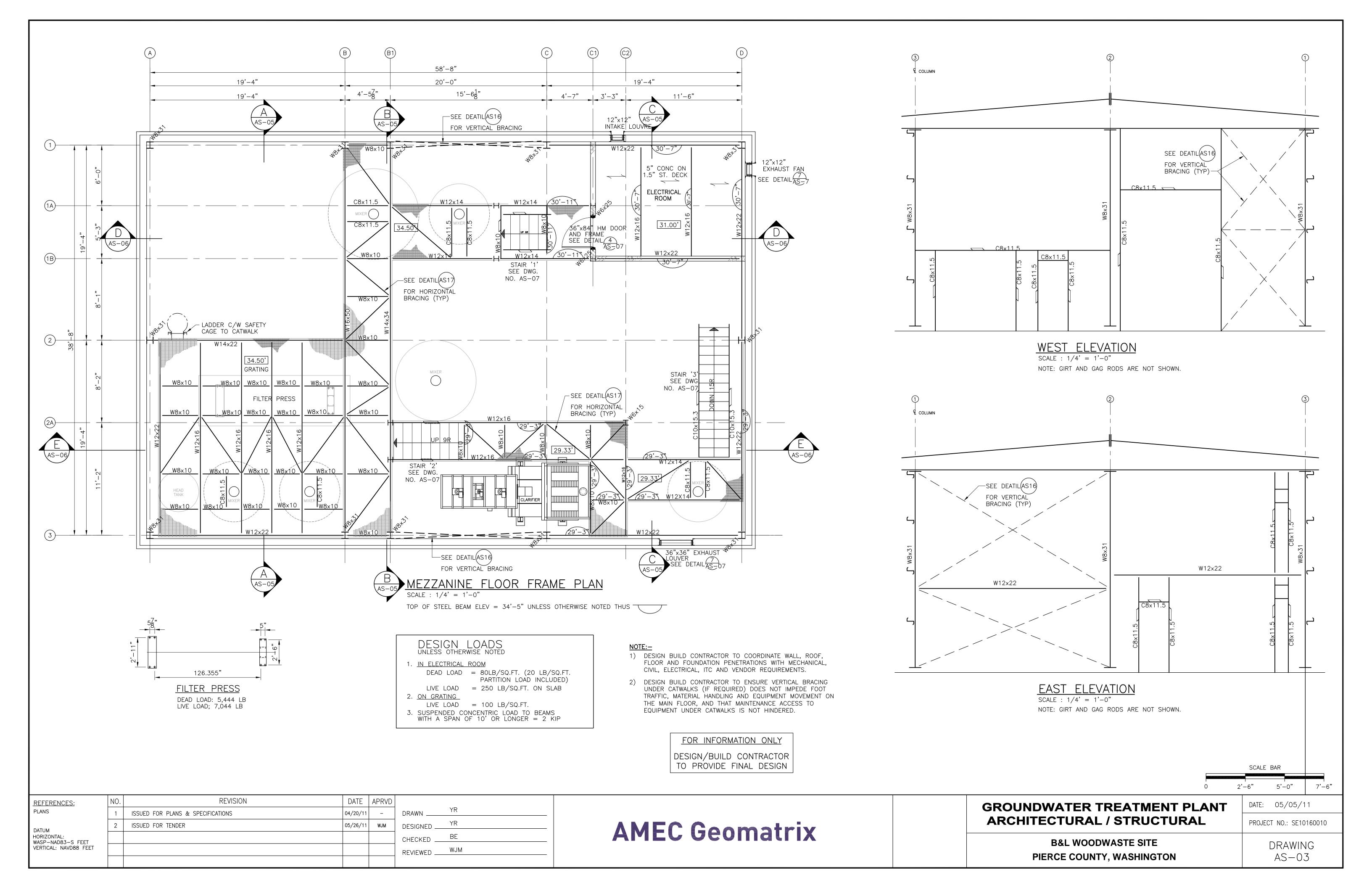
AMEC Geomatrix

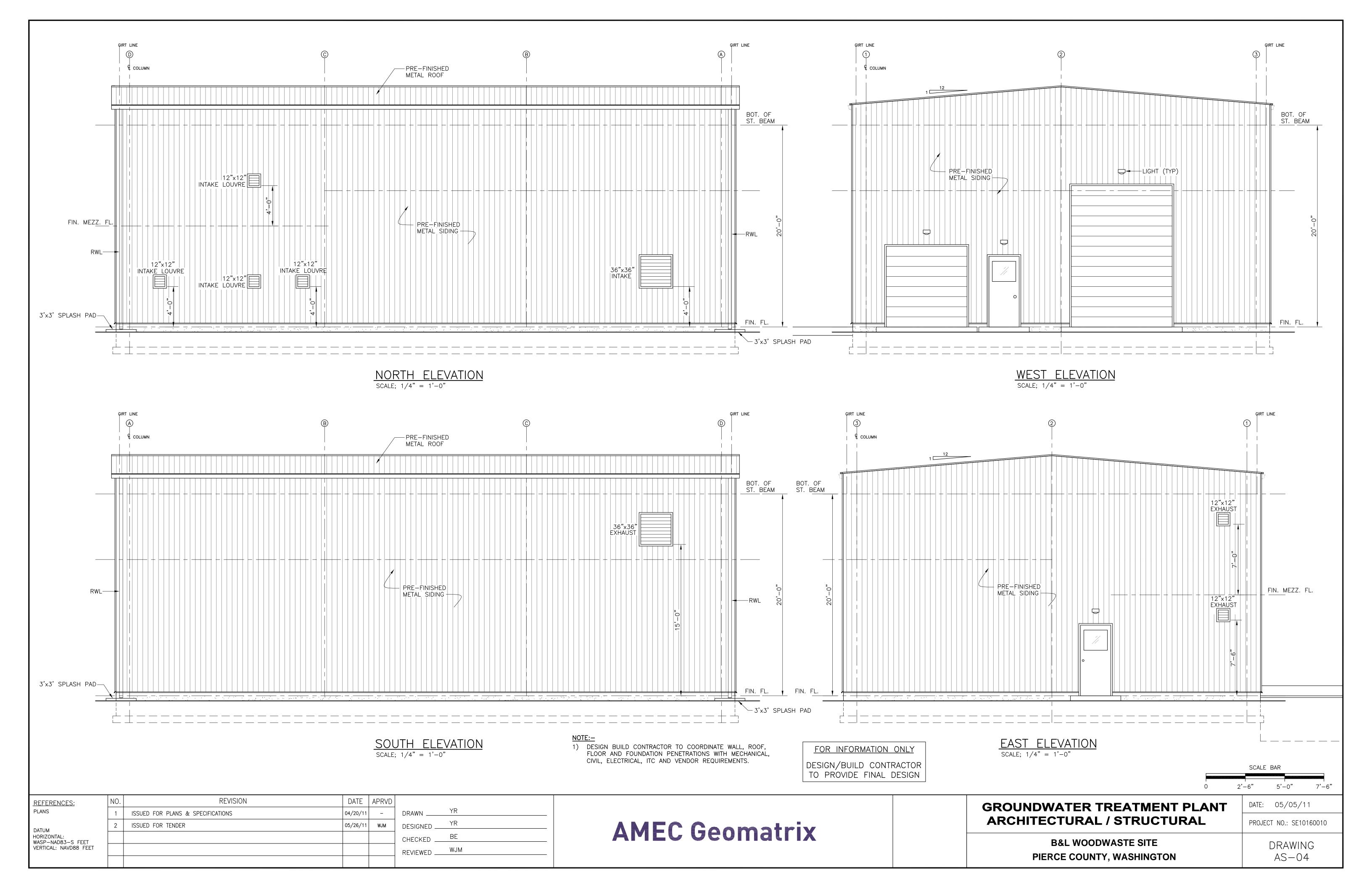
GROUNDWATER TREATMENT PLANT	DATE: 05/05/11
ARCHITECTURAL / STRUCTURAL	PROJECT NO.: SE10160010
B&L WOODWASTE SITE	DRAWING

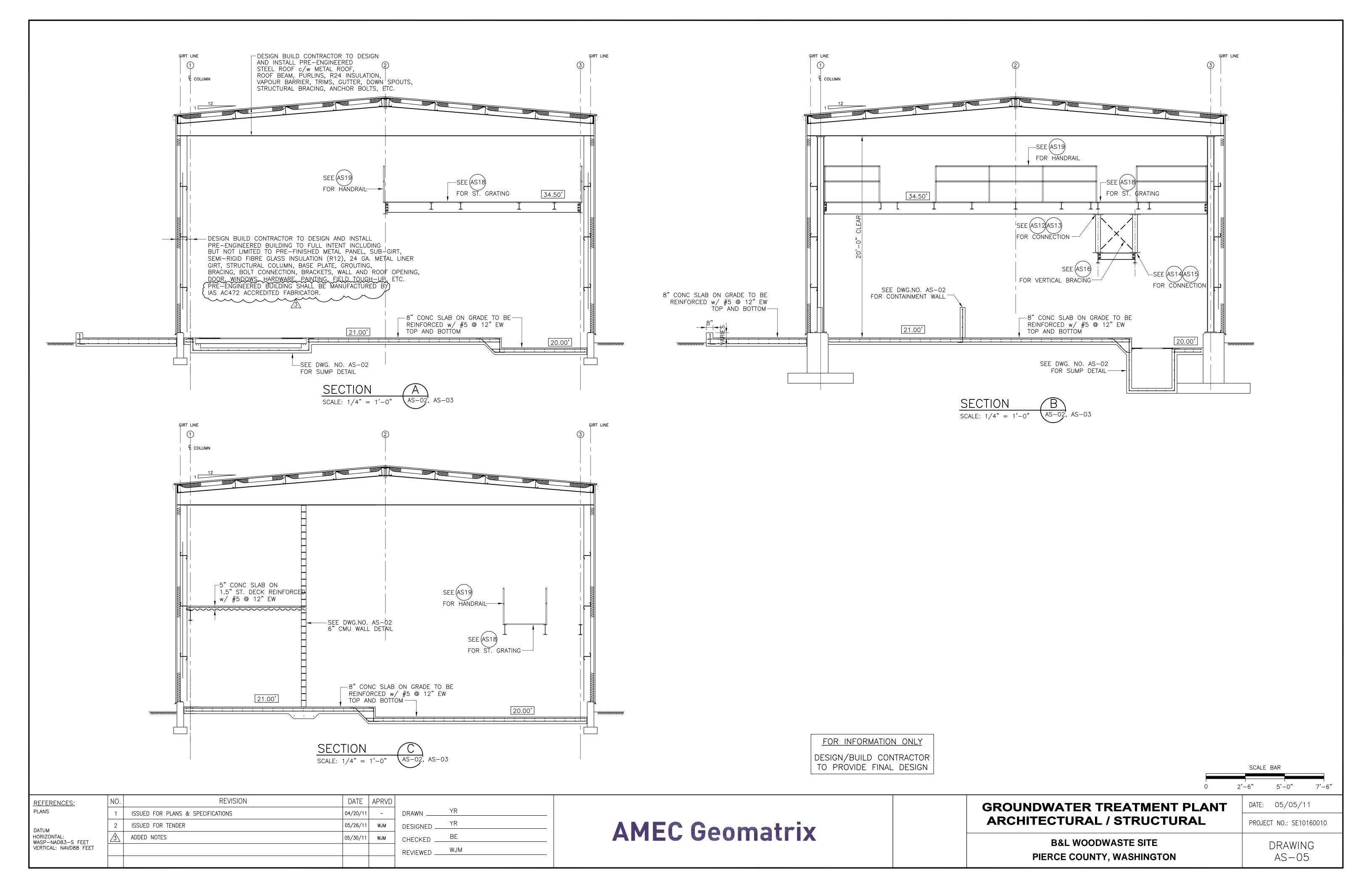
AS - 01

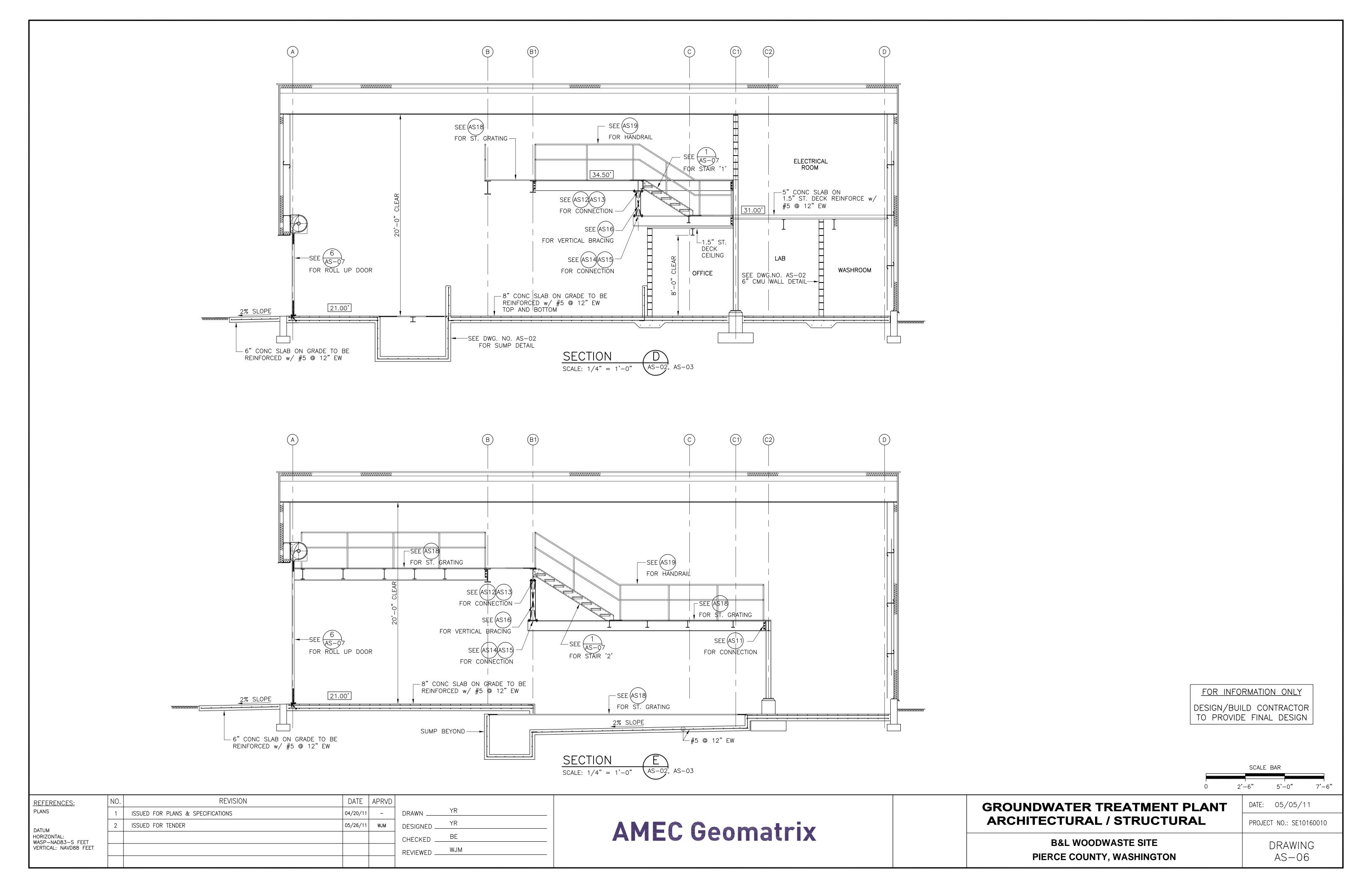
PIERCE COUNTY, WASHINGTON

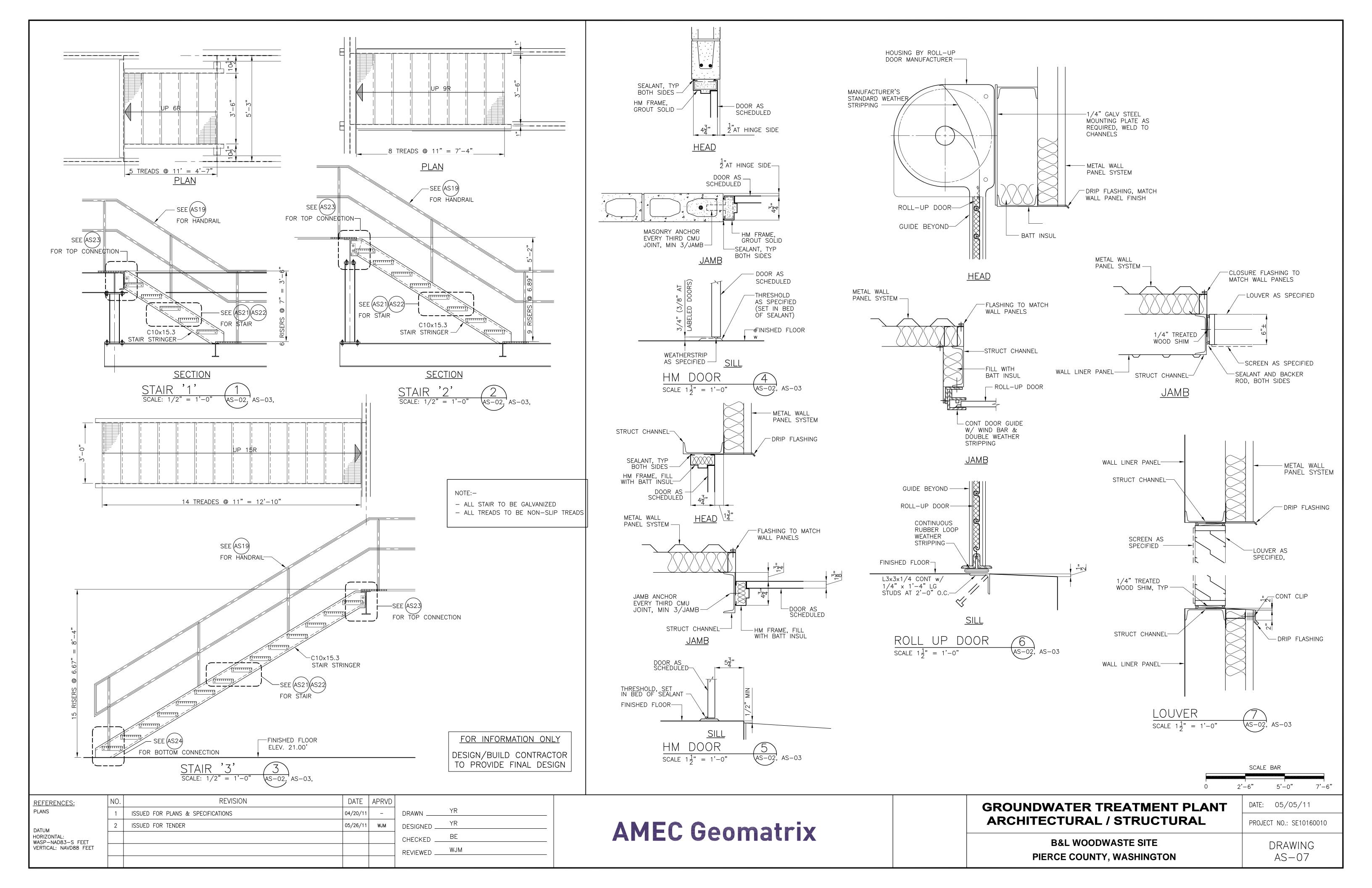












GENERAL STRUCTURAL NOTES

GENERAL

1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO LATEST EDITION OF THE INTERNATIONAL BUILDING CODE (IBC).

LOADS

SNOW LOAD 18 PSF

WIND LOAD 85 MPH, EXPOSURE "C" SEISMIC SITE CLASS D

PEAK GROUND ACCELERATION: 0.292g

SITE COEFFICIENT Fa; 1.035 SITE COEFFICIENT Fv; 1.619

FLOOR LIVE LOADS AS NOTED ON DWGS. EQUIPMENT LOADS AS NOTED ON DWGS.

FOUNDATIONS

- 1. REFER TO GEOTECHINICAL INVESTIGATION REPORT (MARCH 25, 2011)
 PREPARED BY AMEC EARTH & ENVIRONMENTAL INC.

 AMEC EARTH & ENVIRONMENTAL INC. DOES NOT GUARANTEE THE
 ACCURACY OF THIS REPORT.
- 2. FOUNDATIONS OVER STRUCTURAL FILL HAVE BEEN
 DESIGNED FOR AN ALLOWABLE BEARING PRESSURE OF 1000 PSF.

CONCRETE

- 1. ALL CAST-IN-PLACE CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 4000 PSI, EXCEPT THAT CONCRETE SPECIFICALLY DETAILED AS CONCRETE FILL SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2500 PSI. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
- 2. REINFORCING STEEL FOR CONCRETE SHALL CONFORM TO ASTM A615, GRADE 60
 DEFORMED BARS. FURNISH AND ERECT IN ACCORDANCE WITH ACI MANUAL OF
 STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, ACI 315.
- 3. THE MINIMUM REINFORCING FOR ALL CONCRETE WALLS AND SLABS SHALL BE AS FOLLOWS:

WALL THICKNESS	<u>REINF EACH WAY</u>	<u>LOCATION</u>
6"	#4@12"	CENTERED
8"	#5@12 "	CENTERED
10"	#4@12 "	FACH FACE

PROVIDE LARGER SIZES AND MORE REINFORCING IN ALL SECTIONS OF CONCRETE WHERE REQUIRED BY THE DETAILS ON THE DRAWINGS OR BY THE SPECIFICATIONS.

EACH FACE

______ 1 1/2"

- 4. CLEARANCE FOR REINFORCEMENT BARS, UNLESS SHOWN OTHERWISE,
 - WHEN PLACED ON GROUND:

#6 BAR OR LARGER

- ALL OTHER CONCRETE SURFACES: #5 BAR OR SMALLER
- 5. REFER TO WALL CORNER AND WALL INTERSECTION REINFORCING DETAIL. IN GENERAL, THE WALL CORNER REINFORCING SIZES AND SPACINGS SHALL BE CALLED OUT ON THE PLANS AND
- 6. ALL BENDS, UNLESS OTHERWISE SHOWN, SHALL BE A 90 DEGREE STANDARD HOOK AS DEFINED IN LATEST EDITION OF AC1 318.

REFERENCED TO THESE DETAILS AND THE TYPICAL HORIZONTAL

WALL REINFORCING SHALL LAP WITH THE HORIZONTAL REINFORCING.

- 7. ALL WALL CORNER AND WALL INTERSECTION REINFORCEMENT BARS SHALL BE CONTINUOUS AROUND CORNERS AND THROUGH COLUMNS OR PILASTERS. REINFORCEMENT SHALL BE EXTENDED INTO CONNECTING WALLS AND LAPPED ON THE OPPOSITE FACE OF THE CONNECTING WALLS, AS INDICATED ELSEWHERE ON THIS SHEET.
- 8. VERTICAL WALL BARS SHALL BE LAPPED WITH DOWELS FROM BASE SLABS AND EXTENDED INTO THE TOP FACE OF ROOF SLABS AND LAPPED WITH TOP SLAB REINFORCEMENT. PROVIDE A MINIMUM OF TWO FULL HEIGHT VERTICAL BARS WITH MATCHING DOWELS AT WALL ENDS, CORNERS AND INTERSECTIONS WITH SIZE TO MATCH TYPICAL VERTICAL REINFORCING STEEL SHOWN OR REQUIRED BY NOTES ABOVE.

9. UNLESS INDICATED OTHERWISE, ALL REINFORCEMENT BENDS
AND LAPS, UNLESS OTHERWISE NOTED, SHALL SATISFY THE FOLLOWING
MINIMUM REQUIREMENT:

DETAIL OF REINFORCEMENT — LAP LENGTHS							
BAR SIZ	Έ	#6 OR SMALLER	#7	#8	#9		
CONC DESIGN STRENGTH		4000 PSI					
GR 40	TOP BAR	32 DIA, MIN 1'-6"	2'-6"	3'-3"	4'-0"		
	OTHER BAR	22 DIA, MIN 1'-0"	1'-9"	2'-4"	3'-0"		
OD 00	TOP BAR	45 DIA, MIN 2'-0"	3'-8"	4'-9"	6'-0"		
GR 60	OTHER BAR	32 DIA, MIN 1'-6"	2'-8"	3'-6"	4'-4"		

10. ALL CONCRETE WORKS AND STRUCTURAL GROUT SHALL BE INSPECTED BY THE WASHINGTON ASSOCIATION BUILDING OFFICIALS (WABO) CERTIFIED SPECIAL INSPECTORS EMPLOYED BY WABO CERTIFIED INSPECTION AGENCIES.

MASONRY

- 1. MORTAR SHALL CONFORM TO ASTM C270, TYPE S. MASONRY CEMENT SHALL NOT BE USED.
- 2. GROUT SHALL CONFORM TO ASTM C476 AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI CONTAINING NO MASONRY CEMENT.
- 3. CONCRETE BLOCK UNITS SHALL CONFORM TO ASTM C90 GRADE N-1 AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 1900 PSI.
- 4. THE MASONRY ASSEMBLY SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF $f' =_m 1500$ PSI, NO SPECIAL INSPECTION (USE HALF ALLOWABLE DESIGN STRESSES).
- 5. REINFORCING STEEL FOR MASONRY SHALL CONFORM TO ASTM A615, GRADE 60 FOR DEFORMED BARS.
- 6. MASONRY WALLS SHALL BE GROUTED AND BE REINFORCED AS SCHEDULED UNLESS OTHERWISE SHOWN:

S SCHEDULED UNLESS OTHERWISE SHOWN:

WALL THICKNESS VERTICAL REINF HORIZ REINF

6" #5 @ 36" OC 2 - #4 @ 4'-0" OC

8" #6 @ 48" OC 2 - #4 @ 4'-0" OC

7. HORIZONTAL REINFORCING SHALL BE PLACED AT THE BOTTOM OF WALLS, ROOF LEVEL, TOP AND BOTTOM OF ALL OPENINGS. AT THE TOP OF WALLS, AT THE MAXIMUM SPACING INDICATED IN SCHEDULE, AND ELSEWHERE AS INDICATED ON THE DRAWINGS. REINFORCING SHALL BE CONTINUOUS EXCEPT AT THE TOP AND BOTTOM OF OPENINGS WHERE IT SHALL EXTEND A MINIMUM OF 2'-0" BEYOND THE FACE OF THE OPENING OR TERMINATE IN A STANDARD HOOK AS SHOWN IN THE STANDARD DETAIL.

REINFORCING BARS SHALL BE CONTINUOUS AROUND WALL CORNERS AND THROUGH WALL INTERSECTIONS AND HOOKED WALL ENDS PER STANDARD DETAIL. ALL HORIZONTAL BARS SHALL BE PLACED IN A BOND BEAM UNIT.

- 8. VERTICAL REINFORCING SHALL BE PLACED AT CORNERS, EACH SIDE OF OPENINGS, END WALLS (INCLUDING EACH SIDE OF CONTROL JOINTS), AT MAXIMUM SPACING INDICATED IN THE SCHEDULE, AND ELSEWHERE AS INDICATED ON THE DRAWINGS. BARS SHALL BE CONTINUOUS FROM FOUNDATION TO TOP OF WALL AND LAP SPLICED A MINIMUM OF 2'-6" TO FOUNDATION DOWELS.
- 9. GROUT ALL REINFORCED CELLS AND THOSE ADDITIONAL CELLS INDICATED IN THE PLANS AND DETAILS.
- 10. LAP REINFORCING 44 BAR DIAMETERS UNLESS OTHERWISE SHOWN.
- 11. GROUT ALL CELLS WITH ANCHOR BOLTS.
- 12. CMU WALL CONTROL JOINTS SHALL BE CONTINUOUS FROM TOP OF FOUNDATION TO TOP OF WALL OR PARAPET.

STRUCTURAL STEEL

- 1. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 UNLESS SHOWN OTHERWISE ON THE PLANS. STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500, GRADE B.
- 2. ALL STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN CONFORMANCE WITH THE AISC MANUAL OF STEEL CONTRUCTION, LATEST EDITION.
- 3. ALL BOLTS SHALL BE HIGH STRENGTH BOLTS CONFORMING TO ASTM A325—F(SC) UNLESS SHOWN OTHERWISE. USE DIRECT TENSION INDICATORS AT ALL HIGH STRENGTH BOLTS. BOLTS INDICATED AS MACHINE BOLTS OR ANCHOR BOLTS SHALL CONFORM TO ASTM A307 FOR CARBON STEEL, A193 FOR STAINLESS STEEL AND A153 FOR GALVANIZED STEEL.
- INSPECTION IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.

 ALL WELDING SHALL BE PERFORMED WITH E70XX

 ELECTRODES EXCEPT THAT E60XX ELECTRODES SHALL BE USED FOR METAL

 DECKING. ALL WELDS SHALL BE PERFORMED BY CERTIFIED WELDERS.

 ALL WELDS AND WELD INSPECTIONS SHALL BE DONE BY THE WASHINGTON ASSOCIATION

 BUILDING OFFICIALS (WABO) CERTIFIED WELDERS AND INSPECTORS EMPLOYED BY

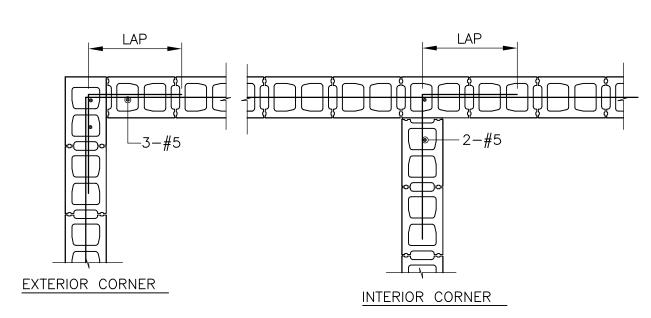
4. ALL WELDS SHALL CONFORM TO THE CURRENT EDITION OF AWS D1.1. PERIODIC

- 6. ALL STRUCTURAL STEEL TO BE EMBEDDED IN CONCRETE OR TO RECEIVE FIREPROOFING MATERIAL SHALL BE CLEAN AND FREE OF PAINT, OIL OR DIRT. ALL OTHER STRUCTURAL STEEL SHALL RECEIVE ONE COAT OF RUST INHIBITING PRIMER. SEE SPECIFICATIONS FOR ADDITIONAL PAINTING REQUIREMENTS.
- 7. SEE THE SPECIFICATIONS FOR ADDITIONAL STRUCTURAL STEEL REQUIREMENTS.

METAL DECKING

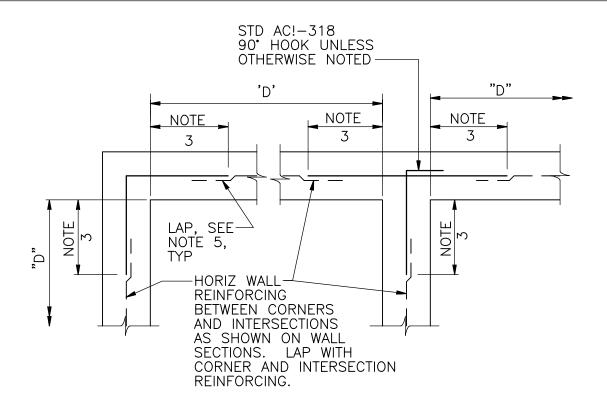
WABO CERTIFIED INSPECTION AGENCIES.

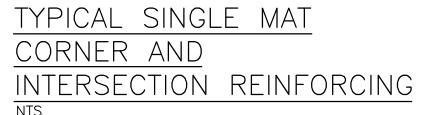
1. ROOF DECKING SHALL BE 1 1/2" DEEP x 22 GA (I=0.183 IN /FT, S=0.209 IN /FT) GALV STEEL DECK, FLOOR DECKING SHALL BE 1 1/2" DEEP x 18 GA (I=0.338 IN /FT, S=0.395 IN /FT) AS SPECIFIED. PROVIDE (5) 3/4" DIA PUDDLE WELDS PER 36" PANEL AT ALL SUPPORTS PERPENDICULAR TO DECK RIBS. PROVIDE 3/4" DIA. PUDDLE WELDS AT 12" ON CENTER AT ALL SUPPORTS PARALLEL TO DECK RIBS. SIDE SEAMS SHALL BE BUTTON PUNCHED AT 18" ON CENTER AND AS RECOMMENDED BY THE MANUFACTURER, UNLESS OTHERWISE NOTED.



NOTE:- LAP = 30 BAR DIAMETERS OR 2'-0" MIN. UNLESS OTHERWISE NOTED.









CORNER AND INTERSECTION REINFORCING DETAILS

- 1. TYPICAL HORIZONTAL WALL CORNER AND INTERSECTION REINFORCING LAYOUT IS SHOWN TO AVOID CONGESTION AND PERMIT PROPER PLACEMENT, FOR SIZE AND SPACING SEE PLANS. ALL HORIZONTAL REINFORCING AT CORNERS AND INTERSECTIONS SHALL BE FABRICATED AND INSTALLED WITH SPLICES LOCATED WHERE SHOWN REGARDLESS OF BAR SIZE AND SPACING.
- 2. WHERE THE CORNER OR INTERSECTION REINFORCING SIZE AND SPACING IS NOT SHOWN, NOTED OR TABULATED ON THE PLANS, THE SIZE AND SPACING SHALL BE THE SAME AS THE WALL HORIZONTAL REINFORCING SHOWN ON THE WALL SECTIONS OR AS NOTED FOR THE REINFORCING BETWEEN THE CORNERS OR INTERSECTIONS.
- 3. EXCEPT WHERE OTHERWISE SHOWN ON THE DRAWINGS, THE LENGTH INDICATED AS "NOTE 3" SHALL BE THE LESSER OF D/4, 10 FEET, OR 1.0 TIMES THE HEIGHT OF THE WALL, EXCEPT THAT IN NO CASE SHALL IT BE LESS THAN 2.0 FEET.
- 4. D = LENGTH OF WALL PARALLEL TO THE BAR LENGTH IN QUESTION.
- 5. EXCEPT WHERE OTHERWISE SHOWN ON THE DRAWINGS, THE LENGTH INDICATED AS "NOTE 5" SHALL BE EQUAL TO ONE "LAP LENGTH" AS REQUIRED BY THE GENERAL STRUCTURAL NOTES. USE THE LAP LENGTH AS REQUIRED FOR THE SMALLER OF THE TWO REINFORCING BARS BEING SPLICED.
- 6. UNLESS OTHERWISE NOTED, "B" AND "C" BARS ARE THE SAME SIZE AND SPACING AND, "F" AND "G" BARS ARE THE SAME SIZE AND SPACING.

REFERENCES:	NO.	REVISION	DATE	APRVD		
PLANS	1	ISSUED FOR PLANS & SPECIFICATIONS	04/20/11	-	DRAWN	YR
DATUM HORIZONTAL: WASP-NAD83-S FEET VERTICAL: NAVD88 FEET	2	ISSUED FOR TENDER	05/26/11	WJM	DESIGNED	YR
	3	ADDED NOTES	05/30/11	WJM	CHECKED	BE
					REVIEWED	WJM

AMEC Geomatrix

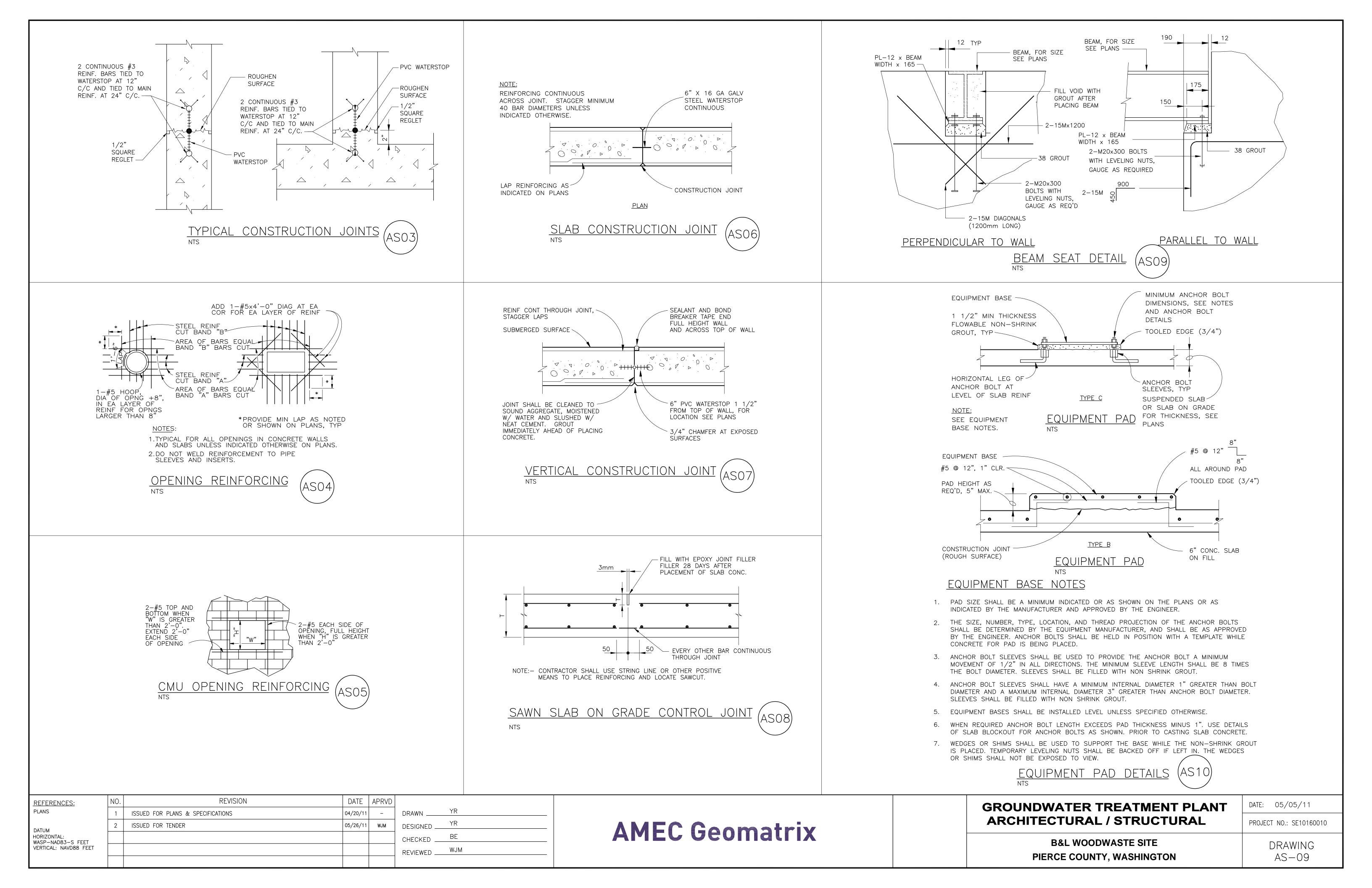
GROUNDWATER TREATMENT PLANT ARCHITECTURAL / STRUCTURAL

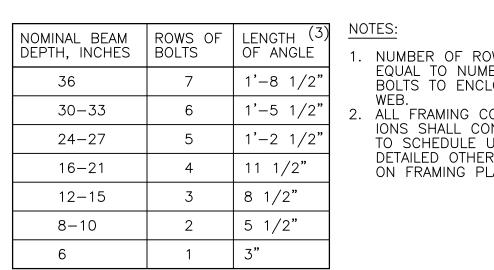
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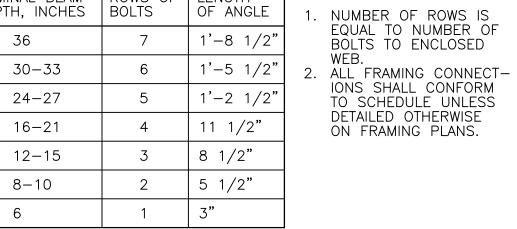
B&L WOODWASTE SITE
PIERCE COUNTY, WASHINGTON

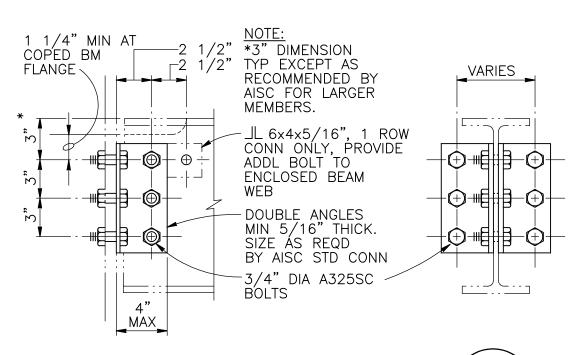
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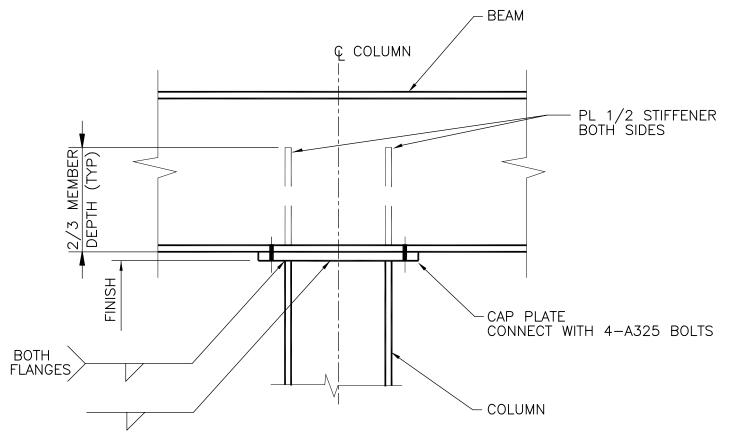




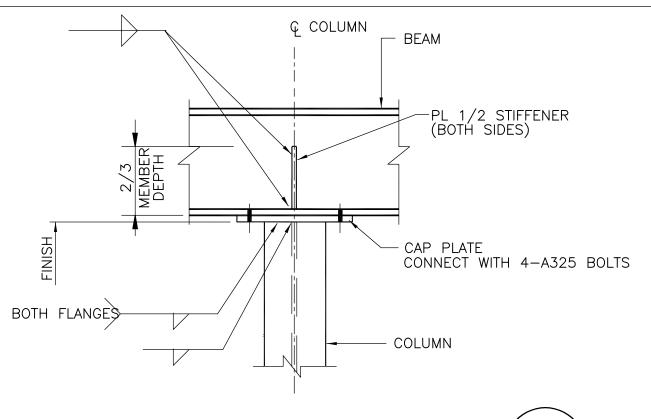




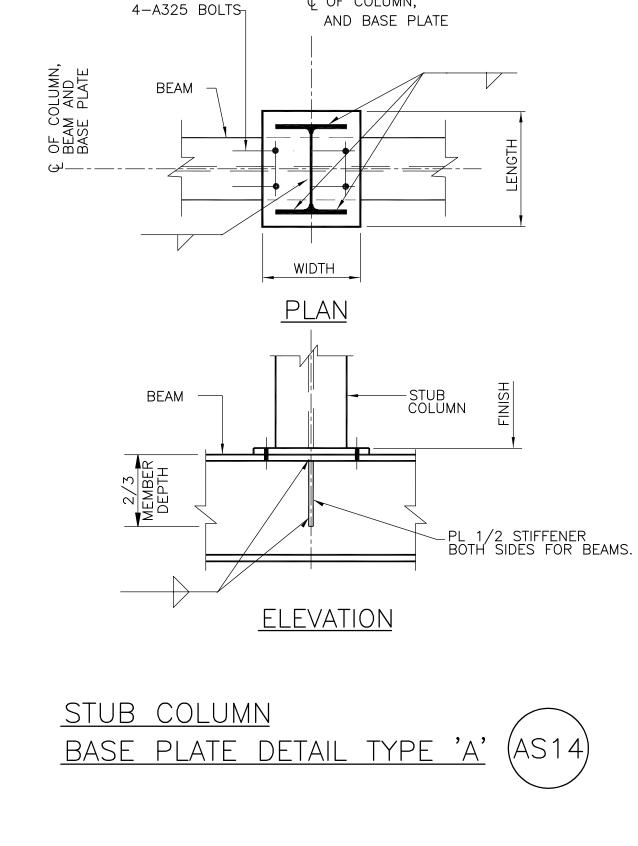
TYPICAL FRAMING CONNECTION NTS



BEAM CONNECTION TO COLUMN CAP PLATE TYPE 'A'

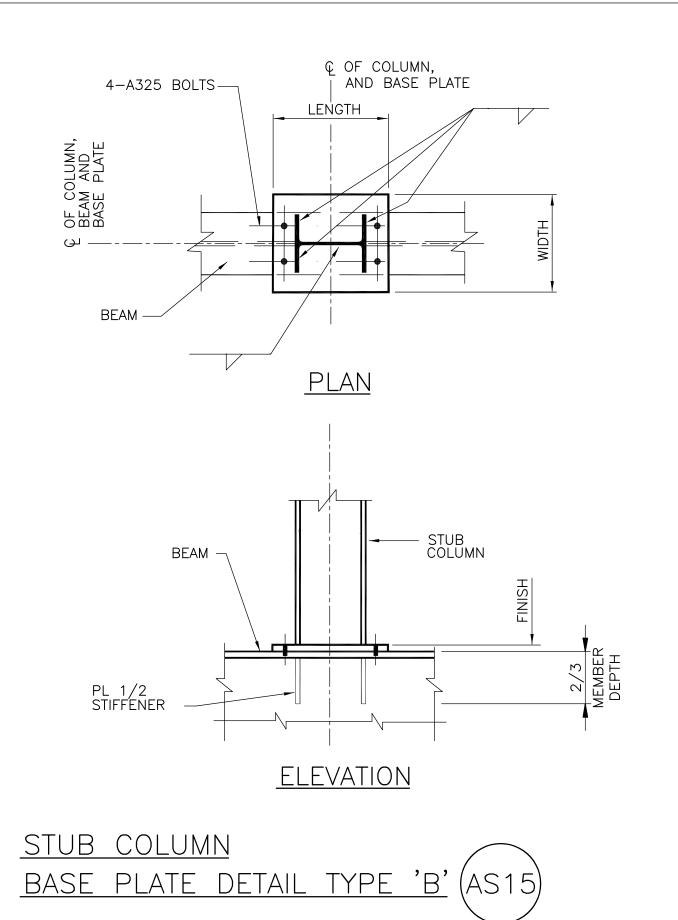


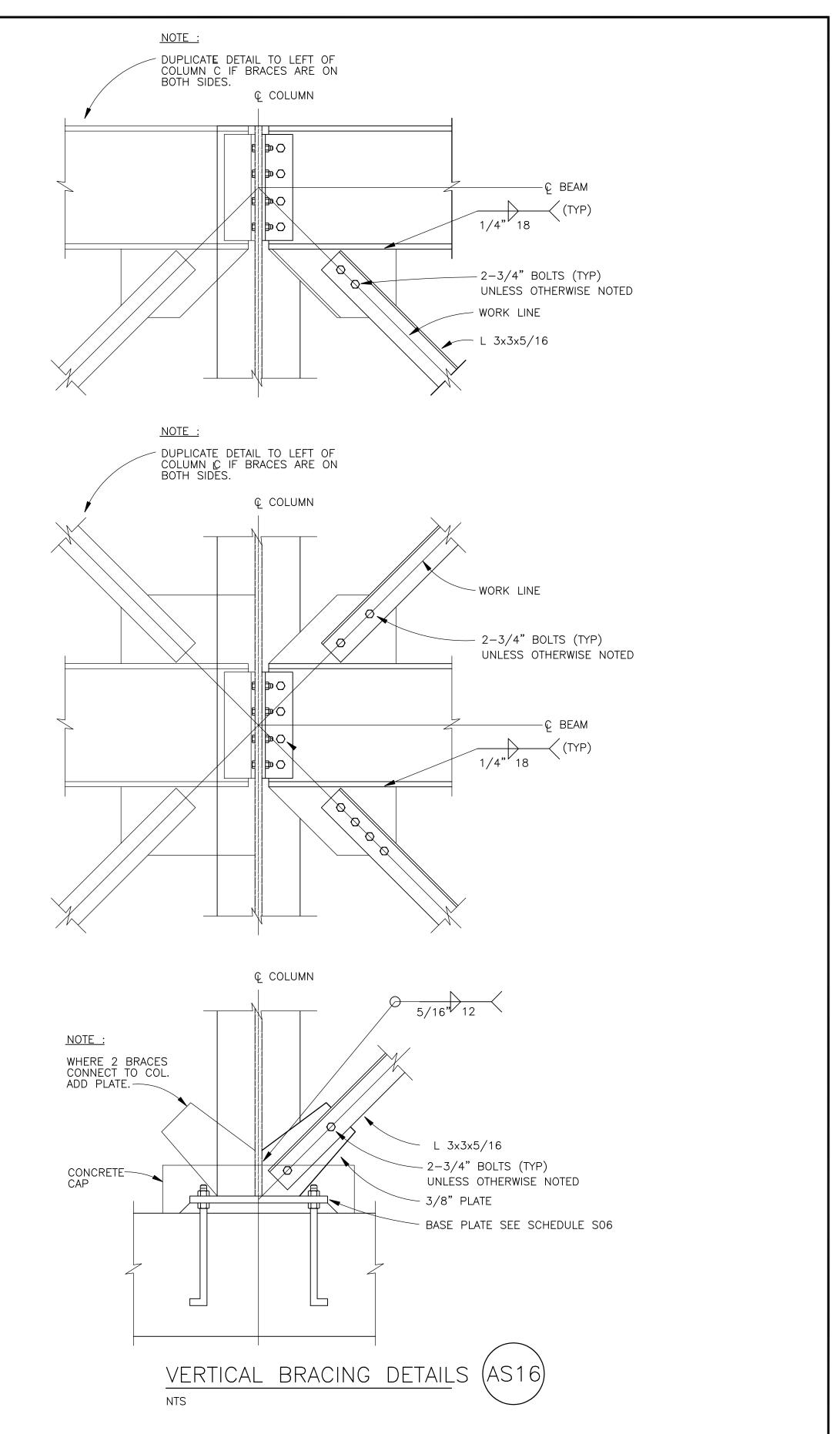
BEAM CONNECTION TO COLUMN CAP PLATE TYPE 'B'





Ç OF COLUMN,

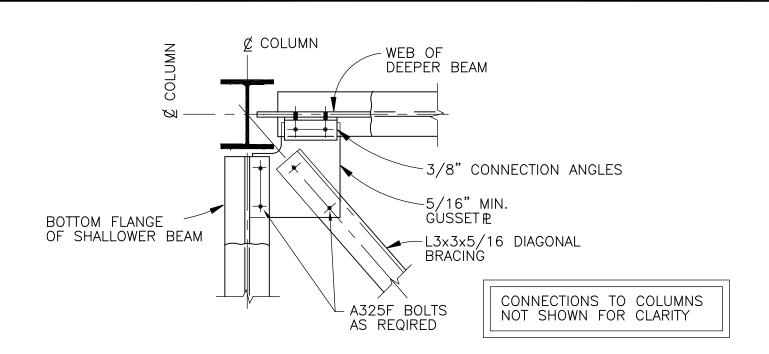




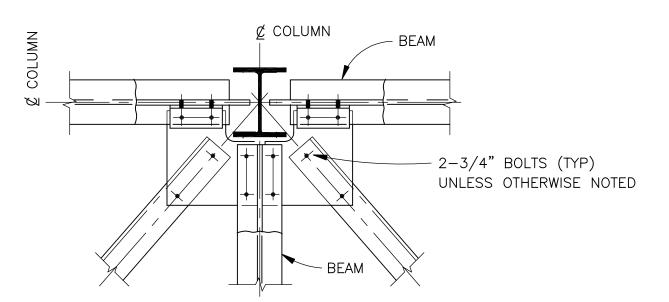
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DATUM	2	ISSUED FOR TENDER	05/26/11	WJM	
HORIZONTAL: WASP-NAD83-S FEET VERTICAL: NAVD88 FEET					
] F

DRAWN _ DESIGNED _ CHECKED BE REVIEWED ____WJM

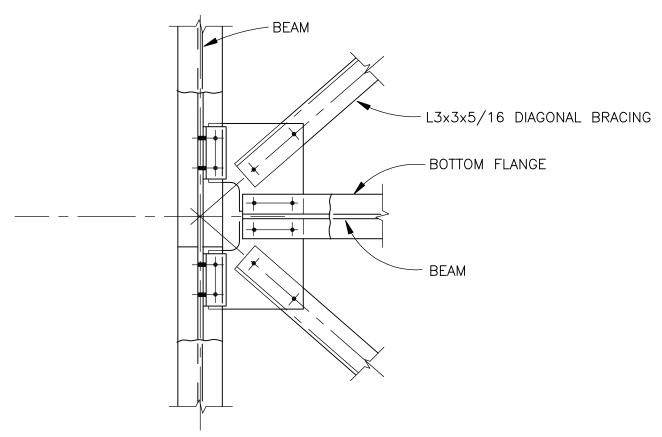
GROUNDWATER TREATMENT PLANT	DATE: 05/05/11
ARCHITECTURAL / STRUCTURAL	PROJECT NO.: SE10160010
B&L WOODWASTE SITE	DRAWING
PIERCE COUNTY, WASHINGTON	AS-10



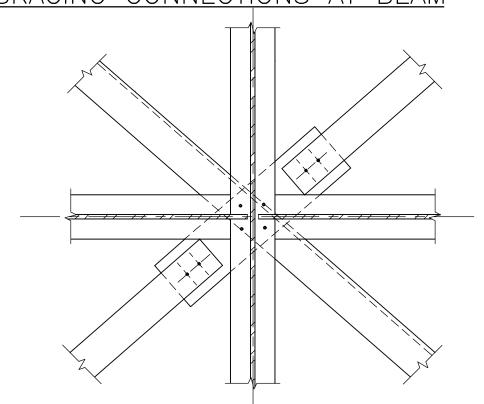
BRACING CONNECTIONS AT CORNERS



BRACING CONNECTIONS AT COLUMNS

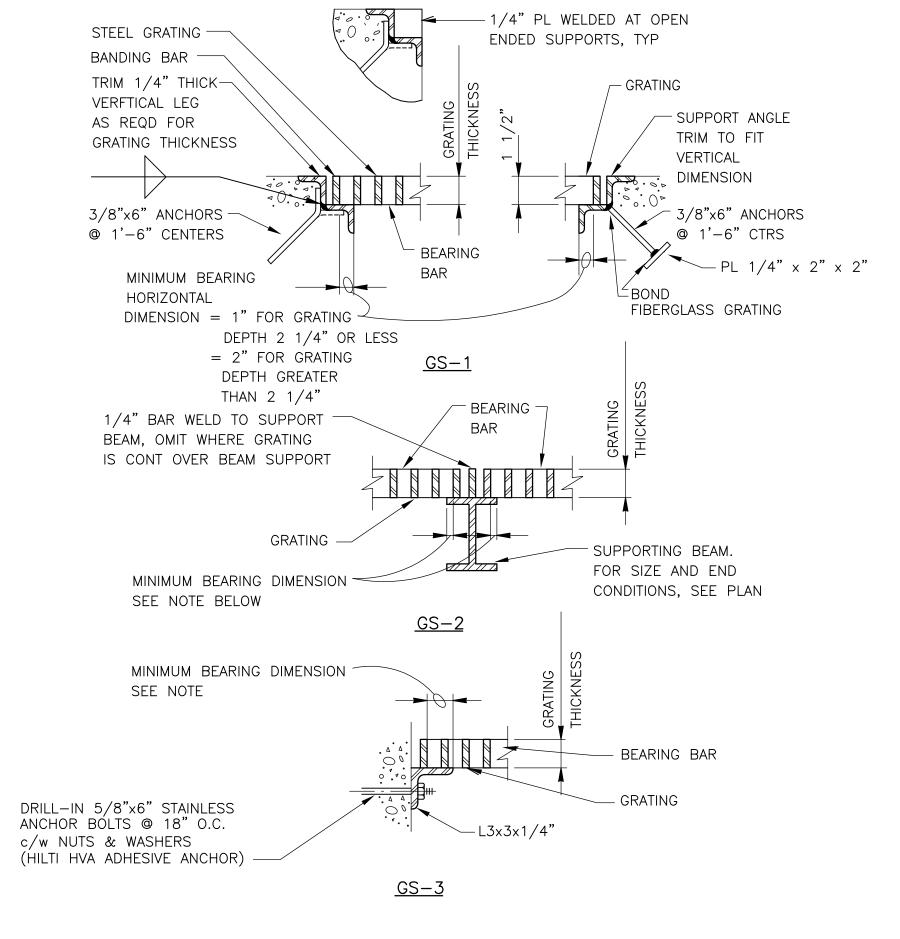


BRACING CONNECTIONS AT BEAM



BRACING CONNECTIONS AT BEAM

HORIZONTAL BRACING DETAIL



GRATING SUPPORT

ALL STEEL GRATING TO BE GALVANIZED

	FOOT TRAFFIC GRATING THICKNESS TAE	BLE
MAXIMUM SPAN	STEEL (IN.)	FIBERGLASS (IN.)
3'-6"	1"	1 1/2"
4'-0"	1"	1 1/2"
4'-6"	1"	MAXIMUM ALLOWABLE
5'-0"	1 1/4"	SPAN IS 4'-0"
5'-6"	1 1/4"	DEFLECTION TO
6'-0"	1 1/2"	1/4" MAXIMUM.

SHALL BE SPACED AT 1 7/8" OC.

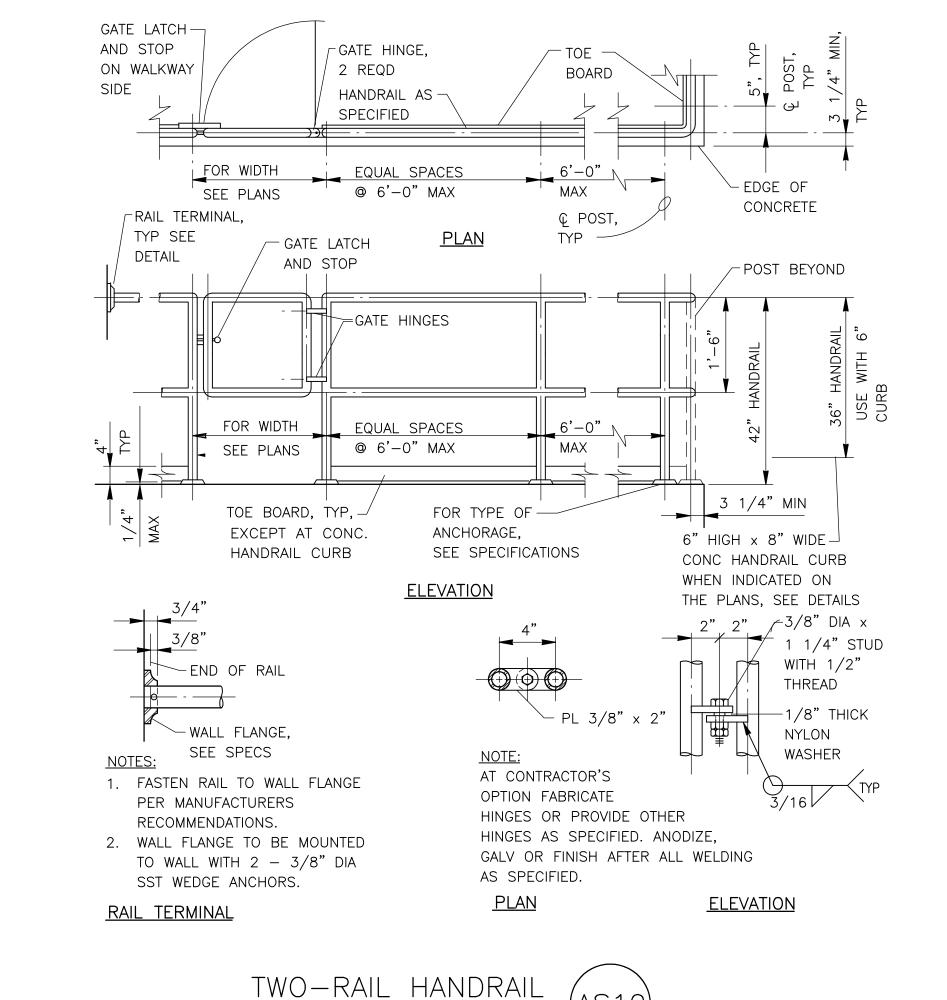
STEEL GRATING BEARING BARS FOR HEAVY VEHICULAR TRAFFIC

HEAVY VEHICL (HS 20-	
MAXIMUM SPAN	STEEL (IN.)
1'-8"	2 1/2"x 1/4"

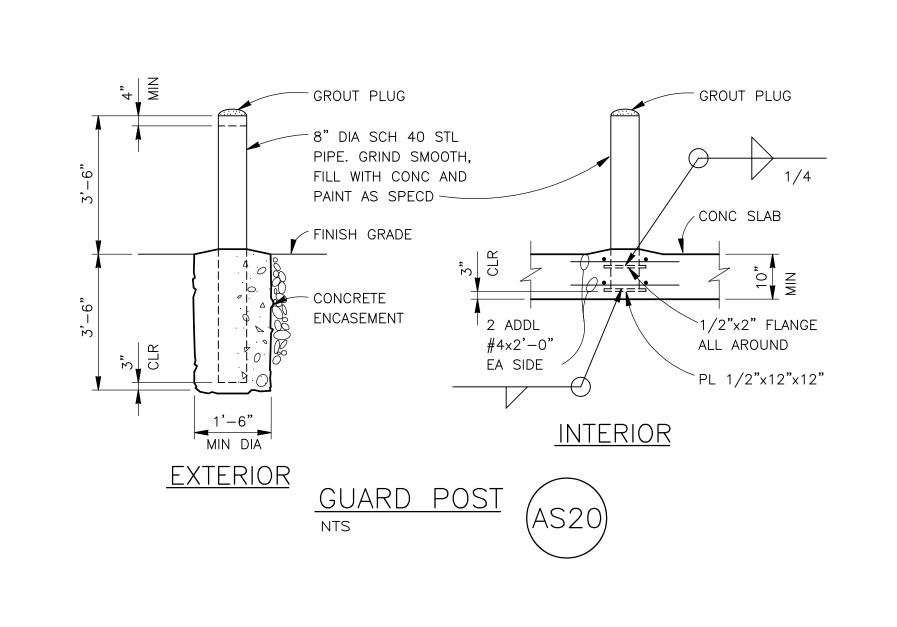
STEEL GRATING NOTES

- 1. EXTEND GRATING CONTINUOUSLY OVER GATE GUIDES AND GATES.
- NOTCH GRATING SUPPORTS AT GATES AS REQUIRED.
- GRATING SPAN SEE PLAN.
- WIDTH OF GRATING SECTIONS SHALL NOT EXCEED 3'-0". SHOP DRAWINGS BASED ON FIELD DIMENSIONS SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO FABRICATION.
- MATERIAL FOR SUPPORTS OF STEEL GRATING TO BE SAME AS GRATING.
 UNLESS NOTED OTHERWISE ON PLANS, GRATING THICKNESS SHALL BE AS TABULATED IN "GRATING THICKNESS TABLE" FOR APPLICABLE TRAFFIC.
- 8. BEARING BAR THICKNESS FOR GRATING TO BE 3/16" MINIMUM. BAND ALL EDGES WITH 3/16" x DEPTH OF BEARING BAR.
- 10. PROVIDE MISCELLANEOUS GRATING FASTENERS AS REQUIRED.
- 11. THE HORIZONTAL CLEARANCE BETWEEN THE GRATING AND GRATING SUPPORTS SHALL NOT BE LESS THAN 1/4" NOR GREATER THAN 1/2" AND AS SPECIFIED.
- 12. ALL GRATING SECTIONS, WHEN IN PLACE, SHALL ALWAYS BE FIRMLY ANCHORED TO THEIR SUPPORTS AS SPECIFIED.





NOTE:- ALL HANDRAIL TO BE GALVANIZED.



NOTE:							
USF 4	-3/4"	DIA.	A325	BOLTS	FOR	CONNECTION	

REFERENCES:	NO.	REVISION	DATE	APRVD	
PLANS	1	ISSUED FOR PLANS & SPECIFICATIONS	04/20/11	_	DR
DATUM	2	ISSUED FOR TENDER	05/26/11	WJM	DE
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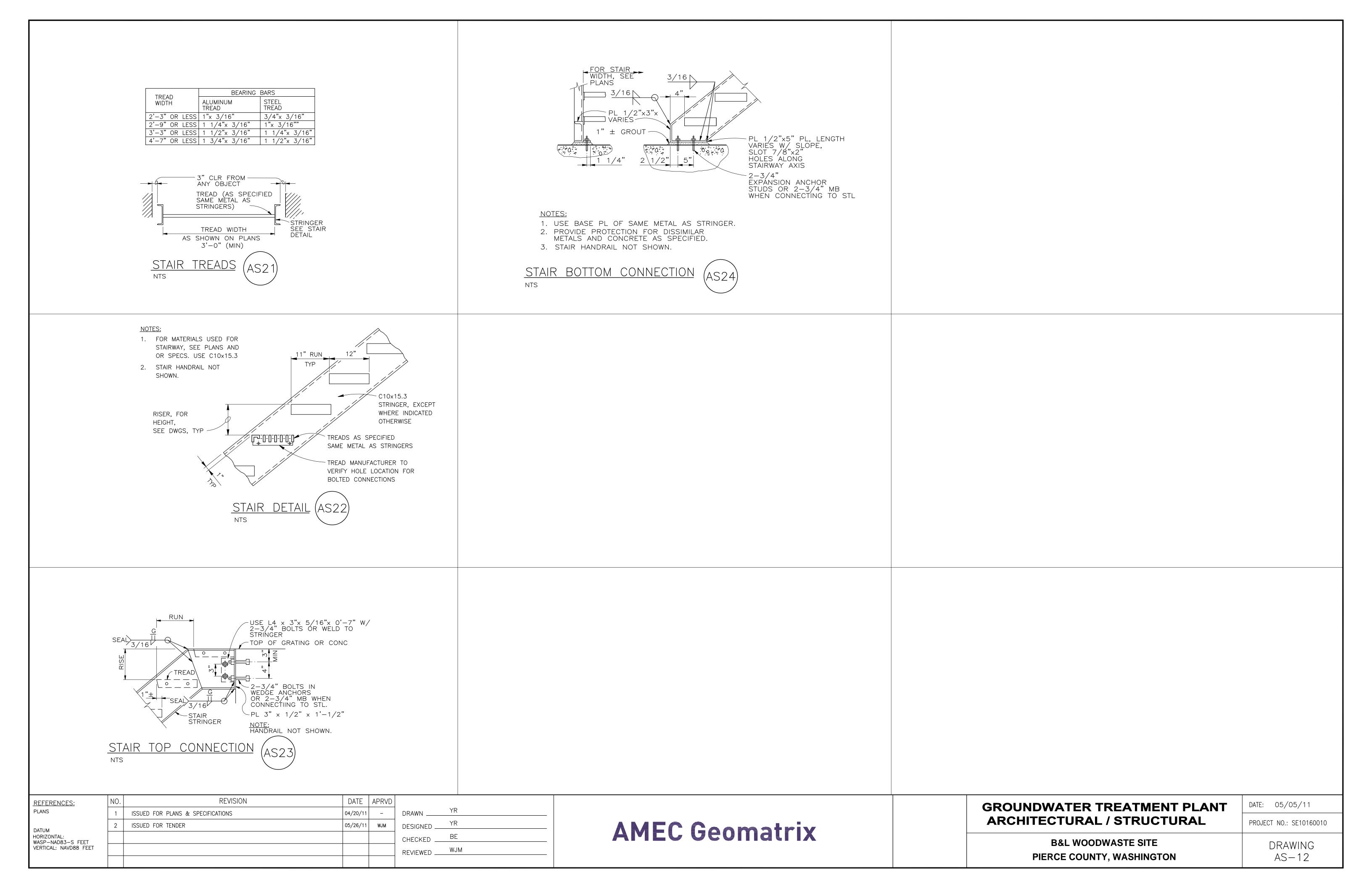
DESIGNED _ CHECKED . REVIEWED ___

AMEC Geomatrix

GROUNDWATER TREATMENT PLANT
ARCHITECTURAL / STRUCTURAL

B&L WOODWASTE SITE PIERCE COUNTY, WASHINGTON PROJECT NO.: SE10160010 DRAWING AS-11

DATE: 05/05/11





IOTES:

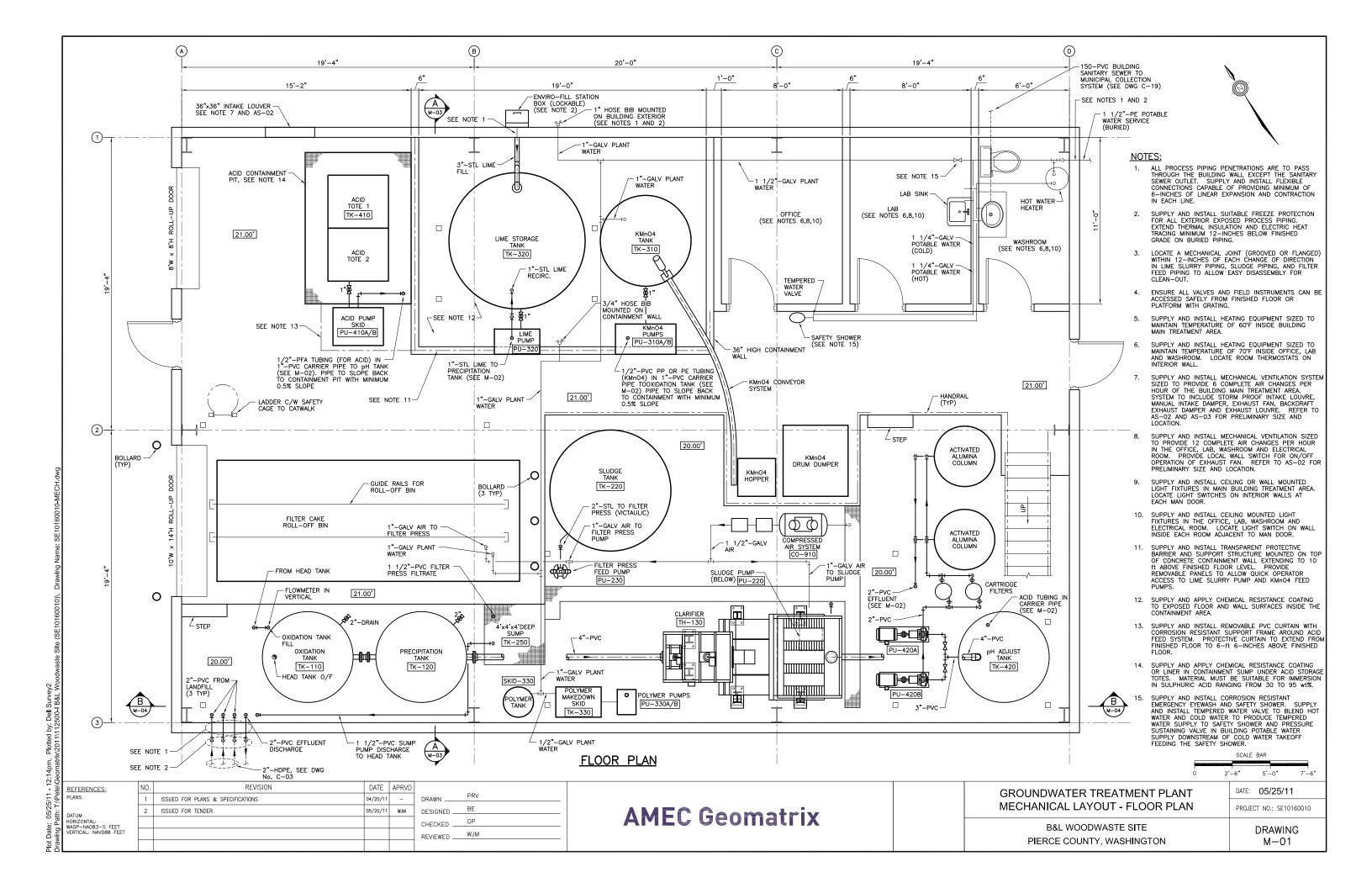
- RENDERING PROVIDED TO SHOW AESTHETIC CONCEPTS ONLY. AESTHETIC DETAILS DESIRED: TWO-TONE COLOR SCHEME, CUPOL/ ROOF OVERHANG, AWNING OVER MAN-DOOR. THIS RENDERING DOES NOT REFLECT PROPER LOCATION OR NUMBER OF DOORS, EXHAUST FANS ETC. WHERE THIS RENDERING CONFLICTS WITH OTHER DRAWINGS, OTHER DRAWINGS SHALL TAKE PRECEDENCE.
 - CONTRACTOR TO SUBMIT DRAWINGS, RENDERING, AND COLOR SAMPLES OF BUILDING FOR APPROVAL BY OWNER BEFORE ANY BUILDING MATERIALS ARE ORDERED AND ANY CONSTRUCTION BEGINS.

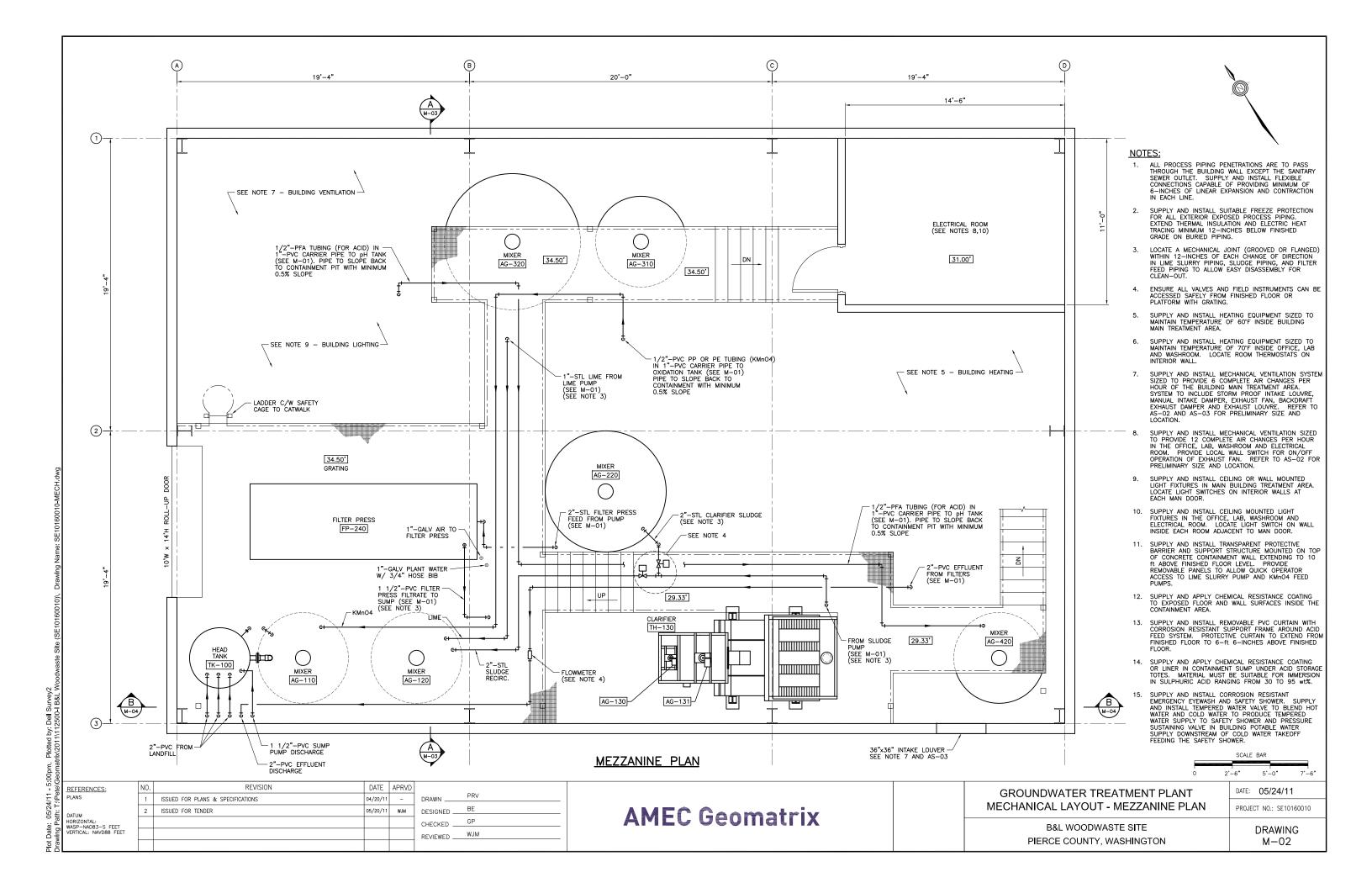
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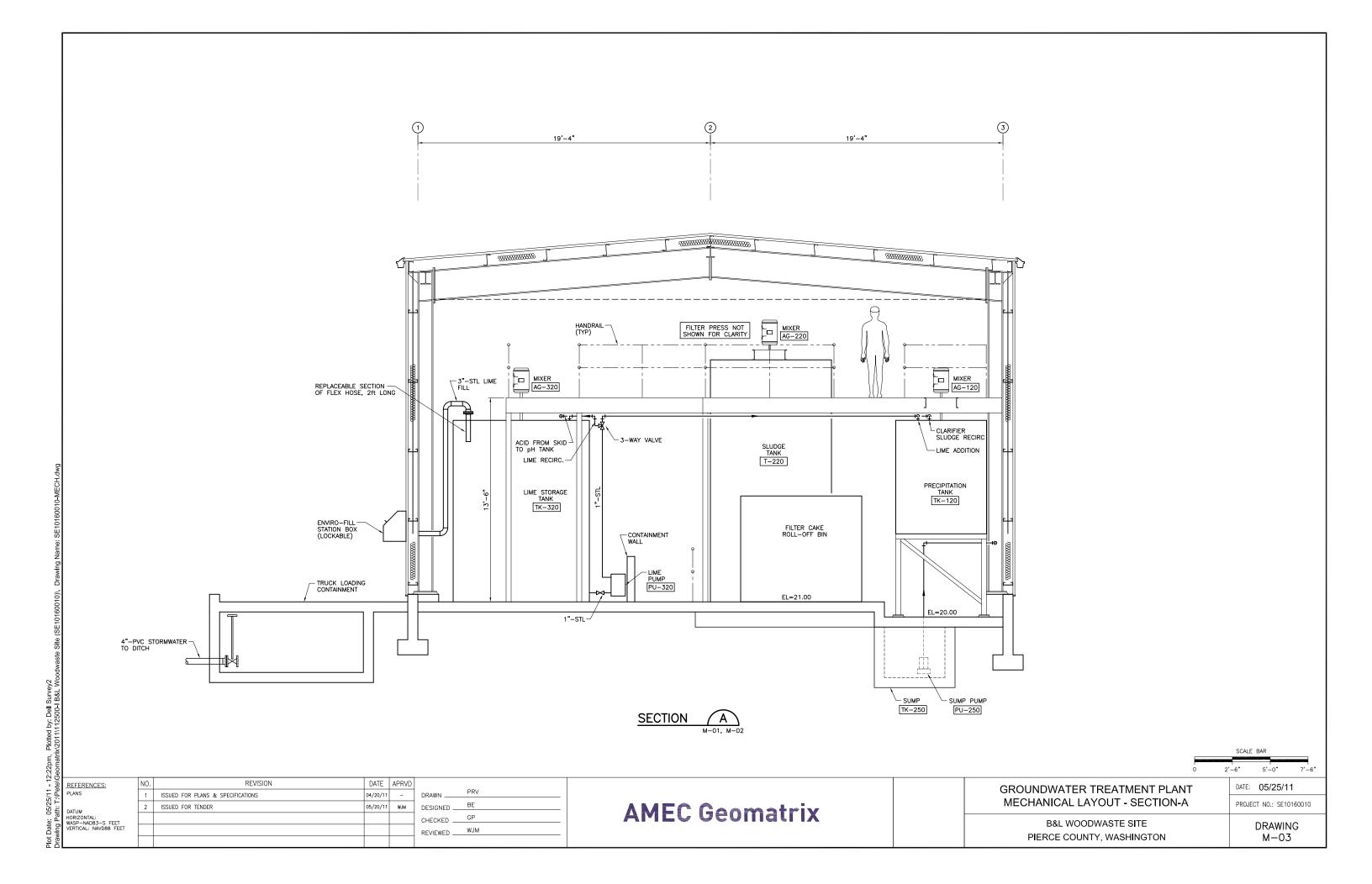
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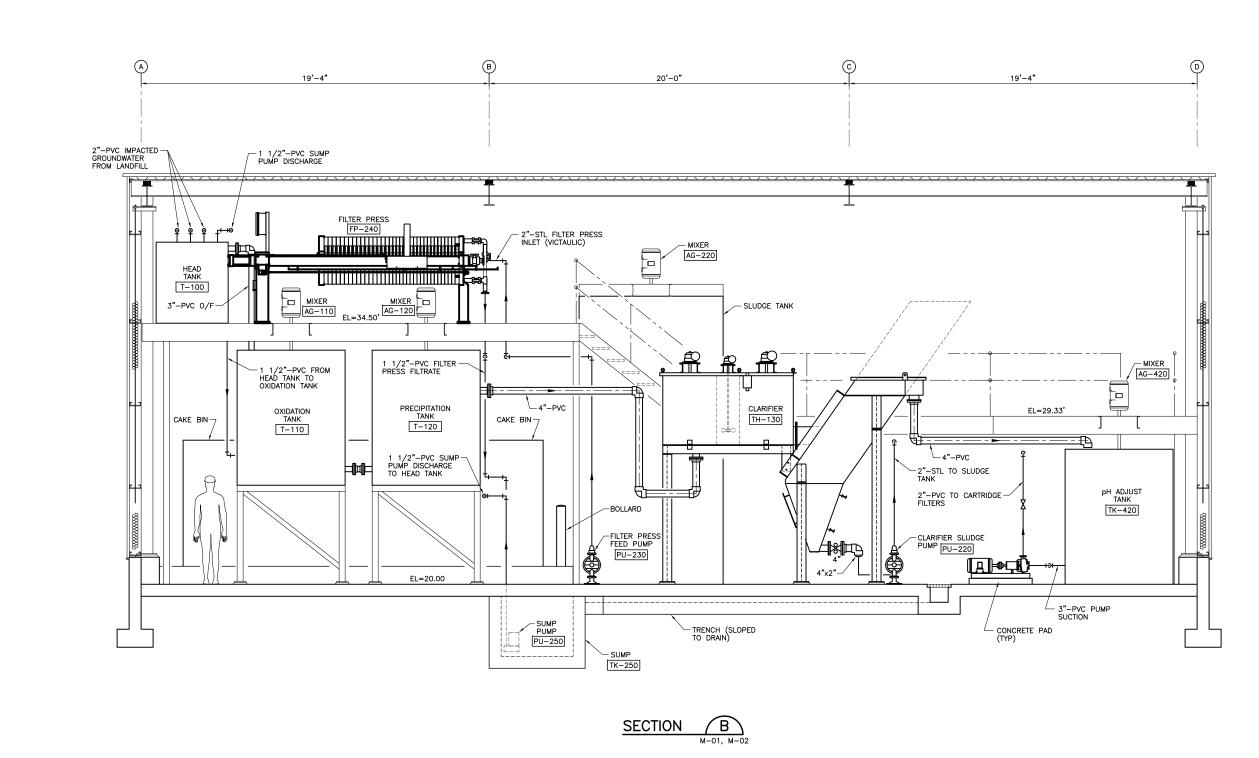
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GROUNDWATER TREATMENT PLANT	DATE:	05/05/
BUILDING CONCEPTUAL RENDERING	PROJEC	CT NO.: SE1
B&L WOODWASTE SITE		DRAWII
PIERCE COUNTY, WASHINGTON		AS-1









REFERENCES:	NO.	REVISION	DATE	APRVD		
PLANS	1	ISSUED FOR PLANS & SPECIFICATIONS	04/20/11	-	DRAWN	PRV
	2	ISSUED FOR TENDER	05/20/11	WJM	DESIGNED	BE
DATUM HORIZONTAL:					CHECKED	GP
WASP-NAD83-S FEET VERTICAL: NAVD88 FEET					REVIEWED	WJM

AMEC Geomatrix

GROUNDWATER TREATMENT PLANT	DATE: 05/25/11
MECHANICAL LAYOUT - SECTION-B	PROJECT NO.: SE10160010
B&L WOODWASTE SITE PIERCE COUNTY, WASHINGTON	DRAWING M-04

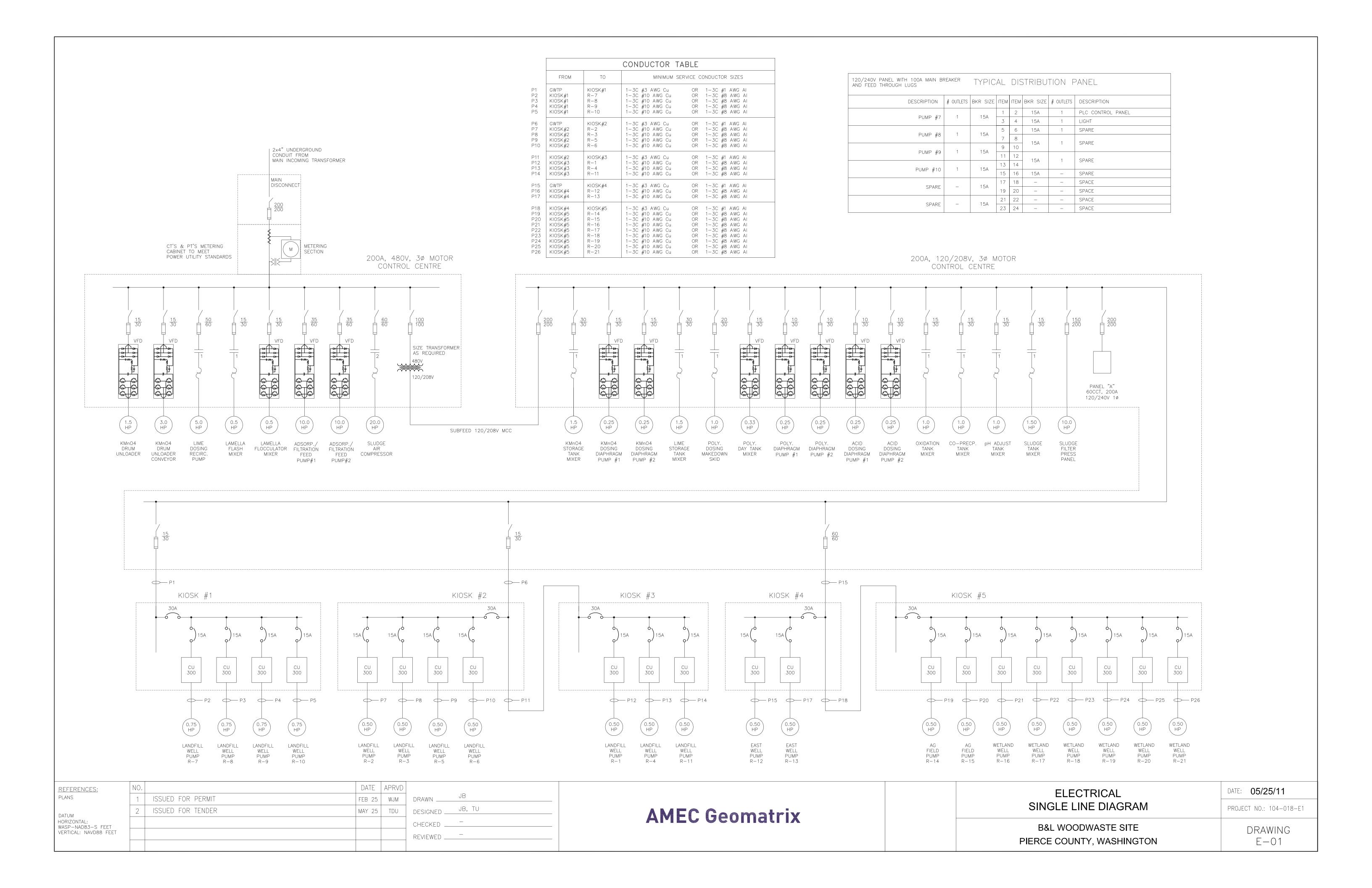
ot bate. 1905/011 12.5-0pm; 1100ed by Dell Sativeys. rawing Path: T:Pete\Geomatrix\2011/112500-I B&L Woodwaste Site (SE10160010)), Drawing Name:

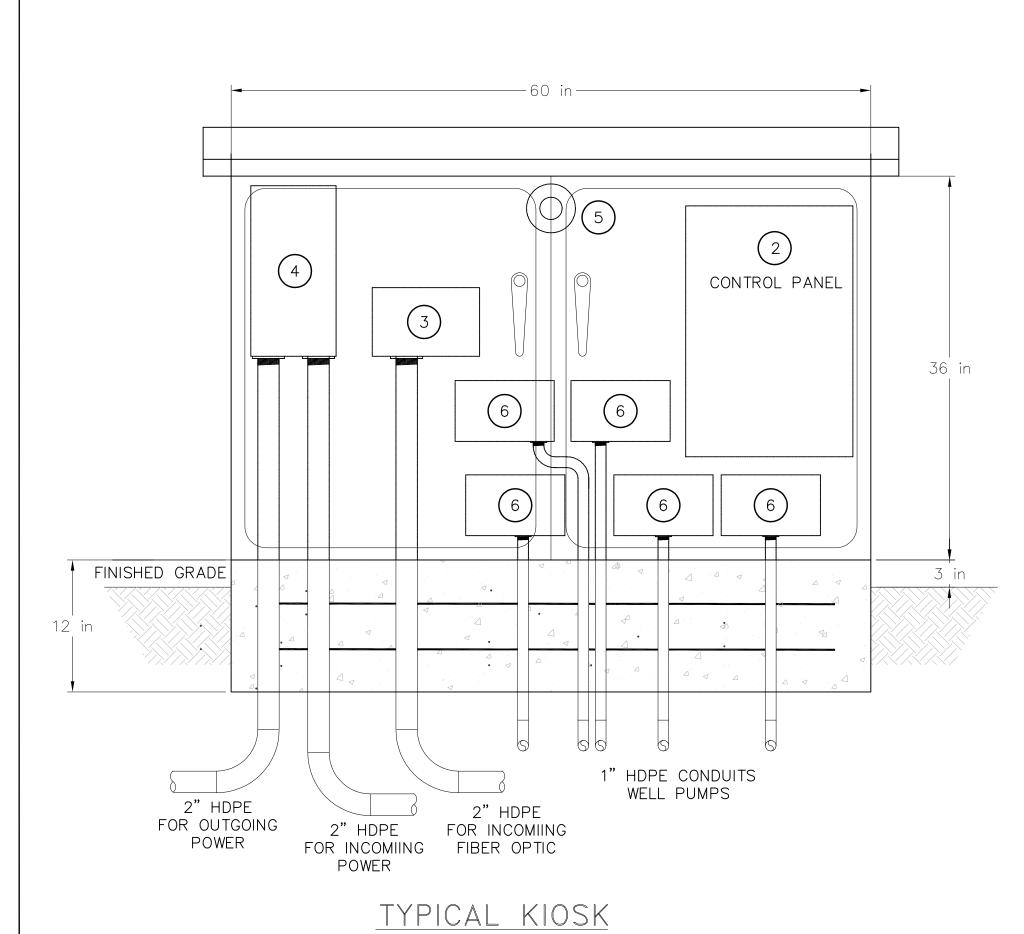
Service	Pipe Material	Туре	Pressure Rating	Joint Type/Joining Method	Minimum Support Spacing	Anchor Rod Size	Maximum Operating Temperature	Maximum Operating Pressure	Test Pressure	Test Fluid	Test Duration	Remarks
Impacted Groundwater - buried	HDPE	DR11	160 psi	butt fusion welded	continuous	n/a	100F	100 psi	150 psi	water	2 hrs	provide suitable freeze protection on any section with less than 12-inches of cover (minimum 1-inch closed cell foam insulation, self regulated heat tracing cable and watertight jacket)
Impacted Groundwater - exposed	LIDDE	DD44	160 psi	butt fusion welded and flanged	5 feet	2/9 in ab	1005	100 psi	150 psi		2 h.m.	provide suitable freeze protection (minimum 1-inch closed cell foam insulation, self regulated heat tracing cable and watertight jacket); use flanged end connections as required to mate with valves and process equipment
Impacted Groundwater/Process - pressurized	HDPE PVC	DR11 Sch 40	280 psi @ 73F	socket solvent welded and flanged	5 feet	3/8-inch 3/8-inch	100F 100F	100 psi	150 psi	water	2 hrs	use flanged end connections as required to mate with valves and process equipment
Process - gravity	PVC	Sch 40	280 psi @ 73F	socket solvent welded or flanged	5 feet	3/8-inch	100F	5 psi	n/a	water	2 hrs	use flanged end connections as required to mate with valves and process equipment
Potassium Permanganate Solution (3.5% KMnO4)	braided PVC, PP or polyethylene tubing	0.062-inch wall thickness	100 psig	compression fitting	continuous	n/a	100F	50 psi	75 psi	water	2 hrs	use single continuous section of tubing run inside Sch 40 PVC containment pipe
Lime Slurry	Steel	Sch 40	150 psi @ -40/+250F	grooved coupling or flanged	6 feet	3/8-inch	100F	100 psi	150 psi	water	2 hrs	use long radius bends; provide coupling or flange within 1 foot of any change in direction; use flanged end connections as required to mate to valves or process equipment
Polymer Solution	braided PVC, PP or polyethylene tubing	0.062-inch wall thickness	100 psig	compression fitting	continuous	n/a	100F	50 psi	75 psi	water	2 hrs	use single continuous section of tubing
Clarifier Sludge/Filter Press Feed	Steel	Sch 40	150 psi @ -40/+250F	grooved coupling or flanged	6 feet	3/8-inch	100F	100 psi	150 psi	water	2 hrs	use long radius bends; provide coupling or flange within 1 foot of any change in direction; use flanged end connections as required to mate to valves or process equipment
Compressed Air	Galvanized Steel	Sch 40	150 psi @ -40/+250F	threaded	6 feet	3/8-inch	150F	125 psi	187.5 psi	air	2 hrs	provide pre-formed fiberglass insulation on first 10 feet of piping from compressor discharge for personnel protection
Acid	PFA Teflon tubing	0.062-inch wall thickness	100 psig	compression fitting	continuous	n/a	100F	50 psi	75 psi	air	2 hrs	use single continuous section of tubing run inside Sch 40 PVC containment pipe
Potable Water - bldg supply	HDPE	DR11	160 psi	butt fusion welded	n/a	n/a	100F	100 psi				test as required by local regulatory authority; use flanged end connections as required to mate to valves
Potable Water - Cold	Galvanized Steel	Sch 40	150 psi @ -40/+250F	threaded	6 feet	3/8-inch	100F	100 psi	150 psi	water	2 hrs	provide closed cell insulation to reduce condensation
Potable Water - Hot	Galvanized Steel	Sch 40	150 psi @ -40/+250F	threaded	6 feet	3/8-inch	150F	100 psi	150 psi	water	2 hrs	provide pre-formed fiberglass insulation for heat conservation and personal protection
Potable Water - Tempered	Galvanized Steel	Sch 40	150 psi @ -40/+250F	threaded	6 feet	3/8-inch	150F	100 psi	150 psi	water	2 hrs	
Plant Water - cold	PVC or	Sch 40	280 psi @ 73F	socket solvent welded	5 feet	3/8-inch	100F	100 psi	150 psi	water	2 hrs	
Fire Protection Water	Galvanized Steel	Sch 40	150 psi @ -40/+250F	threaded	6 feet	3/8-inch	100F	100 psi	150 psi	water	2 hrs	provide closed cell insulation to reduce condensation
Sanitary Sewer	PVC			-	-			-				test as required by local regulatory authority

Plot Date: 05/25/11 - 2:54pm, Plotted by: Dell Survey2 Drawing Path: T:\Pete\Geomatrix\2011\112500-I B&L Woodwaste Site (SE101600

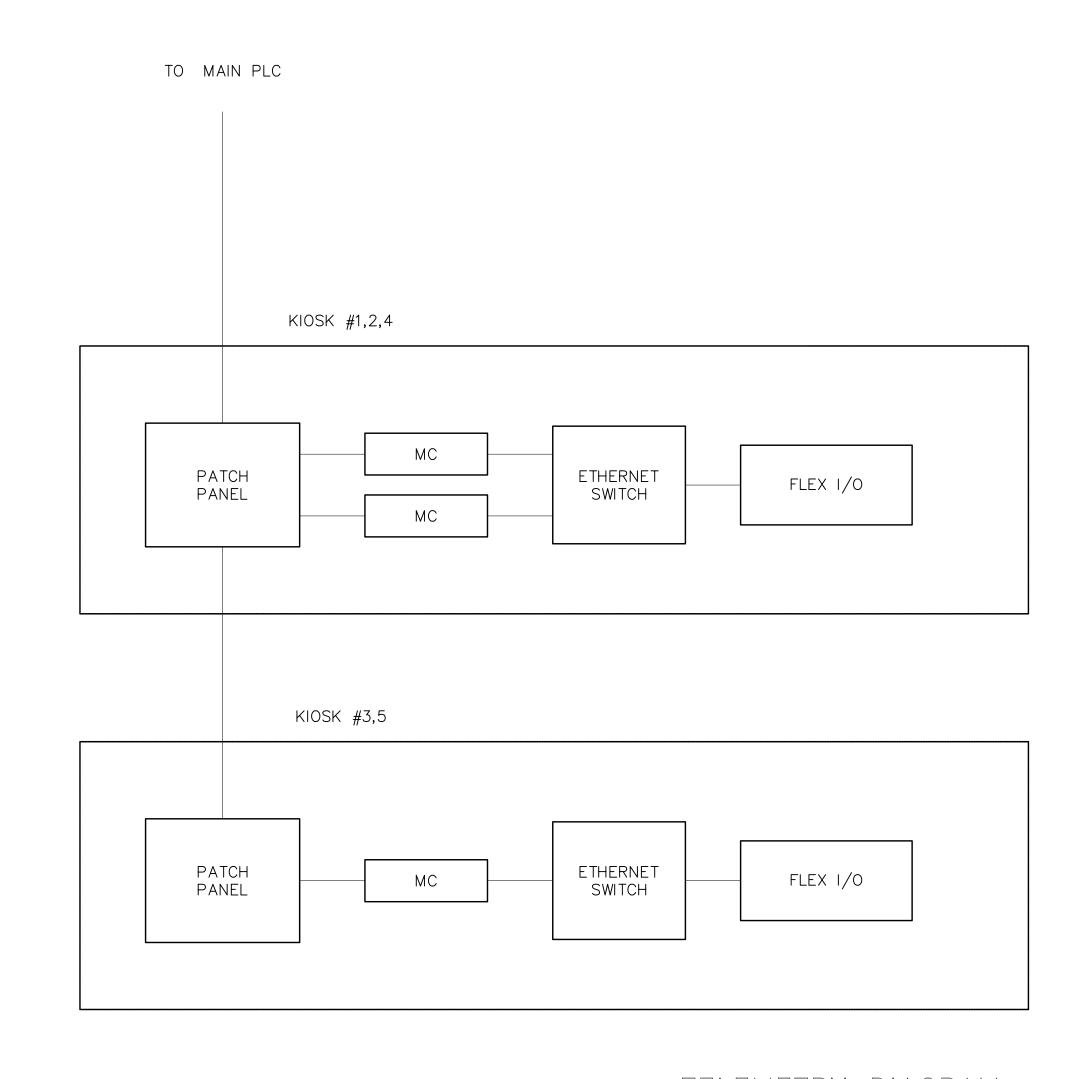
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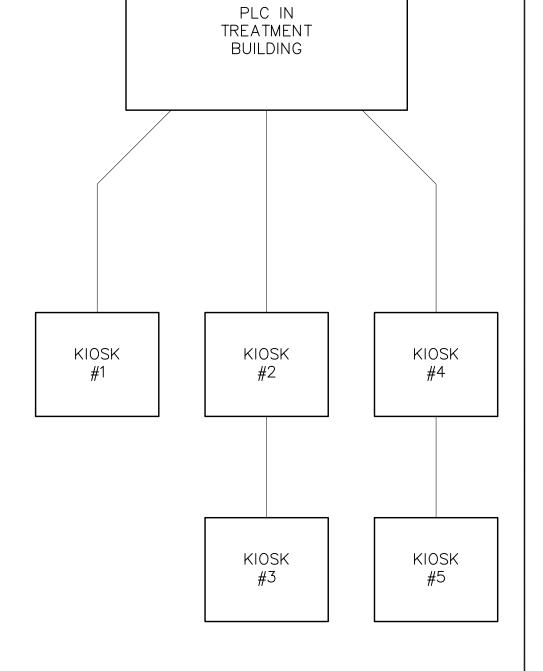
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PIPING SCHEDULE	PROJECT NO.: SE10160010
B&L WOODWASTE SITE	DRAWING
PIERCE COUNTY, WASHINGTON	M-05





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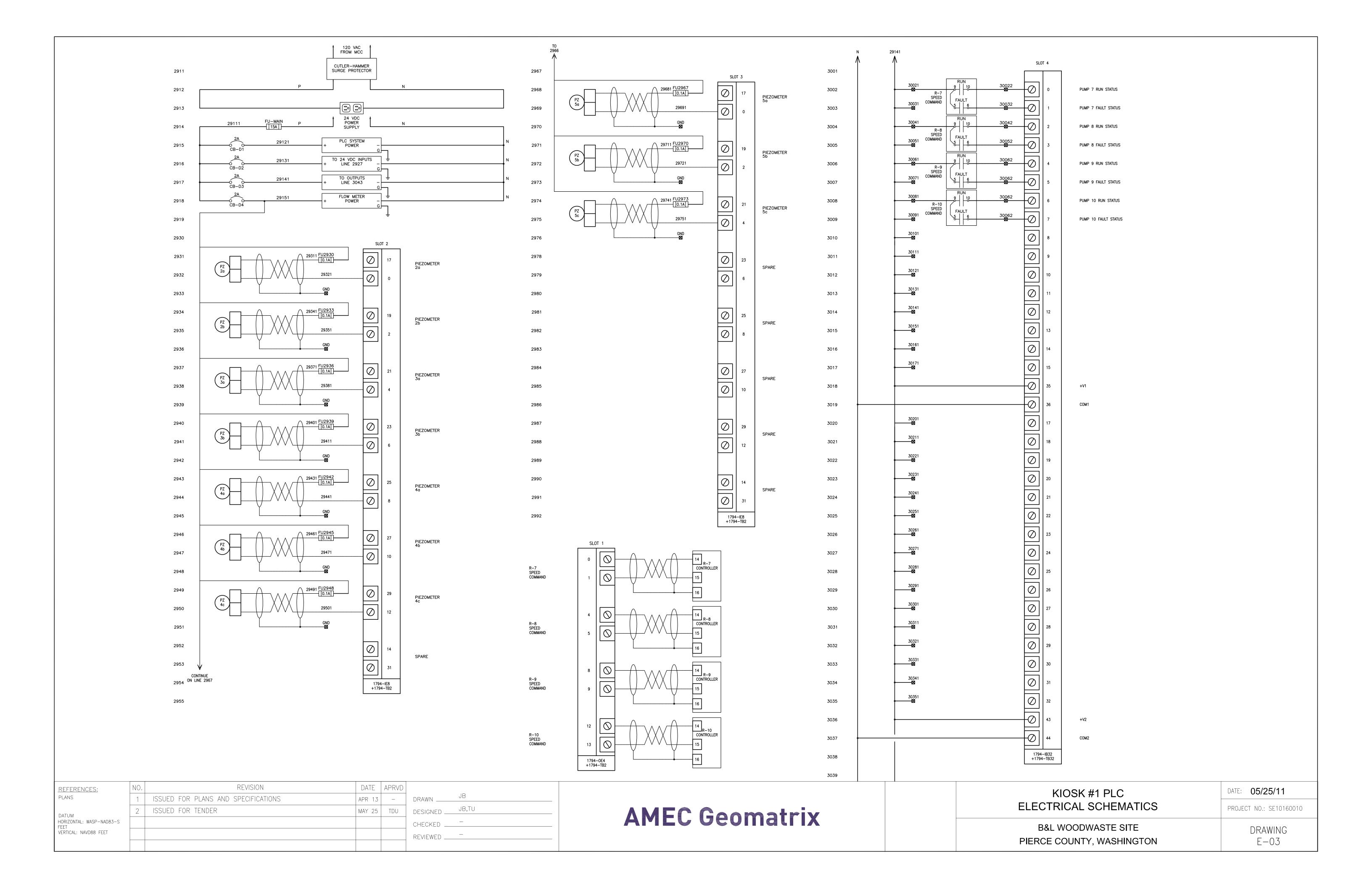


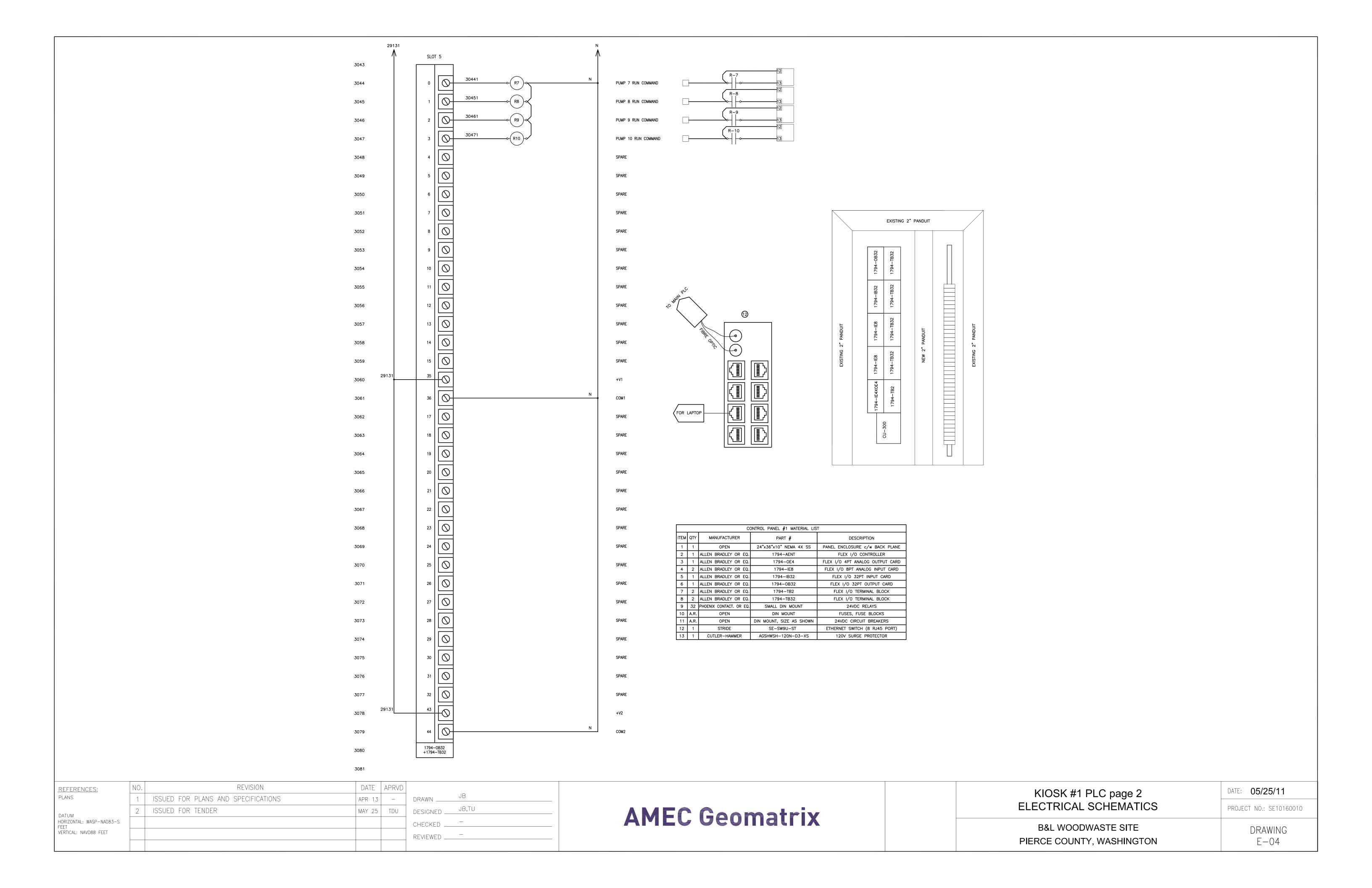
TELEMETRY DIAGRAM

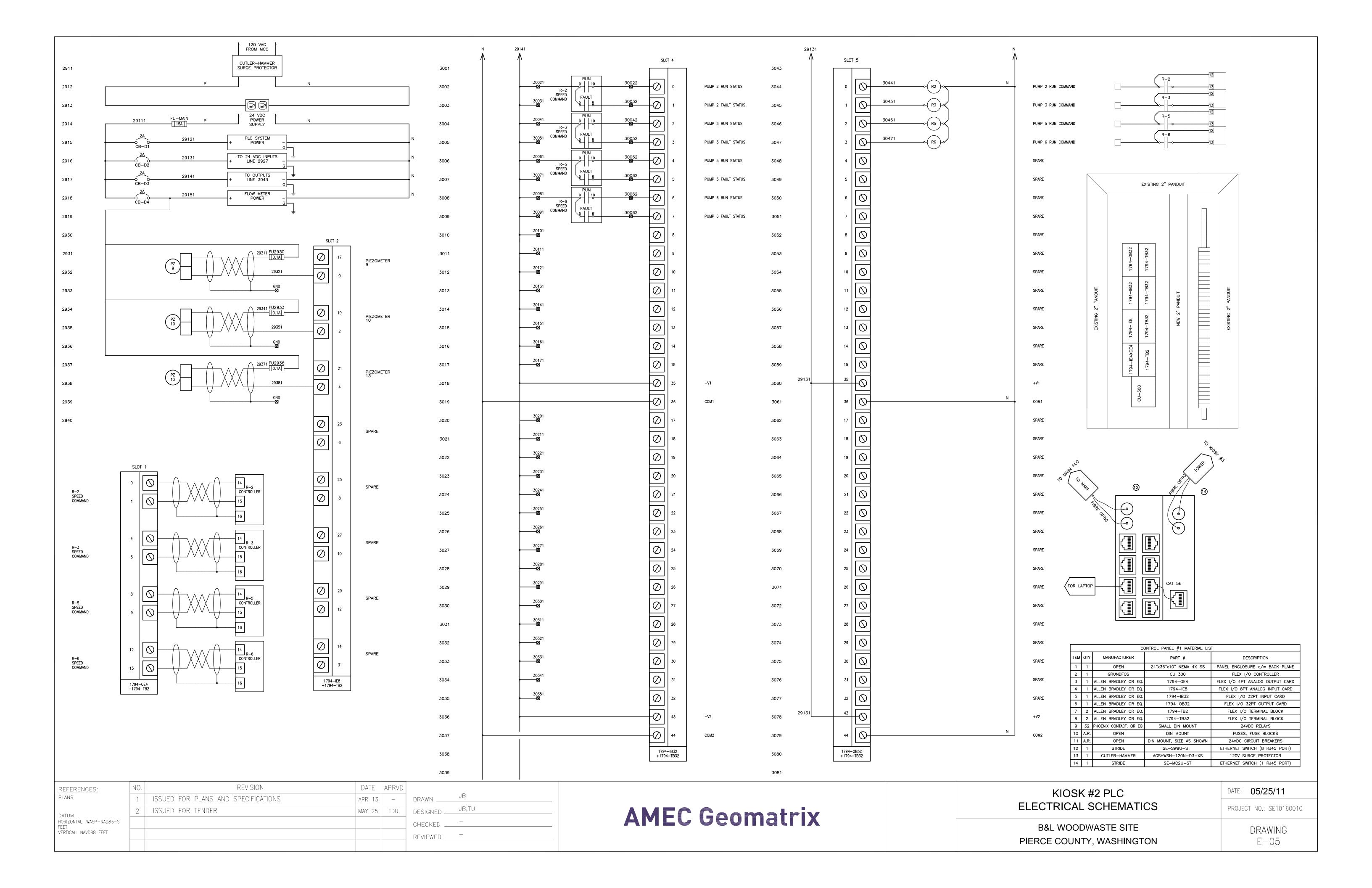
	KIOSK MATERIAL LIST								
ITEM	QTY	MANUFACTURER	PART #	DESCRIPTION					
1	1	OPEN	36"x72"x12" NEMA 4X SS	PANEL ENCLOSURE c/w BACK PLANE					
2	1	OPEN	OPEN	PLC CONTROL PANEL					
3	1	OPEN	OPEN	FIBER OPTIC PATCH PANEL					
4	1	OPEN	OPEN	120/240V BREAKER PANEL					
5	1	OPEN	OPEN	120V LIGHT					
6	1	GRUNDFOS	CU-300	PUMP MOTOR CONTROLLER					

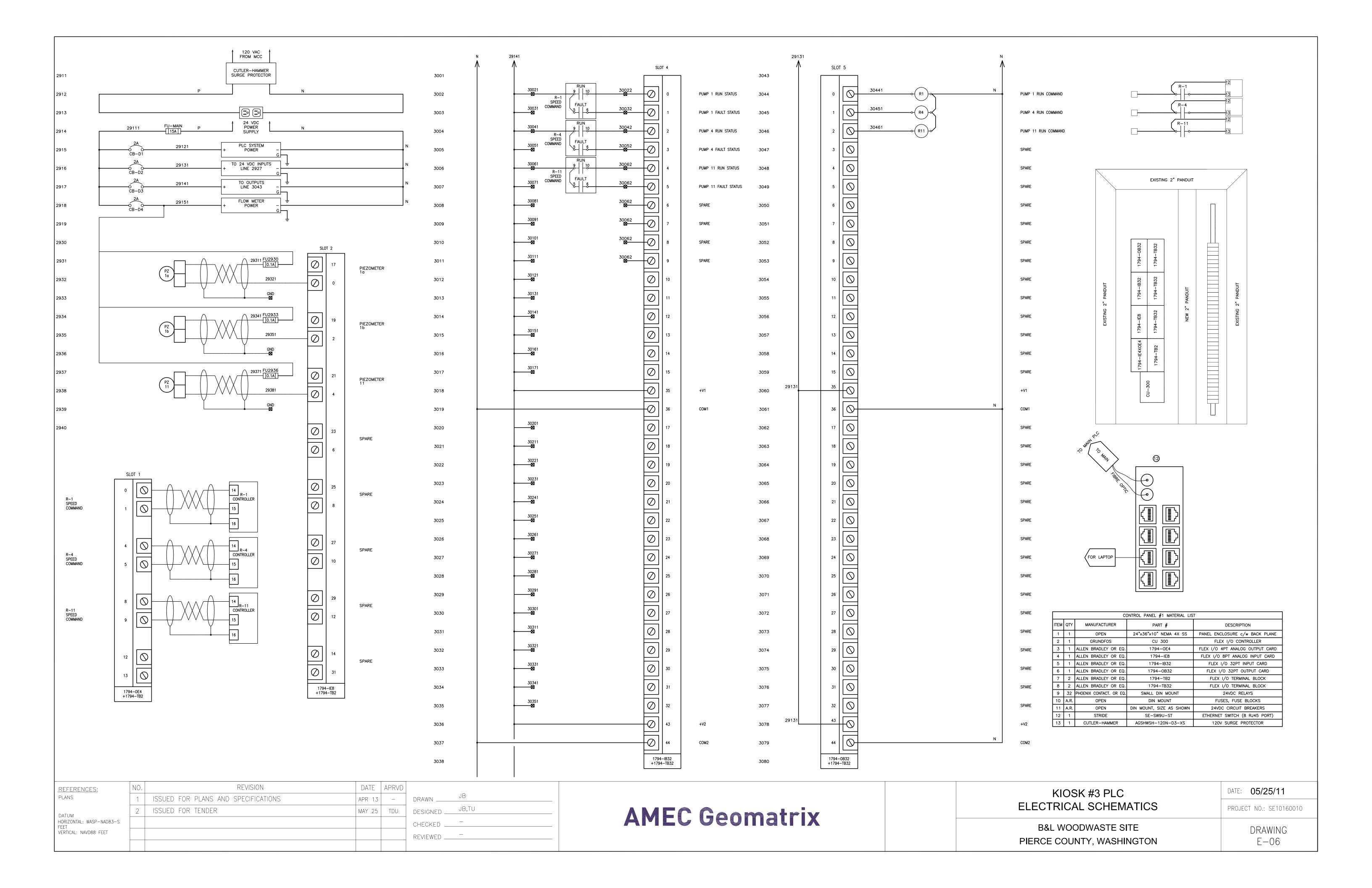
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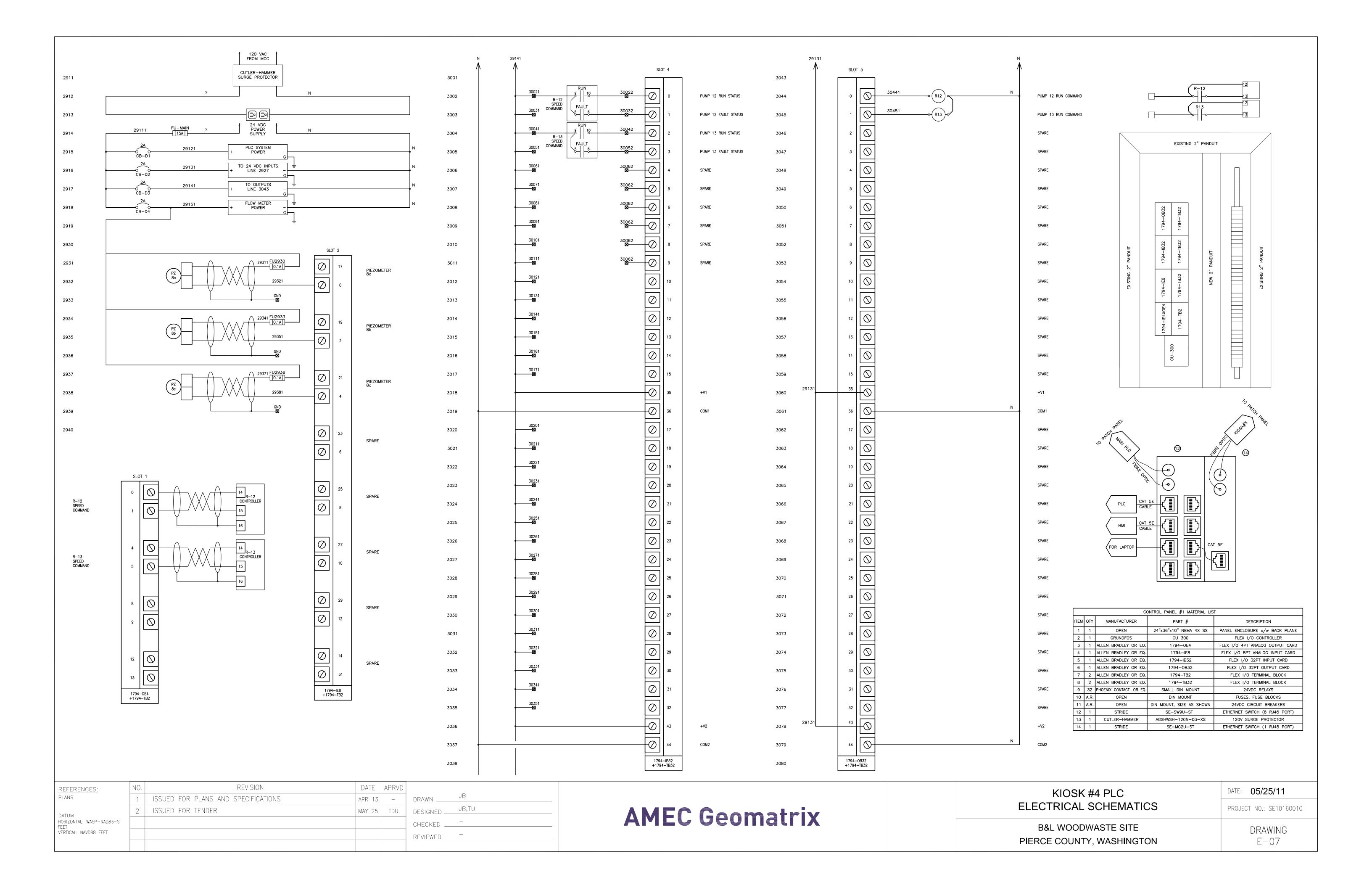
KIOSK DETAILS, CONDUCTOR TABLE	DATE: 05/25/11			
& TELEMETRY SCHEMATICS	PROJECT NO.: SE10160010			
B&L WOODWASTE SITE	DRAWING			
PIERCE COUNTY, WASHINGTON	E-02			

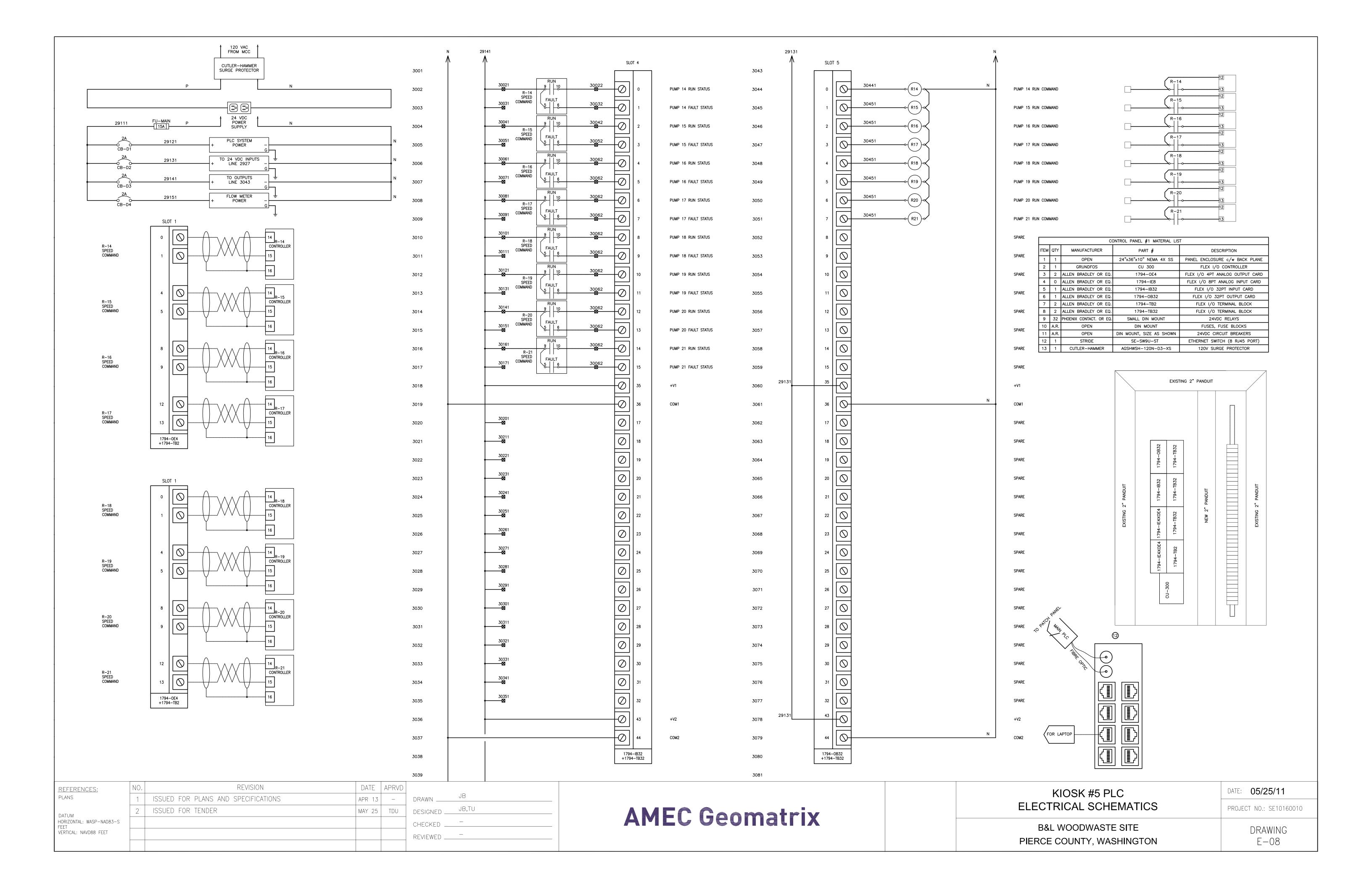




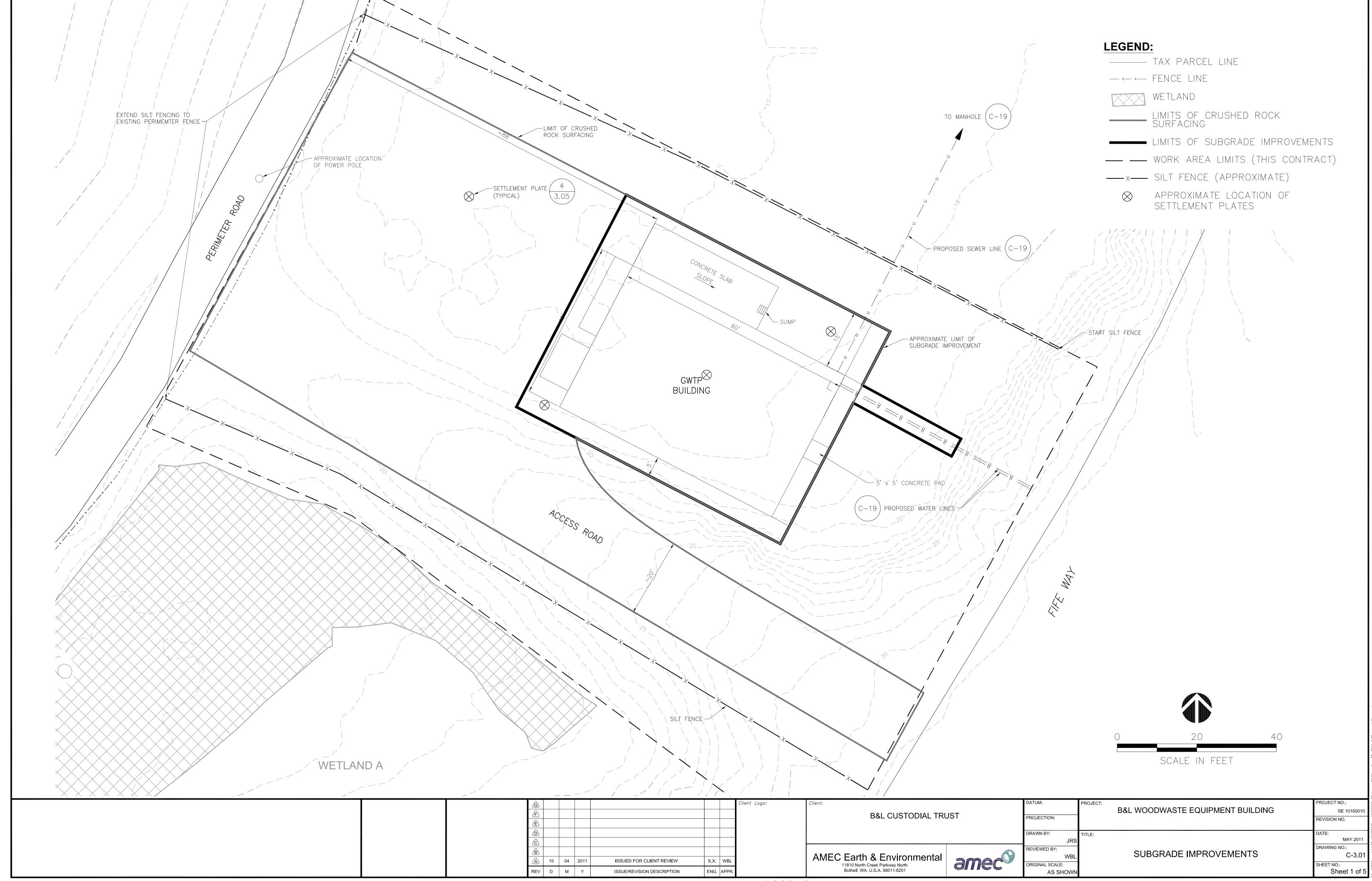


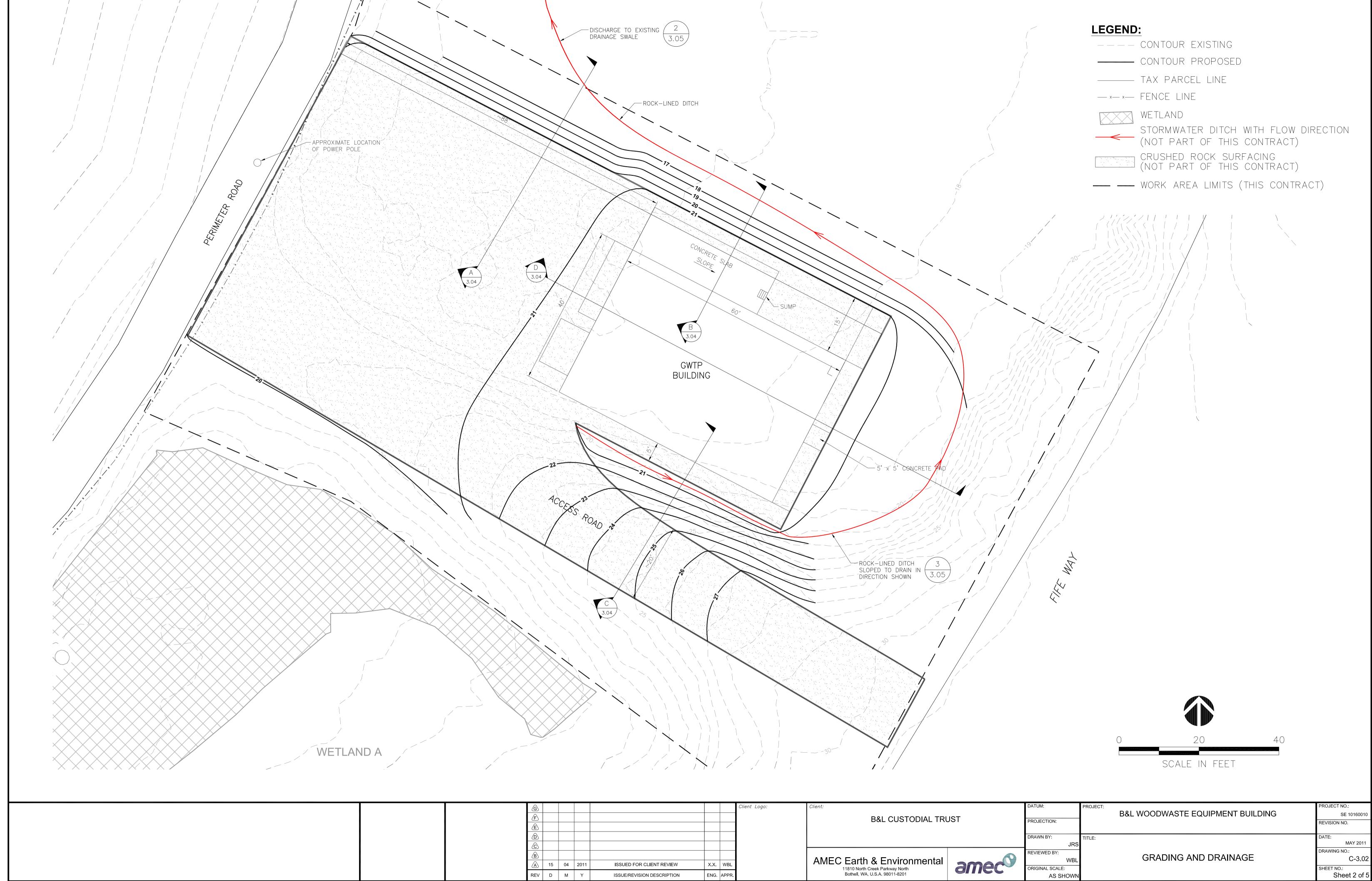












GENERAL NOTES

- 1. IT IS THE INTENT OF THIS SPECIFICATION THAT THE CONTRACTOR PROVIDE THE WORK DEFINED HEREIN, COMPLETE IN EVERY RESPECT, AND IN ACCORDANCE WITH THE GOOD PRACTICES OF THE TRADES INVOLVED IN THE EXCAVATION, TRANSPORT, PLACEMENT, GRADING, BACKFILLING AND COMPACTION OF EARTHEN MATERIALS AND THE REQUIREMENTS OF THE SPECIFICATION, REGARDLESS OF WHETHER OR NOT FULL DETAILS OF SUCH COMPLETENESS, WORKMANSHIP, OR PRACTICES ARE CONTAINED HEREIN.
- 2. ALL WORK SHALL COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES, ORDINANCES AND REGULATIONS. NOTHING IN THE SPECIFICATIONS OR DRAWINGS IS TO BE CONSTRUED TO ALLOW WORK NOT CONFORMING TO SUCH CODES. CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH THE REGULATIONS AND CODE REQUIREMENTS.

EROSION CONTROL NOTES

- 1. PROVIDE, INSTALL AND MAINTAIN EROSION CONTROL MEASURES AS INDICATED IN THE PROJECT PLANS AND AS REQUIRED BY PIERCE COUNTY AND THE WASHINGTON STATE DEPARTMENT OF ECOLOGY, PRIOR TO BEGINNING ANY WORK WHICH DISTURBS THE EXISTING SITE SOILS OR VEGETATION. PROVIDE, INSTALL AND MAINTAIN ADDITIONAL TEMPORARY EROSION CONTROL MEASURES AT NO ADDITIONAL COST TO THE OWNER AS NECESSARY DUE TO WEATHER OR ENVIRONMENTAL CONDITIONS AND TO COMPLY WITH PIERCE COUNTY, AND THE WASHINGTON STATE DEPARTMENT OF ECOLOGY.
- 2. ALL STORMWATER WITHIN THE WORK AREAS SHALL BE CONTAINED ONSITE AND INFILTRATED. EROSION AND SEDIMENT CONTROL MEASURES ARE CONCEPTUAL AND MAY BE ALTERED AS NECESSARY DURING DIFFERENT STAGES OF CONSTRUCTION TO PREVENT SEDIMENT OR STORMWATER MIGRATING OFFSITE.
- 3. THE EXISTING WETLANDS SOUTH OF THE ACCESS ROAD AND NORTH OF THE PROJECT SITE SHALL BE PROTECTED FROM ADVERSE IMPACT BY SITE ACTIVITIES. PROVIDE SUMPS, PUMPS AND HOSES TO COLLECT AND REMOVE ANY TURBID STORMWATER BEFORE IT ENTERS THESE AREAS.
- 4. ANY SEDIMENT THAT IS TRACKED ON TO FIFE WAY SHALL BE REMOVED IMMEDIATELY BY SWEEPING. THE PAVEMENT SHALL NOT BE CLEANED BY WASHING DOWN THE STREET, EXCEPT WHEN SWEEPING IS INEFFECTIVE AND THERE IS NO THREAT TO PUBLIC SAFETY
- 5. IF SEDIMENT FROM THE WORK SITE IS TRACKED ONTO FIFE WAY, THEN ALTERNATIVE MEASURES TO KEEP STREETS FREE OF SEDIMENT SHALL BE USED. THIS MAY INCLUDE CONSTRUCTION OF A STABILIZED CONSTRUCTION ENTRANCE, REGULARLY SCHEDULED STREET SWEEPING OR CONSTRUCTION OF A WHEEL WASH LOCATED ONSITE.
- 6. DUST CONTROL SHALL BE IMPLEMENTED WHEN EXPOSED SOILS ARE DRY TO THE POINT THAT WIND TRANSPORT IS POSSIBLE AND ROADWAYS, DRAINAGE WAYS OR SURFACE WATERS ARE LKELY TO BE IMPACTED. WATER IS THE MOST COMMON DUST CONTROL METHOD, WHEN USING WATER FOR DUST CONTROL THE EXPOSED SOILS SHALL BE SPRAYED UNTIL WET, BUT RUNOFF SHALL NOT BE GENERATED BY SPRAYING.

SITE EARTHWORK NOTES AND SPECIFICATIONS

1. WSDOT SPECIFICATION CODES CITED HEREIN REFER TO WSDOT PUBLICATION M41-10, 2010 STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION, PREPARED BY WASHINGTON STATE DEPARTMENT OF TRANSPORTATION.

- 2. CLEARING SHALL CONSIST OF REMOVAL OF ALL TREES, BRUSH, GRASS AND OTHER VEGETATION. CUT OFF FLUSH WITH OR BELOW THE ORIGINAL GROUND SURFACE, TREES, STUMPS, ROOTS, BRUSH AND OTHER VEGETATION IN AREAS TO BE CLEARED. CLEARING SHALL INCLUDE THE DISPOSAL OF ANY RUBBISH OR MAN-MADE MATERIALS ENCOUNTERED. REMOVE AND DISPOSE OF ROOTS LARGER THAN 3 INCHES IN DIAMETER AND THE MATTED ROOTS FROM THE INDICATED GRUBBING AREAS. EXCAVATE THIS MATERIAL TOGETHER WITH ORGANIC AND METALLIC DEBRIS, BRUSH AND REFUSE AND REMOVE TO A DEPTH OF NOT LESS THAN 6 INCHES BELOW THE ORIGINAL SOIL SURFACE. ALL MATERIALS GENERATED FROM CLEARING AND GRUBBING ACTIVITIES ARE TO BE HAULED OFFSITE AND DISPOSED AT AN APPROVED LOCATION. FILL DEPRESSIONS CREATED BY GRUBBING WITH SUITABLE MATERIAL AND IN ACCORDANCE WITH PROJECT SPECIFICATIONS.
- 3. EXCAVATE SURFACES WITHIN WORK AREAS TO INDICATED CROSS-SECTIONS, ELEVATIONS AND GRADES SHOWN ON PROJECT PLANS. NOTIFY PROJECT GEOTECHNICAL ENGINEER WHEN EXCAVATION HAS REACHED REQUIRED SUBGRADE. PRIOR TO PLACEMENT OF FILL ALL AREAS TO RECEIVE STRUCTURAL FILL SHALL BE PROOF-ROLLED TO EVALUATE SUBGRADE CONDITIONS.
- 4. PROOF-ROLL EXPOSED SUBGRADE SOILS UNDER THE OBSERVATION OF THE PROJECT GEOTECHNICAL ENGINEER, WHEN WEATHER CONDITIONS PERMIT. CONTRACTOR SHALL PROOF-ROLL USING A FULLY LOADED DUAL-AXLE DUMP TRUCK TO IDENTIFY SOFT SUBGRADE AREAS. SOFT OR YIELDING SUBGRADE SOILS SHALL BE EXCAVATED TO REMOVE THE SOFT SOILS TO EXPOSE SUITABLE SUBGRADE SOILS, AS DETERMINED BY THE PROJECT GEOTECHNICAL ENGINEER. AREAS WHERE SOFT SOILS HAVE BEEN REMOVED SHALL BE BACKFILLED WITH STRUCTURAL FILL PLACED AND COMPACTED IN ACCORDANCE WITH PROJECT SPECIFICATIONS.
- 5. SETTLEMENT PLATES TO BE INSTALLED AT LOCATIONS SHOWN ON DRAWINGS PRIOR TO FILL PLACEMENT. ALL SETTLEMENT PLATES TO BE SURVEYED IMMEDIATELY AFTER INSTALLATION.
- 6. ALL IMPORTED MATERIALS SHALL BE PROVIDED BY EITHER LLOYD ENTERPRISES MILTON PIT OR CTI SUMNER PIT NO. 3 WHICH ARE PRE-APPROVED SOURCES FOR THE MATERIAL SPECIFIED. SUBMIT EITHER RECENT GRADATION TESTS OF SOIL MATERIALS TESTED AT THE SOURCE OR PROVIDE 60-POUND SAMPLES WITHIN AIRTIGHT CONTAINERS OF EACH PROPOSED SOIL MATERIAL TO THE DESIGN ENGINEER A MINIMUM OF 1 WEEK BEFORE ANTICIPATED USE. ADDITIONAL CHEMICAL ANALYSES MAY BE REQUIRED
- 7. DRAINAGE LAYER: SUPPLY AND PLACE 6-INCH THICK, UNIFORM LAYER OF BALLAST MATERIAL MEETING WSDOT 9-03.9(2) "PERMEABLE BALLAST" OVER THE SUBGRADE TREATMENT AREA, EXTENDING THE BALLAST MATERIAL TO THE NORTH EDGE OF THE FILL PAD SO THAT ANY ACCUMULATED SEEPAGE CAN DRAIN FREELY TO THE EDGE OF THE PAD. ALSO EXTEND BALLAST PLACEMENT 1-FOOT BEYOND THE OUTSIDE EDGE OF ANY RAMMED AGGREGATE PIER LOCATIONS.
- 8. <u>FILL PAD</u>: SUPPLY AND PLACE FILL PAD MATERIAL CONSISTING OF GRANULAR MATERIAL MEETING WSDOT 9-03.14(2) "SELECT BORROW". GRADE AND COMPACT TO MEET PROJECT PLANS AND SPECIFICATIONS.
- 9. SETTLEMENT OF THE FILL PAD OUTSIDE OF THE SUBGRADE TREATMENT AREA IS ANTICIPATED. THE CONTRACTOR SHOULD ASSUME THAT UP TO 6-INCHES OF SETTLEMENT MAY OCCUR AFTER PLACEMENT OVER THE COURSE OF BUILDING CONSTRUCTION. FINE GRADING AND ADDITIONAL FILL PAD MATERIAL SHALL BE PLACED NO SOONER THAN SIX WEEKS AFTER COMPLETION OF ROUGH GRADING TO ALLOW FOR MAJORITY OF SETTLEMENT TO OCCUR.

- 10. GRAVEL SURFACING (NOT PART OF THIS CONTRACT):
 SUPPLY AND PLACE 6-INCH THICK, UNIFORM LAYER OF
 CRUSHED SURFACING MATERIAL MEETING WSDOT
 9-03.9(3) "CRUSHED SURFACING TOP COURSE" WITHIN
 THE AREAS INDICATED ON THE PROJECT PLANS. SHAPE
 CRUSHED SURFACING TO REQUIRED CROWN
 ELEVATIONS AND CROSS-SLOPE GRADES TO PROMOTE
 POSITIVE DRAINAGE AWAY FROM BUILDING AREA.
- 11. PLACE FILL MATERIALS IN UNIFORM, LEVEL CONTINUOUS LAYERS NOT EXCEEDING 6-INCHES IN LOOSE LIFT THICKNESS AND COMPACT TO A FIRM AND UNYIELDING CONDTION TO ATTAIN A MINIMUM OF 90 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D1557. MAINTAIN OPTIMUM MOISTURE CONTENT OF FILL MATERIALS TO ATTAIN REQUIRED COMPACTION VALUES. EARTHWORK PERFORMED DURING WET WEATHER IS AT THE CONTRACTORS OWN RISK. MATERIALS WHICH BECOME UNUSABLE DUE TO HIGH MOISTURE CONTENT SHALL BE EITHER REMOVED AND REPLACED WITH SUITABLE STRUCTURAL FILL OR SCARIFIED AND AERATED TO DRY THE MATERIAL TO ITS OPTIMUM MOISTURE CONTENT AT NO COST TO THE OWNER.
- 12. GRANULAR SPOILS GENERATED FROM PRE-DRILLING OF RAMMED AGGREGATE PIERS ARE TO BE PLACED IN A THIN LIFT NEAR THE BASE OF THE FILL AREA OUTSIDE OF THE BUILDING PAD FOOTPRINT. SHOULD THE MATERIAL CONSIST OF HIGH PLASTICITY CLAYEY SOILS OR HAVE PREVALENT ORGANIC DEBRIS, THE SPOILS SHALL BE EXPORTED OFF-SITE.
- 13. EXCAVATE DRAINAGE DITCH (NOT PART OF THIS CONTRACT) AT THE APPROXIMATE LOCATION INDICATED ON THE PROJECT PLANS. SLOPE DITCH INVERT SUCH THAT POSITIVE DRAINAGE IS MAINTAINED WITH THE DISCHARGE POINT LOCATED AS SHOWN. LINE DITCH WITH "PERMEABLE BALLAST", WSDOT 9-03.4(2) TO ARMOR THE DRAINAGE DITCH SIDEWALLS.

RAMMED AGGREGATE PIER FOUNDATION NOTES AND SPECIFICATIONS

- 1. WORK SHALL CONSIST OF DESIGNING, FURNISHING AND INSTALLING RAMMED AGGREGATE PIER FOUNDATIONS TO THE LINES AND GRADES DESIGNATED ON THE SUBGRADE IMPROVEMENT PLAN AND AS SPECIFIED HEREIN. PROVIDE ALL EQUIPMENT, MATERIAL, LABOR, AND SUPERVISION TO DESIGN AND INSTALL RAMMED AGGREGATE PIER ELEMENTS. DESIGN SHALL RELY ON SUBSURFACE INFORMATION PRESENTED IN THE PROJECT GEOTECHNICAL REPORT.
- 2. THE AGGREGATE PIERS SHALL BE CONSTRUCTED BY DRIVING A HOLLOW MANDREL TO THE DESIGN DEPTH AND VERTICALLY RAMMING LIFTS OF AGGREGATE USING THE SPECIALLY DESIGNED TAMPER HEAD AND HIGH-ENERGY IMPACT DENSIFICATION EQUIPMENT TO CREATE THE COMPACTED AGGREGATE PIER. THE RAMMED AGGREGATE PIER ELEMENTS SHALL BE IN A COLUMNAR-TYPE CONFIGURATION AND SHALL BE USED TO IMPROVE THE EXISTING SUBGRADE SOILS BENEATH THE FOOTPRINT OF THE PROPOSED BUILDING AND CONCRETE TRUCK PAD AND ASSOCIATED UTILITIES INCLUDING: DOMESTIC WATER, FIRE AND SANITARY SEWER LINES TO THE LIMITS OF THE FILL PAD AS SHOWN.
- 3. THE RAMMED AGGREGATE PIER INSTALLER (THE INSTALLER) CURRENTLY APPROVED FOR THIS WORK IS GEOPIER-NORTHWEST, INC., BELLEVUE, WA. WITHOUT EXCEPTION, NO ALTERNATE INSTALLER WILL BE ACCEPTED UNLESS APPROVED BY THE OWNER'S ENGINEER AT LEAST TWO (2) WEEKS PRIOR TO BID OPENING.
- 4. DESIGN CALCULATIONS THE INSTALLER SHALL SUBMIT DETAILED DESIGN CALCULATIONS AND CONSTRUCTION DRAWINGS PREPARED BY THE RAMMED AGGREGATE PIER DESIGNER (THE DESIGNER) FOR REVIEW AND APPROVAL BY THE OWNER OR OWNER'S ENGINEER. ALL PLANS SHALL BE SEALED BY A WASHINGTON STATE

PROFESSIONAL ENGINEER.

- 5. THE DESIGN OF THE RAMMED AGGREGATE PIER SYSTEM SHALL BE BASED ON THE SERVICE LOAD BEARING PRESSURE AND THE ALLOWABLE TOTAL AND DIFFERENTIAL SETTLEMENT CRITERIA OF ALL FOOTINGS INDICATED BY THE DESIGN TEAM FOR SUPPORT BY THE RAMMED AGGREGATE PIER SYSTEM. THE RAMMED AGGREGATE PIER SYSTEM SHALL BE DESIGNED IN ACCORDANCE WITH GENERALLY-ACCEPTED ENGINEERING PRACTICE. THE DESIGN LIFE OF THE STRUCTURE SHALL BE 50 YEARS AND SHALL MEET THE FOLLOWING CRITERIA:
- MAXIMUM ALLOWABLE BEARING PRESSURE FOR REINFORCED SOILS FOOTINGS SUPPORTED BY RAMMED AGGREGATE PIER: 2.000 PSF

ESTIMATED TOTAL LONG-TERM SETTLEMENT: ≤1-INCH

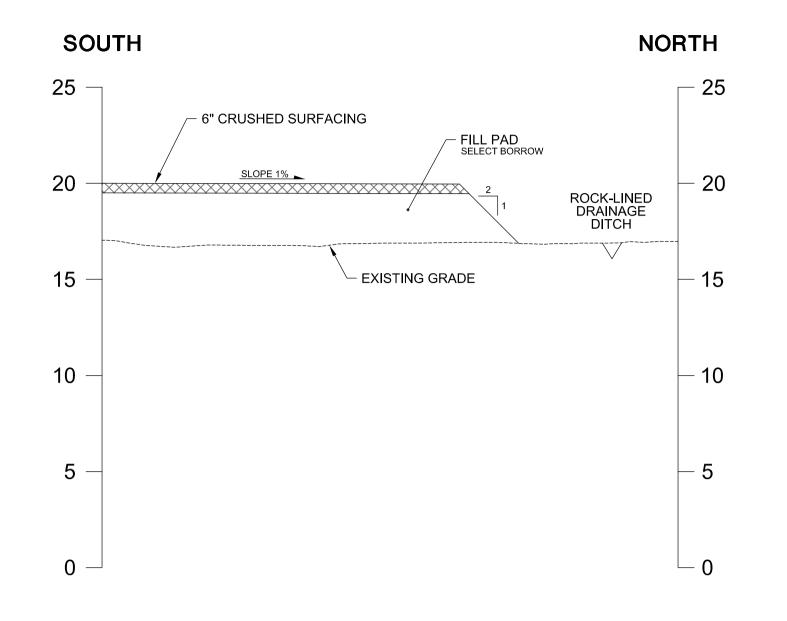
ESTIMATED LONG-TERM DIFFERENTIAL SETTLEMENT: ≤½-INCH

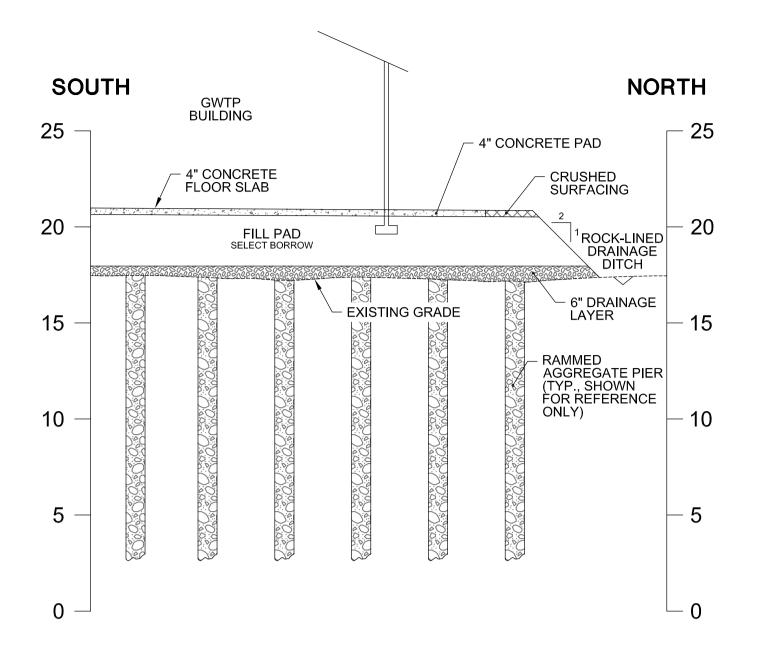
- 6. THE INSTALLER SHALL SUBMIT DETAILED DESIGN CALCULATIONS, CONSTRUCTION DRAWINGS, AND SHOP DRAWINGS, (THE DESIGN SUBMITTAL), FOR APPROVAL AT LEAST 2 WEEK(S) PRIOR TO THE BEGINNING OF CONSTRUCTION. A DETAILED EXPLANATION OF THE DESIGN PARAMETERS FOR SETTLEMENT CALCULATIONS SHALL BE INCLUDED IN THE DESIGN SUBMITTAL. ALL COMPUTER-GENERATED CALCULATIONS AND DRAWINGS SHALL BE PREPARED AND SEALED BY A PROFESSIONAL ENGINEER, LICENSED IN WASHINGTON STATE.
- 7. THE LOCATION OF RAMMED AGGREGATE
 PIER-SUPPORTED FOUNDATIONS FOR THIS PROJECT,
 INCLUDING LAYOUT OF INDIVIDUAL RAMMED
 AGGREGATE PIER ELEMENTS, SHALL BE MARKED IN THE
 FIELD USING SURVEY STAKES OR SIMILAR MEANS AT
 LOCATIONS SHOWN ON THE DRAWINGS. THE AS-BUILT
 CENTER OF EACH PIER SHALL BE WITHIN 6 INCHES OF
 THE LOCATIONS INDICATED ON THE PLANS. PIERS
 INSTALLED OUTSIDE OF THE ABOVE TOLERANCES AND
 DEEMED NOT ACCEPTABLE SHALL BE REBUILT AT NO
 ADDITIONAL EXPENSE TO THE OWNER.
- 8. DISPLACEMENT RAMMED AGGREGATE PIER SYSTEMS SHALL BE CONSTRUCTED BY ADVANCING A SPECIALLY DESIGNED MANDREL WITH A MINIMUM 15 TON STATIC FORCE AUGMENTED BY DYNAMIC VERTICAL RAMMING ENERGY TO THE FULL DESIGN DEPTH. THE HOLLOW-SHAFT MANDREL, FILLED WITH AGGREGATE, IS INCREMENTALLY RAISED, PERMITTING THE AGGREGATE TO BE RELEASED INTO THE CAVITY, AND THEN LOWERED BY VERTICALLY ADVANCING AND/OR RAMMING TO DENSIFY THE AGGREGATE AND FORCE IT LATERALLY INTO THE ADJACENT SOIL. THE CYCLE OF RAISING AND LOWERING THE MANDREL IS REPEATED TO THE TOP OF PIER ELEVATION. THE CYCLE DISTANCE SHALL BE DETERMINED BY THE RAMMED AGGREGATE PIER DESIGNER, SPECIAL HIGH-ENERGY IMPACT DENSIFICATION APPARATUS SHALL BE EMPLOYED TO VERTICALLY DENSIFY THE RAMMED AGGREGATE PIER ELEMENTS DURING INSTALLATION OF EACH CONSTRUCTED LIFT OF AGGREGATE. DENSIFICATION SHALL BE PERFORMED USING A MANDREL/TAMPER. THE MANDREL/TAMPER FOOT IS REQUIRED TO ADEQUATELY INCREASE THE LATERAL EARTH PRESSURE IN THE MATRIX SOIL DURING INSTALLATION. COMPACTION **EQUIPMENT THAT INDUCES HORIZONTAL VIBRATORY** ENERGY (SUCH AS VIBROFLOT EQUIPMENT) IS NOT PERMITTED. DOWNWARD CROWD PRESSURE SHALL BE APPLIED TO THE MANDREL DURING INSTALLATION.
- 9. PRE-AUGERING THROUGH THE EXISTING GRANULAR FILL USING MECHANICAL DRILLING OR EXCAVATION EQUIPMENT IS PERMITTED PROVIDED THE MAXIMUM DEPTH OF DRILLING IS LESS THAN 10 FEET. SPOILS FROM PRE-AUGERING TO BE COLLECTED AND STOCKPILED FOR REUSE AS FILL OUTSIDE OF THE BUILDING PAD.
- 10. DAILY RAMMED AGGREGATE PIER PROGRESS REPORTS THE INSTALLER SHALL FURNISH A COMPLETE AND
 ACCURATE RECORD OF RAMMED AGGREGATE PIER
 INSTALLATION TO THE GENERAL CONTRACTOR. THE

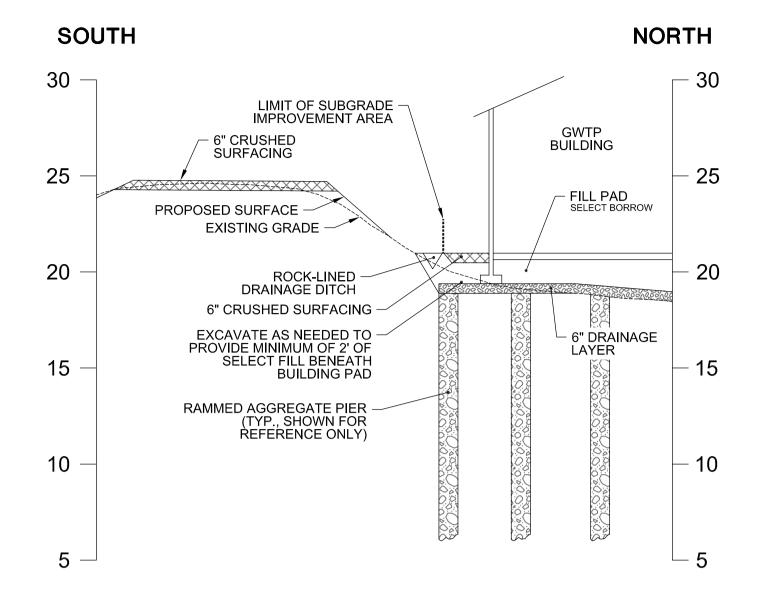
- RECORD SHALL INDICATE THE PIER LOCATION, LENGTH, VOLUME OF AGGREGATE USED OR NUMBER OF LIFTS, DENSIFICATION FORCES DURING INSTALLATION, AND FINAL ELEVATIONS OR DEPTHS OF THE BASE AND TOP OF PIERS. THE RECORD SHALL ALSO INDICATE THE TYPE AND SIZE OF THE INSTALLATION EQUIPMENT USED, AND THE TYPE OF AGGREGATE USED. THE INSTALLER SHALL IMMEDIATELY REPORT ANY UNUSUAL CONDITIONS ENCOUNTERED DURING INSTALLATION TO THE GENERAL CONTRACTOR, THE DESIGNER AND TO THE TESTING AGENCY.
- 11. THE RAMMED AGGREGATE PIER INSTALLER SHALL PROVIDE FULL-TIME QUALITY CONTROL MONITORING OF RAMMED AGGREGATE PIER CONSTRUCTION ACTIVITIES. THE OWNER'S ENGINEER WILL PROVIDE QUALITY ASSURANCE SERVICES.
- 12. SITE GRADES FOR RAMMED AGGREGATE PIER INSTALLATION SHALL BE WITHIN 1 FOOT OF THE TOP OF FOOTING ELEVATION OR FINISHED GRADE ELEVATION TO MINIMIZE RAMMED AGGREGATE PIER INSTALLATION DEPTHS. GROUND ELEVATIONS AND BOTTOM OF FOOTING ELEVATIONS SHALL BE PROVIDED TO THE RAMMED AGGREGATE PIER INSTALLER IN SUFFICIENT DETAIL TO ESTIMATE INSTALLATION DEPTH ELEVATIONS TO WITHIN 3 INCHES.
- 13. THE EARTHWORK CONTRACTOR WILL PROVIDE SITE ACCESS TO THE INSTALLER, AFTER EARTHWORK IN THE AREA HAS BEEN COMPLETED. A WORKING SURFACE SHALL BE ESTABLISHED AND MAINTAINED BY THE EARTHWORK CONTRACTOR TO PROVIDE WET WEATHER PROTECTION OF THE SUBGRADE AND TO PROVIDE ACCESS FOR EFFICIENT OPERATION OF THE RAMMED AGGREGATE PIER INSTALLATION. PRIOR TO, DURING AND FOLLOWING RAMMED AGGREGATE PIER INSTALLATION, THE EARTHWORK CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE TO PROTECT THE SITE FROM WET WEATHER AND SURFACE PONDING OF WATER. IF SPOILS ARE GENERATED BY RAMMED AGGREGATE PIER INSTALLATION, SPOIL REMOVAL FROM THE RAMMED AGGREGATE PIER WORK AREA IN A TIMELY MANNER TO PREVENT INTERRUPTION OF RAMMED AGGREGATE PIER INSTALLATION IS REQUIRED. SPOILS TO BE STOCKPILED FOR REUSE AS STRUCTURAL FILL OUTSIDE OF THE BUILDING FOOTPRINT
- 14. THE EARTHWORK CONTRACTOR SHALL COORDINATE ALL EXCAVATIONS MADE SUBSEQUENT TO RAMMED AGGREGATE PIER INSTALLATIONS SO THAT EXCAVATIONS DO NOT ENCROACH ON THE PIERS AS SHOWN IN THE RAMMED AGGREGATE PIER CONSTRUCTION DRAWINGS. PROTECTION OF COMPLETED RAMMED AGGREGATE PIER ELEMENTS IS THE RESPONSIBILITY OF THE FARTHWORK CONTRACTOR. IN THE EVENT THAT UTILITY EXCAVATIONS ARE REQUIRED IN CLOSE PROXIMITY TO THE INSTALLED RAMMED AGGREGATE PIER ELEMENTS. THE EARTHWORK CONTRACTOR SHALL CONTACT THE RAMMED AGGREGATE PIER DESIGNER IMMEDIATELY TO DEVELOP CONSTRUCTION SOLUTIONS TO MINIMIZE IMPACTS ON THE INSTALLED AGGREGATE PIER ELEMENTS.
- 15. PRIOR TO PLACEMENT OF THE DRAINAGE LAYER OVER THE SUBGRADE IMPROVEMENT AREA, THE TOPS OF ALL RAMMED AGGREGATE PIER ELEMENTS AND SUBGRADE AREAS BETWEEN THE AGGREGATE PIERS SHALL BE CLEARED OF LOOSE SOILS OR DEBRIS. COMPACTION OF THE TOP OF RAMMED AGGREGATE PIER ELEMENTS SHALL BE PERFORMED USING A MOTORIZED IMPACT COMPACTOR ("WACKER", "JUMPING JACK," OR SIMILAR). LOOSE OR SOFT SURFICIAL SOIL OVER THE TREATMENT AREA SURFACE SHALL BE RECOMPACTED OR REMOVED.

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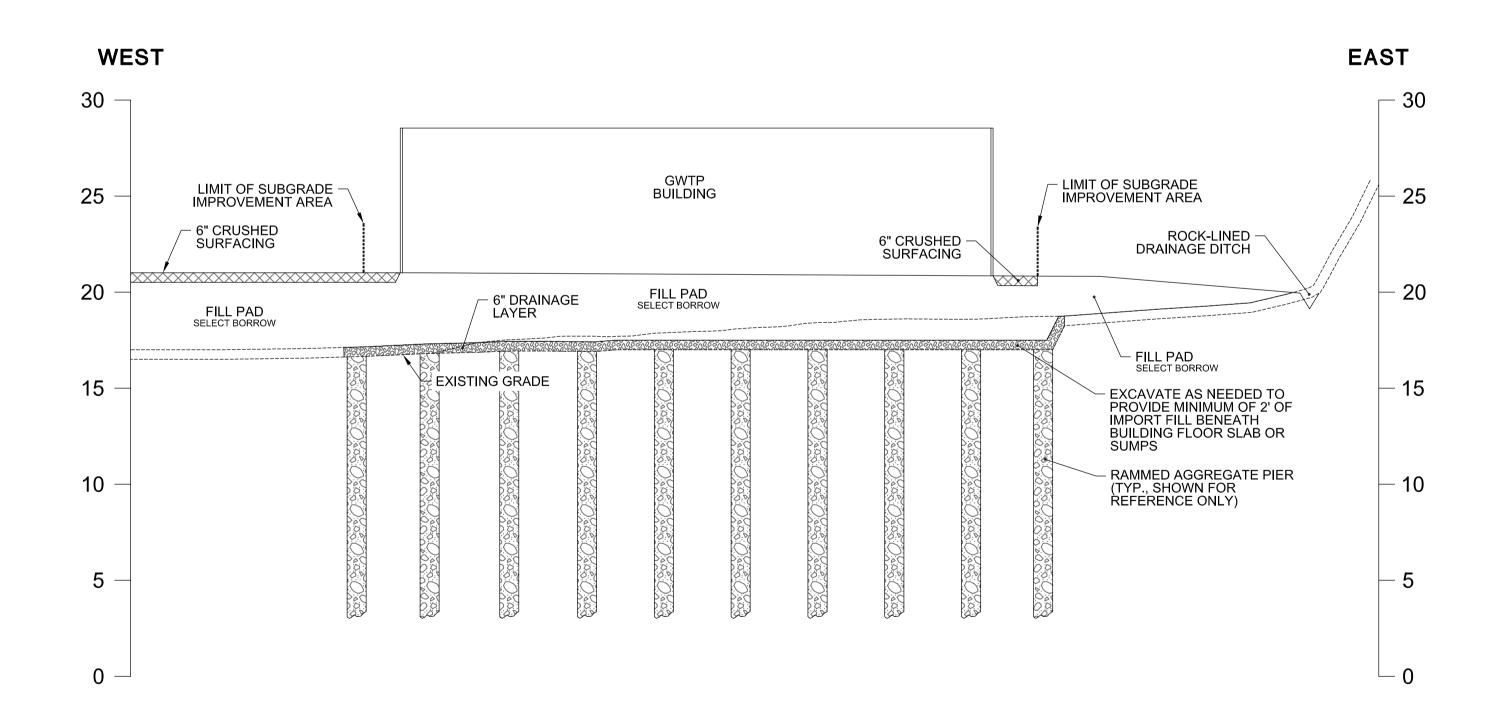








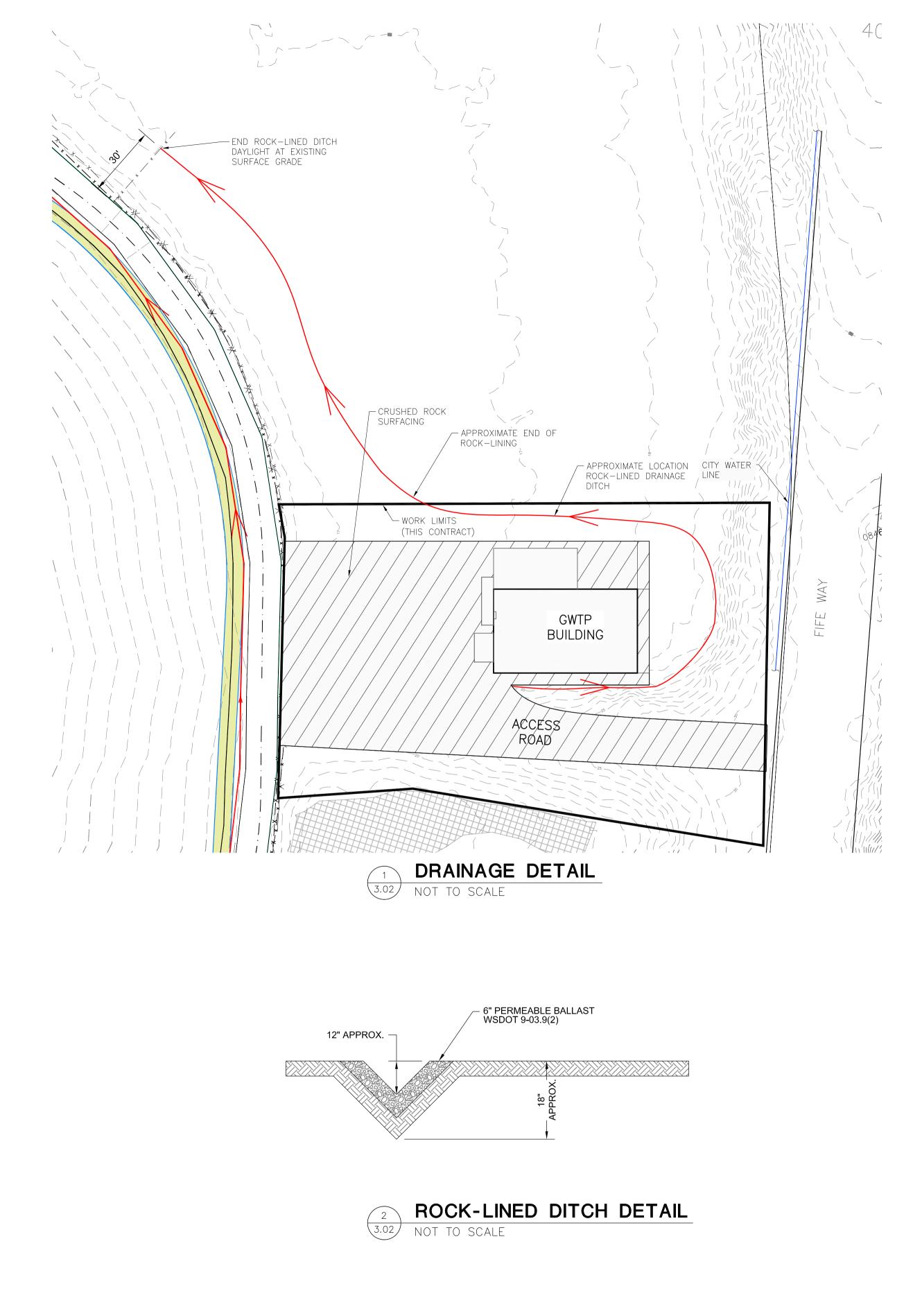






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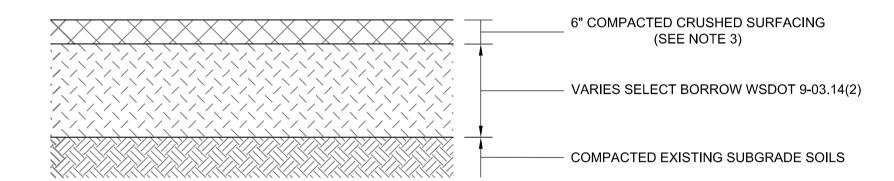
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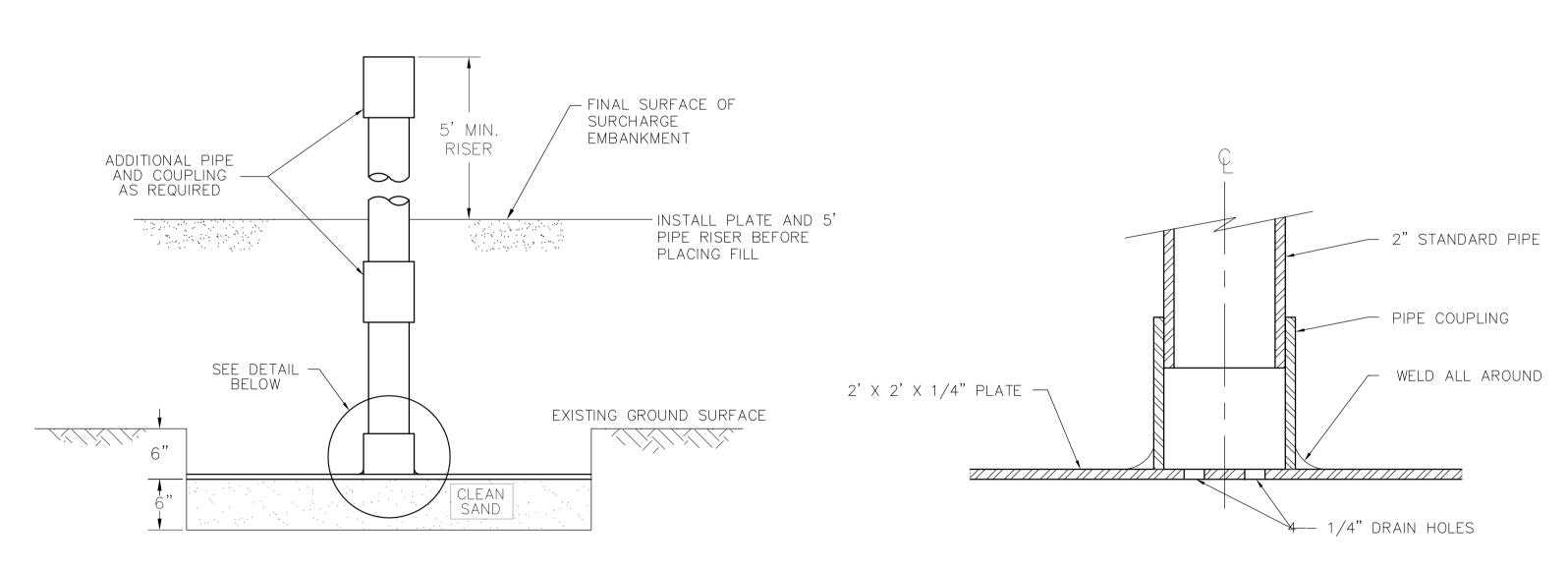
NOTES

- 1. DEPTHS ARE COMPACTED THICKNESS.
- 2. ALL UTILITY TRENCHING AND EXCAVATION TO BE COMPLETED AND BACKFILLED PRIOR TO PLACING CRUSHED SURFACING.
- 3. CRUSHED SURFACING TO MEET THE FOLLOWING REQUIREMENTS:

SIEVE	PERCENT PASSING
3/4	100
No. 4	50-78
No. 8	37-67
No. 40	13-35
No. 200	4-15



3 CRUSHED ROCK SURFACING DETAIL NOT TO SCALE





<u>6</u>			Client Logo:	Client: B&L CUSTODIAL TRU	JST	DATUM: PROJECTION:	PROJECT:	B&L WOODWASTE EQUIPMENT BUILDING	PROJECT NO.: SE 10160010
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