

PROJECT MEMORANDUM

TO:	Eric C. Evans / Shelter Holdings
PREPARED BY:	Donald Huling, P.E. / HWA GeoSciences Inc.
SUBJECT:	Proposed Pile Foundation Types Everett Riverfront Development Everett, WA
PROJECT NO.:	2015-061-21 Task 600
DATE:	May 09, 2018

This memorandum provides a description of deep foundation types proposed to support development of the Riverfront property in Everett, Washington. The project site is located within the limits of the Everett Landfill/ Tire Fire Site as shown in the vicinity map, Figure 1. The deep foundation types outlined herein have been proposed to satisfy the intent of the Consent Decree (State of Washington v. Everett, 2001) both inside and outside of the identified Pile Restriction Zone. This memorandum is meant to provide information and facilitate approval of the proposed deep foundation types for development on the landfill property.

Project Understanding

It is our understanding that Shelter Holdings, LLC. proposes to develop the Everett Landfill/ Tire Fire Site with a mixed-use development. The approximate current geometry of the proposed development is shown in the site and exploration plan, Figure 2. As shown in Figure 2, access to the site would be provided by a newly constructed Riverfront Drive, bisecting the site and connecting the current 41st Street to the south to 36th Street to the north. A series of multistory structures are proposed along both sides of Riverfront Drive with retail and parking on the first floor and residential units above. All buildings will be supported on deep foundations bearing in the very dense sandy soils at depth to avoid settlement related damage over the life of the structures. Parking outside the buildings, roadways and plazas will be constructed at grade.

Existing Available Geotechnical Data

In support of past development concepts for the Everett Landfill/ Tire Fire Site, two main geotechnical exploration programs have been completed. Most recently, HWA drilled a series of geotechnical borings, designed BH-1 through BH-35, in support of Project Evergreen along the north side of the site (HWA, 2017). The approximate locations of these explorations are shown on the site and exploration plan, Figure 2. Copies of the logs of these borings are included in Appendix A. Earlier, GeoEngineers drilled a series of geotechnical borings, designated GEI-1 through GEI 47, in 2007 in support of Oliver McMillan's 21312 30th Drive SE Suite 110 Bothell, WA 98021.7010

21312 30^{ac} Drive SE Suite 110 Bothell, WA 98021.7010 Tel: 425.774.0106 Fax: 425.774.2714 www.hwageo.com Federal & State Certified DBE / MWBE May 9, 2018 HWA Project No. 2015-061-21 Task 600

explorations are shown on the site and exploration plan, Figure 2. Copies of the logs of these borings are included in Appendix B. It should be noted that Geoengineer's borings were drilled prior to preloading of the Everett Landfill/ Tire Fire Site. Therefore, the soil contacts presented in the Geoengineer's borings should be adjusted to account for observed preloading settlements.

Summary of Soil and Groundwater Conditions

The previous soil investigations at the project site have encountered a general soil sequence consisting of fill underlain by refuse, fine grained alluvium, coarse grained alluvium and glacially consolidated soils at depth. The thicknesses of each of these layers vary across the site but the general sequence is relatively consistent. The refuse and alluvial deposits are compressible in nature and are expected to undergo settlement over time, even after preloading. The dense glacial soils at depth will provide bearing for the proposed deep foundations.

Two groundwater aquifers have been observed across the site. The shallow aquifer consists of water perched on the top of the fine grained alluvial soils found below the refuse. The shallow aquifer flows east through the refuse and is intercepted by the leachate collection system along the eastern boundary of the landfill. The deeper aquifer flows under the entire site through the dense glacial soils. The deep aquifer is hydraulically connected to the Snohomish River located to the east. Where present, the fine grained alluvial soils act as a barrier between the shallow and deeper aquifer. As defined in previous reports, the aquitard separating the shallow and deeper aquifers is not present on the western portion of the Everett Landfill/ Tire Fire Site.

Regulatory Background

The Consent Decree for the site provides requirements for future development of the landfill property. Per the requirements of the Ecology Cleanup Action Plan, a Shallow Aquifer Characterization study was completed (HWA, 2005). This study defined a Pile Restriction Zone across portions of the landfill site and set restrictions for pile foundations installed within the zone. The geometry and extent of the pile restriction zone is shown in Figure 2 and was identified based on the presence or absence of an aquitard layer below the refuse and the presence or absence of observed contamination within the refuse.

The Shallow Aquifer Characterization report states with respect to the Pile Restriction Zone that "Deep foundation systems in these areas should be restricted to augercast piles or other equivalent piling types that minimize the potential to create new groundwater flowpaths parallel to the pile. The rationale for using augercast piles instead of driven piles is that the likelihood of creating a hydraulic connection between the shallow and deep aquifer is higher with the use of driven piles. Driven pile systems employing some form of external grouting would also decrease the potential for cross-connection of shallow and deep zones." (HWA, 2005). Therefore, deep foundations systems installed within the Pile Restriction Zone are required to consist of augercast piles or an equivalent piling system that minimize the potential to create new groundwater flowpaths between the shallow and deep aquifer.

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Challenges Associated with Augercast Pile Foundations

Our explorations suggest that the refuse across the site is highly variable in composition and relatively porous. During our 2017 drilling operation, loss of drilling fluid into the refuse was documented at several boring locations across the site. This observed migration of drilling fluid suggests that grout from construction of augercast piles has a high potential for migration into the refuse layer if this foundation system is used. Given the variability in the composition of the refuse, it is impossible to estimate the volume, geometry and depth of potential grout migration. From a construction cost prospective, grout migration potential for augercast piles could be addressed through construction contingencies. However, from a design perspective, grout migration can influence anticipated downdrag loading and affect localized pile capacity, representing a significant design concern.

Downdrag loads are loads that develop on deep foundations when the compressible soils settle relative to the pile. Downdrag loads generally develop as friction along the pile surface. As long-term settlements are expected across the landfill site, downdrag loads are expected to develop on all pile foundations. However, grout migration into the refuse, associated with the use of augercast piles, would result in significant increases in the magnitude of the anticipated downdrag loads. We calculate that downdrag loads associated with grout migration into the refuse could be large enough to fail the piles in some areas. Due to potential downdrag induced augercast pile failures, we do not recommend the use of augercast piles at this site. We recommend that alternative deep foundation systems, which provide equivalent minimization of potential to create new groundwater flowpaths between the shallow and deep aquifer, be utilized.

Literature Review

To identify alternative deep foundation systems to be used within the pile restriction zone, HWA conducted a literature review with respect to pile foundations driven at Brownfield sites. Through this review HWA identified several research papers focused on evaluating contaminate transport along pile foundations driven through aquitards. Attempts were made to identify case histories that documented contaminate migration associated with various piles types driven through landfill refuse and penetrating underlaying aquitards. However, no such case histories were identified. The lack of available case histories is likely due to the relatively recent push for development of Brownfield sites. Due to the lack of relevant case histories, alternative deep foundation selection was based on guidance from available research on the topic.

Available research associated with contaminate migration due to deep foundations penetrating aquitard layers is focused on numerical modeling and scale testing of multiple pile types driven through a modeled aquitard (Satyamurthy, 2005). This research indicated that contamination migration through an aquitard layer, due to deep foundation penetrations, can be described by three mechanisms. These mechanisms include Direct transfer of soil at the pile tip (one-time event), flow along the perimeter of the pile, and flow through the pile. Each of these mechanisms is described below.

Direct transfer

Direct transfer is a mechanism of contaminate migration that is associated with the pushing of a plug of contaminated soils through the aquitard when driving displacement piles. Numerical modeling and conventional soil mechanics suggest that the volume of the plug pushed down with the pile is directly related to the shape of the tip of the pile. For driven displacement piles, a large plug is transported with flat ended piles and a significantly reduced plug is transported with piles installed with a conical tip. Therefore, driving displacement piles with a conical tip is preferred to limit the direct transfer mechanism.

Perimeter Flow

Perimeter flow is the mechanism wherby contamination migrates along the perimeter of the pile element through the aquitard layer. Numerical models and scale testing suggest that the magnitude of anticipated perimeter flow is dependent on the type of pile installed. Satyamurthy's research suggests that perimeter flow is greater for drilled pile systems, such as auger-cast piles and drilled shafts, than for driven displacement piles. The reduced perimeter flow for driven displacement piles is associated with the increased pressure developed during driving. Numerical models and scale testing suggest that the pressure caused by driving of the pile results in the soils within the aquitard squeezing the pile, helping to prevent perimeter flow. This increased pressure is not present for drilled deep foundation systems. Satyamurthy's research also suggests that perimeter flow is greater for low displacement driven piles such as H-piles or square and octagonal displacement piles, due to the increased surface area of the pile and reduced increase in pressure during driving. Therefore, driving circular displacement deep foundation systems will result in less perimeter flow of contaminates than drilled foundation elements.

Flow Through Foundation Element

Flow through the foundation element occurs when a foundation system is installed that is porous enough to allow for the flow of contaminates through the foundation element itself. Research suggests that this is only a consideration when using timber displacement piles. Therefore, timber piles should not be used as a foundation system for this site. Steel pile piles or grout piles do not experience flow through the foundation element.

Based on these three contaminate migration mechanisms, the ideal deep foundation system for use within the pile restriction zone would be steel or grout, driven, circular displacement piles, with conical tips.

Proposed Deep Foundation Systems

Based on available research, we propose to utilize two alternate deep foundation systems to support proposed structures within the Pile Restriction Zone. The first of these systems is driven steel pipe piles utilizing post grouting procedure to minimize the potential to create new groundwater flowpaths between the shallow and deep aquifer. The second system consists of driven grout piles with a sleeve installed through the refuse layer. Based on currently available

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research, we believe that these deep foundation systems are superior to augercast piles in that they will minimize the potential for new groundwater flowpaths to develop between the shallow and deep aquifer. A detailed description of each proposed foundation system is provided below.

Steel Pipe Piles with Post Grouting

Driven steel pipe piles have been a widely accepted method of providing deep foundation support for structures over poor soils for decades. Steel pipe piles are generally driven into the ground with a diesel hammer affixed to a large crawler crane. The piles are driven into the underlying bearing soil until the desired load bearing capacity has been achieved. The advantage of steel pipe piles is that they are quick to install, do not generate spoils at the surface and their smooth surface limits downdrag loads with respect to other deep foundation systems. However, it is our understanding that there has been concern that driving steel pipe piles within the Pile Restriction Zone would increase the potential to create new groundwater flowpaths between the shallow and deep aquifer. As stated above, available research has shown that driven steel pipe piles are one of the best functioning deep foundation systems for preventing migration of contaminates when driven through an aquitard. However, to further minimize this potential, we recommend that any steel pipe piles to be driven within the Pile Restriction Zone be fabricated to allow for post grouting below the base of the refuse layer. Post grouting would force grout around the outside of the steel pile, providing a grout to soil seal around each pile below the bottom of the refuse layer.

To allow for post grouting, each pile would be fabricated with a conical tip and maximum half-inch diameter perforations through the side walls of the pile at all four quadrants of the pile. The perforations would be spaced a maximum of 2-feet apart from the tip of the pile to the point on the pile that corresponds to the base of the refuse layer. No perforations would be made above the point on the pile corresponding to the bottom of refuse at the pile location. The upper extent of perforations for each individual pile would be unique and determined based on the elevation of the base of the refuse and the required embedment depths; and, as determined by a test pile program for each structure.

The test pile program would consist of driving test piles at each proposed structure prior to starting production pile driving. The number and location of the test piles for each structure would be determined by the geometry of each structure and expected variability in the bearing soil. Each test pile would be driven to the desired vertical capacity. The first test pile at each building would be considered sacrificial and would be driven with perforations installed over the entire length of the pile. This pile would be pressure grouted after driving and be used to estimate the appropriate extent of perforations on the rest of the piles. The required embedment depth data obtained from each test pile program would be used to develop anticipated embedment depths for the remaining production piles. This data coupled with the soil geometry, obtained from the geotechnical explorations, would allow for reasonable design of the pile perforations to achieve grouting across the fine grained alluvial aquitard at all pile locations.

Pressure grouting of each pile would take place after pile driving is complete or once the piledriving equipment was a sufficient distance to allow for post grouting equipment to access the pile cap. Post grouting would be achieved by placing grout inside the pile. The grout would be tremied to the base of the pile to displace any water than entered the pile through the perforations. Once each pile is filled with grout, a packer will be applied at the top of the pile and pressure would be applied to the grout. Grout for each pile would be pressurized to a level greater than the overburden pressure at the midpoint of the fine grained alluvial aquitard. This pressure level would force the grout out of the perforations and into the surrounding soil unit. The grout extruded from the pile would form a seal around the pile that would minimize the potential to create new groundwater flowpaths between the shallow and deep aquifer. A schematic representation of the pile geometry and the steps involved in the use of a driven pipe pile system is shown in Figure 3.

Driven Grout Piles with Steel Sleeve Through Refuse

Driven Grout Piles are installed using a mandrel that is placed in a "boot". The mandrel is then charged with fluid grout. At this point the driving commences. Once bearing is achieved, grout is injected under pressure as the mandrel is extracted. The design of the mandrel keeps soil from entering the grout column. Upon completion, a grout pile, very similar to an augercast pile, is created. However, unlike a drilled auger cast pile, the driven grout piles will generate the increased pressure within the soil, reducing the potential for migration of contaminates along the perimeter of the foundation system. For use at this site, we recommend that the driven grout pile installation process be modified to include installation of a steel sleeve through the refuse. The sleeve would terminate at the base of the refuse and would be used to minimize grout migration into the refuse. Steel sleeves would be driven with a boot at each location to the base of the refuse. Once the sleeve was in place the mandrel would be installed through the center of the sleeve and driving of pile would continue. Much like augercast piles, driven grout piles could fail under downdrag loads if not sleeved through the refuse. The length of sleeve for each pile would be determined based on the depth of the base of the refuse, observed in the geotechnical borings. Below the sleeved portion of the pile the driven grout pile system would have the same soil to grout interface as an augercast pile system. This grout to soil interface would minimize the potential to create new groundwater flowpaths between the shallow and deep aquifer, as required within the pile restriction zone. A schematic representation of the pile geometry and the steps involved in the use of a driven grout pile system, with a sleeve through the refuse, is shown in Figure 4.

Summary

To avoid potential downdrag failure of deep foundations and reduce the potential for contaminate migration, we recommend that augercast piles not be used at this site. Although the intent of the Cleanup Action Plan with respect to pile types was to reduce cross-contamination from the shallow to deep aquifer, no geotechnical engineering analysis was done at that time; i.e., the infeasibility of auger cast piles in refuse was not considered. Additionally, the Cleanup Action Plan was developed prior to publishing of the most recent research on the topic

(Satyamurthy, 2005). The pile types proposed herein meet the environmental intent of the Cleanup Action Plan and also meet engineering criteria for successful foundation design.

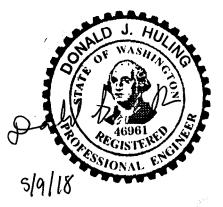
We recommend that post grouted steel pipe piles and/or driven grout piles, sleeved though the refuse, be used as the preferred deep foundations system for development within the Pile Restriction Zone. Both proposed systems would provide equivalent or better minimization of the potential to create new groundwater flowpaths between the shallow and deep aquifer than auger cast piles. We believe that both of these proposed deep foundation systems satisfy the requirements of the consent decree and request that approval for their use be provided.

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We appreciate this opportunity to be of service.

Sincerely,

HWA GEOSCIENCES INC.



Donald Huling, P.E. Principal Geotechnical Engineer

LIST OF FIGURES (FOLLOWING TEXT)

Figure 1 V	<i>Vicinity</i>	Map
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- Figure 2 Site and Exploration Plan
- Figure 3 Steel Pipe Pile with Post Grouting Through Aquitard
- Figure 4 Driven Grout Pile with Sleeve Through Refuse

APPENDICES

Appendix A: HWA Previous Explorations

Appendix B: GeoEngineers Previous Explorations

Proposed Pile Foundation Type

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REFERENCES

- Consent Decree, *State of Washington, Department of Ecology v. City of Everett* (CD-2696-2001-NCN), dated April 02, 2001.
- GeoEngineers geotechnical report entitled *Geotechnical Engineering Services, Foundation Design Report, Everett Landfill Site, Everett Riverfront Redevelopment Project*, dated July 24, 2008.
- HWA GeoSciences Inc. environmental report entitled *Shallow Aquifer Characterization Agency, Everett Landfill and Tire Fire Site, Everett, Washington*, dated June 27, 2005.
- HWA GeoSciences Inc. geotechnical report entitled *Geotechnical Report, Project Evergreen, Everett, Washington,* dated August 18, 2017.
- Satyamurthy, Ranjan, "Investigations of Pile Foundations in Brownfields" (2005). University of New Orleans Theses and Dissertations. 245.



Map not to scale. Excerpt from Google Maps Earth



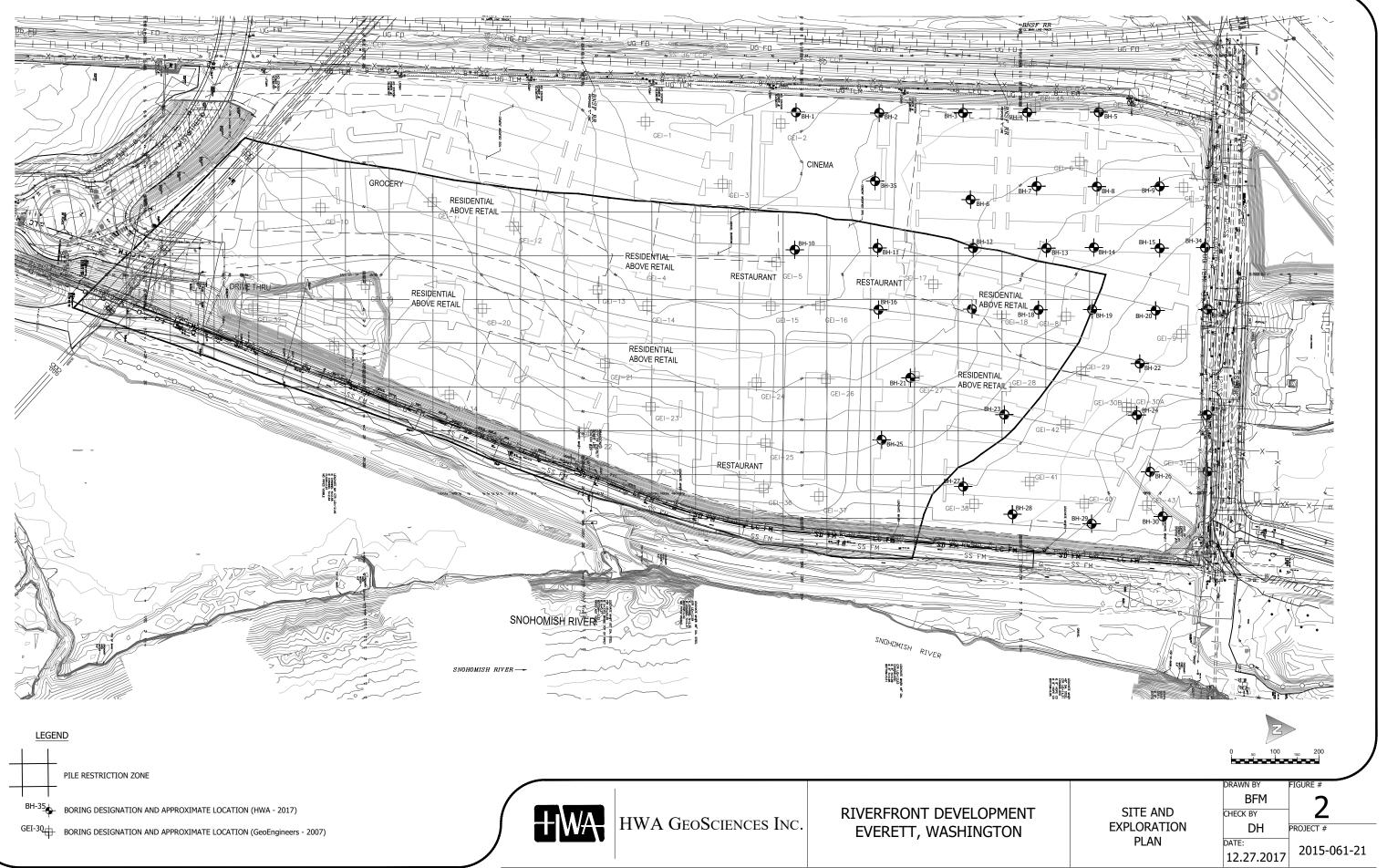
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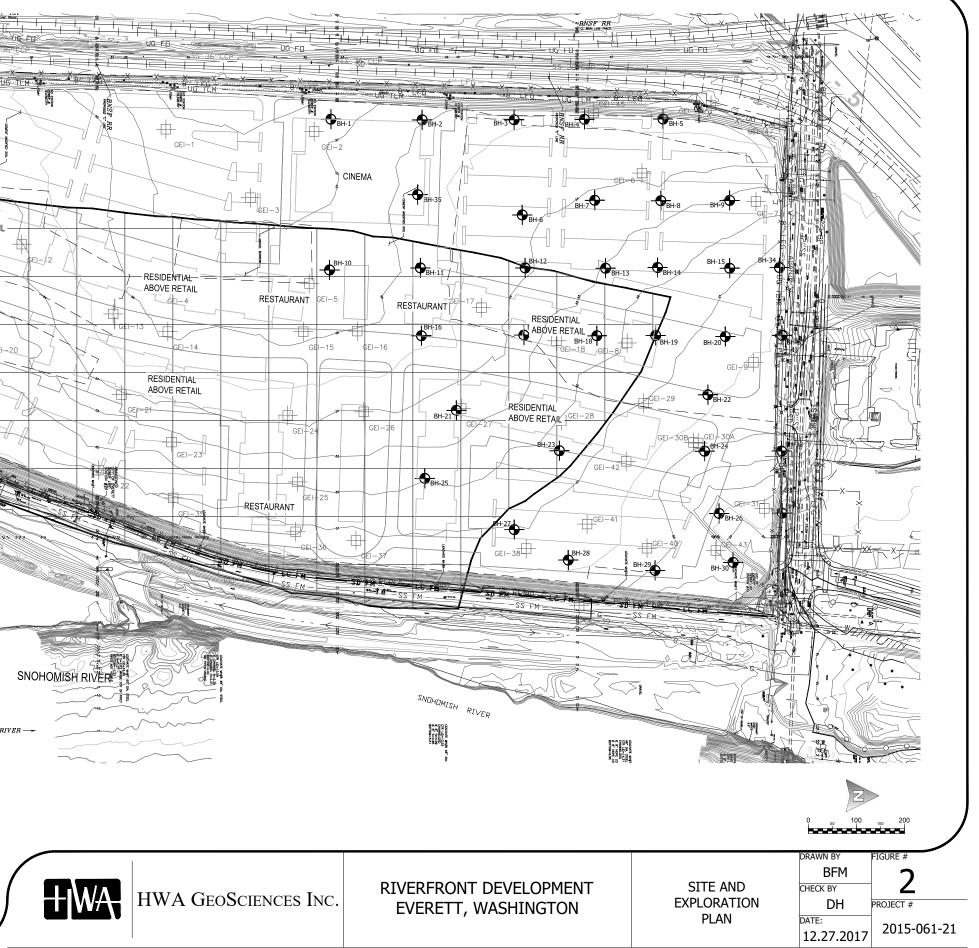
FIGURE NAME

SITE VICINITY MAP EVERETT RIVERFRONT DEVELOPMENT EVERETT, WA FIGURE NO.

PROJECT NO.

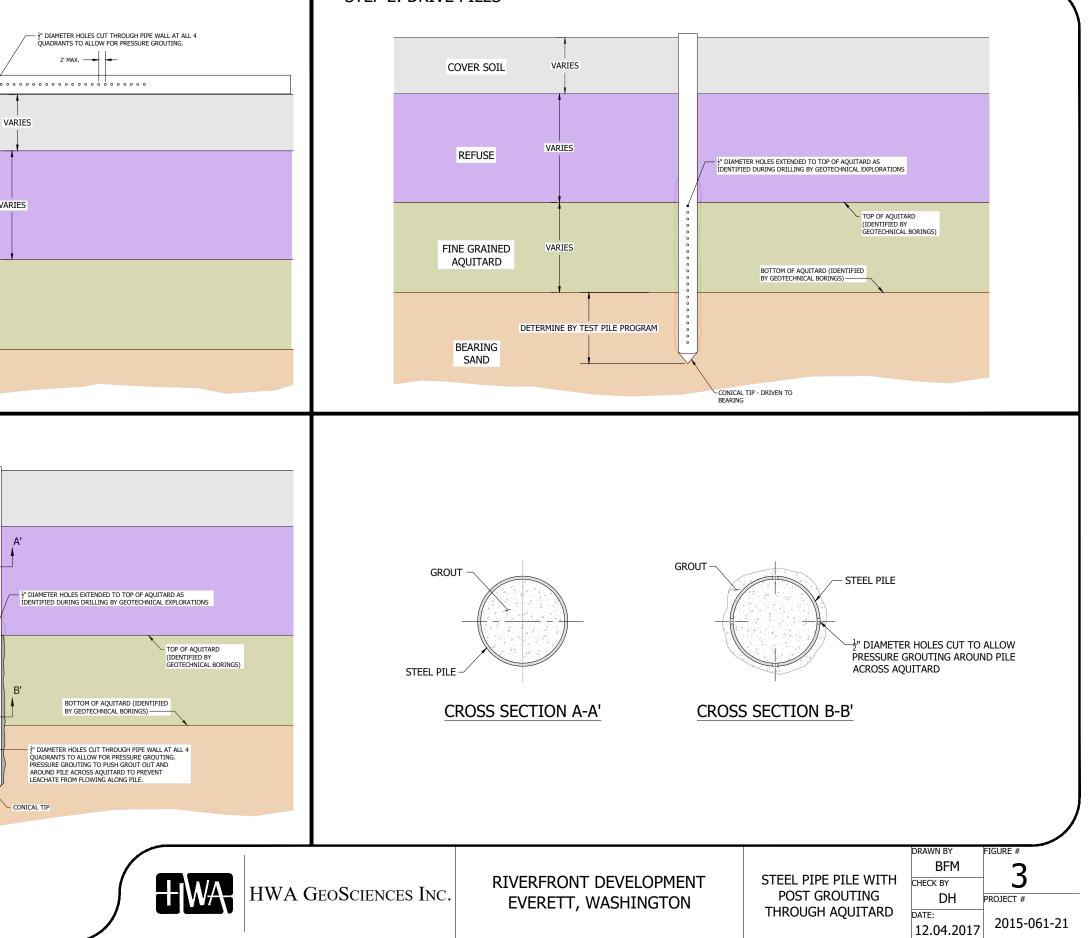
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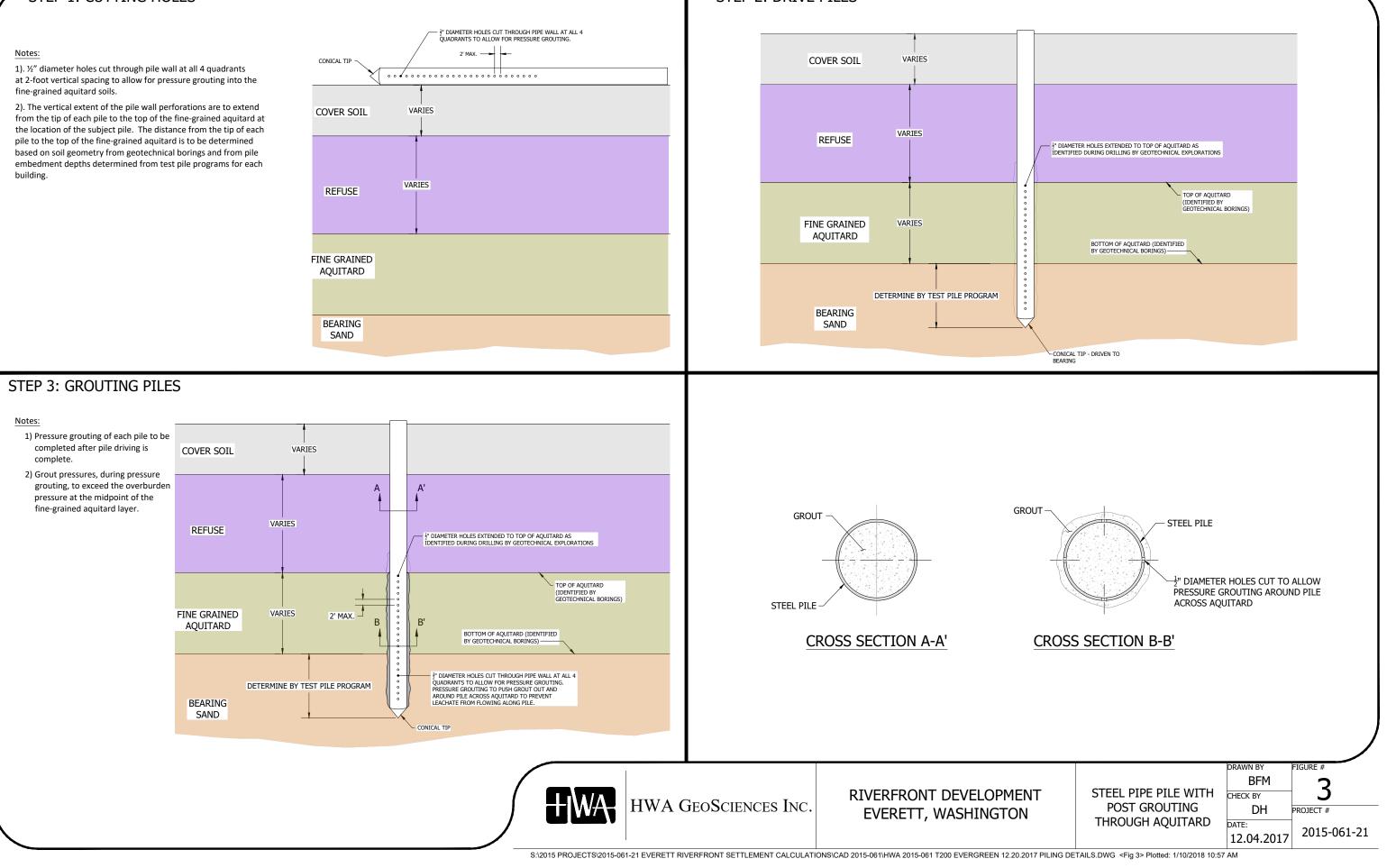


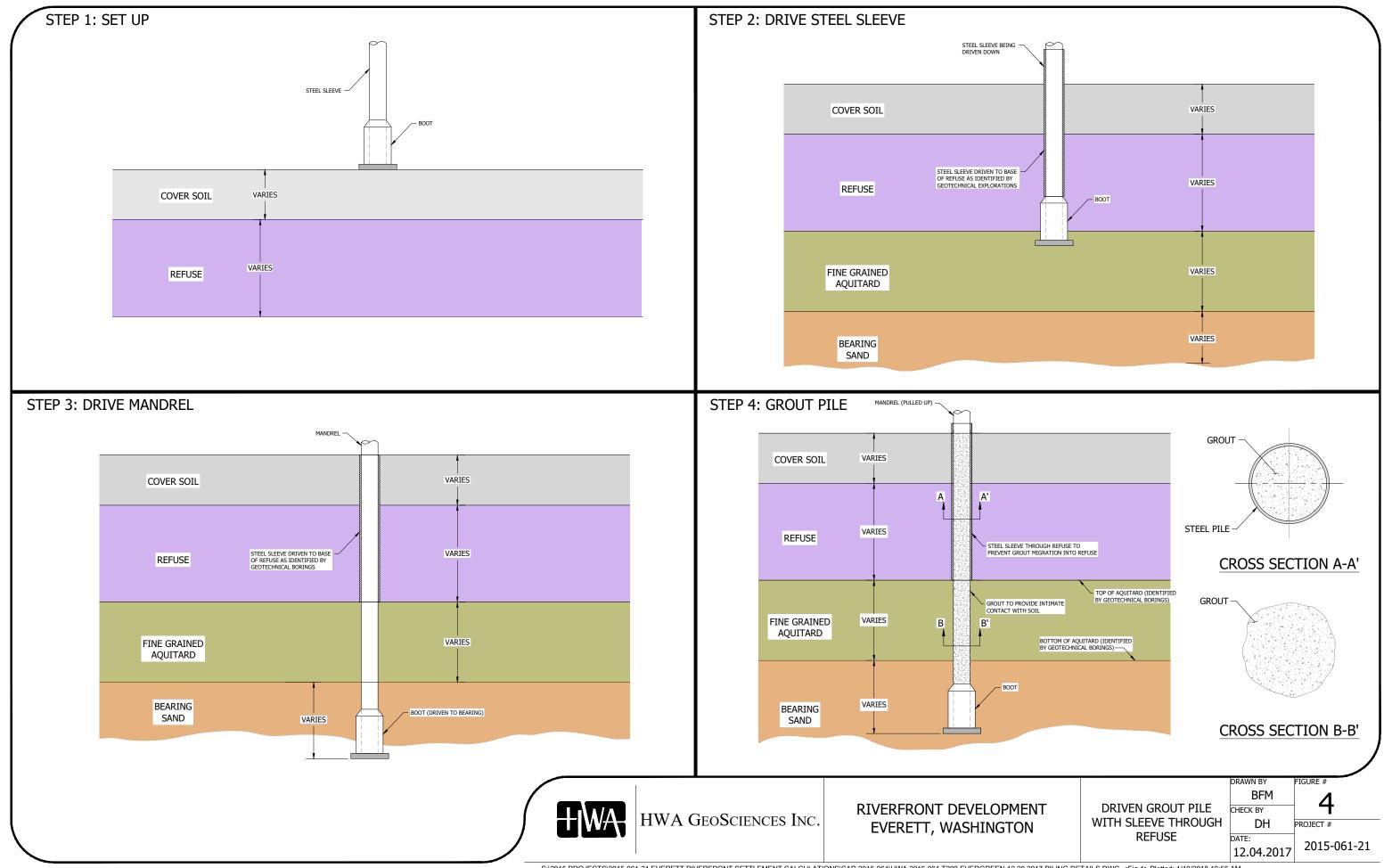


STEP 1: CUTTING HOLES

STEP 2: DRIVE PILES







APPENDIX A

HWA PREVIOUS EXPLORATIONS

APPENDIX A

FIELD INVESTIGATION

The subsurface exploration program consisted of 35 boreholes, drilled to depths ranging from 40 to 141.5 feet below the existing ground surface. Drilling equipment was selected based on site access conditions, and included a truck-mounted drill rig and two track-mounted drill rigs.

Proposed exploration locations were staked in the field by HWA with GPS, per locations chosen and plotted on the site plan by HWA. Upon completion of the boreholes, the actual locations and elevations were subsequently plotted with GPS as some were moved to accommodate access conditions. The approximate exploration locations are shown on the Site and Exploration Plan, Figure 2.

Each of the explorations was completed under the full-time observation of an HWA geologist. HWA personnel recorded pertinent information including soil sample depths, stratigraphy, soil engineering characteristics, and ground water occurrence as the explorations were drilled. Soils were classified in general accordance with the classification system described in Figure A-1, which also provides a key to the exploration log symbols. Soil layers containing refuse are indicated with a dark mottled hatch pattern in the left-hand column for soil symbols. The summary logs of boreholes are presented on Figures A-2 through A-36. The stratigraphic contacts shown on the individual logs represent the approximate boundaries between soil types. The actual transitions may be more gradual.

Under subcontract to HWA, Holocene Drilling, Inc. of Puyallup, Washington, drilled the borings in May and June, 2017. HWA sampled soils to depths of up to 141.5 feet in the borings. Holocene employed a truck-mounted Mobile B-58 drill rig, a track-mounted CME 850 drill rig, and a Diedrich D-50 tracked drill rig. The rigs employed eight-inch outer diameter hollow stem augers. QuikGel bentonite fluid was added to the inside of the auger once a depth of approximately 40 feet had been reached, to provide additional weight to reduce sand heave in the auger. Seven of the deeper borings were drilled using mud rotary methods, after augering to depths of approximately 40 to 45 feet. Mud rotary drilling consisted of advancing a tri-cone bit approximately 4.5 inches in diameter on rotating drill rods. Drilling "mud" consisting of bentonite grout was circulated through the tip and up the borehole into a settling and recirculating tub.

Soil samples were obtained from most of the borings using driven split spoon samplers as well as Shelby tubes pushed into soft soils by hydraulics. 1-inch ring samples were obtained at selected intervals and borings, inside a 3-inch O.D. California sampler driven with either a 140-lb. or 340-lb. hammer (depending on the drill rig).

Standard Penetration Test (SPT) samples were obtained by driving a two-inch split spoon sampler at the end of drilling rods (threaded heavy steel pipe). A 140-pound hammer with a 30inch drop was used to drive the sampler into the subsurface. Soil samples were then retrieved from the sampler. Five of the borings utilized 3-inch diameter modified California sampler split-spoon equipment to collect soil samples. A 340-pound hammer with a 30-inch drop was used to drive the 3-inch O.D. a sampler. Soil samples were then retrieved from the sampler. Summary blow counts from the 340-pound hammer were subsequently converted to SPTequivalent blows.

Soil samples were obtained every 2.5 feet in the upper 20 feet of each boring and then every five feet until boring completion. Extra samples between the five-foot intervals were obtained when necessary for Shelby tubes and 1-inch rings, or if the percent recovery of soil in a split spoon sampler at the regular five-foot interval was low.

Soil boring cuttings were placed in 1-cubic yard heavy cardboard boxes on pallets or in drums, and subsequently buried on site in the landfill by KLB Construction under contract to Shelter Holdings.

Upon completion, each borehole was abandoned in accordance with Department of Ecology requirements. Bentonite grout was mixed and pumped into each boring. Typically, the grout settled several feet, such that on a subsequent day each borehole was topped off to the ground surface with bentonite chips.

RELATIVE DENSITY OR CONSISTENCY VERSUS SPT N-VALUE

COHESIONLESS SOILS				COHESIVE SOIL	s
Density	N (blows/ft)	Approximate Relative Density(%)	Consistency	N (blows/ft)	Approximate Undrained Shear Strength (psf)
Very Loose	0 to 4	0 - 15	Very Soft	0 to 2	<250
Loose	4 to 10	15 - 35	Soft	2 to 4	250 - 500
Medium Dense	10 to 30	35 - 65	Medium Stiff	4 to 8	500 - 1000
Dense	30 to 50	65 - 85	Stiff	8 to 15	1000 - 2000
Very Dense	over 50	85 - 100	Very Stiff	15 to 30	2000 - 4000
			Hard	over 30	>4000

USCS SOIL CLASSIFICATION SYSTEM

MAJOR DIVISIONS			C	GROUP DESCRIPTIONS		
Coarse Grained Soils More than 50% Retained on No. 200 Sieve Size	Gravel and Gravelly Soils	Clean Gravel (little or no fines)	GW			
	More than 50% of Coarse Fraction Retained on No. 4 Sieve	Gravel with Fines (appreciable amount of fines)	GC GM			
	Sand and Sandy Soils	Clean Sand (little or no fines)	SW	Well-graded SAND Poorly-graded SAND		
	50% or More of Coarse Fraction Passing No. 4 Sieve	Sand with Fines (appreciable amount of fines)	SM			
Fine Grained Soils	Silt and Clay	Liquid Limit Less than 50%	CL — OL	SILT Lean CLAY Organic SILT/Organic CLAY		
50% or More Passing No. 200 Sieve Size	Silt and Clay	Liquid Limit 50% or More	мн сн	Fat CLAY		
	Highly Organic Soils		<u>1</u> PT	PEAT		

TEST SYMBOLS

	TEOTOT	NBOLO
%F	Percent Fines	
AL	Atterberg Limits:	PL = Plastic Limit LL = Liquid Limit
CBR	California Bearing Rat	io
CN	Consolidation	
DD	Dry Density (pcf)	
DS	Direct Shear	
GS	Grain Size Distributior	ı
K	Permeability	
MD	Moisture/Density Rela	tionship (Proctor)
MR	Resilient Modulus	
PID	Photoionization Device	e Reading
PP	Pocket Penetrometer Approx. Compres	ssive Strength (tsf)
SG	Specific Gravity	
TC	Triaxial Compression	
TV	Torvane Approx. Shear S	trength (tsf)
UC	Unconfined Compress	sion
	SAMPLE TYPE	
Х	2.0" OD Split Spoon ((140 lb. hammer with	
T		50 m. drop)
Ī	Shelby Tube	
	3-1/4" OD Split Spoon	with Brass Rings
\bigcup	Small Bag Sample	
ľ	Large Bag (Bulk) Sam	ple
	Core Run	
	Non-standard Penetra (3.0" OD split spoon)	tion Test
	GROUNDWATE	R SYMBOLS
$\overline{\Delta}$	Groundwater Level (me time of drilling)	easured at
Ţ	Groundwater Level (me	easured in well or

COMPONENT DEFINITIONS

COMPONENT	SIZE RANGE
Boulders	Larger than 12 in
Cobbles	3 in to 12 in
Gravel Coarse gravel Fine gravel	3 in to No 4 (4.5mm) 3 in to 3/4 in 3/4 in to No 4 (4.5mm)
Sand Coarse sand Medium sand Fine sand	No. 4 (4.5 mm) to No. 200 (0.074 mm) No. 4 (4.5 mm) to No. 10 (2.0 mm) No. 10 (2.0 mm) to No. 40 (0.42 mm) No. 40 (0.42 mm) to No. 200 (0.074 mm)
Silt and Clay	Smaller than No. 200 (0.074mm)

NOTES: Soil classifications presented on exploration logs are based on visual and laboratory observation. Soil descriptions are presented in the following general order:

Density/consistency, color, modifier (if any) GROUP NAME, additions to group name (if any), moisture content. Proportion, gradation, and angularity of constituents, additional comments. (GEOLOGIC INTERPRETATION)

Please refer to the discussion in the report text as well as the exploration logs for a more complete description of subsurface conditions.



ÁMMÁJÜURÒÔVÁÒXÒÜÕÜÒÒÞ EVERETT, WASHINGTUÞ

PROPORTION RANGE DESCRIPTIVE TERMS < 5%</td> Clean 5 - 12% Slightly (Clayey, Silty, Sandy) 12 - 30% Clayey, Silty, Sandy, Gravelly 30 - 50% Very (Clayey, Silty, Sandy, Gravelly) Components arranged in order of increasing quantities.

COMPONENT PROPORTIONS

DRY Absence of moisture, dusty, dry to the touch. MOIST Damp but no visible water. WET Visible free water, usually soil is below water table.

MOISTURE CONTENT

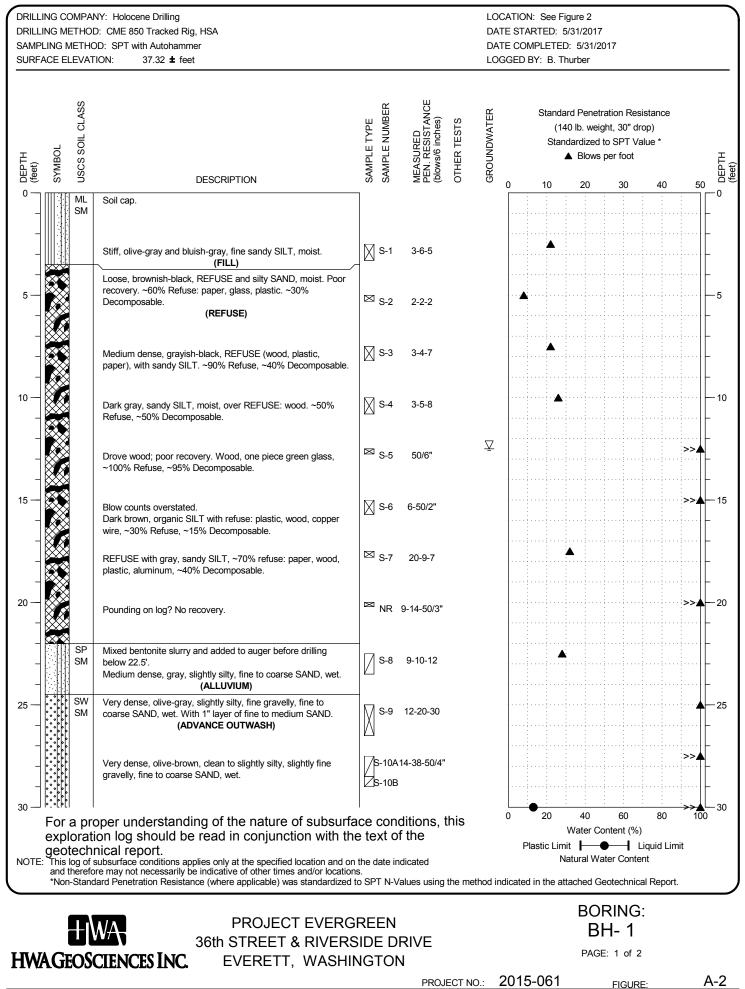
open hole after water level stabilized)

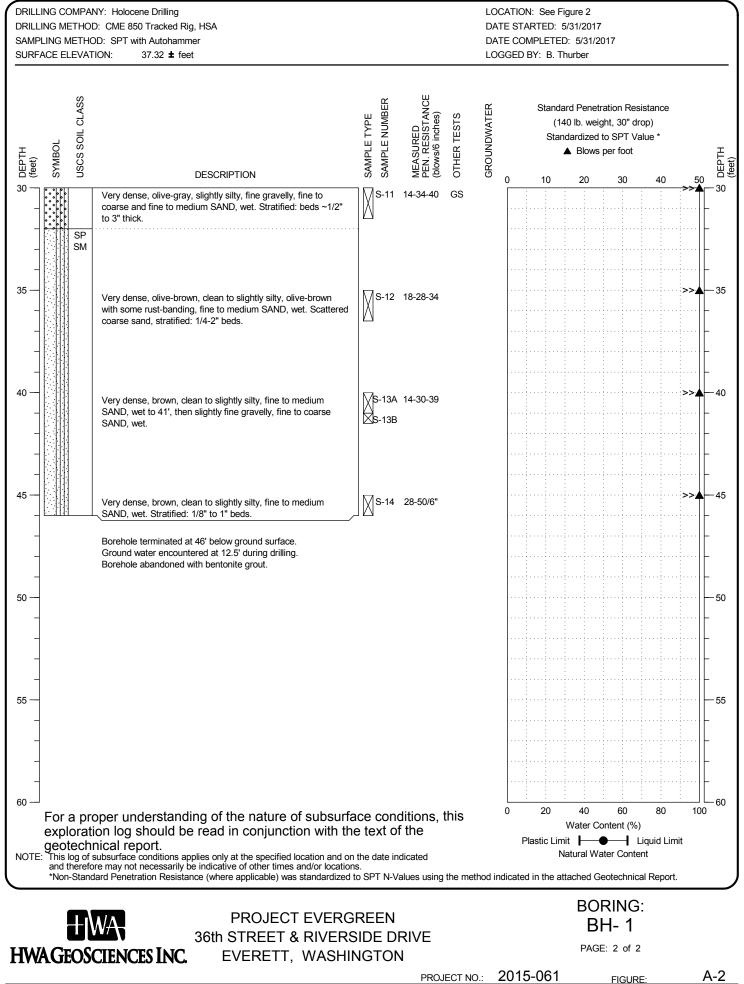
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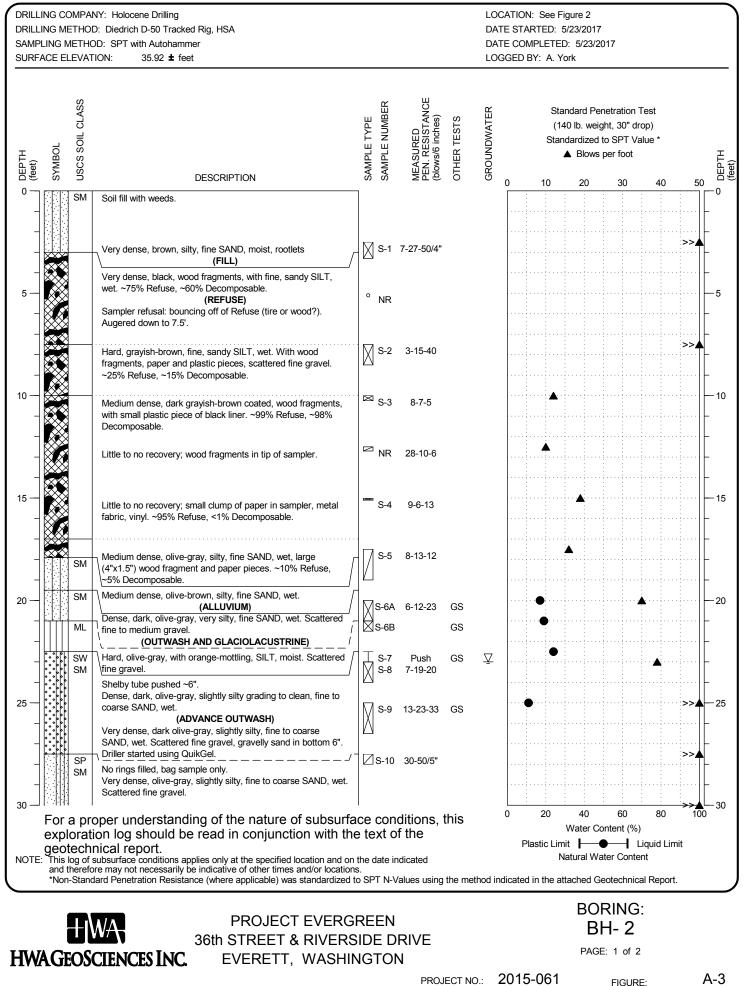
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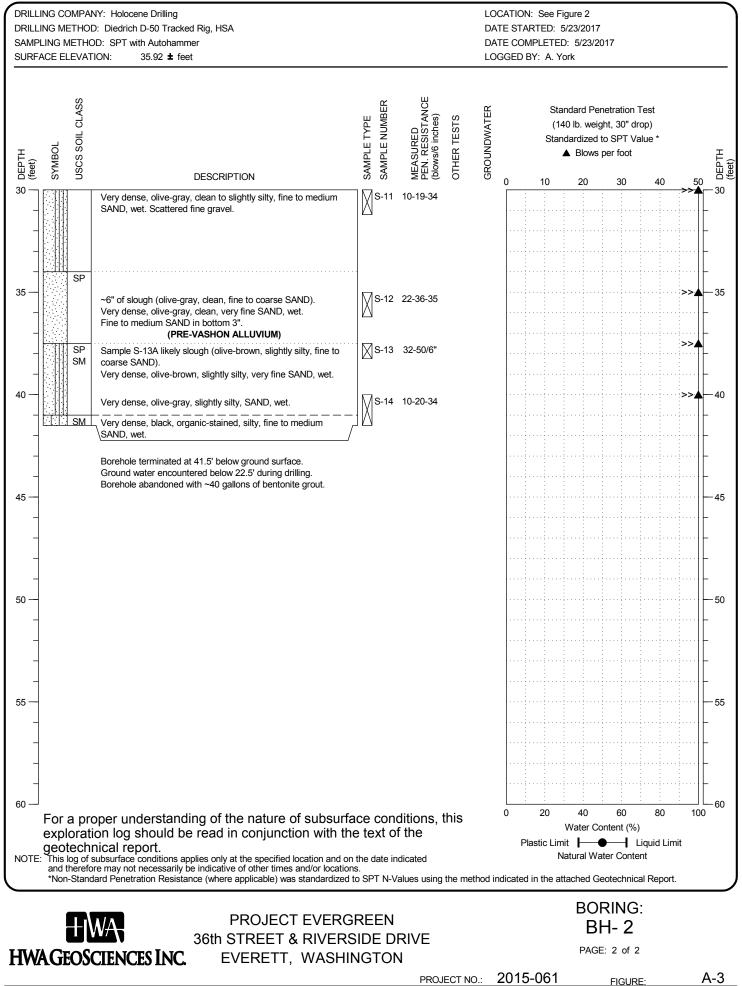
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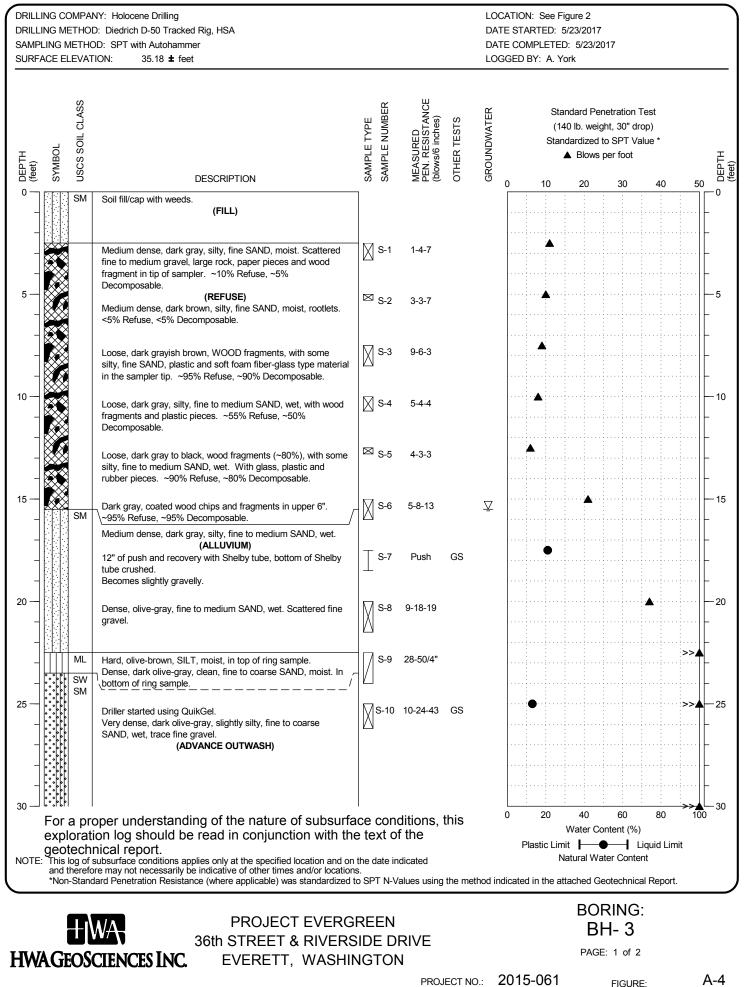






A-3





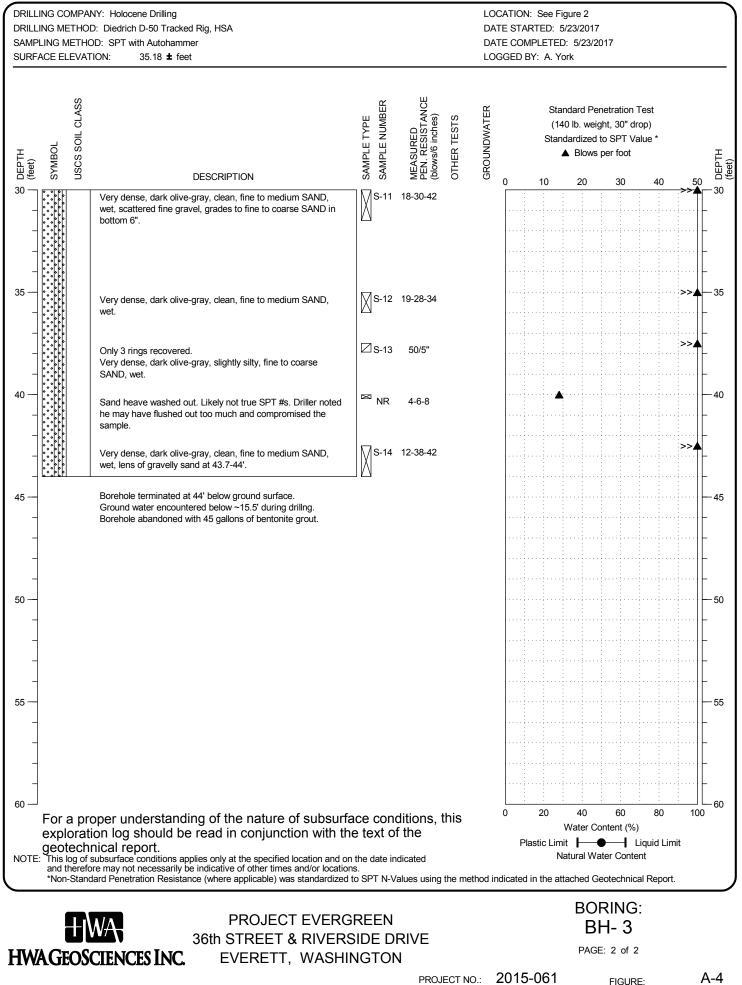
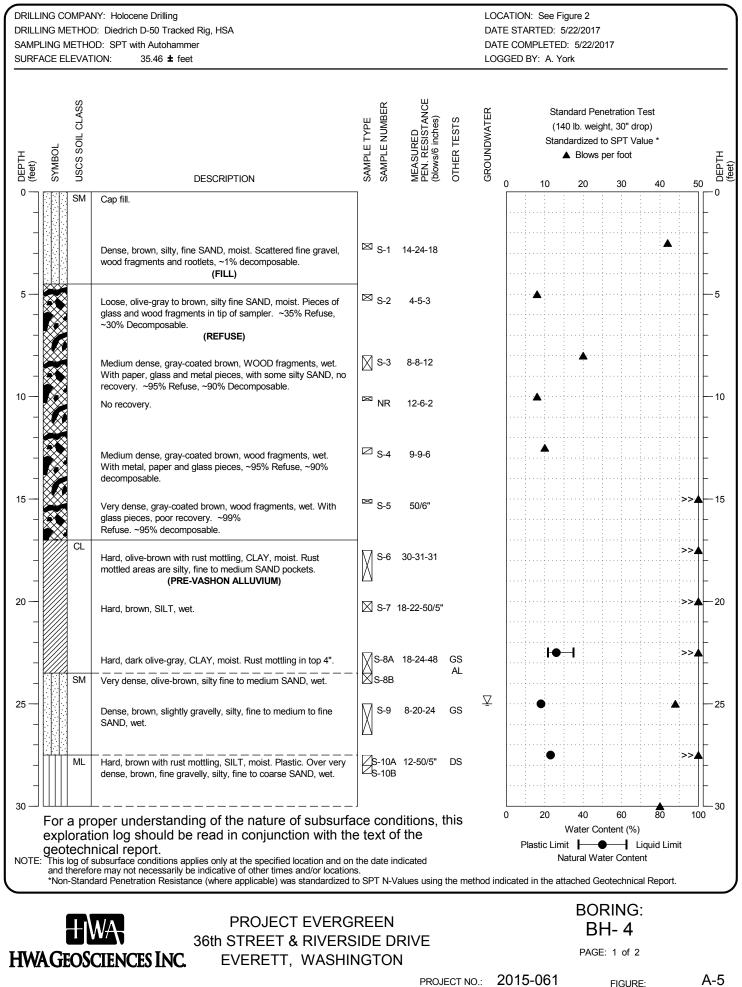
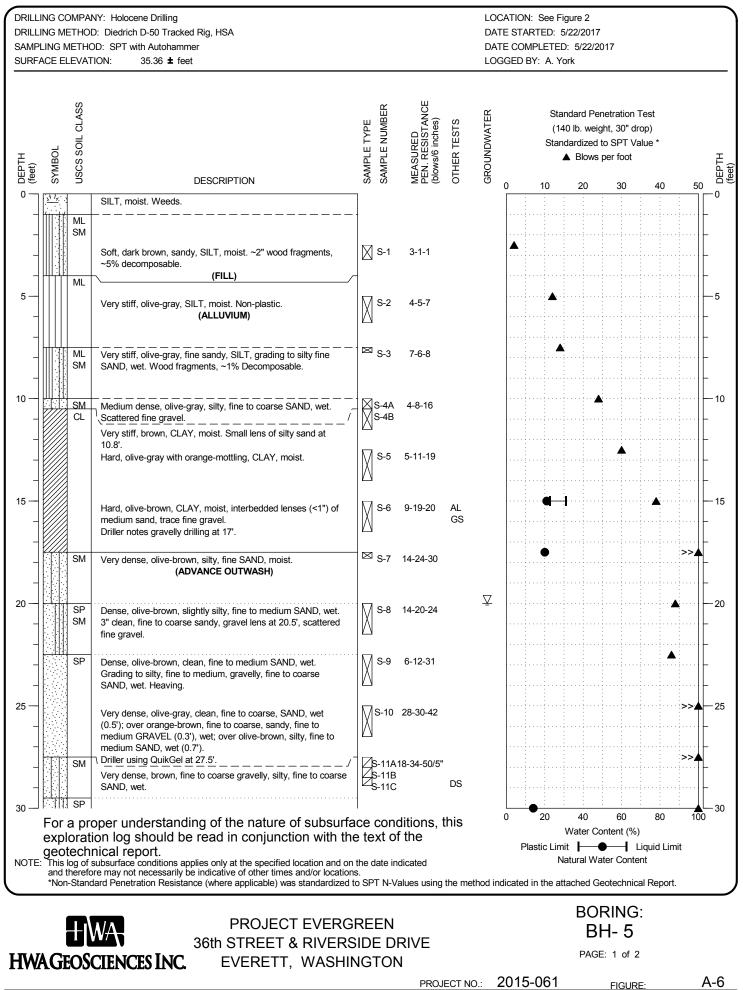
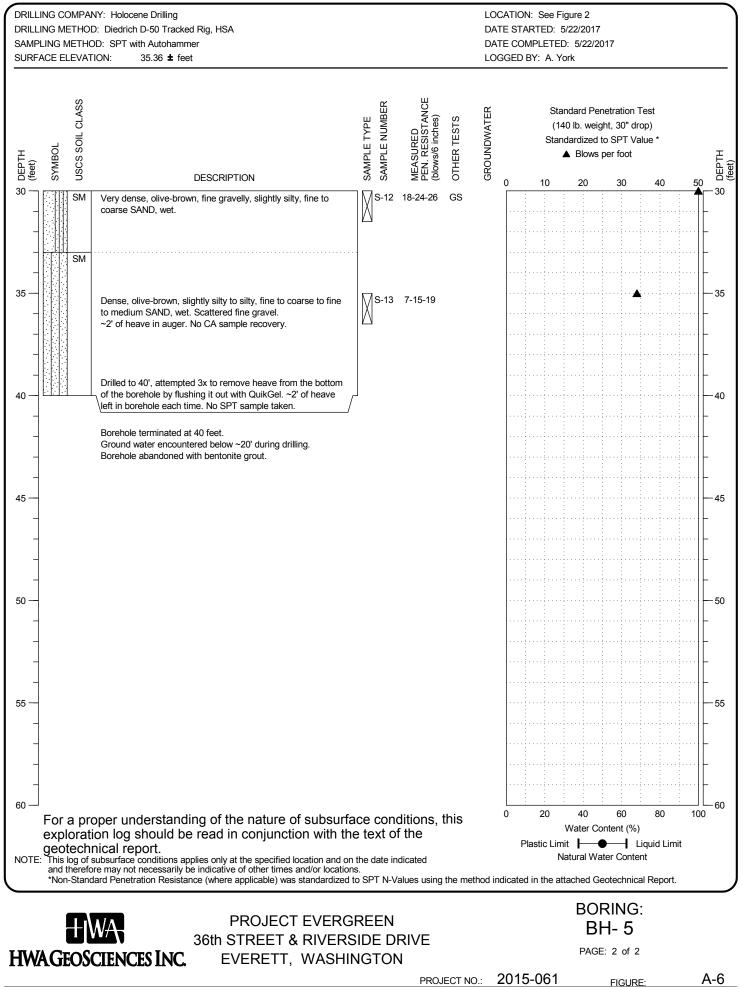


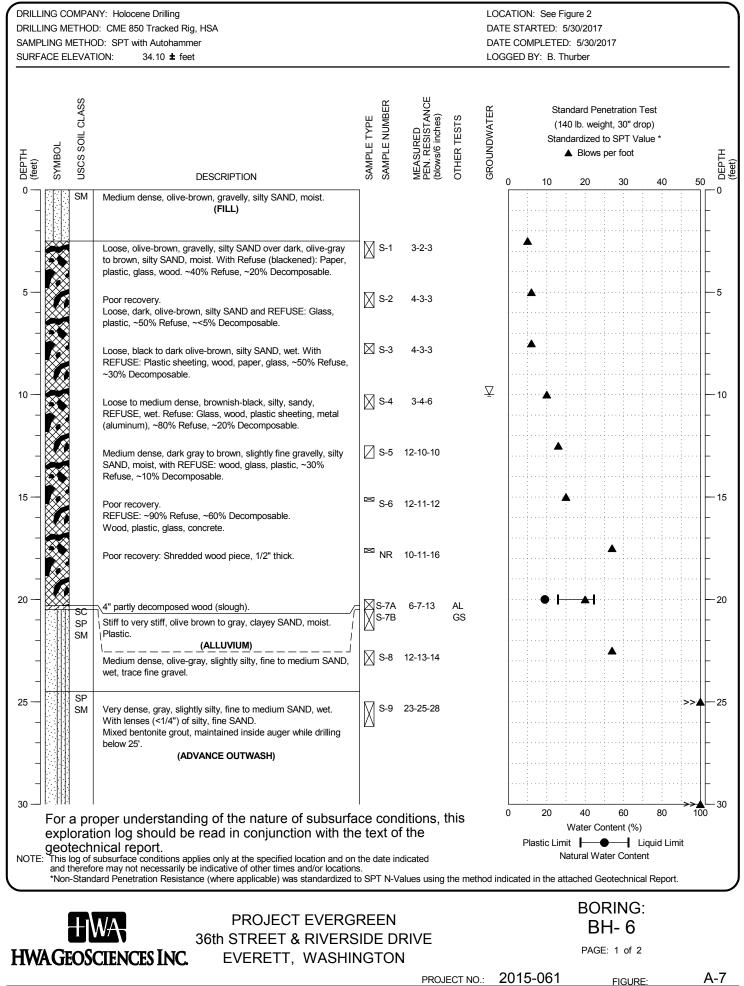
FIGURE:

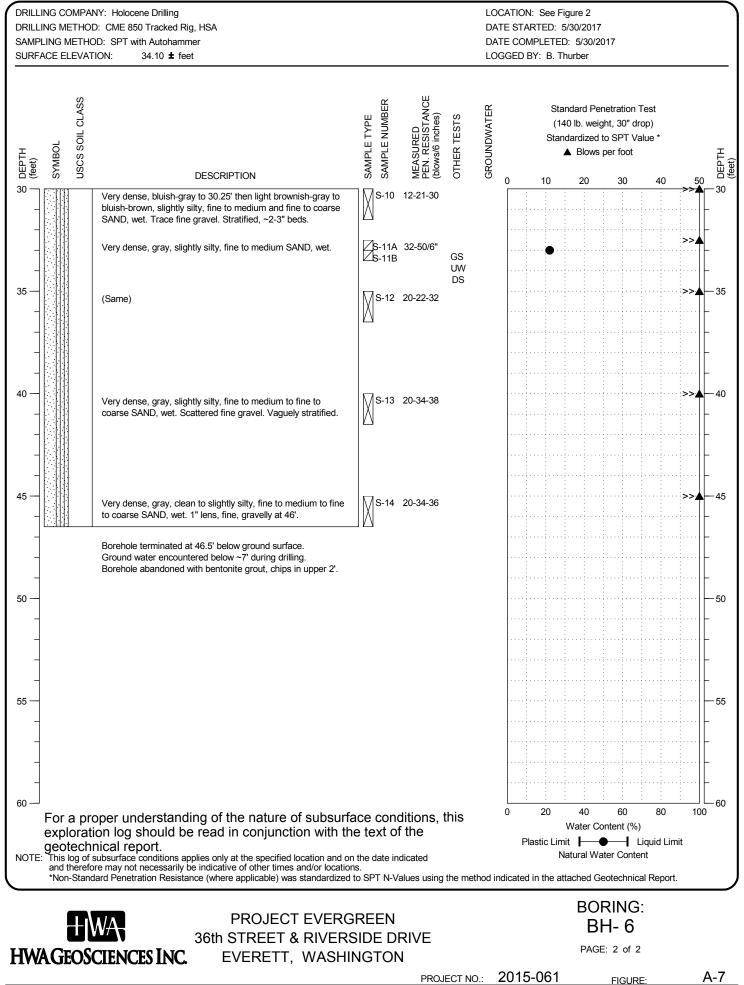


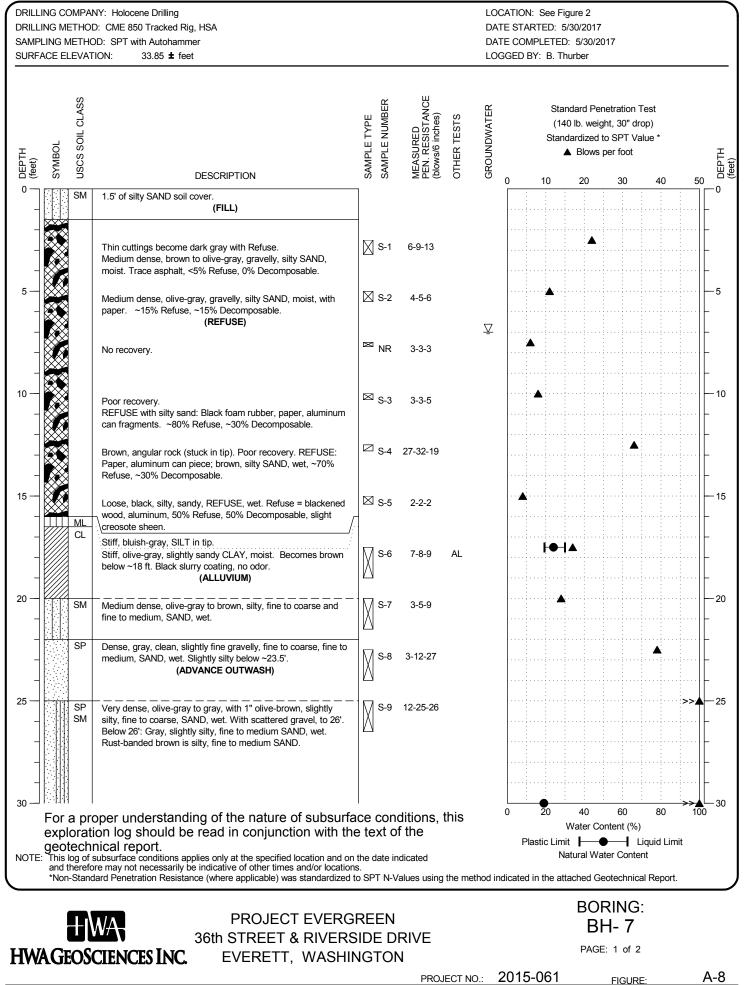




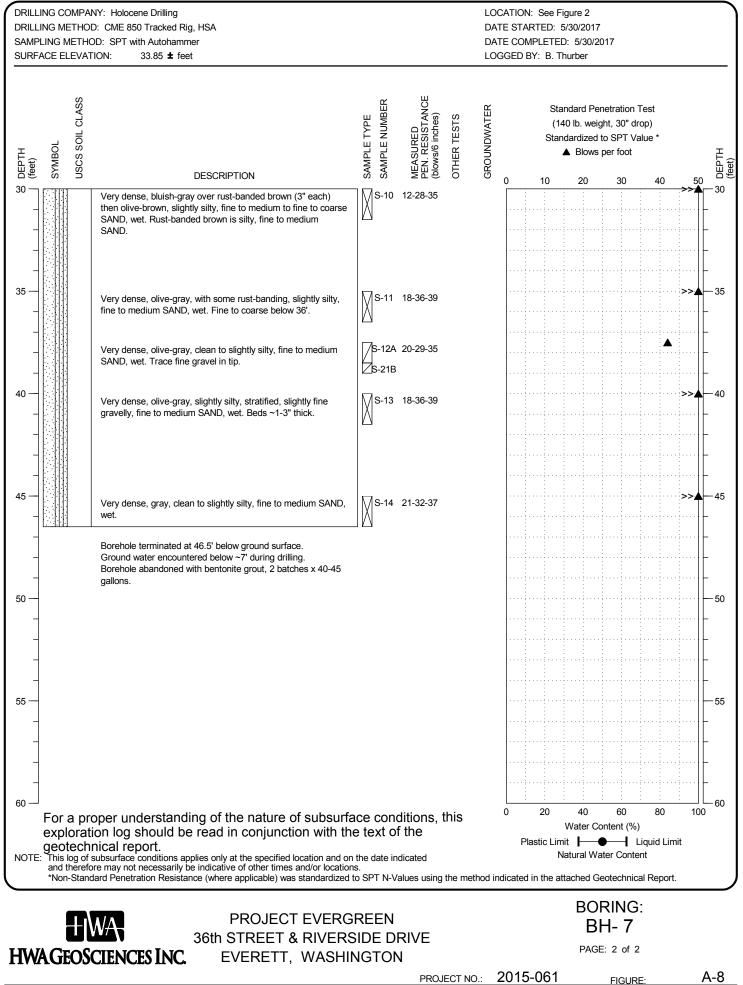


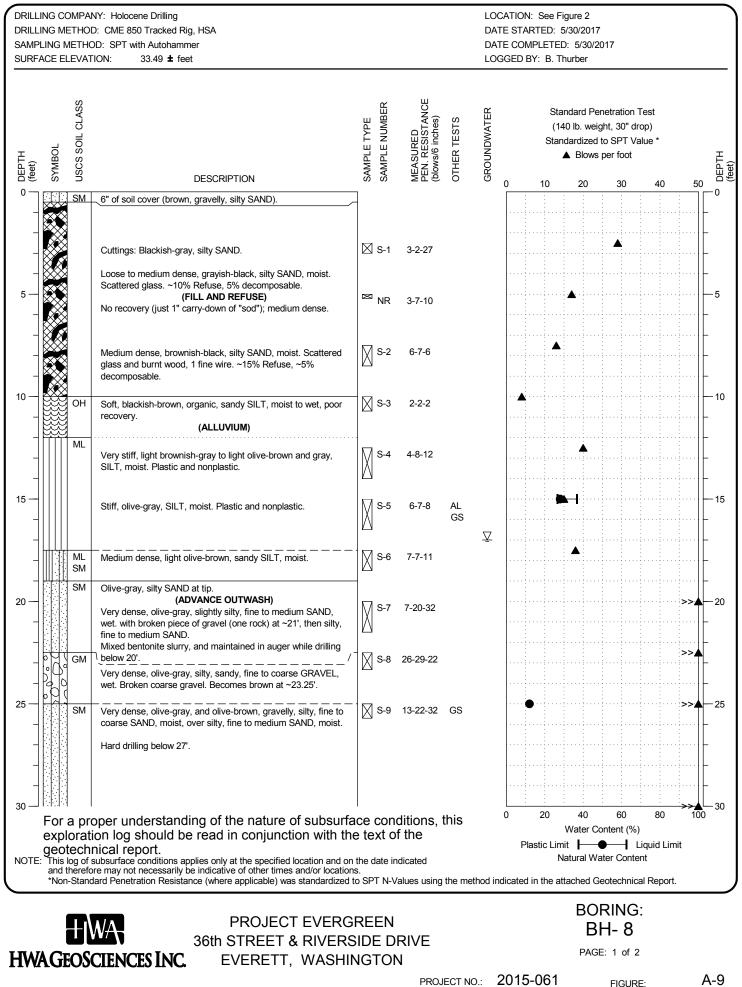


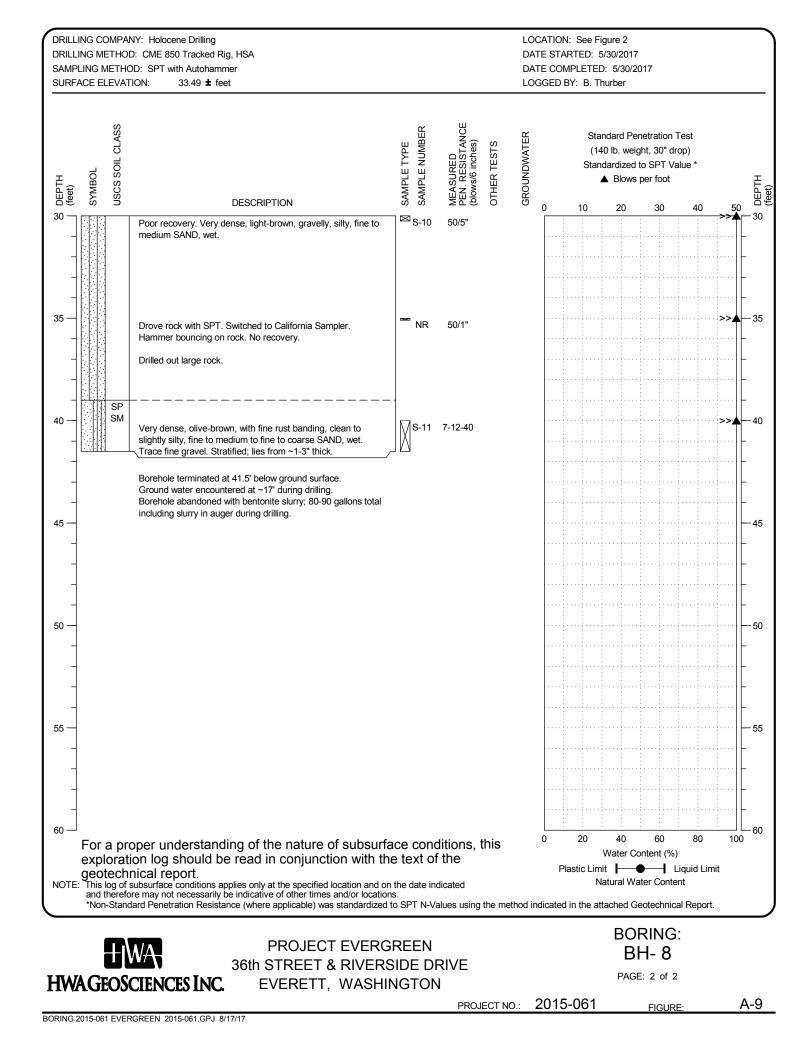


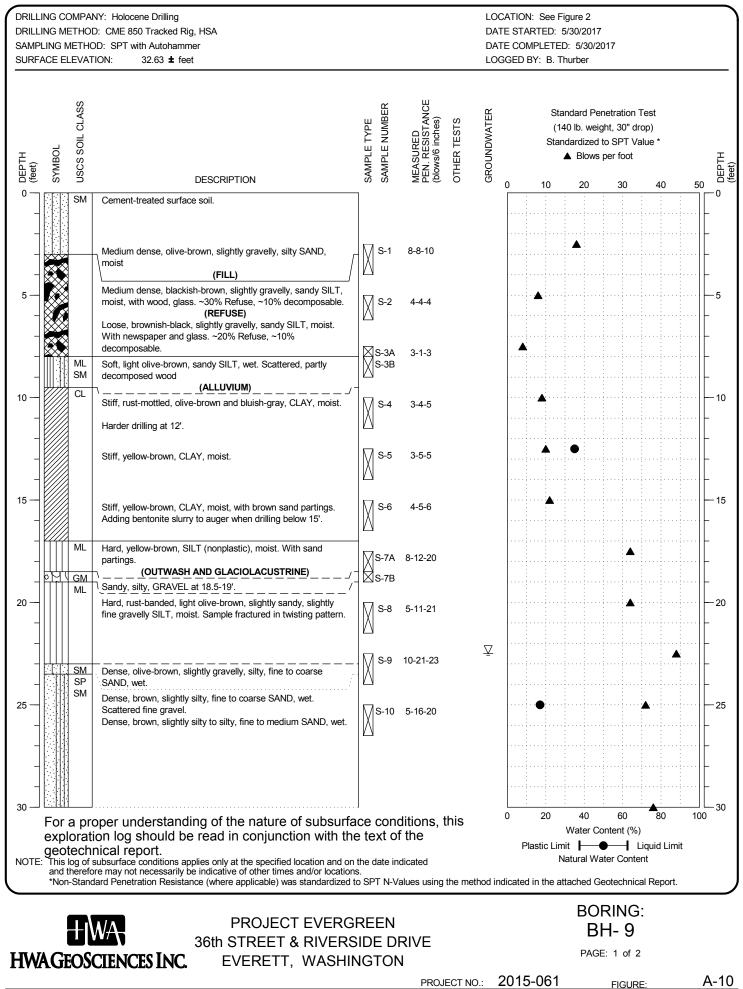


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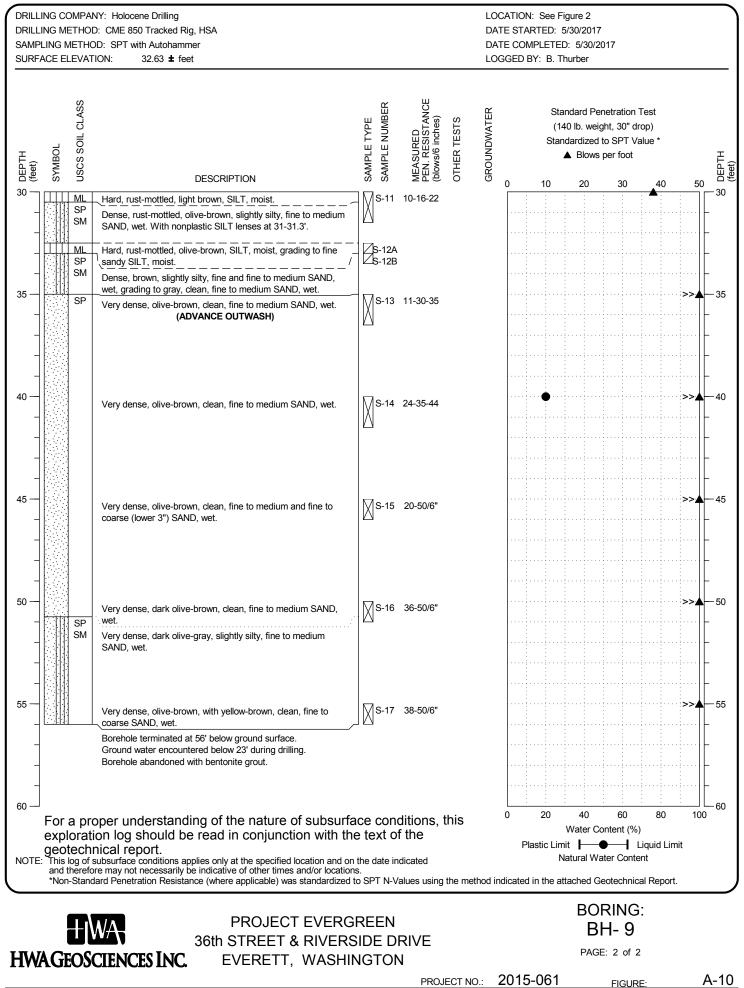


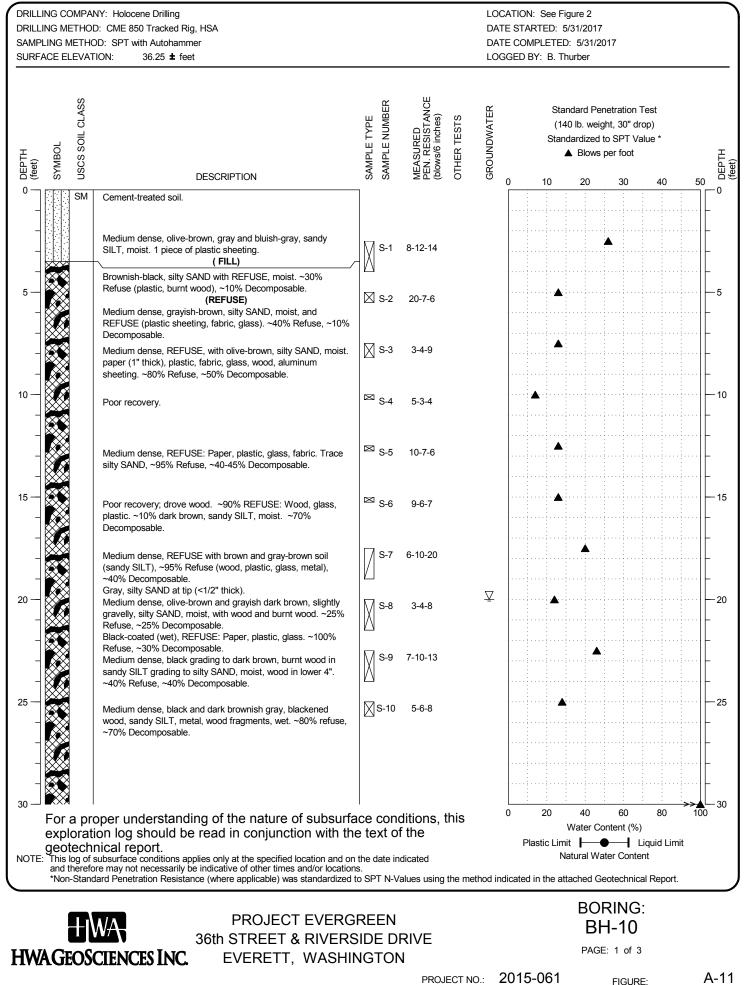


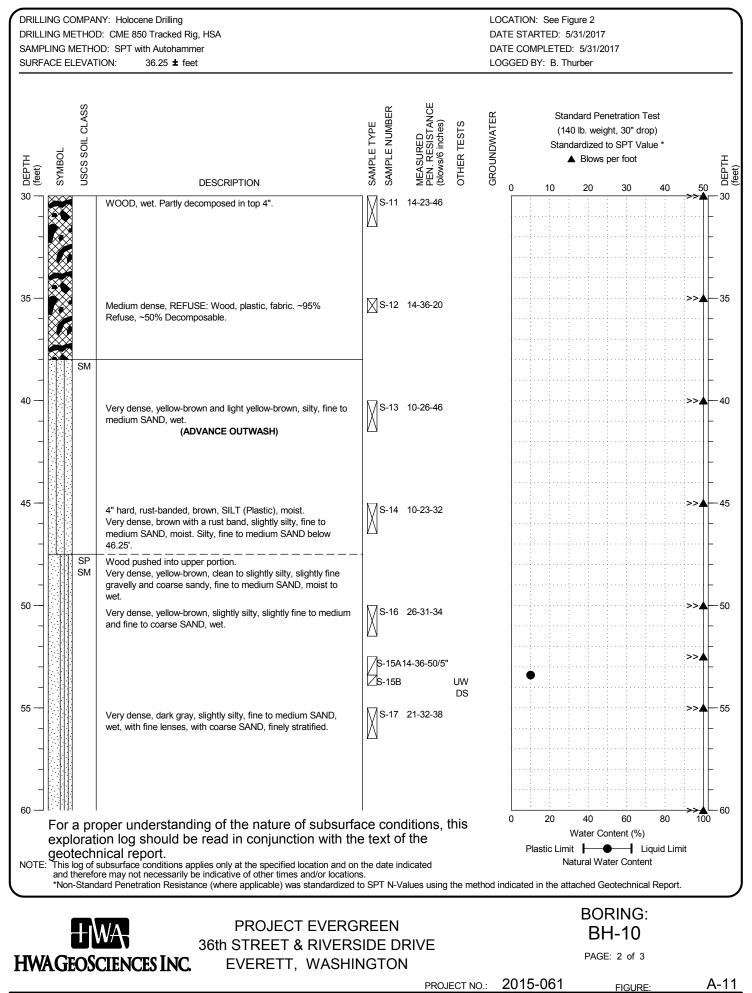


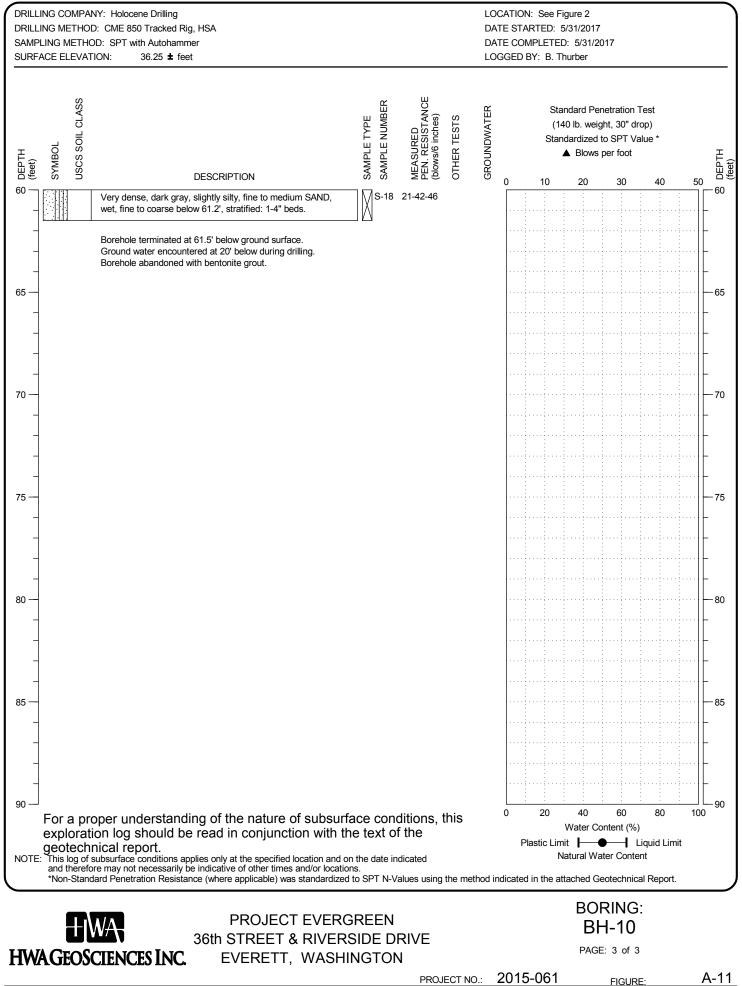


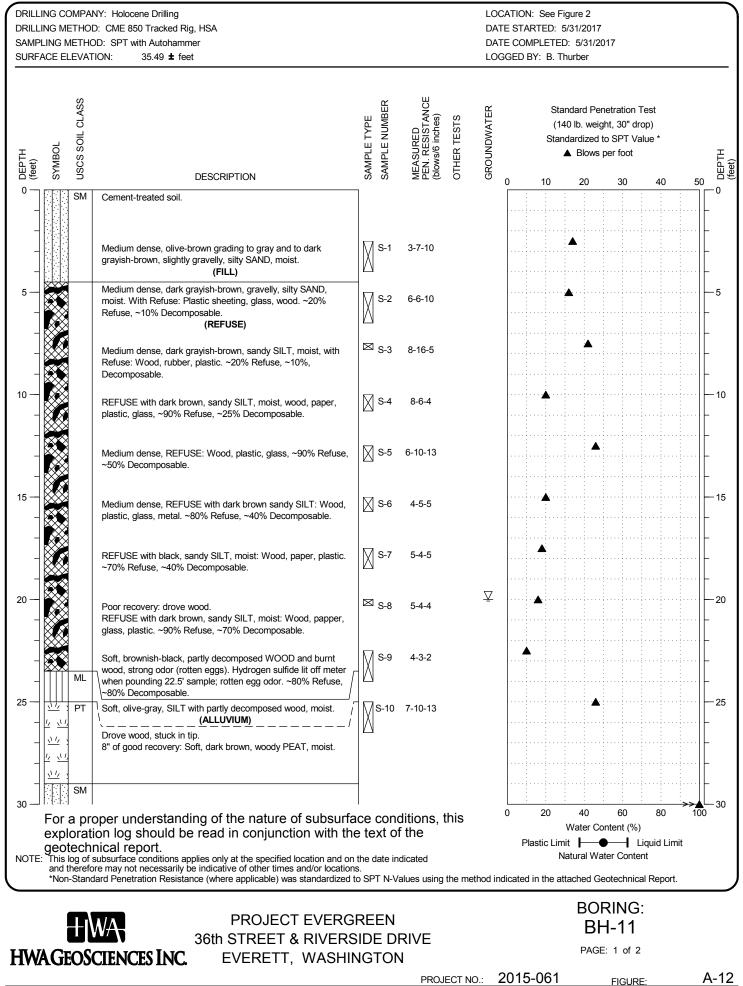
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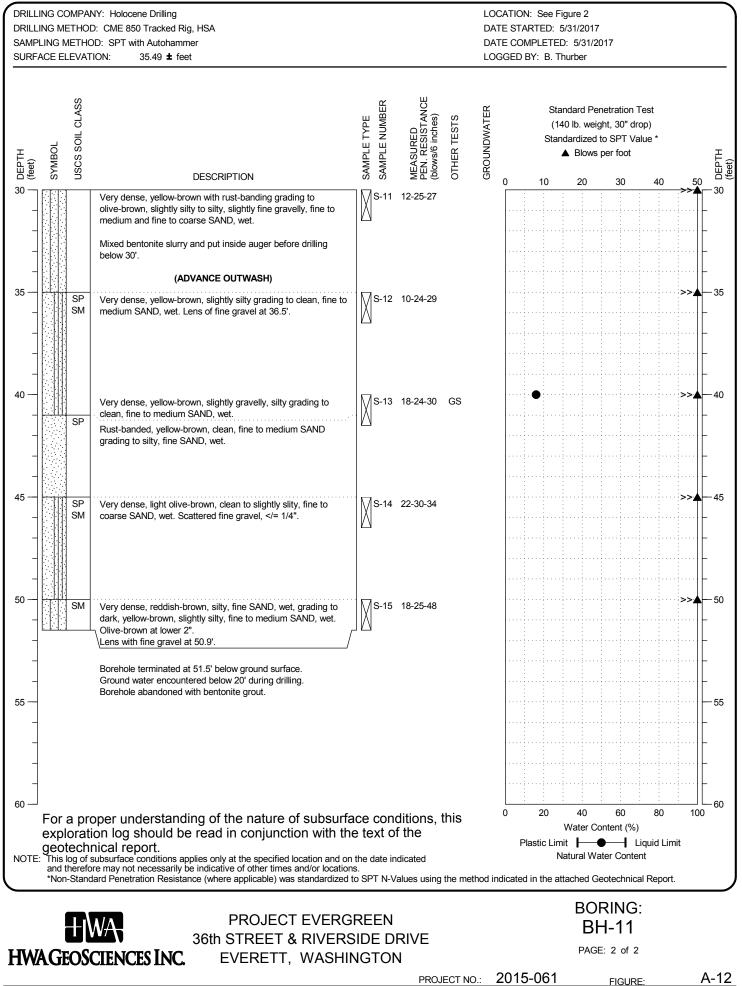


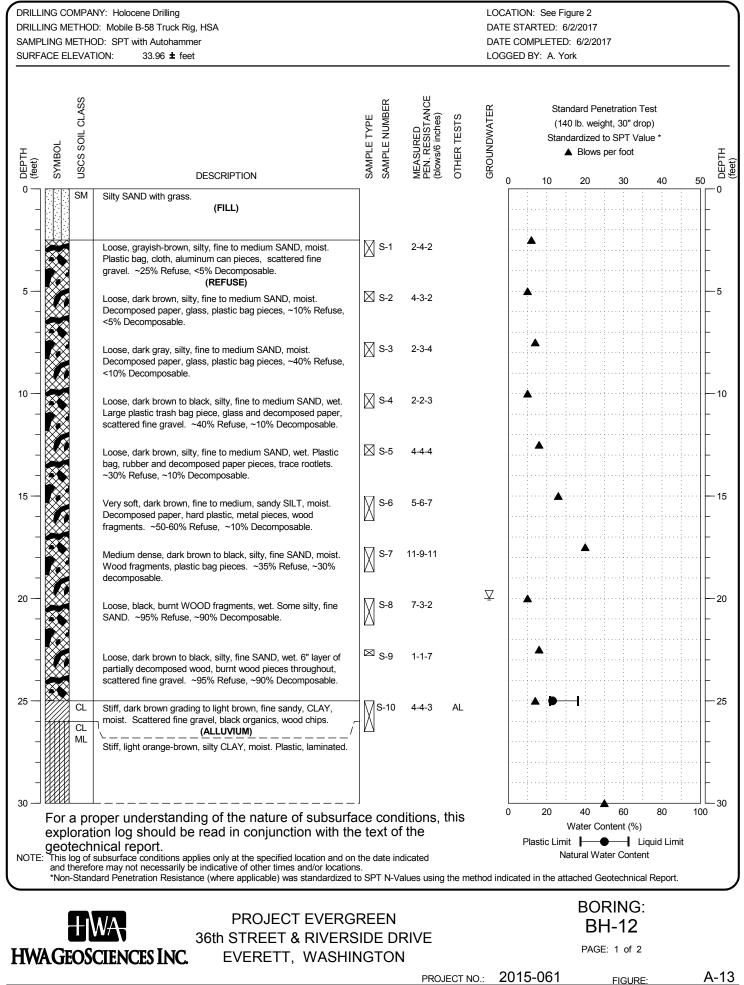


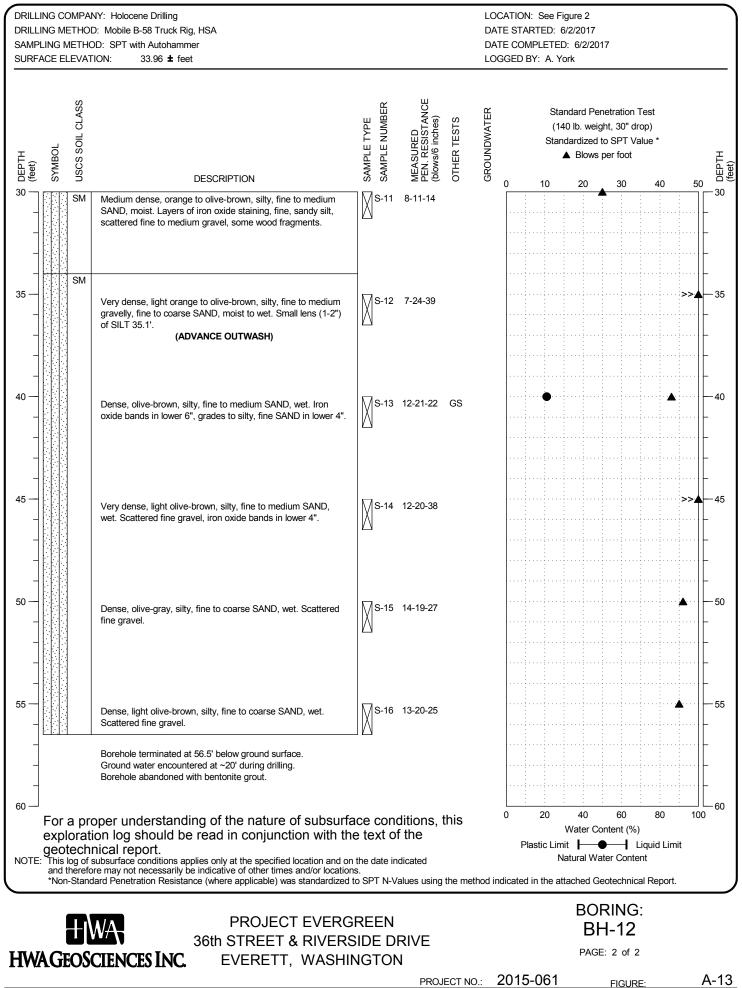


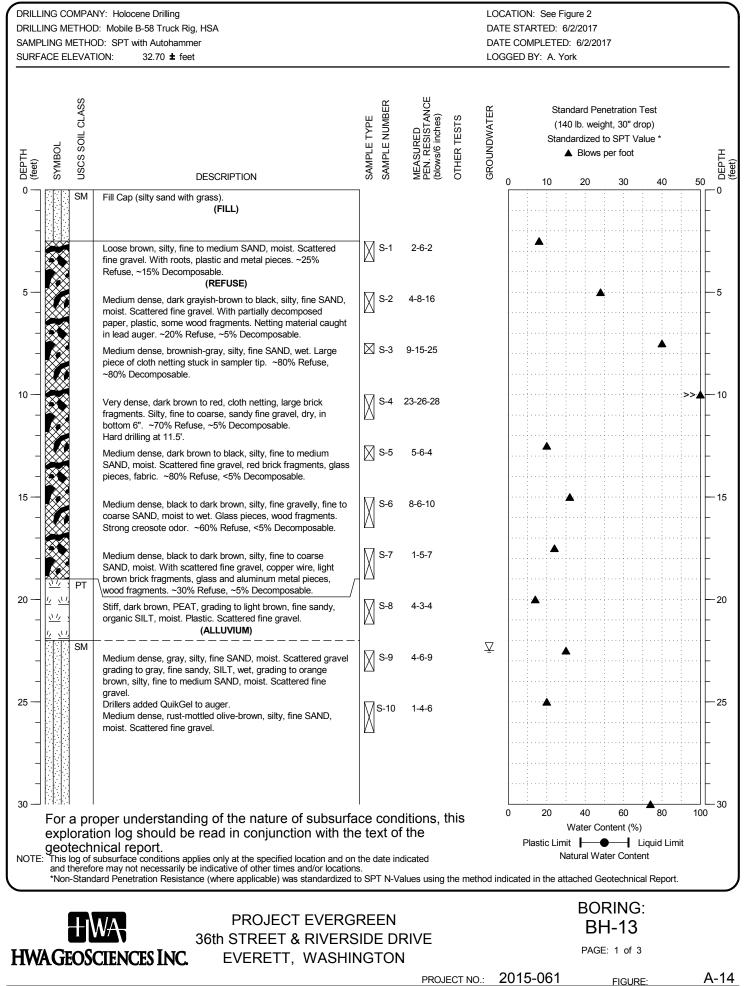


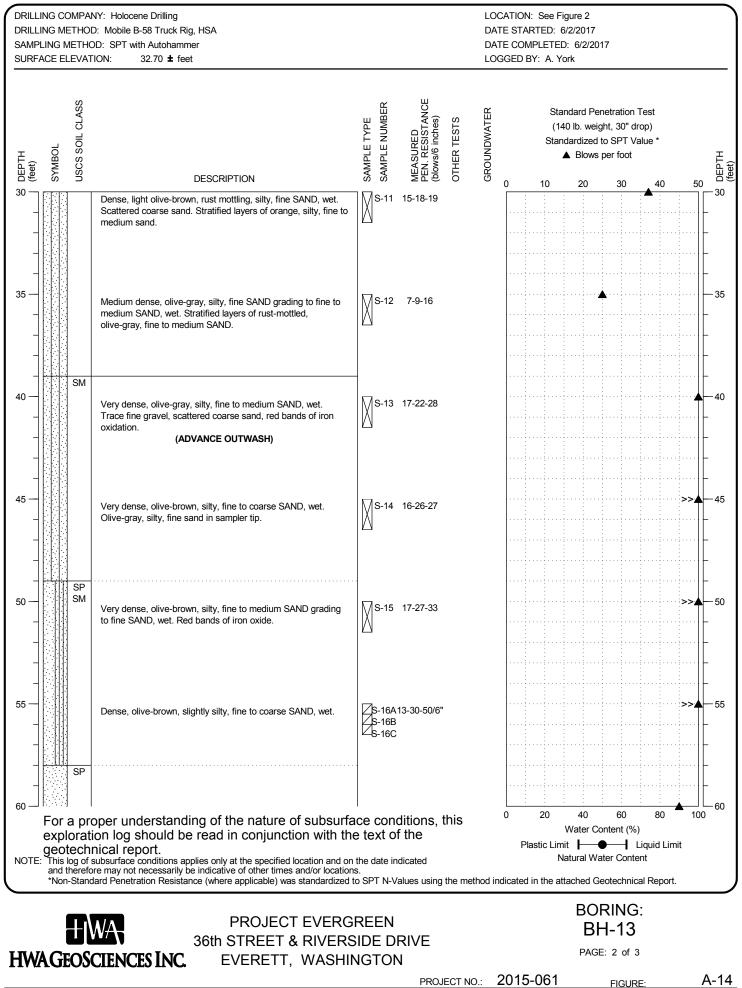


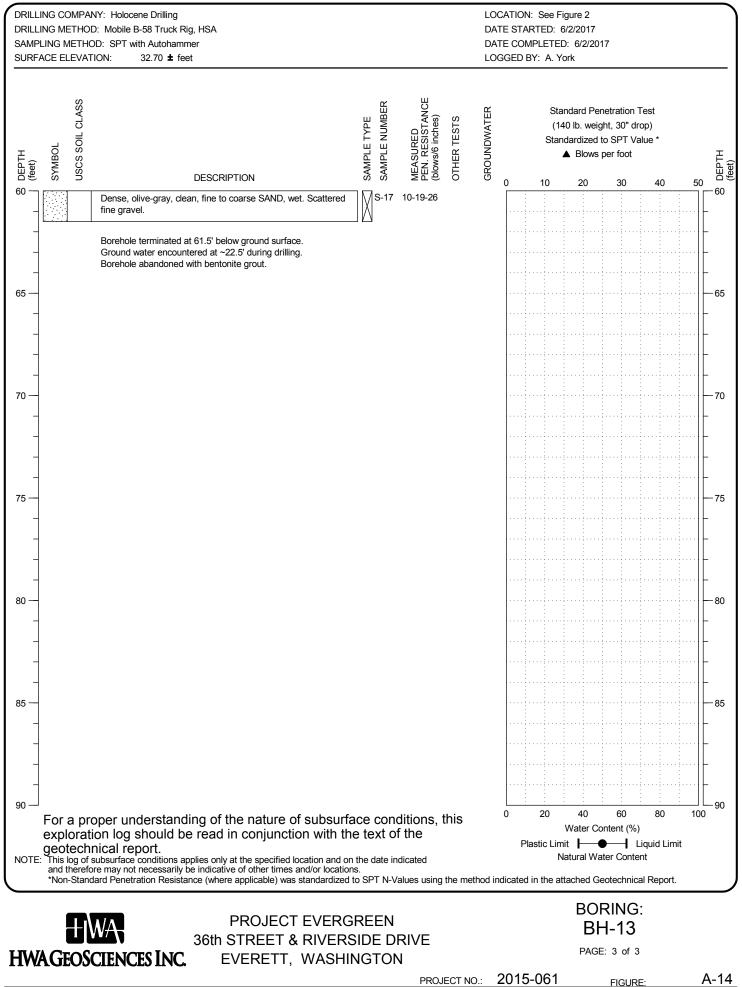


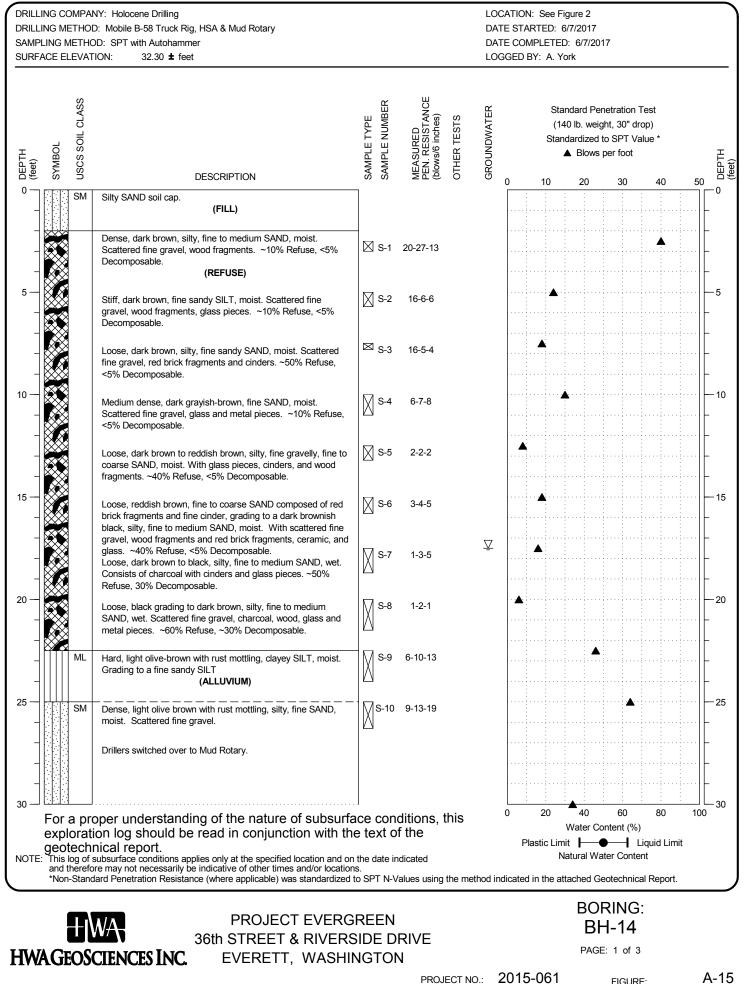






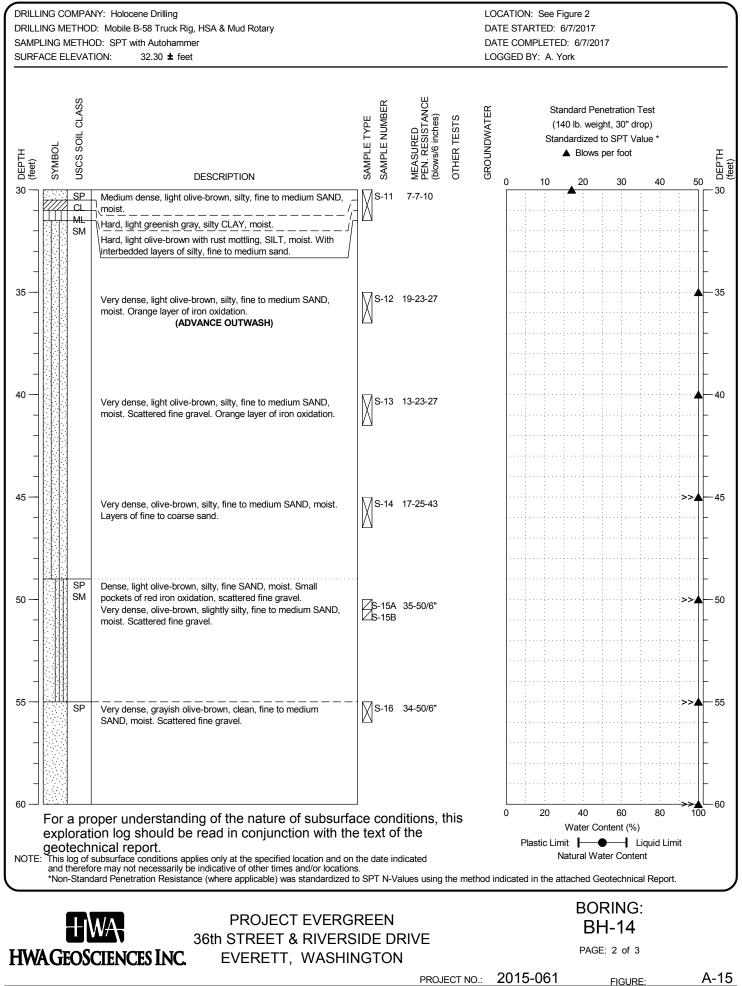


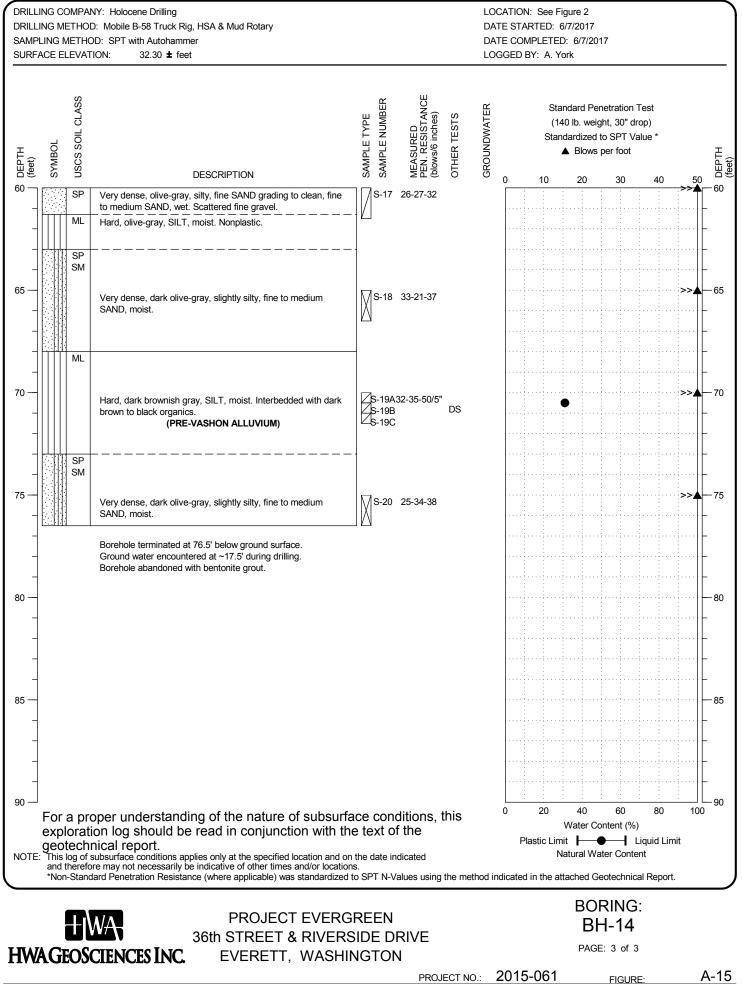


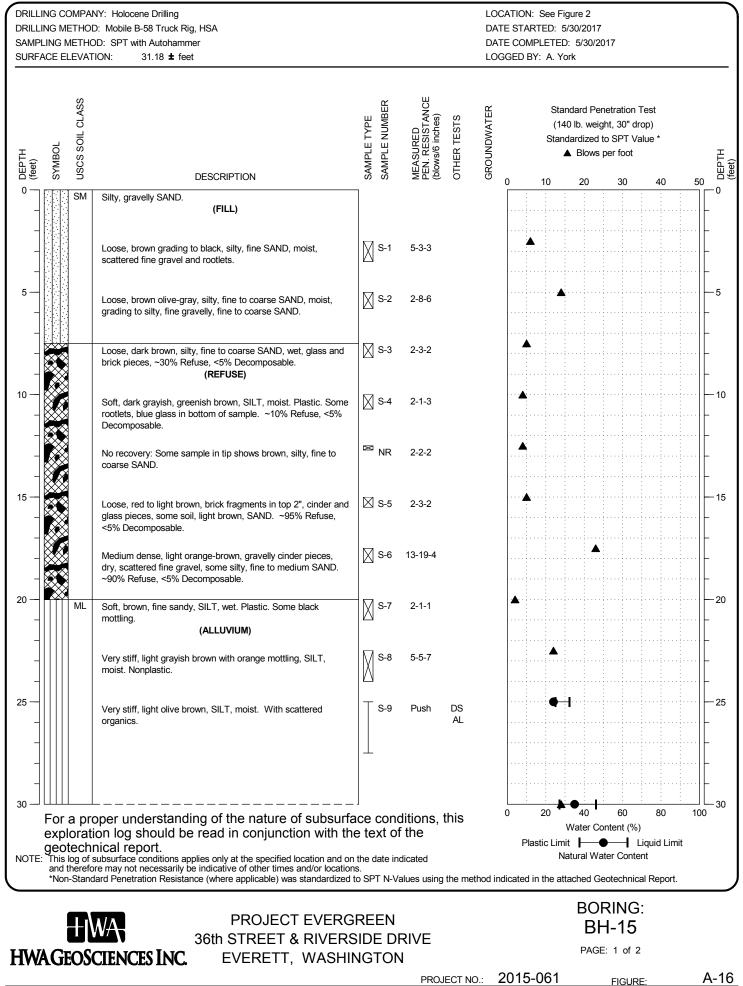


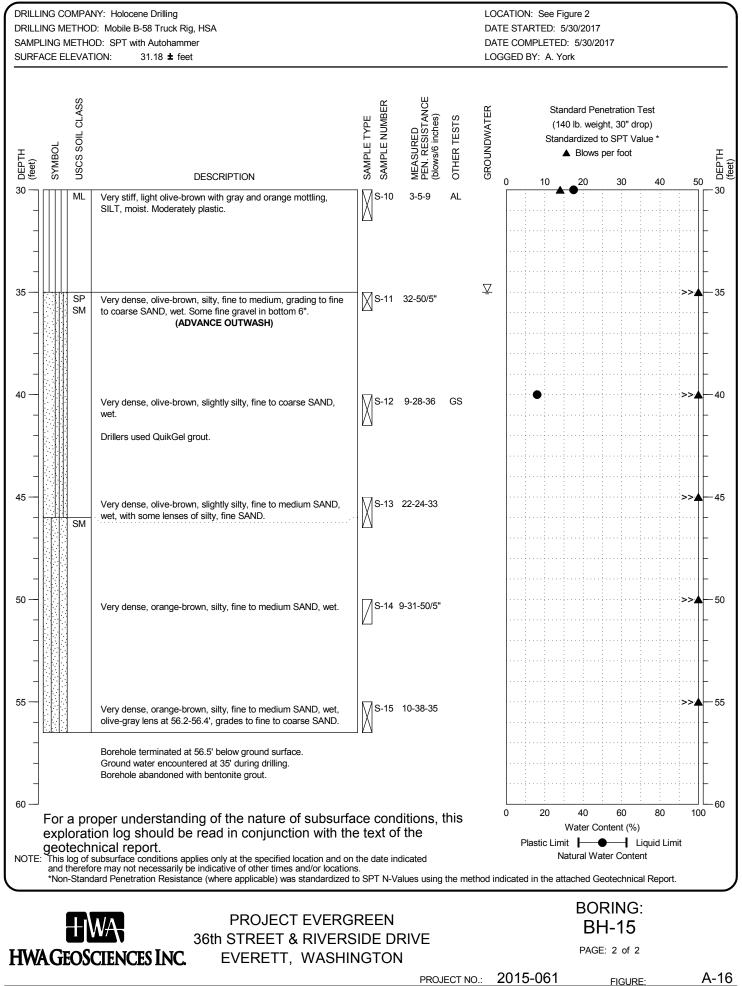
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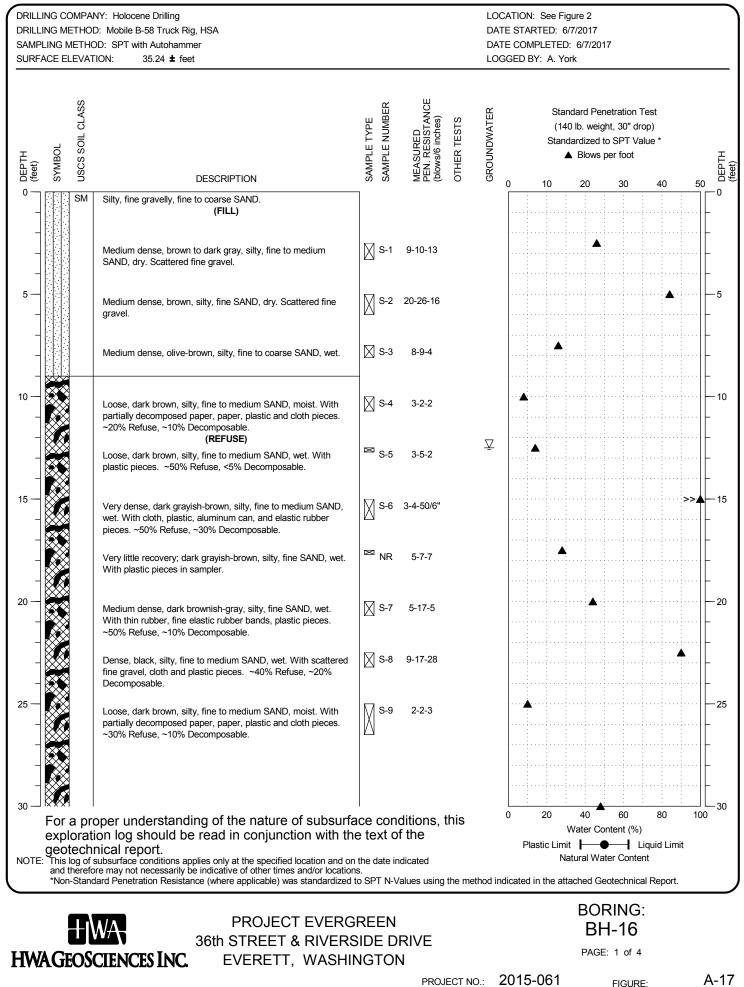
FIGURE:











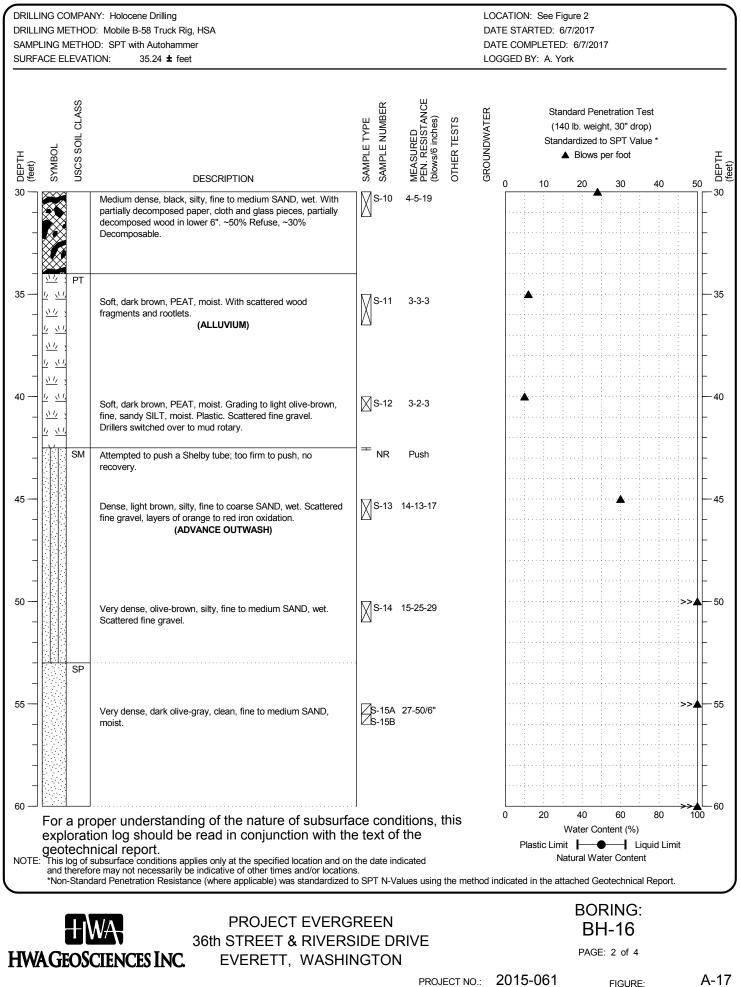
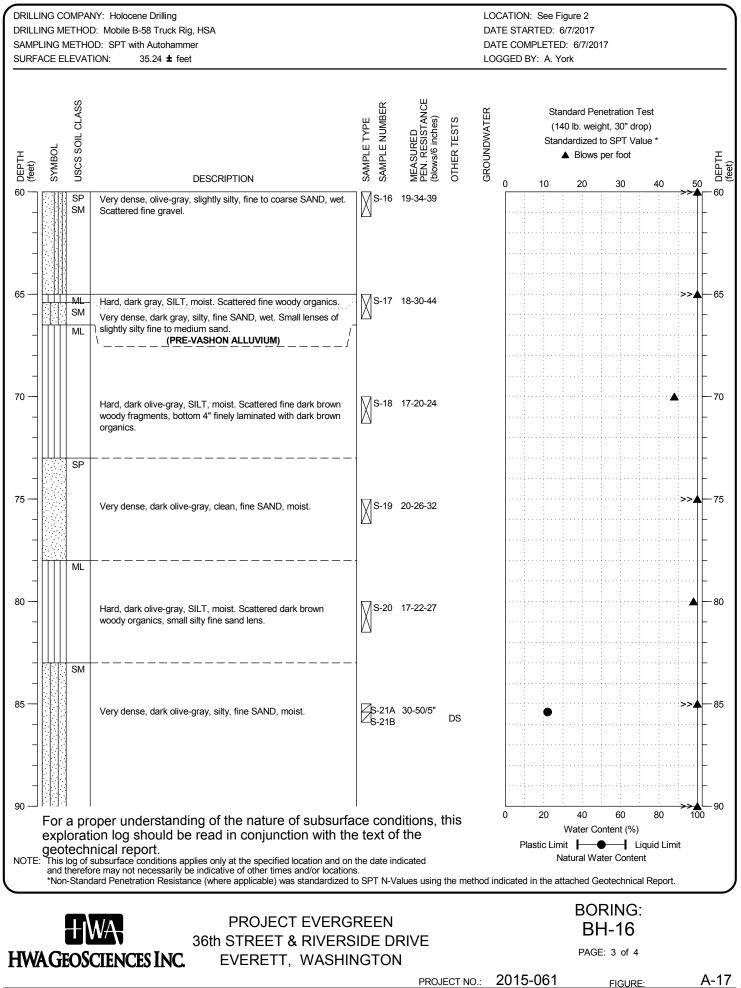
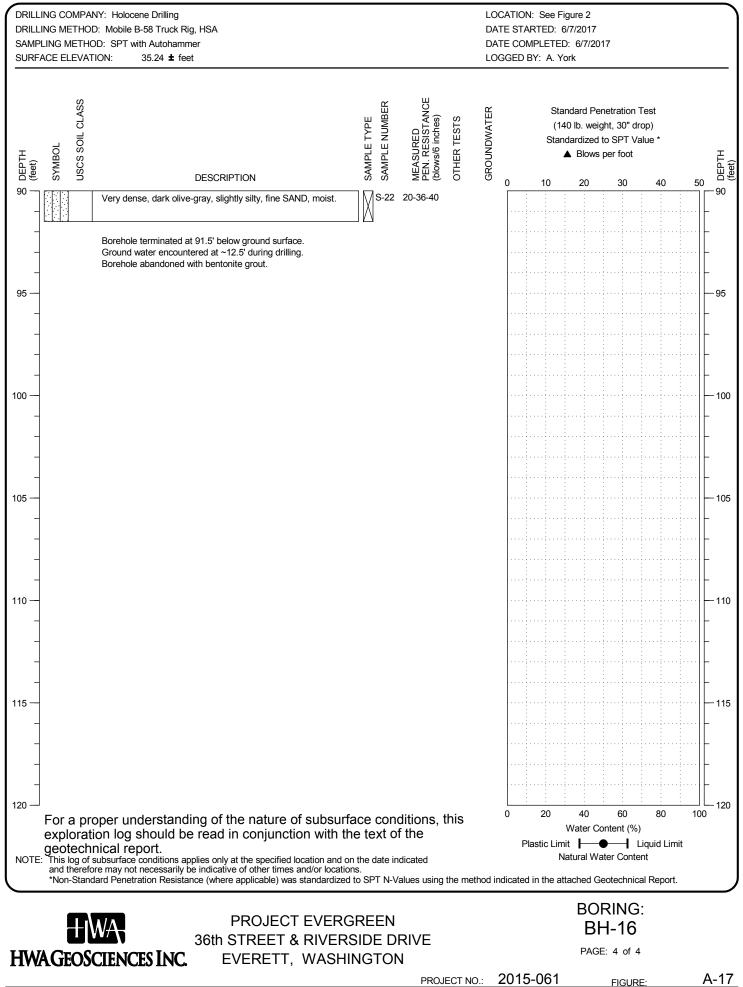
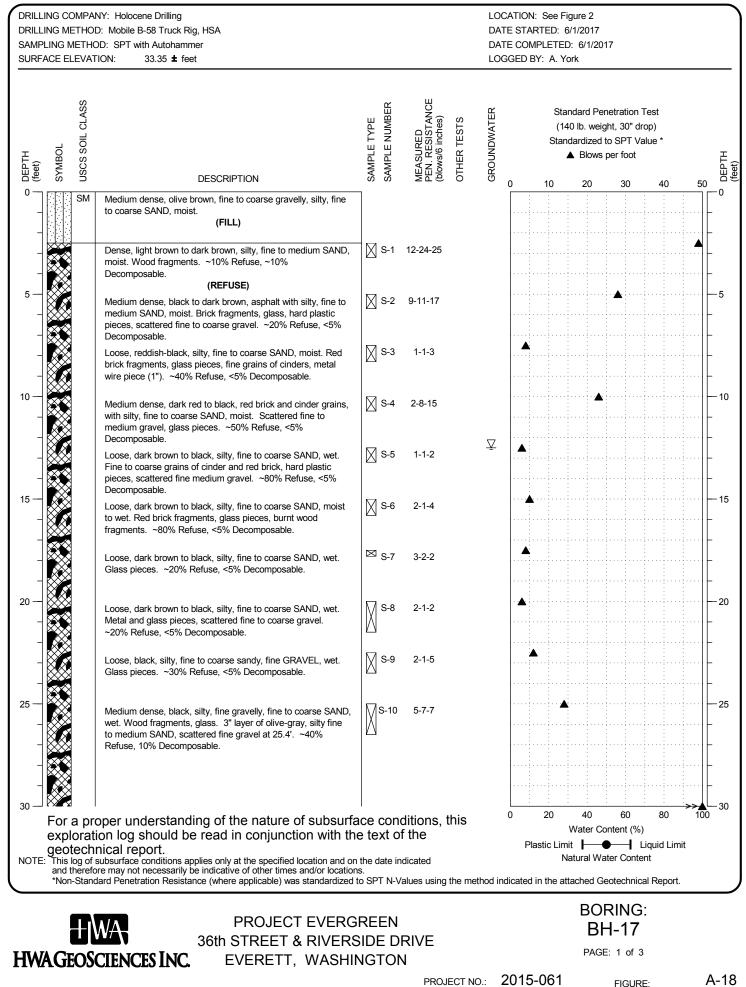


FIGURE:







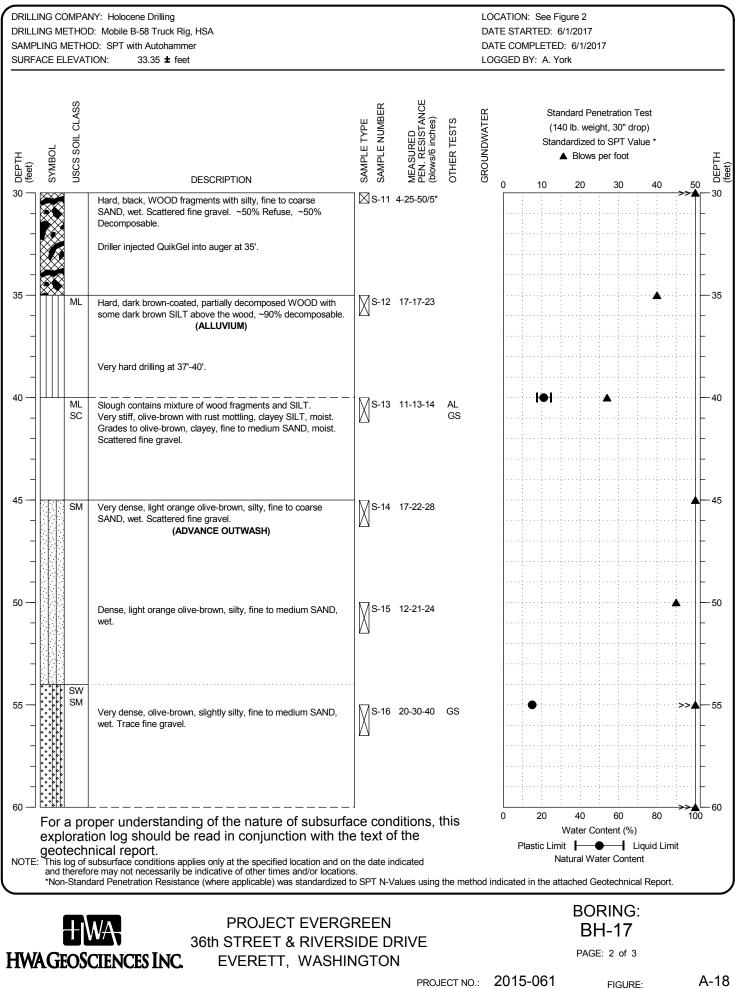
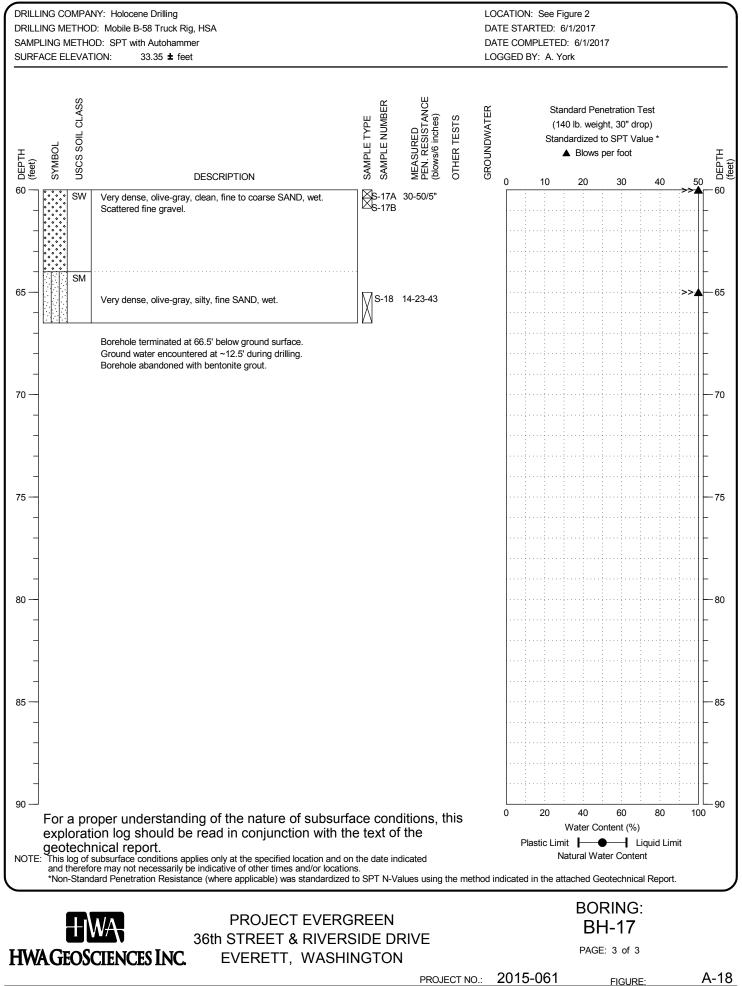
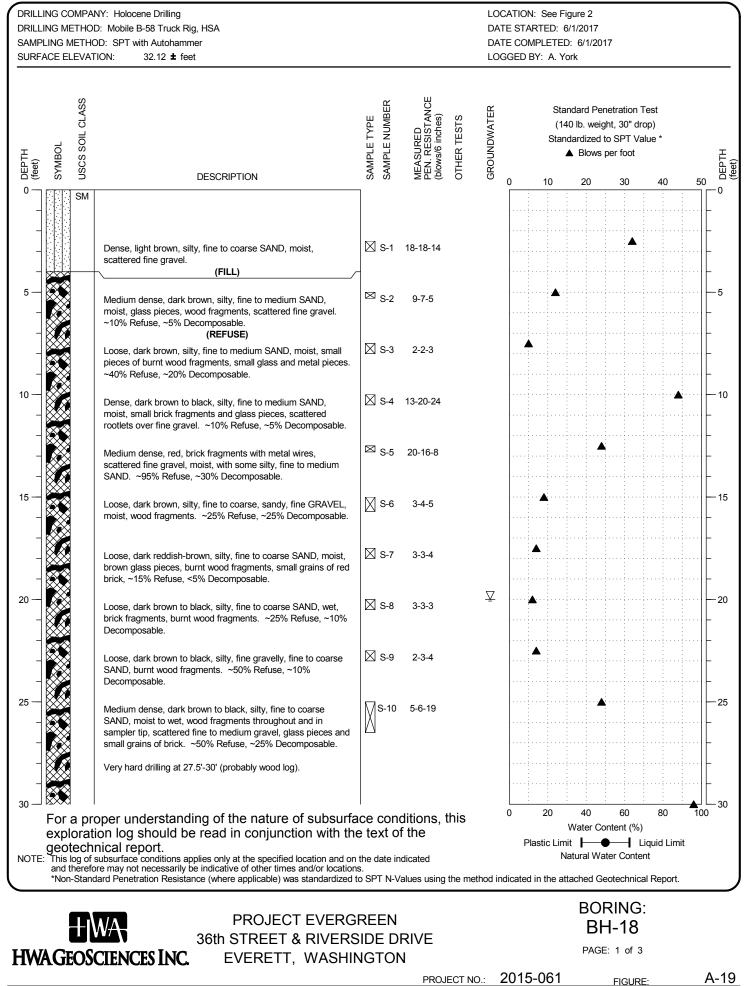
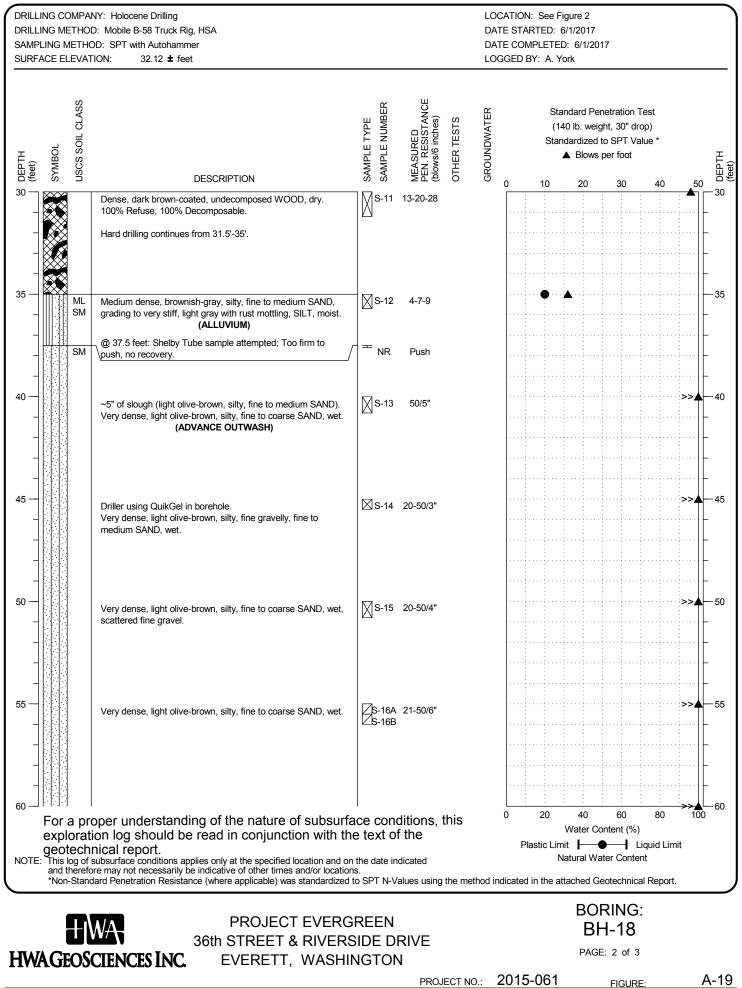
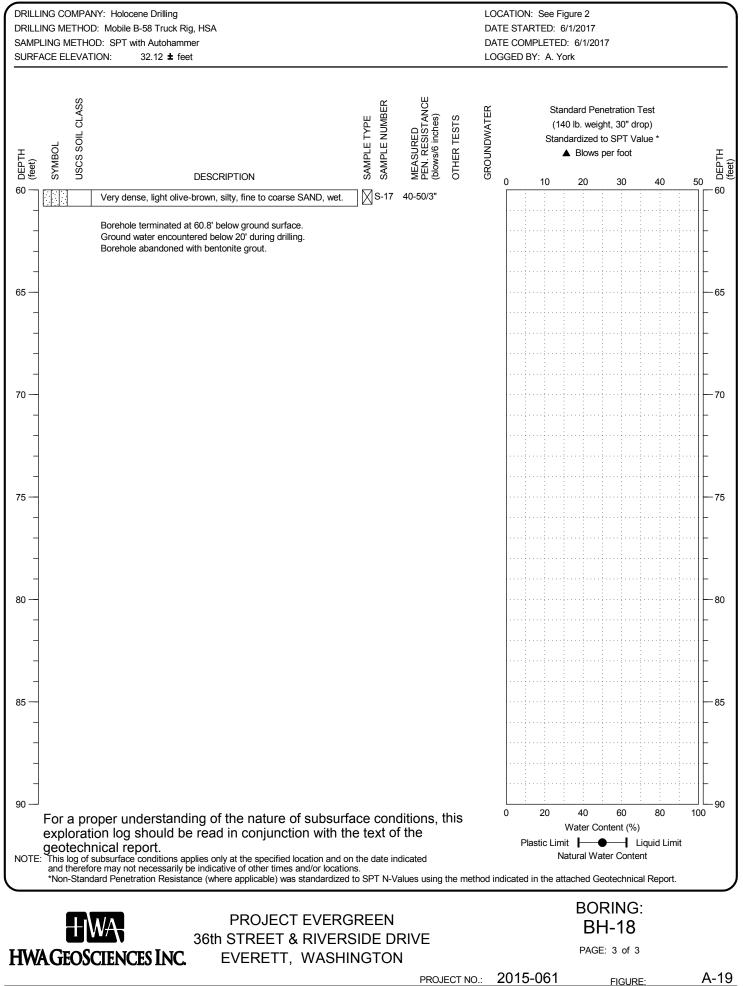


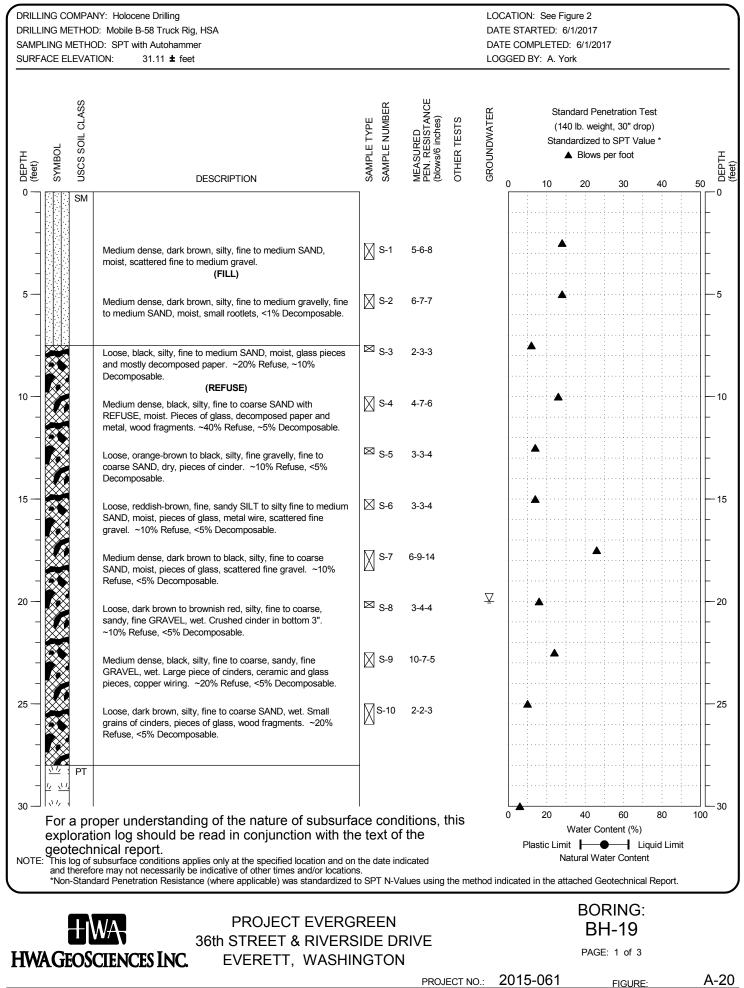
FIGURE:



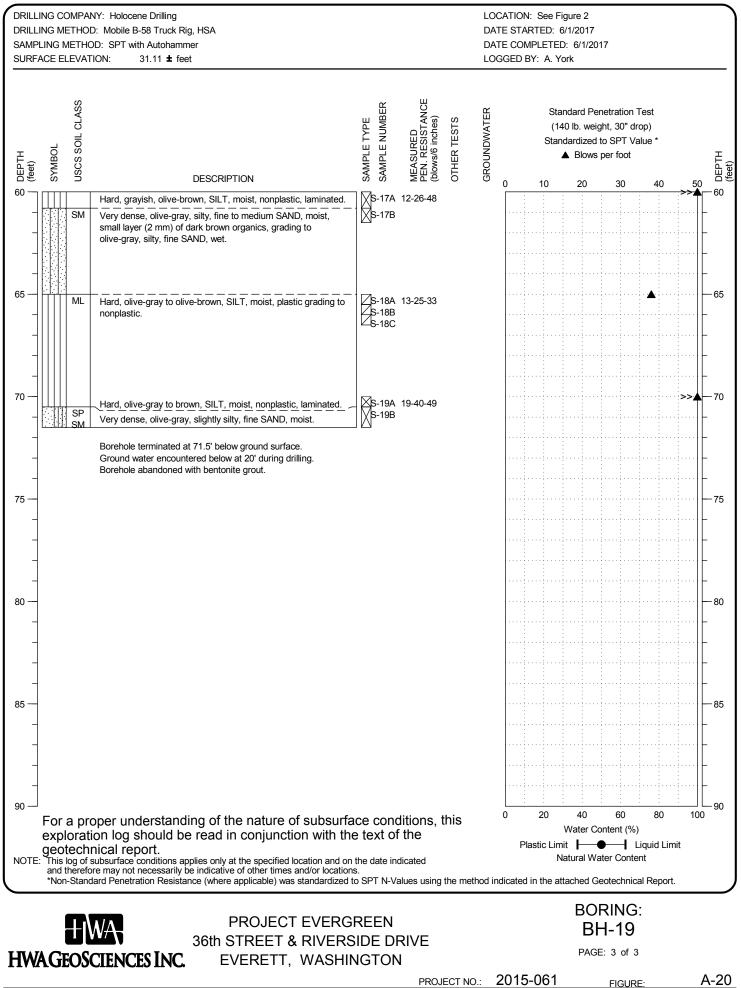


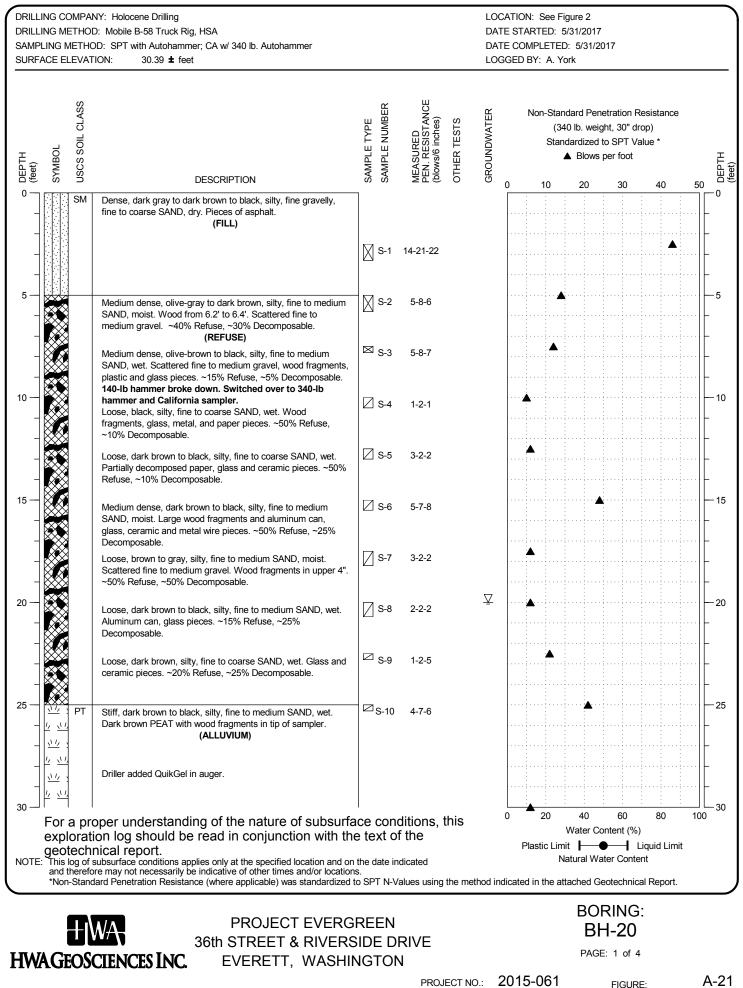


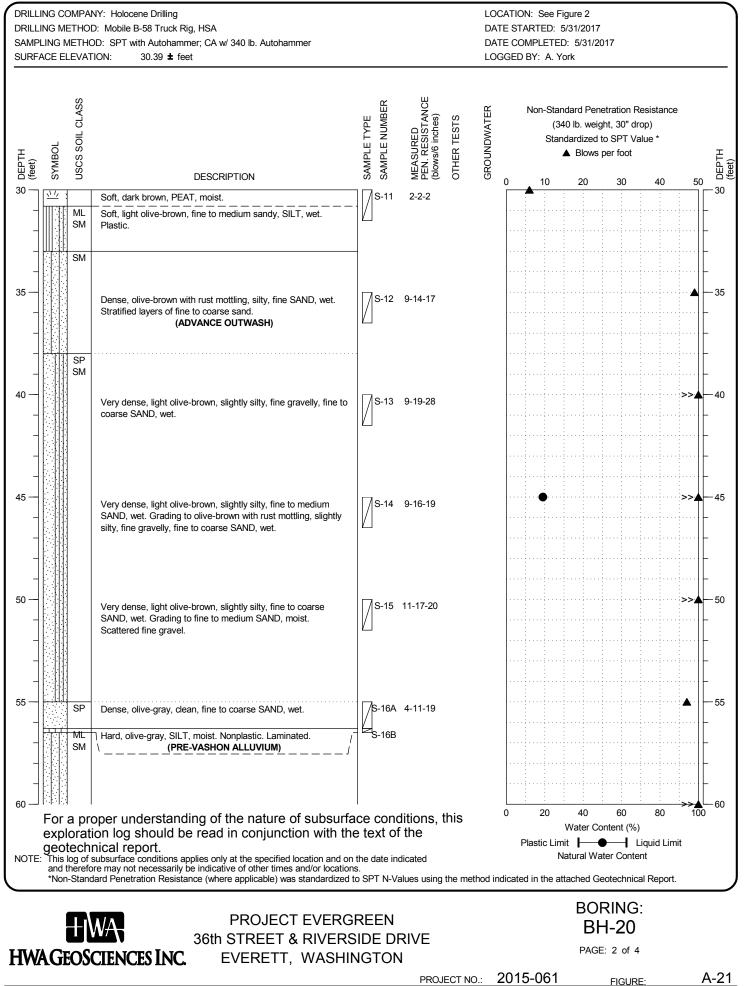




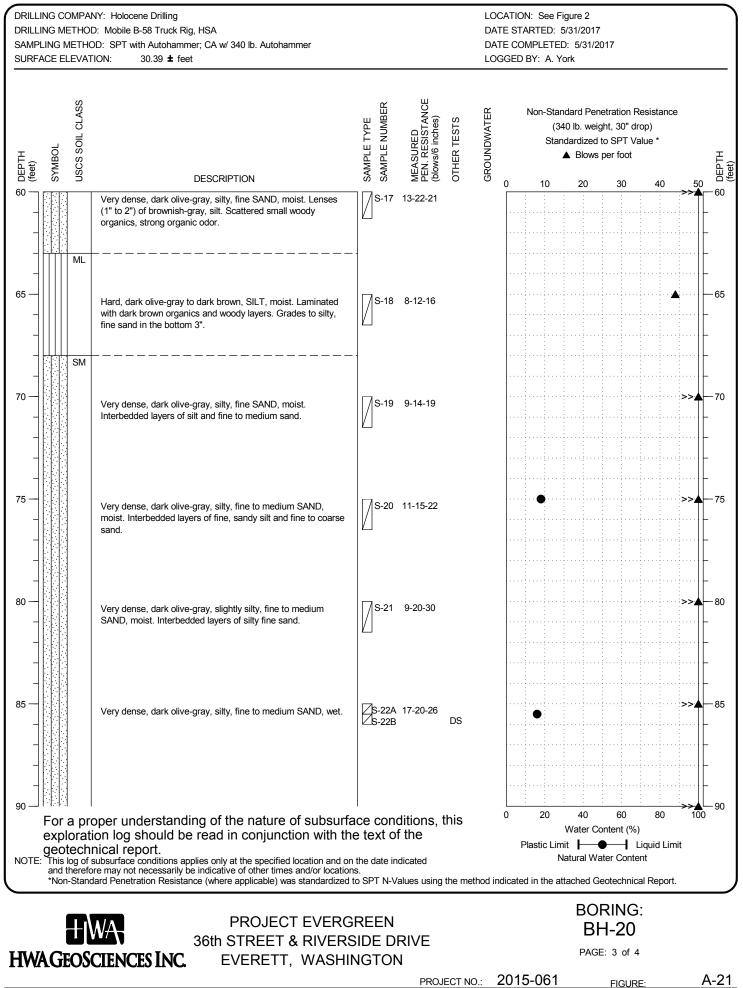
RILLING COMPAI RILLING METHOI AMPLING METHO URFACE ELEVAT		LOCATION: See Figure 2 DATE STARTED: 6/1/2017 DATE COMPLETED: 6/1/2017 LOGGED BY: A. York							
(teet) SXMBOL RSCS SOIL CLASS SVMBOL DESCRIPTION		SAMPLE TYPE SAMPLE NUMBER MEASURED PEN. RESISTANCE (blows/6 inches) OTHER TESTS	GROUNDWATER	Standard Penetration Test (140 lb. weight, 30" drop) Standardized to SPT Value * ▲ Blows per foot					DEPTH
	DESCRIPTION Soft, dark brown, PEAT, moist, with wood chips and rootlets, grading to dark brownish-gray, organic SILT, plastic. (ALLUVIUM)	S-11 1-1-2	0	10		30	40	50	- 30 -
5	Medium dense, olive-brown, silty, fine to medium SAND, wet, trace fine gravel.	S-12 2-5-6		·····	.				- 3; - -
- SP - SM -	Driller added QuikGel to auger. Dense, olive-brown, slightly silty, fine to coarse SAND, wet, grading to silty, fine to medium SAND, scattered fine gravel. (ADVANCE OUTWASH)	S-13 9-14-28					···· •		- 4 -
	Dense, olive-brown, silty, fine to medium SAND, moist, small layers (1mm) of iron oxide.	S-14 9-19-25					· · · · · · · · · · · · · · ·		- 4 - -
	Very dense, olive-brown, silty, fine to coarse SAND, wet, grading to brownish dark red, silty, fine SAND with heavy iron oxide staining, grading to silty, fine to medium SAND.	S-15 11-24-35						>>▲-	- { - -
SP SM ML	Very dense, olive-gray, slightly silty, fine to medium SAND, moist.	S-16 13-24-34						>>:	- 5 -
For a pro explorati geotechu DTE: This log of a and therefo	Hard, olive-brown, SILT, moist. Laminated, nonplastic. (PRE-VASHON ALLUVIUM) oper understanding of the nature of subsurfa on log should be read in conjunction with th nical report. subsurface conditions applies only at the specified location and on re may not necessarily be indicative of other times and/or location	e text of the the date indicated s.	0		Water C Limit Natural W		iquid Limi nt	>> _ 100	- 6
*Non-Stand	PROJECT EVERG 36th STREET & RIVERS CIENCES INC. EVERETT, WASHII	REEN	hod indic	ated in the	BO BI	RING H-19	:		

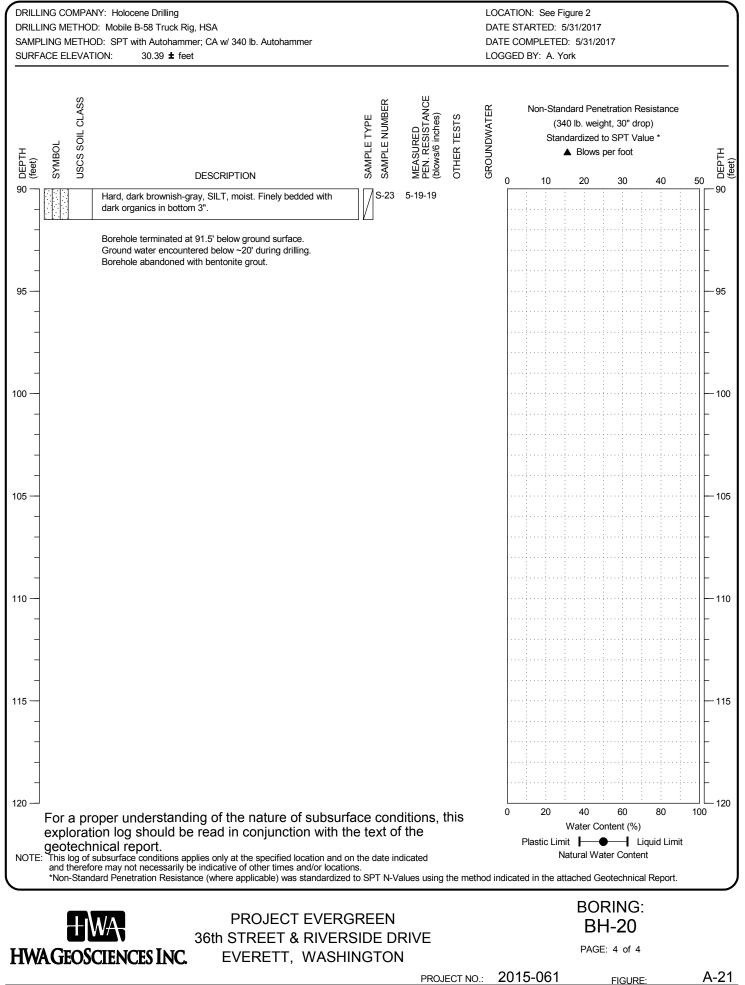


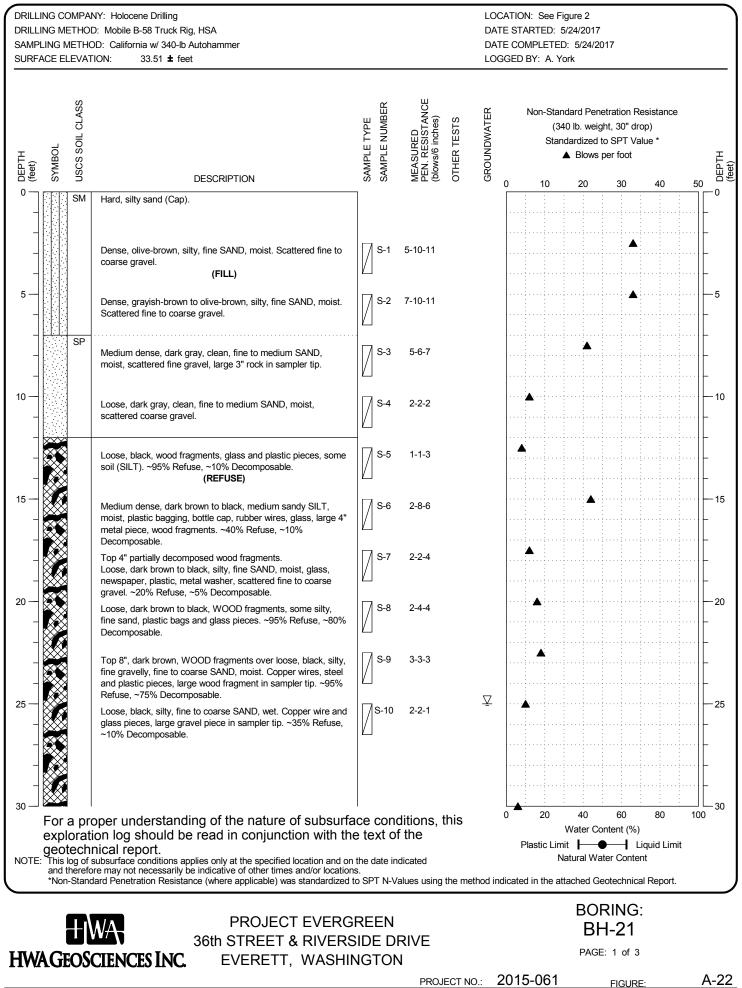




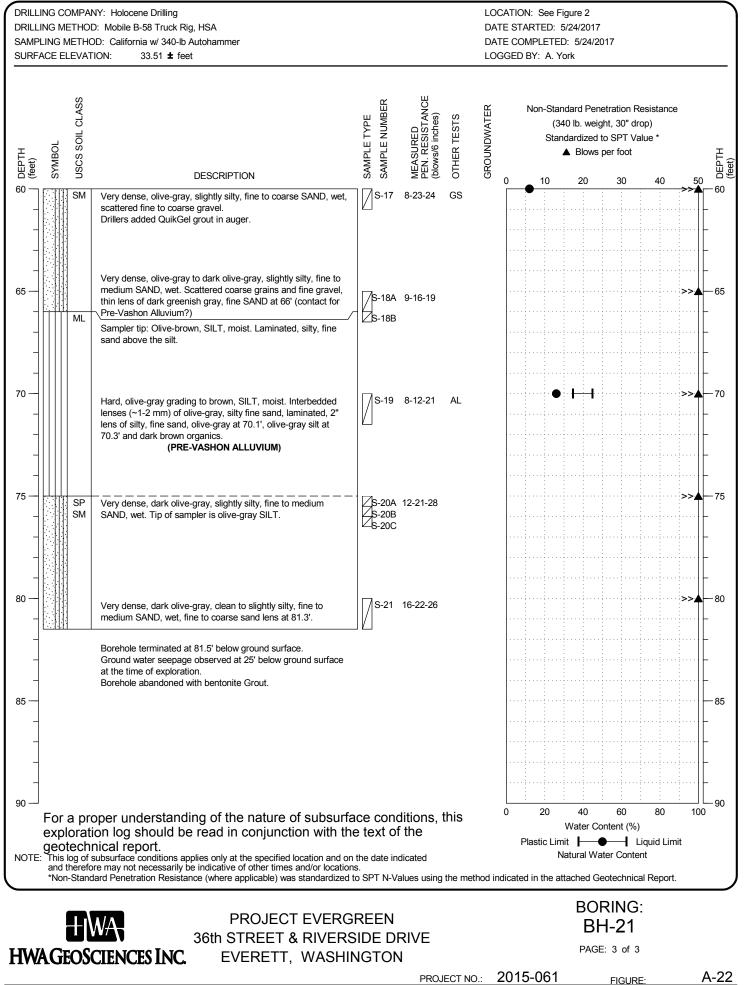
A-21

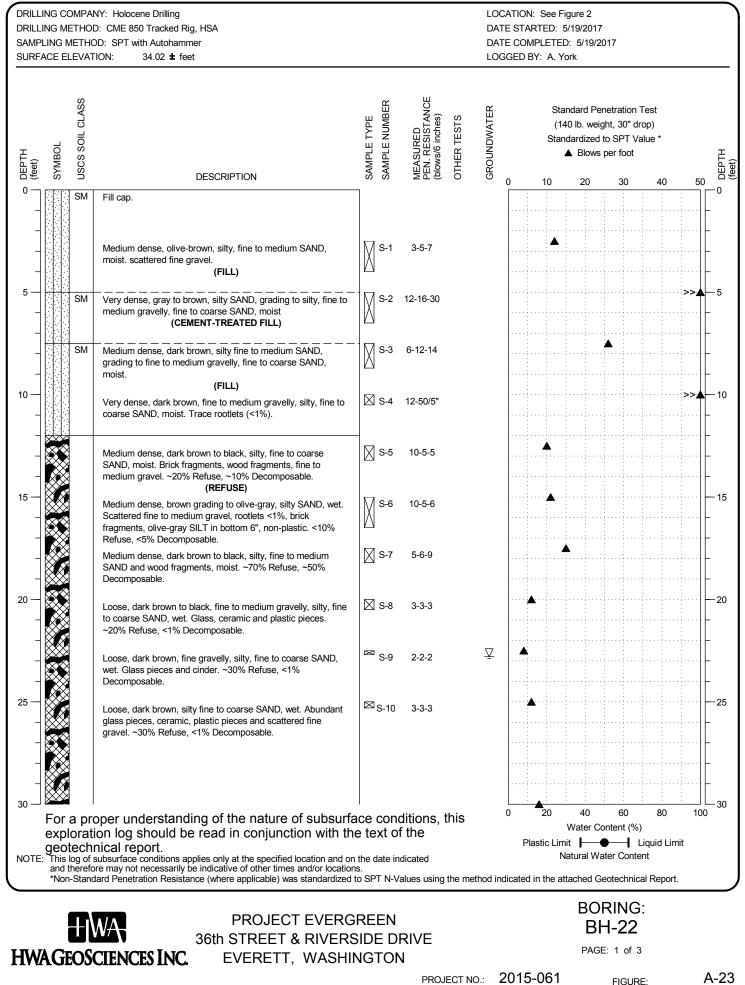


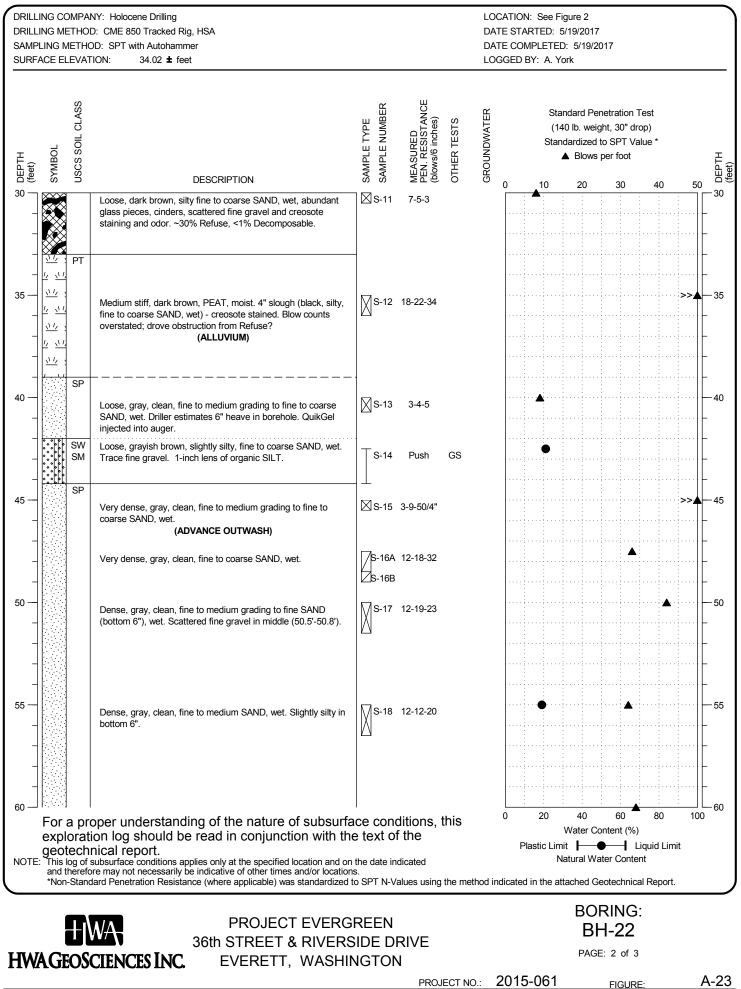


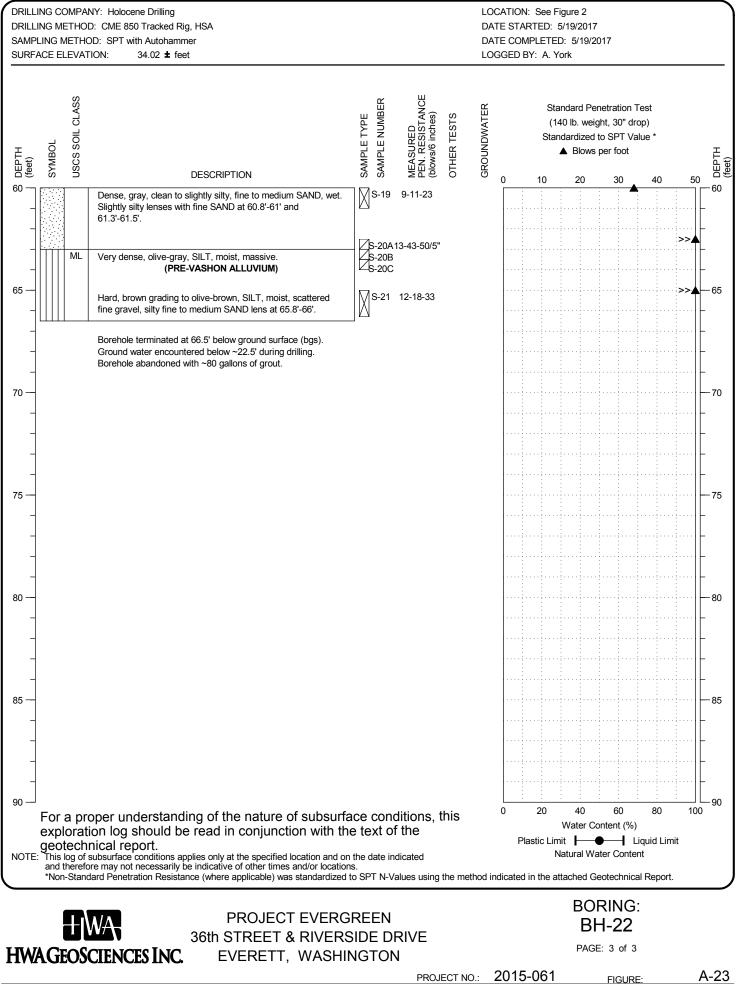


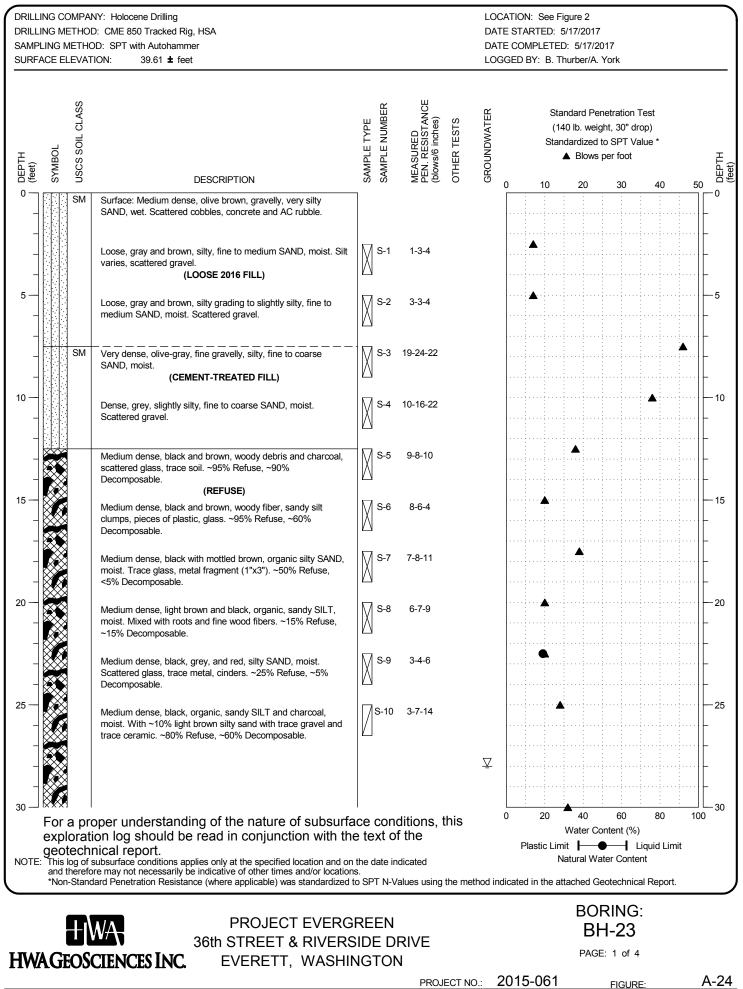
DRILLING METHOD: Mobile B-58 Truck Rig, HSA E SAMPLING METHOD: California w/ 340-lb Autohammer E			LOCATION: See Figure 2 DATE STARTED: 5/24/2017 DATE COMPLETED: 5/24/2017 LOGGED BY: A. York	DATE STARTED: 5/24/2017 DATE COMPLETED: 5/24/2017				
S DEPTH (feet) SYMBOL USCS SOIL CLASS	DESCRIPTION	SAMPLE TYPE SAMPLE NUMBER MEASURED PEN. RESISTANCE (blows/6 inches) OTHER TESTS	Non-Standard Penetration Resistance (340 lb. weight, 30" drop) Standardized to SPT Value * Blows per foot	G DEPTH (feet)				
– 🔆 plasti	e, black, REFUSE, wet. Paper and newspaper pieces, c, glass, and metal wire pieces, with silty, fine to coarse D. ~80% Refuse, <5% Decomposable.	S-11 1-1-2						
$ \begin{array}{c} \underline{\bigcirc} \underline{\checkmark} \\ \underline{\frown} \\ \underline{\checkmark} \\ \underline{\blacksquare} \\ \blacksquare$	covery. Plastic pieces on tip of sampler (carry-down). (ALLUVIUM)	NR 4-5-10		· · · · · · · · · · · · · · · · · · ·				
- <u>v</u> <u>v</u> wood - <u>v</u> <u>v</u> - <u>v</u> <u>v</u> - <u>v</u> <u>v</u> - <u>v</u> <u>v</u> - <u>v</u> <u>v</u>	dark brown, PEAT, moist, scattered rootlets and small chips.	S-12 5-4-4		40				
	covery from Shelby tube. olive-gray, organic SILT, moist. 1/2" diameter x 3" long branch in sample, scattered rootlets and wood chips.	NR Push						
- interb	um stiff, grayish brown, organic SILT, moist, with bedded 1 mm layers of organics, finely laminated, ered small wood chips. recovery, Shelby Tube crushed, sand in bottom of tube. um dense, brown, slightly silty, fine to coarse SAND, wet. ered organics in gravel.	S-14 2-2-3 AL S-15 Push GS DS OC						
	e, olive-brown, silty, fine to medium SAND, wet. (ADVANCE OUTWASH)	S-16A 8-6-14 S-16B		55				
exploration lo geotechnical NOTE: This log of subsurfa and therefore may								
HWAGEOSCIEN	PROJECT EVERGI 36th STREET & RIVERS INC. EVERETT, WASHIN	IDE DRIVE IGTON	BORING: BH-21 PAGE: 2 of 3	 A-22				

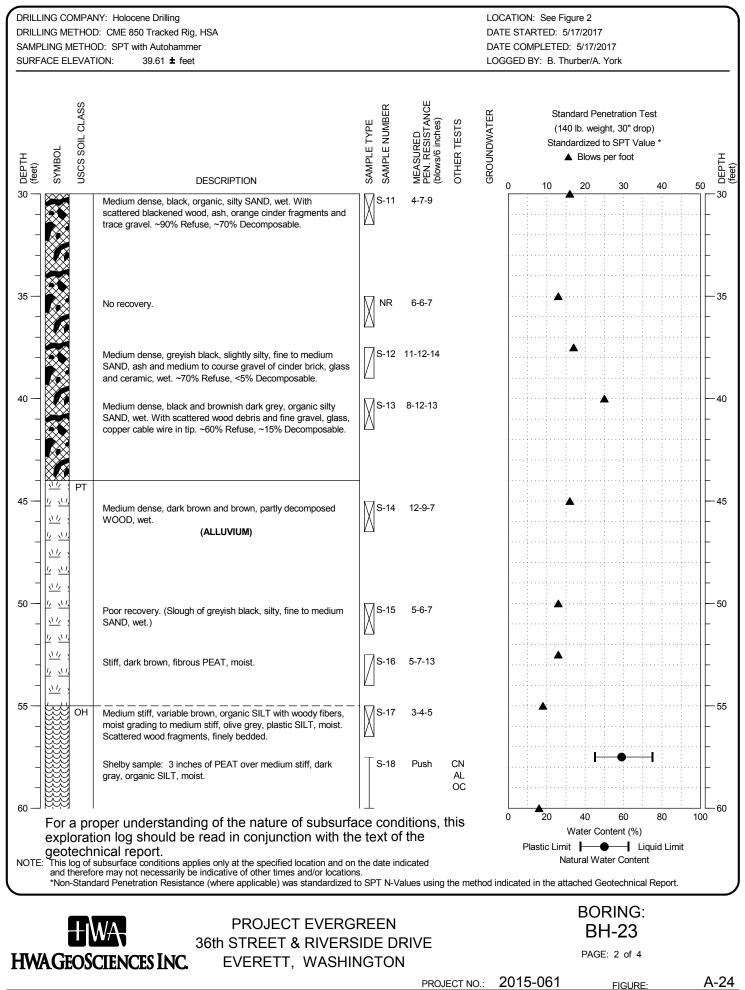






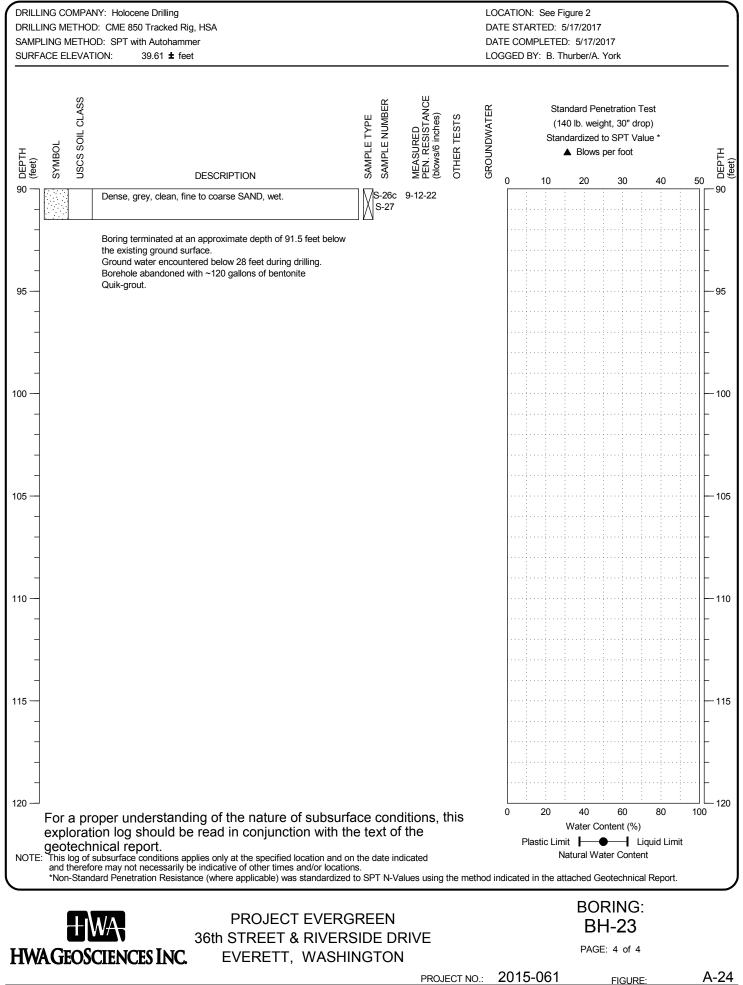


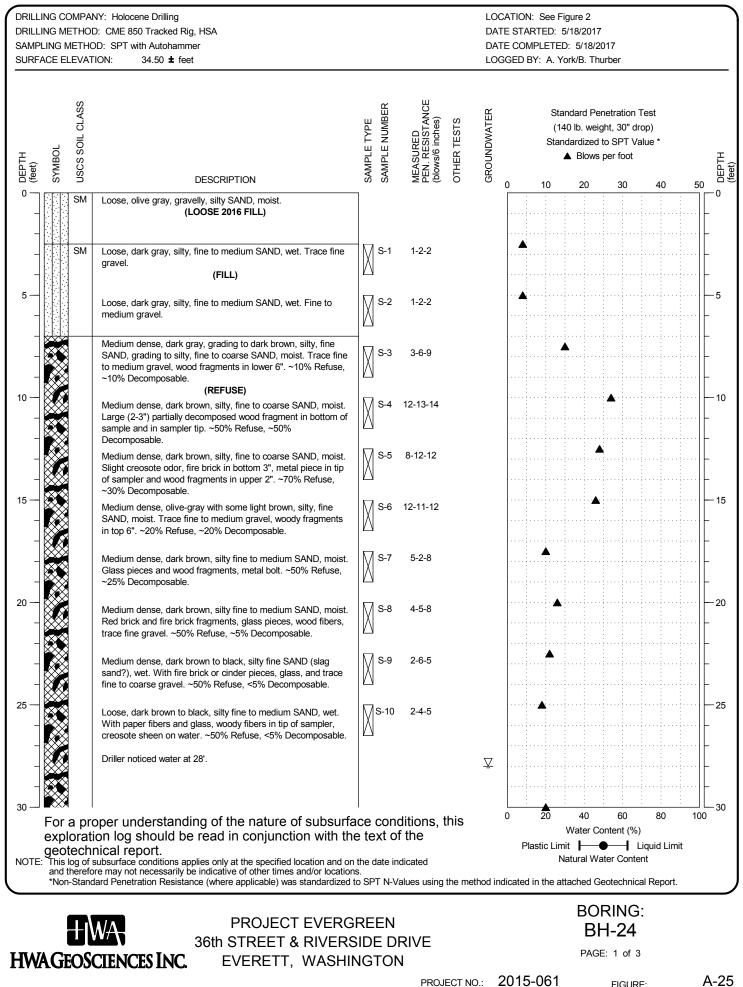


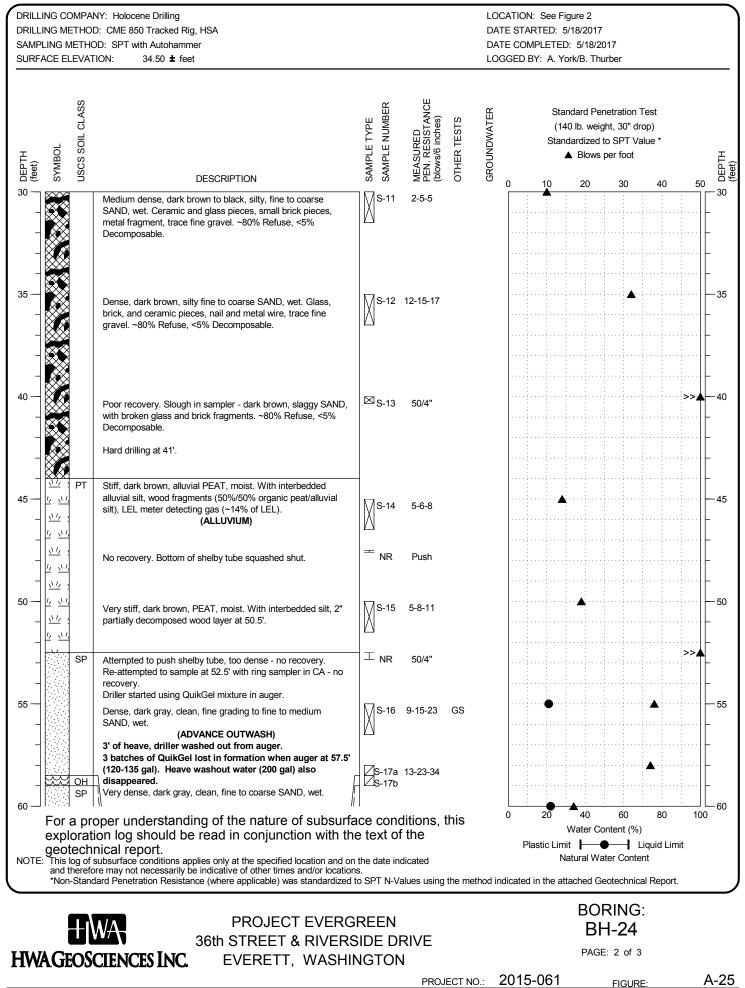


DRILLING COMPANY: Ho DRILLING METHOD: CMB SAMPLING METHOD: SP SURFACE ELEVATION:	E 850 Tracked Rig, HSA		LOCATION: See DATE STARTED: DATE COMPLETE LOGGED BY: B.	5/17/2017 ED: 5/17/201		
t DEPTH (feet) SYMBOL USCS SOIL CLASS	DESCRIPTION	SAMPLE TYPE SAMPLE NUMBER MEASURED PEN. RESISTANCE (blows/6 inches) OTHER TESTS	(,	andard Pene 140 lb. weigh Indardized to ▲ Blows pe 20	t, 30" drop) SPT Value *	0 T DEPTH (feet)
	um stiff, grey and olive grey, laminated SILT with woody nent layers, moist. Roots, partly decomposed 1" layer of I.	S-19 2-3-5				60
- SM Medi With thick	um dense, grey, slightly silty, fine to medium SAND, wet. peat lenses ~2" thick, bluish grey, sandy SILT lenses ~1"	S-20 5-7-8		-		
- Grand deco	e, grey, slightly silty, fine to course SAND, wet. Trace fine el with ~2" lens of silty fine SAND with partially mposed roots.	S-21 5-4-2				
	dense, grey, clean, fine to medium and fine to coarse D, wet. (ADVANCE OUTWASH)	S-22 22-34-48				
non-	dense, olive grey and olive brown, finely bedded, olastic SILT, moist, ~2" dark brown with fine organics in t 80'. 4" piece of partly decomposed wood at 80.6'. (PRE-VASHON ALLUVIUM)	S-23 9-28-35	···· ··· ··· ···			
85 - SM gradi	dense, grey, silty fine gravely, fine to coarse SAND, wet ng to slightly silty, fine SAND, moist.	M_{s-24b}				>>
scatt	ered charcoal lenses and possible ash, off-white.	<u>N</u>				····
90 For a proper exploration lo geotechnical NOTE: This log of subsurf and therefore may	dense, grey, clean, fine to medium SAND, wet. understanding of the nature of subsurfa og should be read in conjunction with the report. ace conditions applies only at the specified location and on not necessarily be indicative of other times and/or locations netration Resistance (where applicable) was standardized to	e text of the the date indicated		Natural Wate	Liquid Ler Content	Limit
HWAGEOSCIEN	PROJECT EVERGI 36th STREET & RIVERS ICES INC. EVERETT, WASHIN	IDE DRIVE IGTON	. 2015-061	BH	21NG: -23 3 of 4 FIGURE:	A-24

<u>A-24</u>

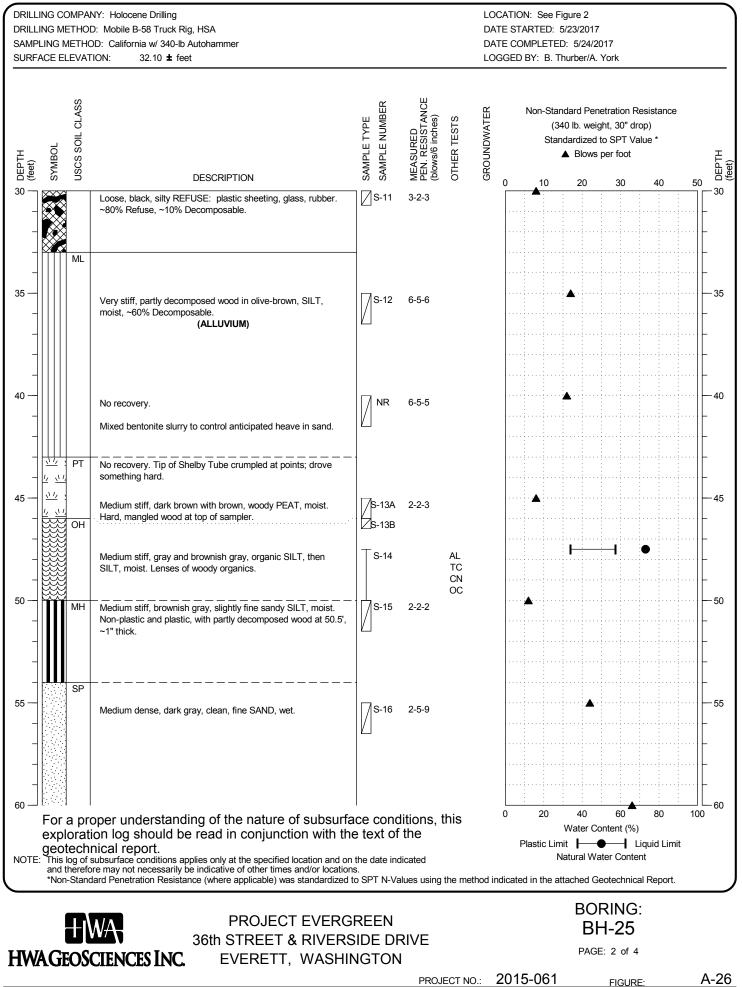


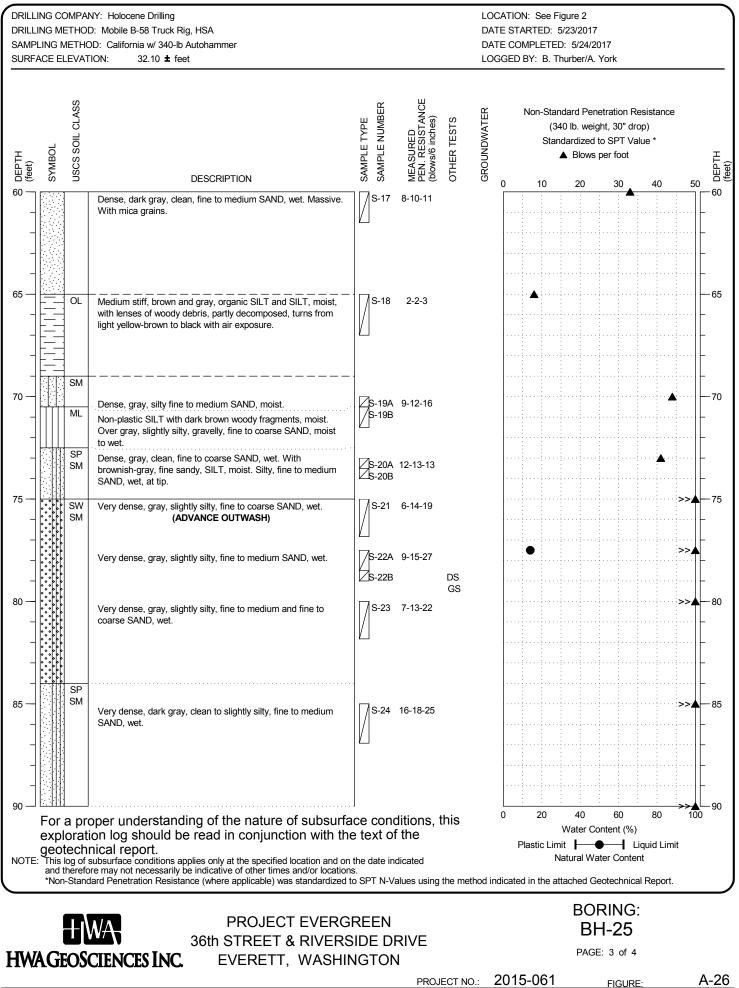


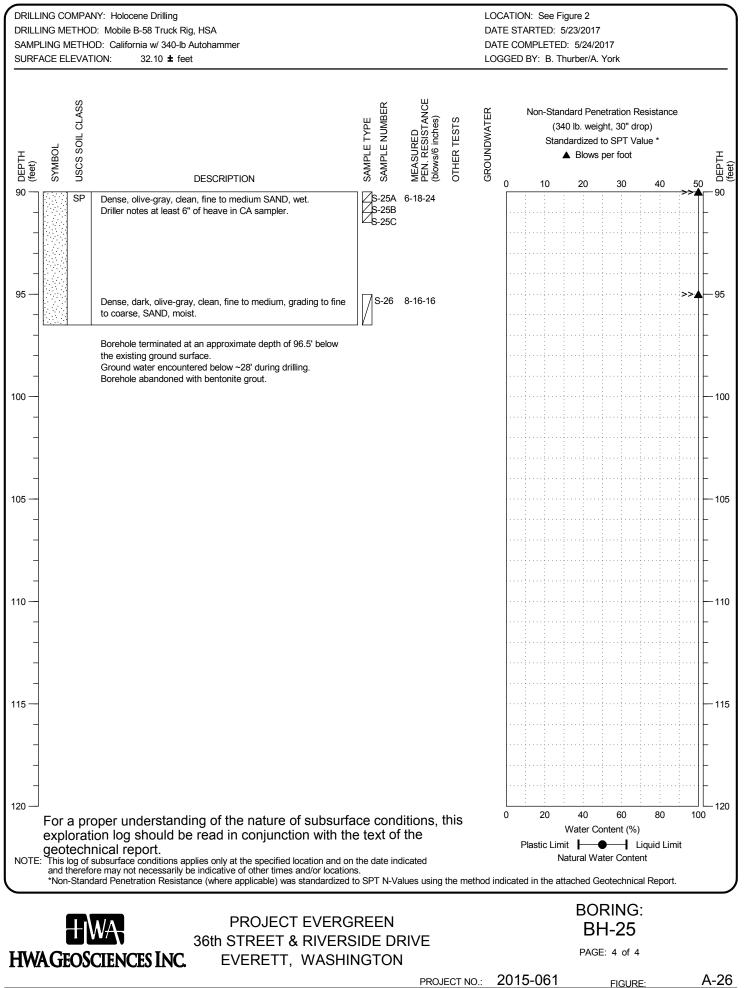


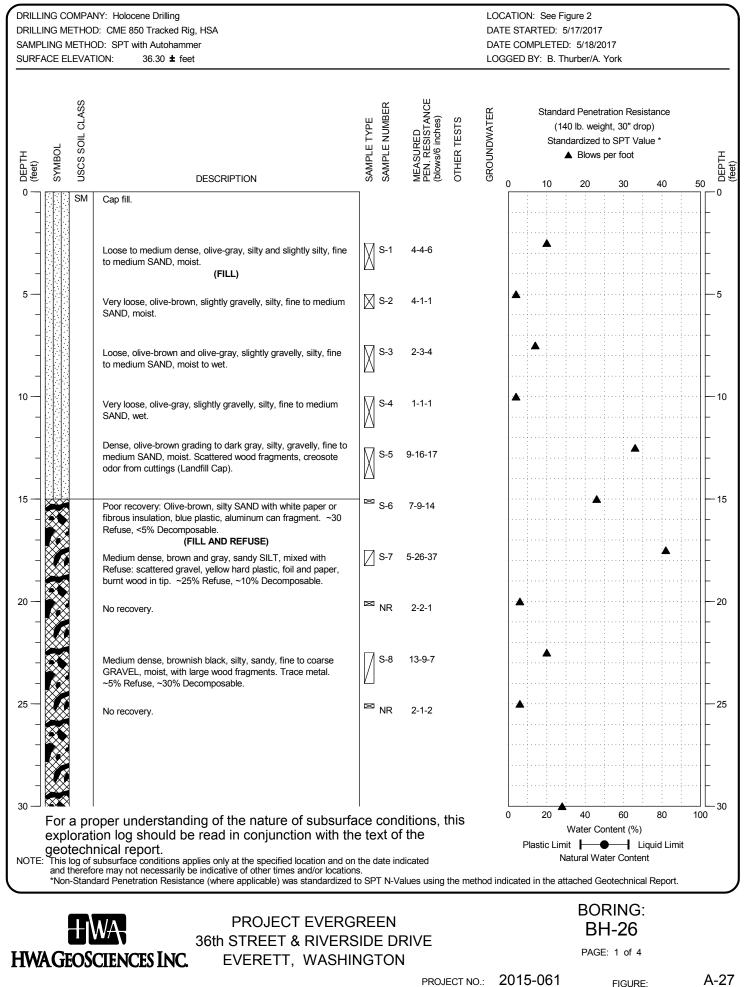
DRILL SAMP	ING METHO	NY: Holocene Drilling D: CME 850 Tracked Rig, HSA DD: SPT with Autohammer ION: 34.50 ± feet				DATE : DATE (TION: See F STARTED: COMPLETE ED BY: A. Y	5/18/2017 D: 5/18/20				_
DEPTH (feet)	SYMBOL USCS SOIL CLASS	DESCRIPTION	SAMPLE TYPE SAMPLE NUMBER	MEASURED PEN. RESISTANCE (blows/6 inches)	OTHER TESTS	GROUNDWATER	(1	40 lb. weig	netration T ght, 30" dro to SPT Va per foot 30	op)	50) DEPTH (feet)
60 — – – –		Heave in top 12". Stiff, brown, organic SILT, moist. (PRE-VASHON ALLUVIUM) Medium dense, gray, clean, fine to medium SAND, wet. Driller notes heaving sands, SPT values likely not true, ~8" of heave. Still losing Quik grout down the borehole.	S-18	5-8-9	GS							-60
65 — - - -	SM	Very dense, gray, silty, fine to coarse SAND, with fine gravel, wet. Small lenses of silt (~1") at 66'.		9-22-36							>>▲	-65
70	ML SM SM	Dense, olive-brown, SILT, with interbedded fine sand and rootlets, moist.	S-20	12-18-23								-70
75 — - -		Very dense, olive-gray, silty, fine to medium SAND, wet. Fine gravel in upper 2-3". Borehole terminated st 76.5' below ground surface.	S-21	8-24-40							>>:	-75
- 80 — - - 85 —		Ground water observed at 28' below ground surface during drilling. 3 batches of QuikGel lost in formation when auger at 57.5' (~120-135 gal). Heave washout water (200 gal) also disappeared. 2.5 barrels (~130 gal) of extra thick grout used to abandon the borehole.										- 80
- - - 90	explorati	oper understanding of the nature of subsurf on log should be read in conjunction with th	ace con ne text c	ditions, t f the	this		20 Plastic Lin		60 ontent (%) ●【 Li	80	100	-90
	This log of and therefo	nical report. subsurface conditions applies only at the specified location and o re may not necessarily be indicative of other times and/or locatio lard Penetration Resistance (where applicable) was standardized	n the date in ns. to SPT N-\	ndicated /alues using	the meth	nod indica	1	Natural Wa	ater Conter	nt Il Report.		
HW	AGEOS	A PROJECT EVERCE 36th STREET & RIVER CIENCES INC. EVERETT, WASHI	SIDE D	N		. 20	15-061	Bł	H-24 E: 3 of 3		٨	-25

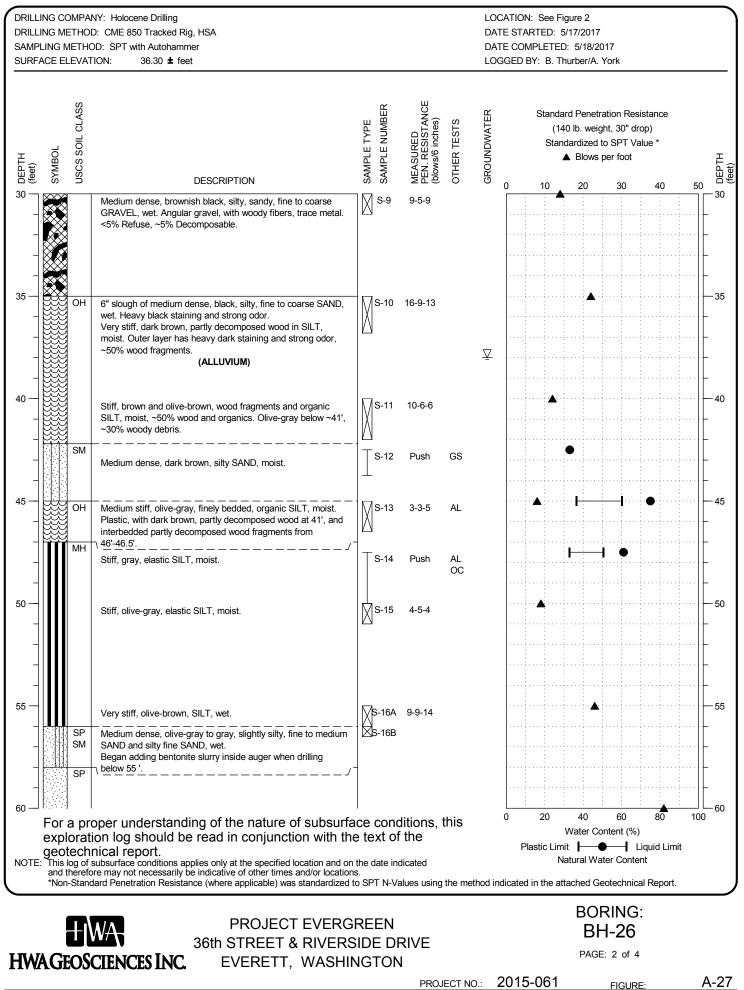
RILLING METHOD: Mobile B-58 Truck Rig, HSADAMPLING METHOD: California w/ 340-lb AutohammerD			DAT DAT	OCATION: See Figure 2 ATE STARTED: 5/23/2017 ATE COMPLETED: 5/24/2017 OGGED BY: B. Thurber/A. York				
(feet)	SYMBOL USCS SOIL CLASS	DESCRIPTION	SAMPLE TYPE SAMPLE NUMBER	MEASURED PEN. RESISTANCE (blows/6 inches)	OTHER TESTS	GROUNDWATER	Non-Standard Penetration Resistance (340 lb. weight, 30" drop) Standardized to SPT Value * ▲ Blows per foot	
 ר'	SN		7	240	0	0	0 10 20 30 40 50 C	
-								
		Dense, olive-gray, fine to coarse gravelly, silty, fine to medium SAND, moist. ~2" of AC on top. (FILL)	S-1	3-3-20				
-		Poor recovery, medium dense, drove ~4" of AC (in tip).	⊠ S-2	8-8-8				
		Poor recovery, medium dense, gray, fine to coarse gravelly, fine to medium SAND, moist.	⊠ _{S-3}	7-10-7				
		Poor recovery, meduim dense, dark gray, slightly gravelly, silty, fine to medium SAND, moist, 0.25" thick x 2" wood fragment and shredded clear plastic sheeting. <10% Refuse, <5% Decomposable.	⊠ _{S-4}	5-6-8				
		(REFUSE) Medium dense, gray, slightly gravelly, silty, fine to medium SAND, moist, over medium dense, dark gray, gravelly, silty SAND with glass and blackened wood fragments. <10% Refuse, <5% Decomposable.	S-5	8-7-11	GS			
		GRAVEL, moist, with ~3" of broken concrete in tip. 100% Refuse, 0% Decomposable.	S-6	20-22-18			>>4-1	
		Gravelly drilling action below 17.5'. Medium dense, dark gray, silty, sandy GRAVEL, moist. With upper 8" of broken concrete, one large sheet of clear visqueen. 100% Refuse, 0% Decomposable.	S-7	14-11-18				
		Medium dense, dark gray and black, organic, gravelly, silty, fine to coarse SAND, moist. Some of the soil is blackened; no odor, sheen, or smear. ~5% Refuse, ~10% Decomposable.	S-8	6-8-6				
		Medium dense, dark brown with mottled brown, slightly gravelly, organic, silty SAND, moist, with Refuse: Glass, metal, concrete, wood fragments, rubber, broken concrete on tip, blow counts overstated. ~30% Refuse, ~10%	S-9	2-3-33				
-		Decomposable. Heavy drilling action at ~23.5'-24'. Medium dense, black and dark brown to black, slightly	S-10	3-6-3				
		gravelly, silty SAND, moist. With wood, rubber ball, glass, cardboard, shredded paper, plastic, and fabric. ~70% Refuse, ~40% Decomposable.				Ā		
	explora	proper understanding of the nature of subsurfa ation log should be read in conjunction with the chnical report.	 ace con e text o	ditions, f the	this			
TE:	This log of and there	of subsurface conditions applies only at the specified location and on efore may not necessarily be indicative of other times and/or location andard Penetration Resistance (where applicable) was standardized	IS.		the me	thod inc	Natural Water Content licated in the attached Geotechnical Report.	
		WA PROJECT EVERG 36th STREET & RIVERS		RIVE			BORING: BH-25	
NA	GEO	SCIENCES INC. EVERETT, WASHI					PAGE: 1 of 4	

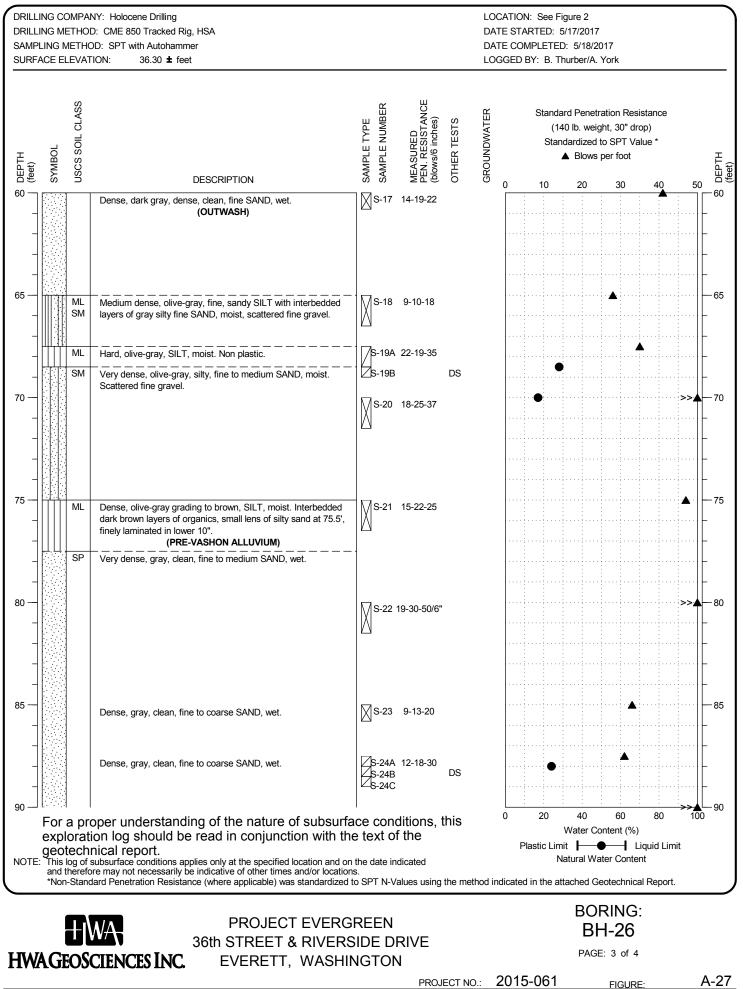


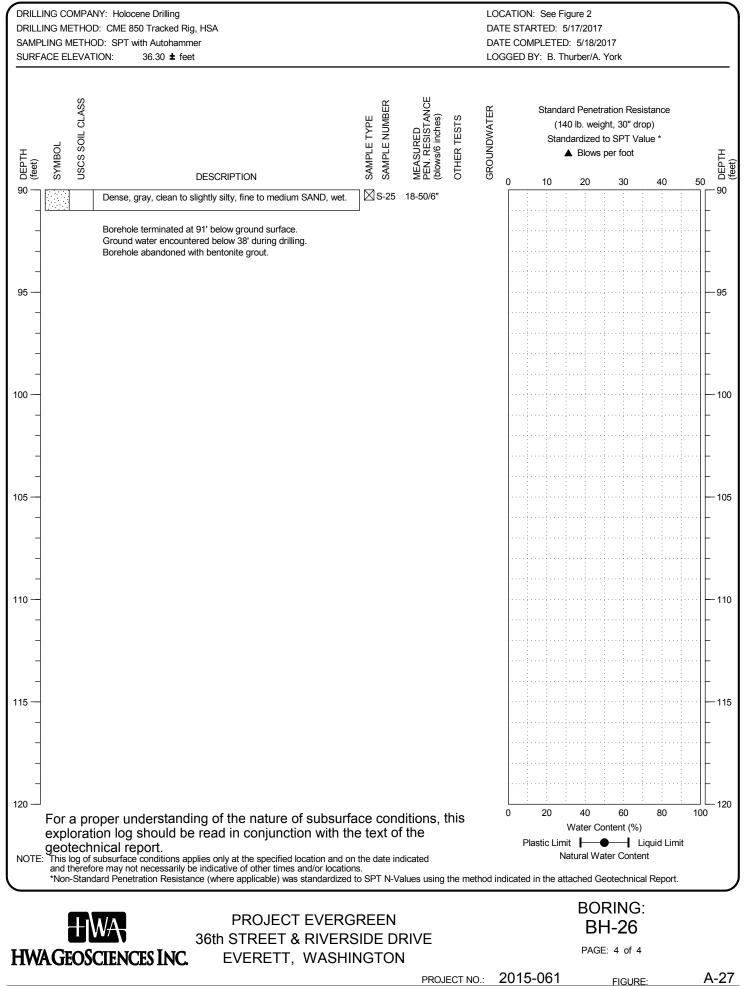


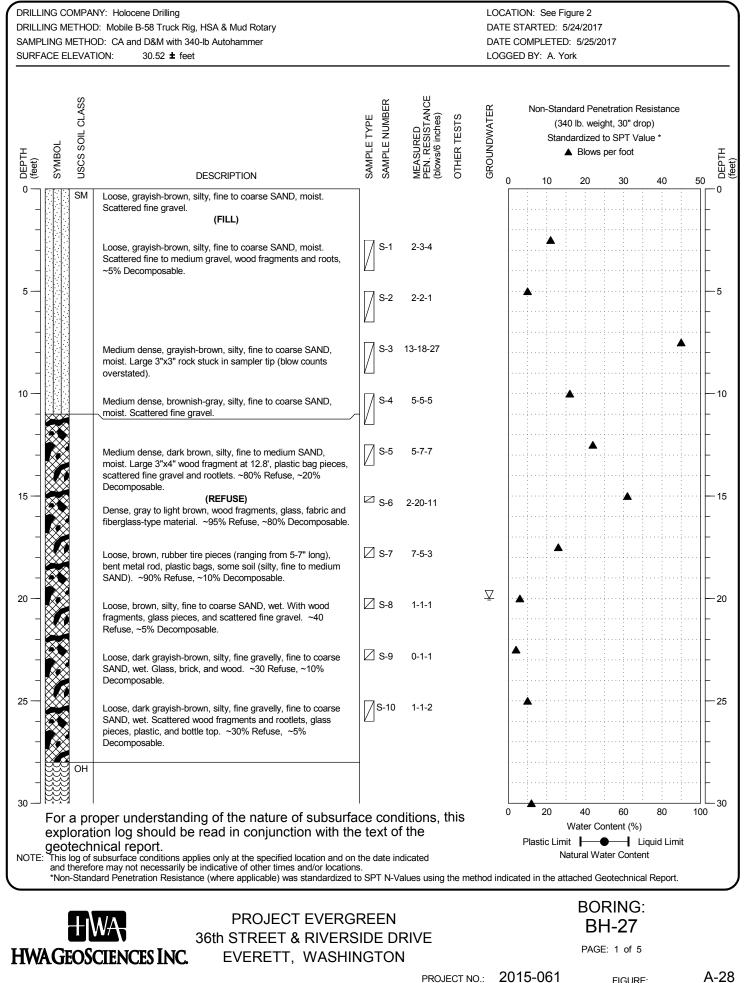


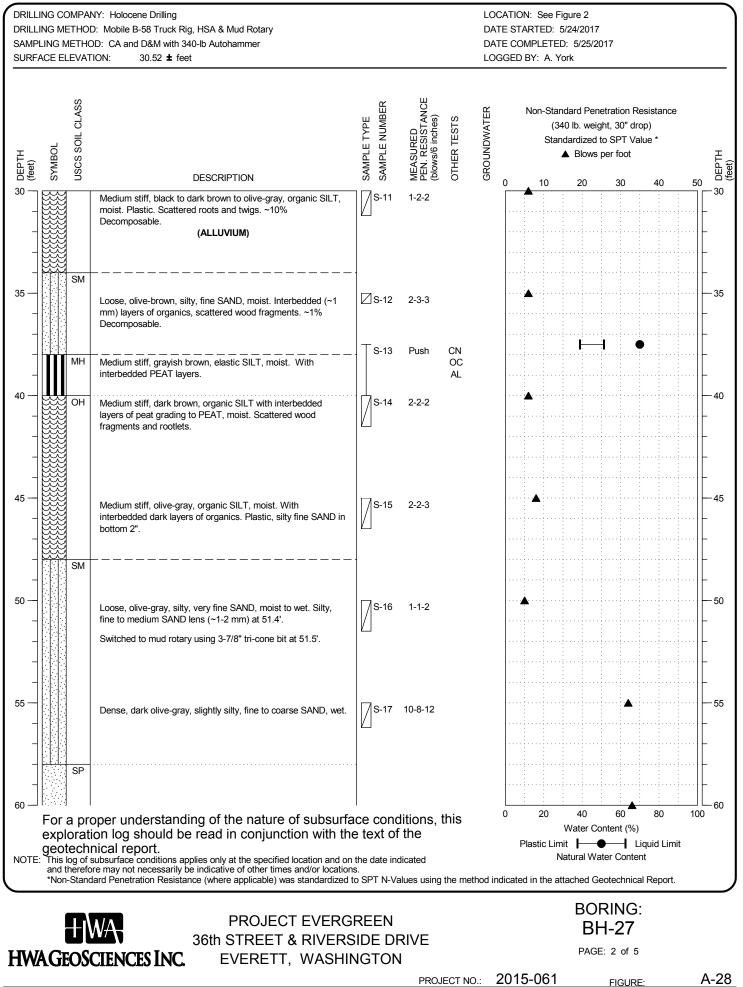


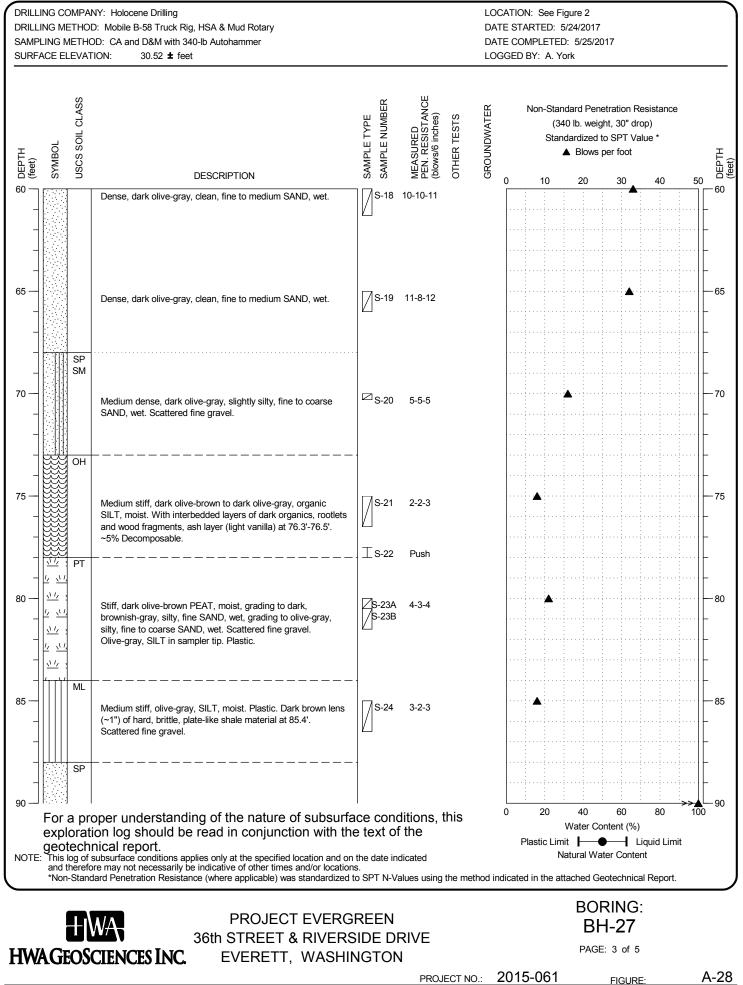


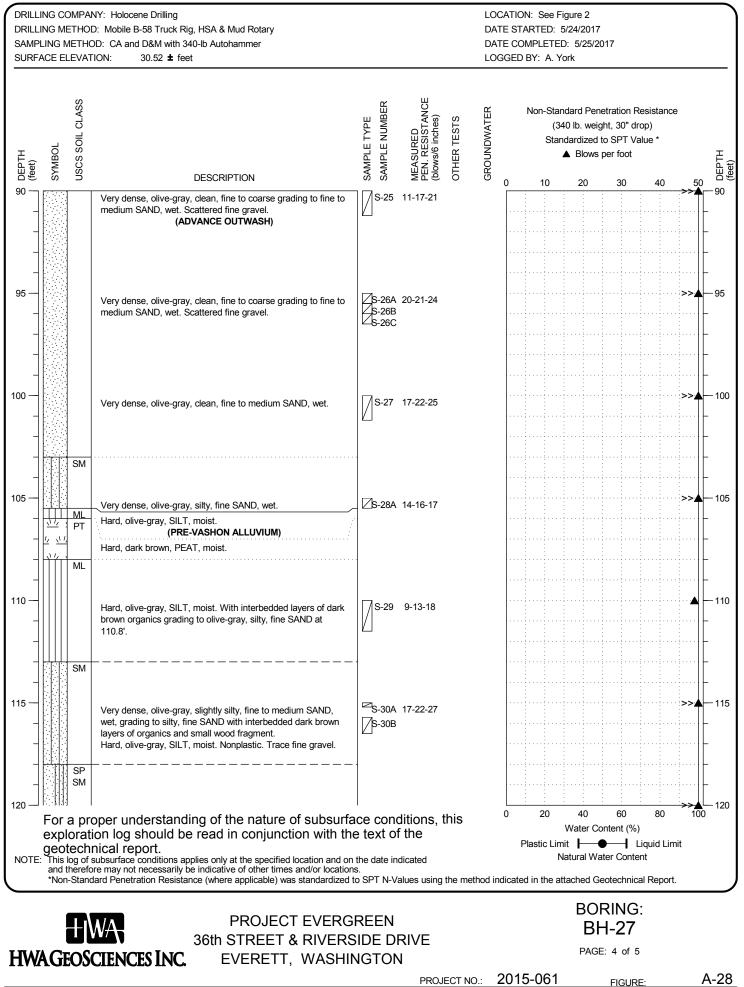


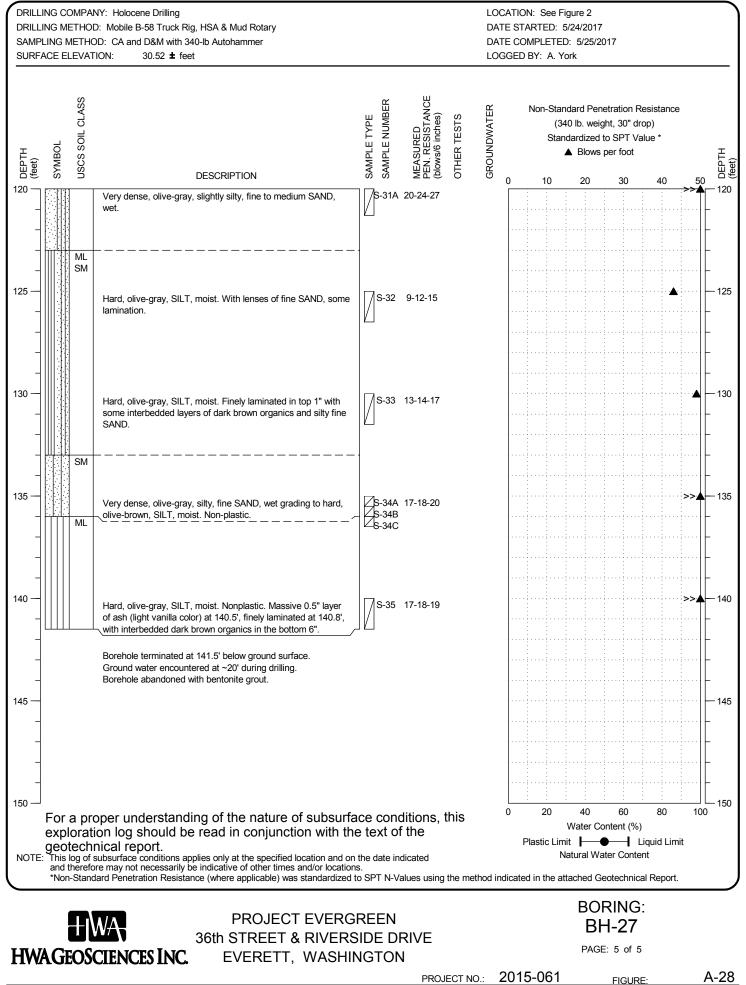


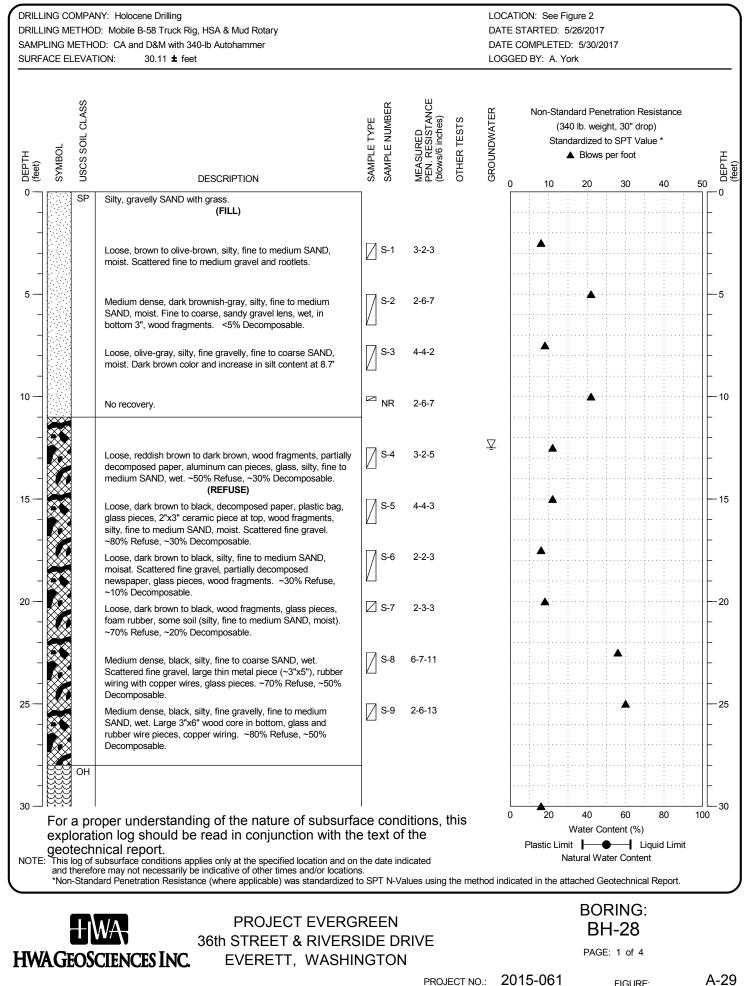


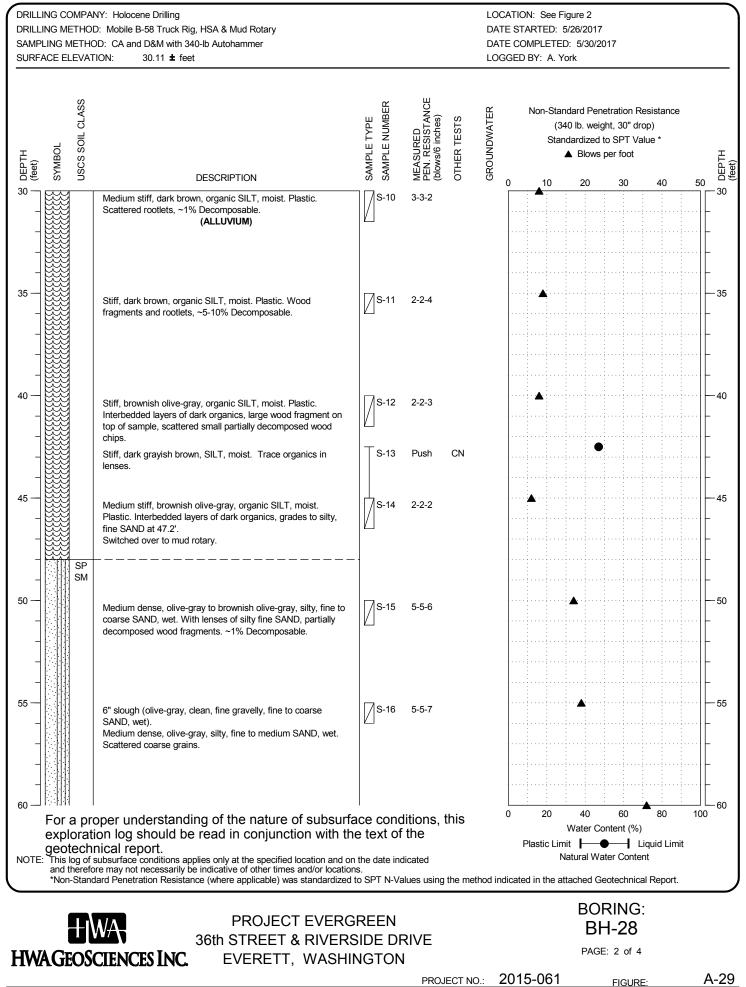


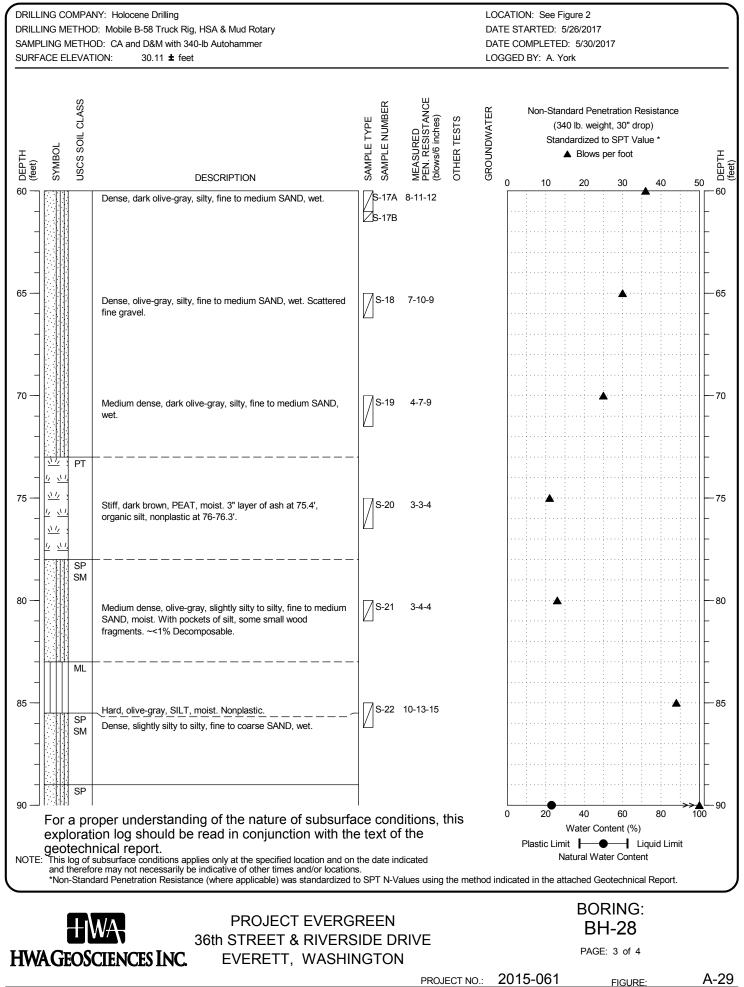


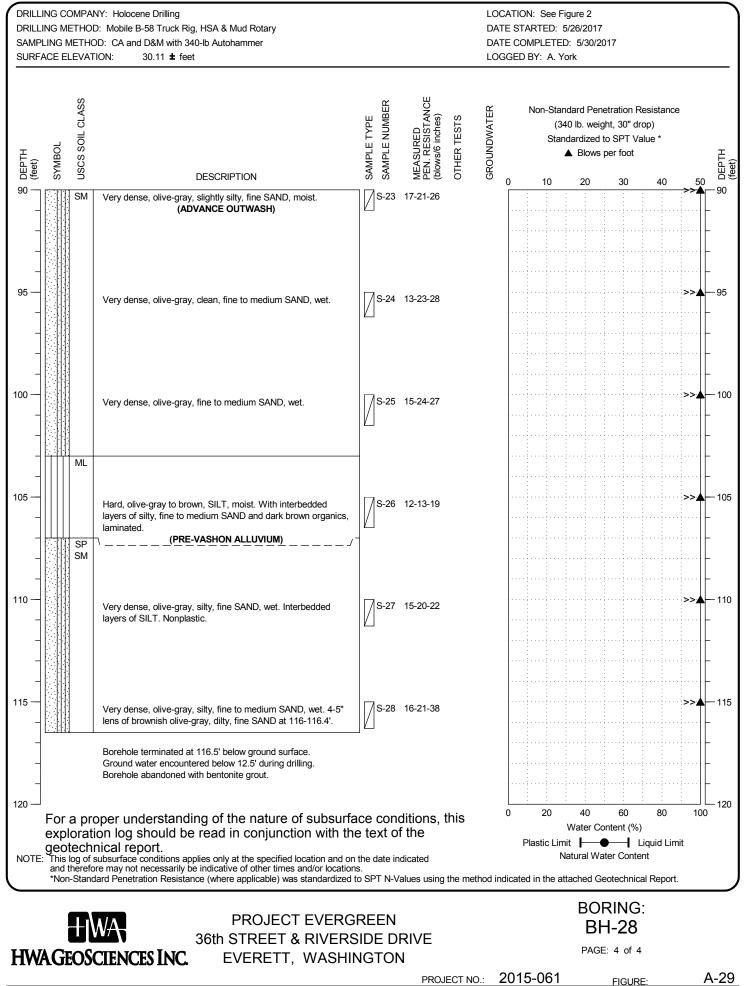




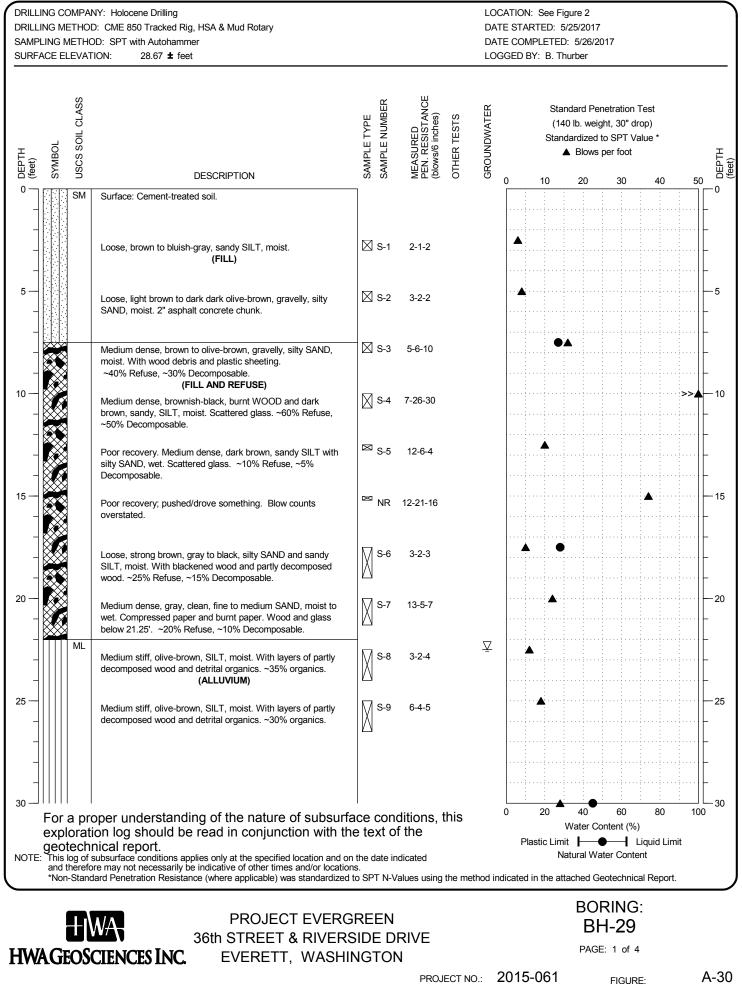


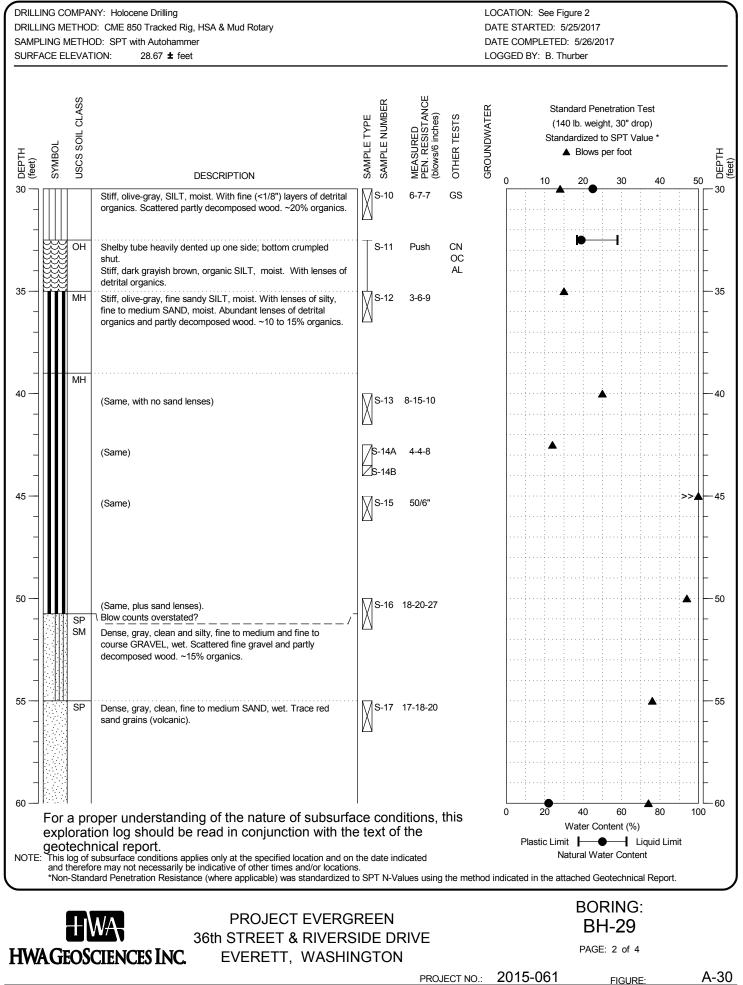


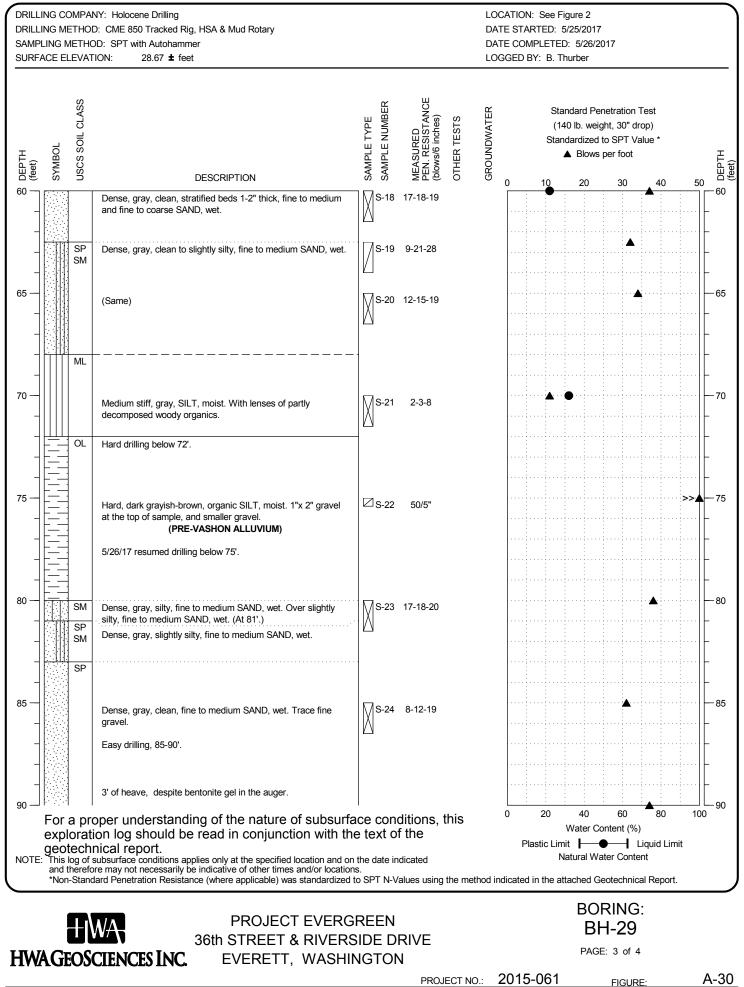


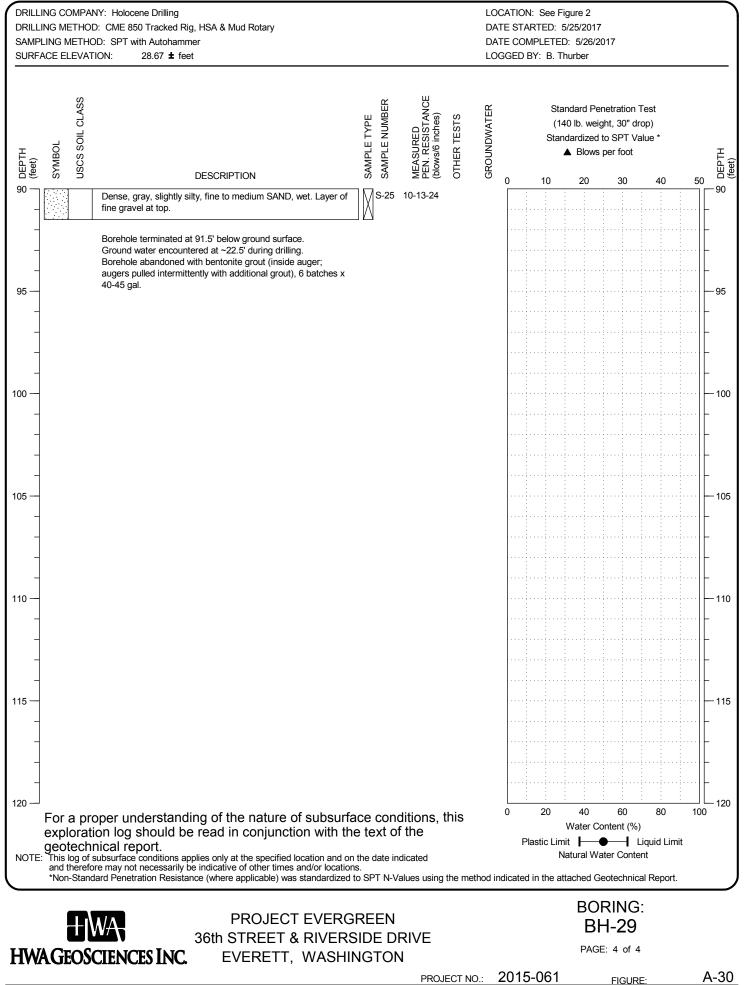


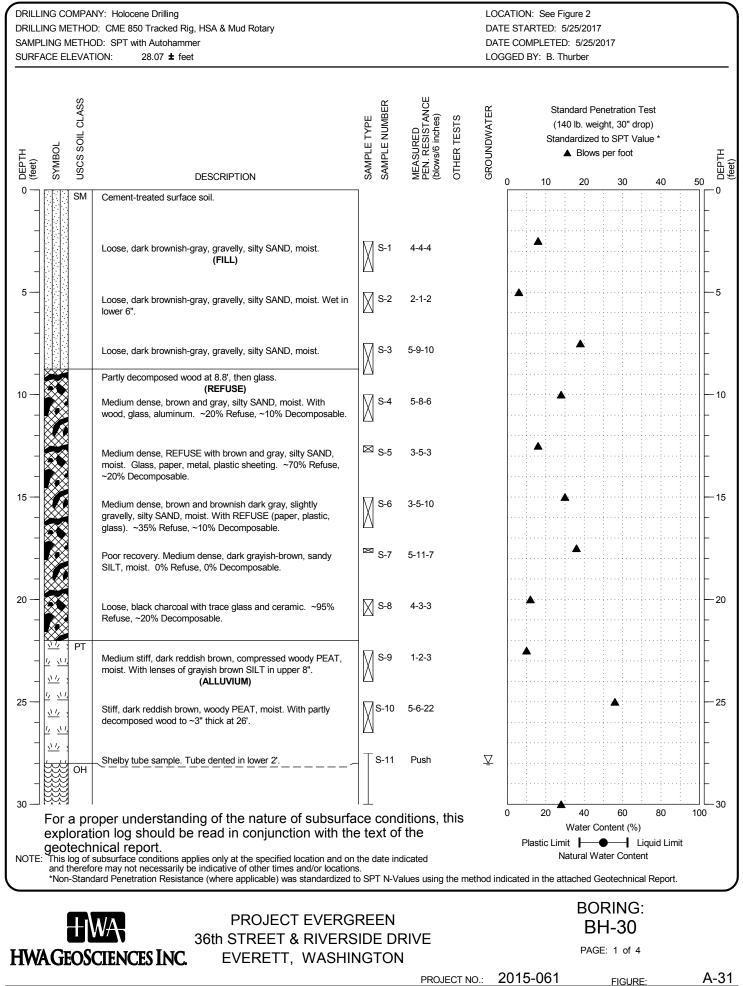
2015-061

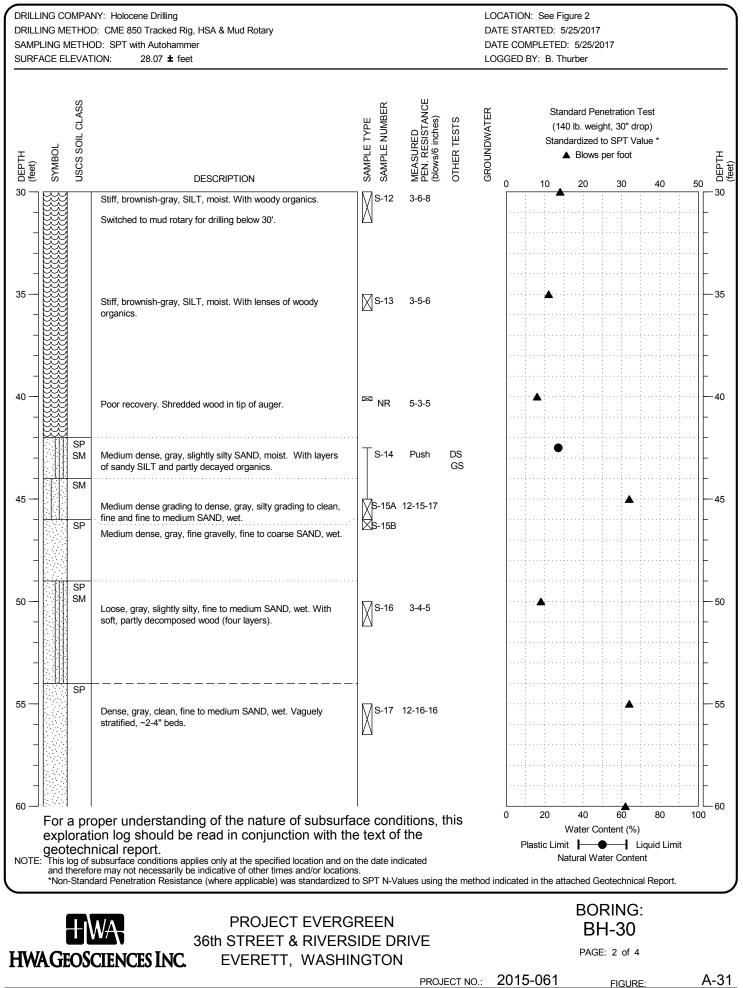


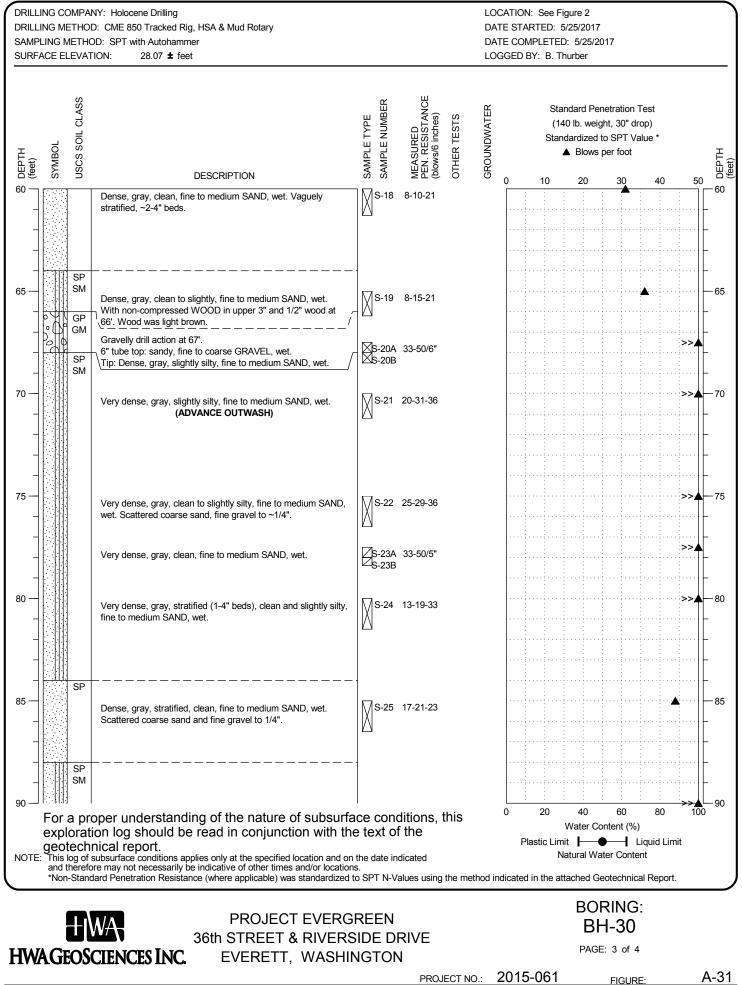


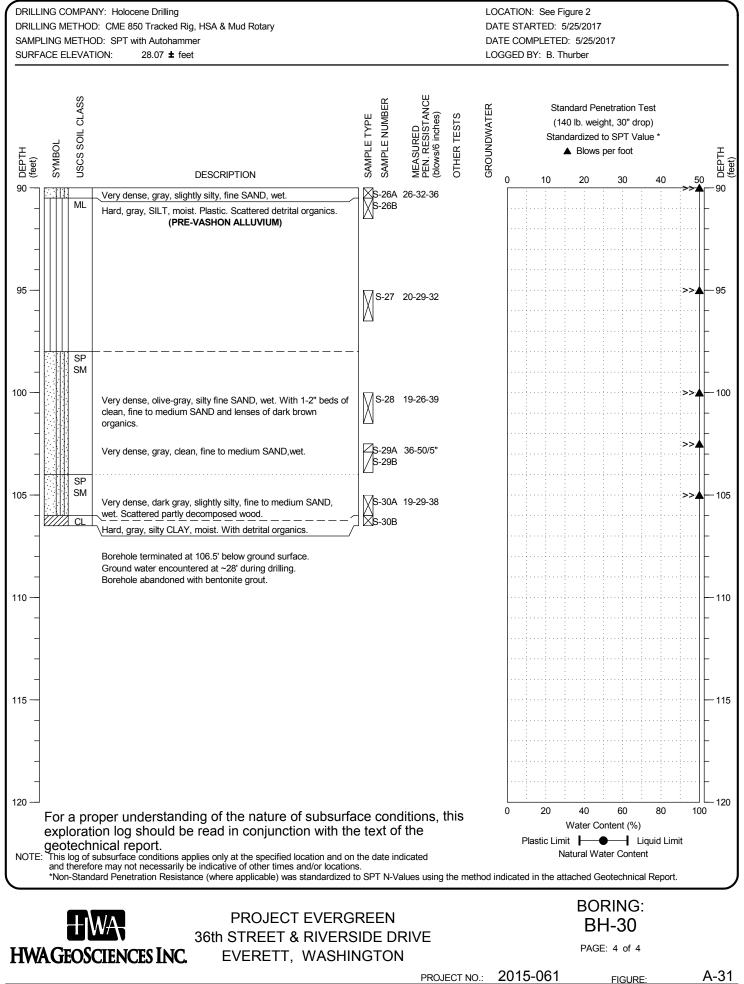


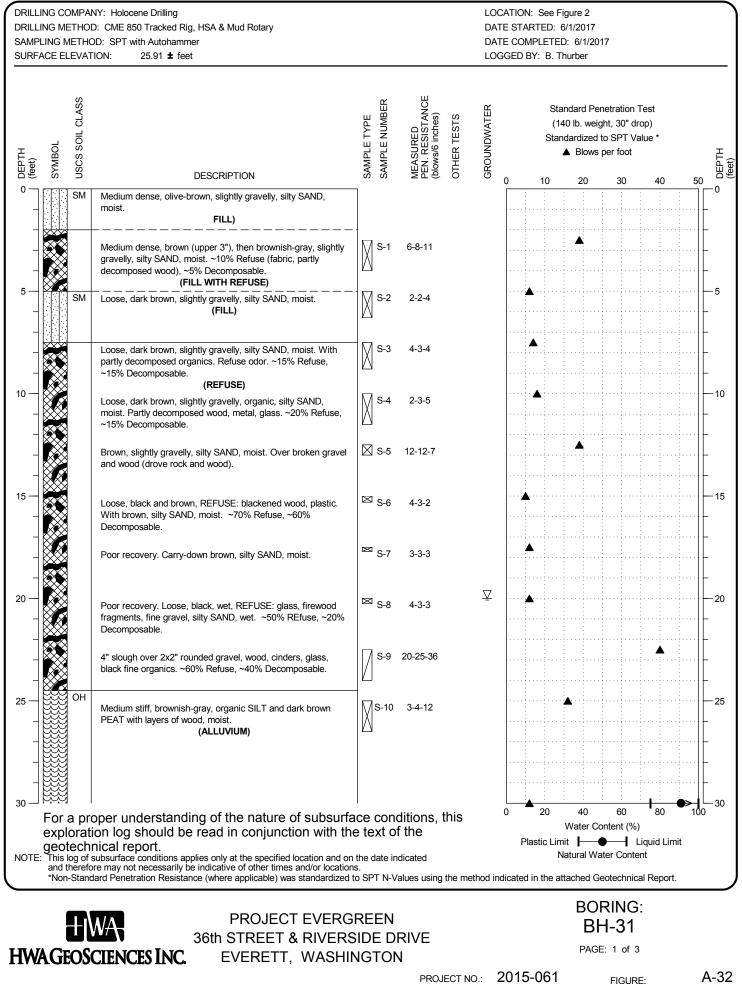


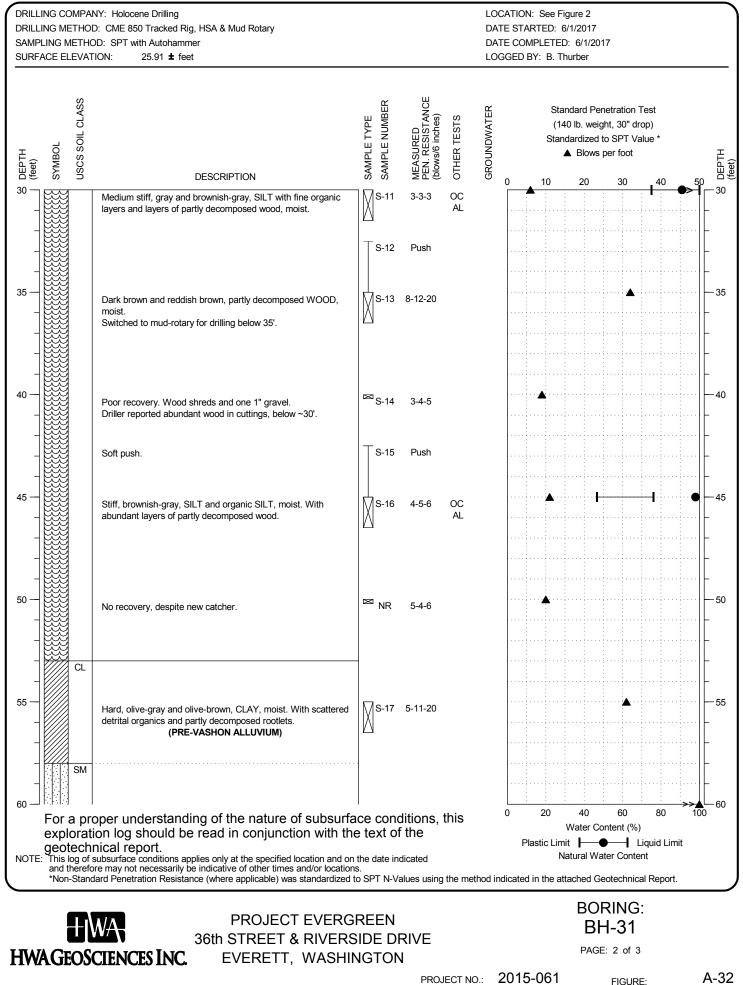


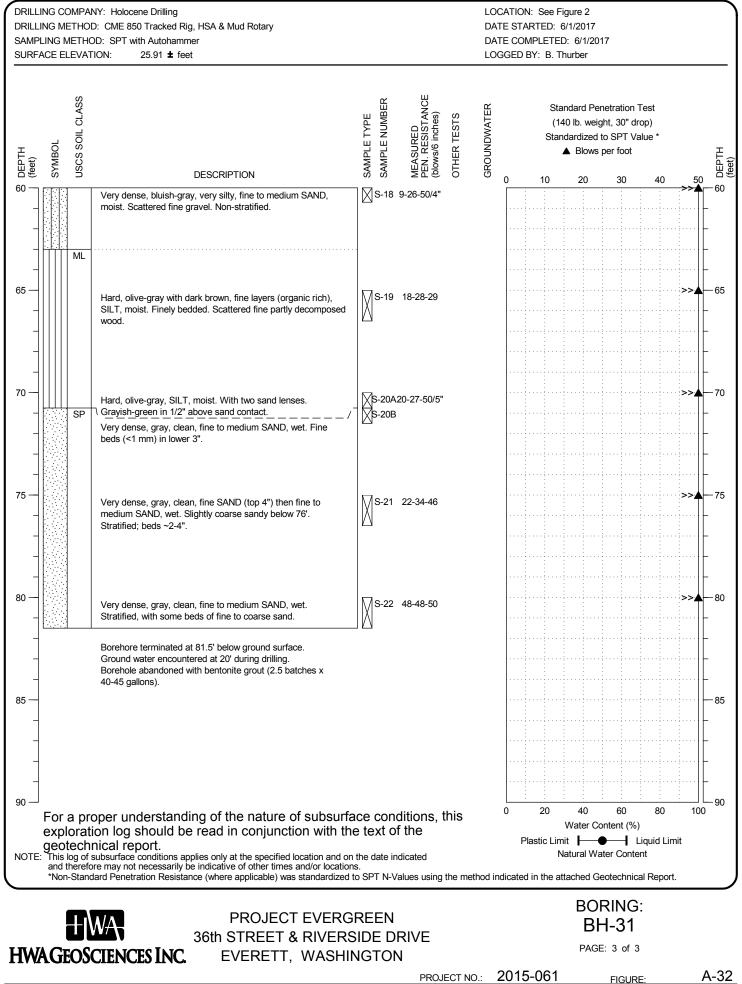


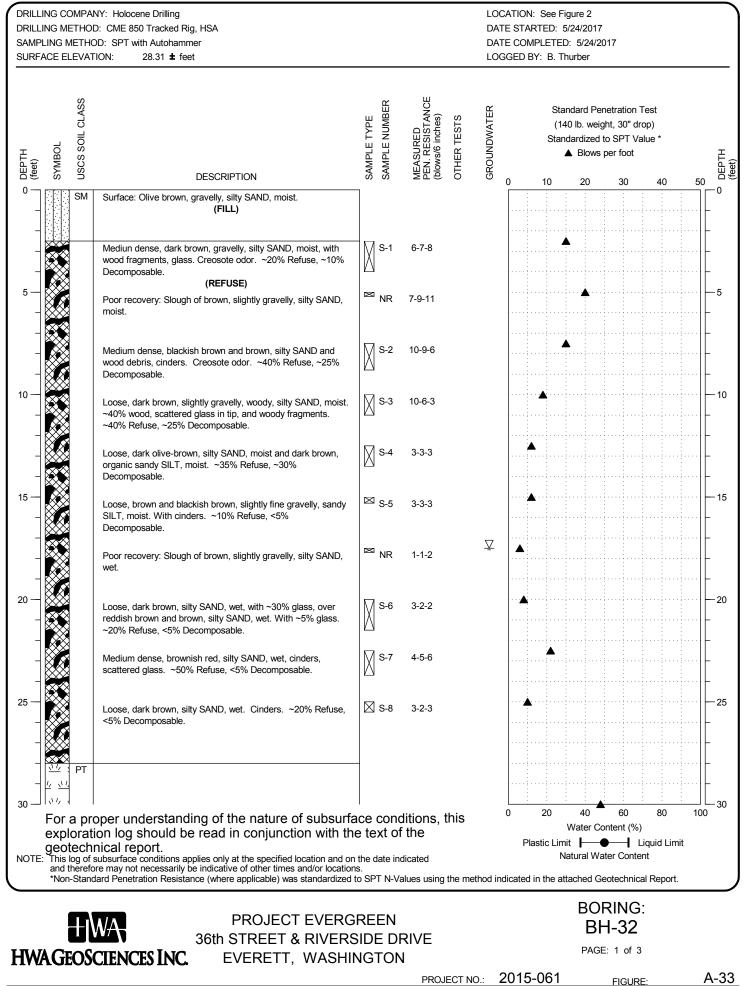






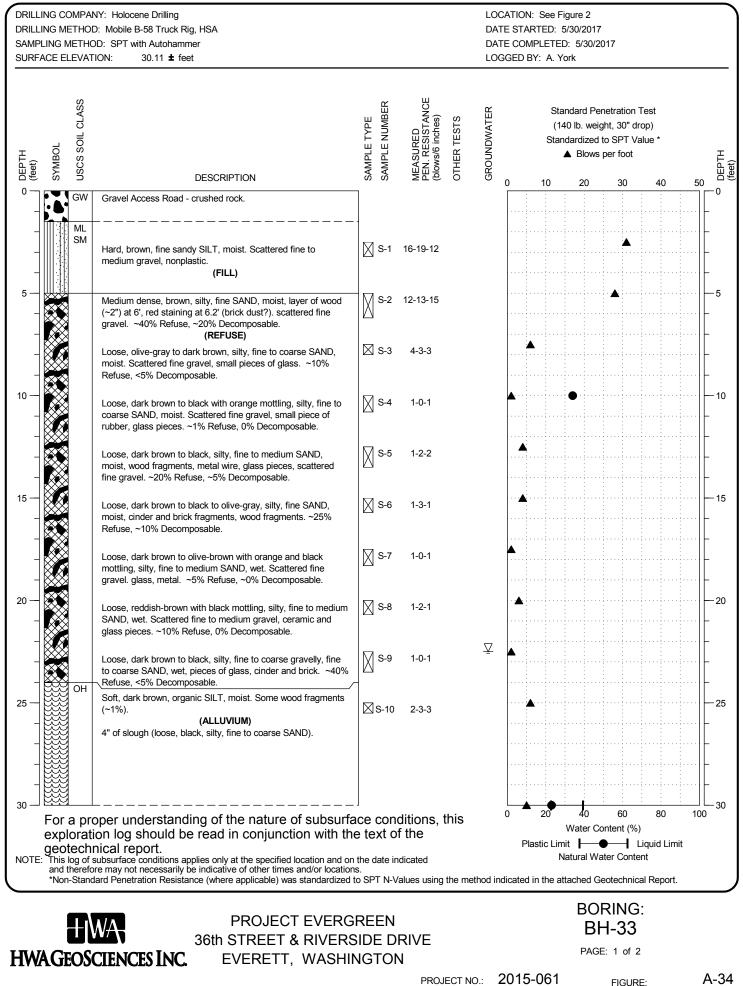




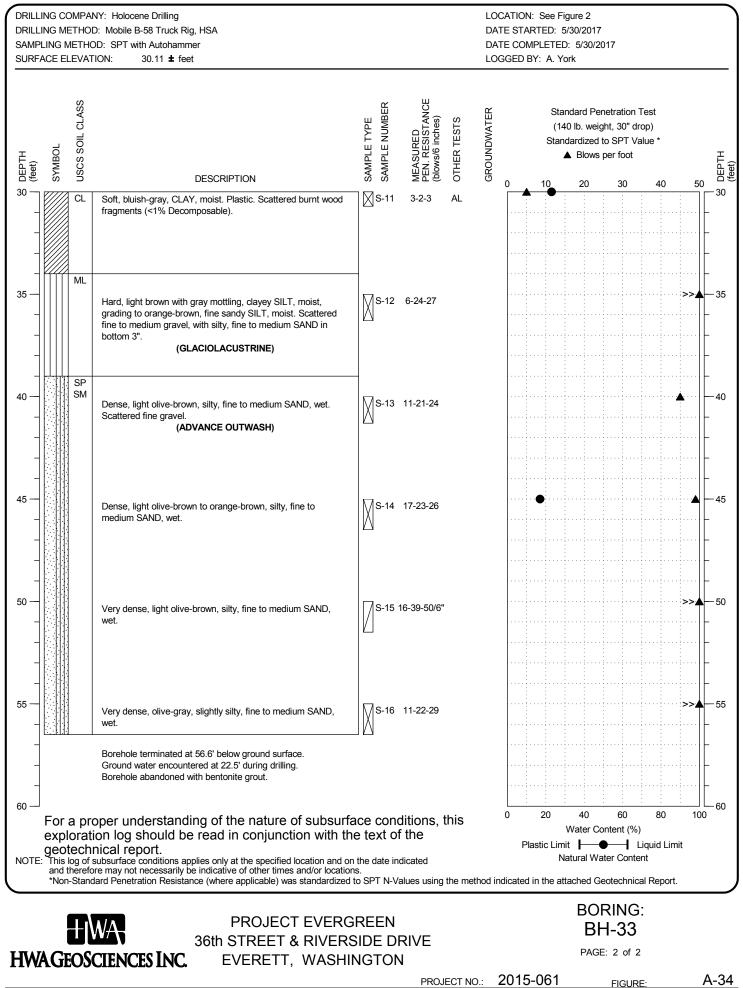


DRILL	ing M Ling N	ETHO METHO	NY: Holocene Drilling D: CME 850 Tracked Rig, HSA DD: SPT with Autohammer FION: 28.31 ± feet				DATE S DATE C	TARTED OMPLE	e Figure 2): 5/24/2017 [ED: 5/24/2 . Thurber				
DEPTH (feet)	SYMBOL	NSCS SOIL CLASS	DESCRIPTION	SAMPLE TYPE SAMPLE NUMBER	MEASURED PEN. RESISTANCE (blows/6 inches)	OTHER TESTS	GROUNDWATER		Standard Pe (140 lb. wei tandardized ▲ Blows 20	ght, 30" dro to SPT Val	op)	50	DEPTH (feet)
30 _	<u>\\/</u> // \\/		No recovery, except 1/8" thick shred of wood.	[™] NR	12-12-12								-30
-			Very stiff, dark brown, PEAT, moist, with scattered wood fragments. (ALLUVIUM)	S-9	12-13-13		•••		A				
35 —	<u> </u>		Reddish brown, oxidizing to dark brown, woody PEAT, moist.	S-10	7-9-12				A				-35
-			Shelby tube tip crumpled; chain link fence wire in tip.		Push	CN OC						185 -	
40	<u> </u>		Medium stiff, dark reddish brown, woody PEAT, moist. Sampler pounding on chain link fencing; blow counts overstated.	⊥ ⊠ _{S-12}	50/4"							>> A	-40
 45 	<u>1, 1,</u> <u>1, 1,</u> <u>1, 1,</u> <u>1, 1,</u>		No recovery; sampler pounding on chain link fencing; blow counts overstated.	⊠ _{NR}	50/3"							>>: A	- 45
-		ML	Heavy wire fragment at top of sampler. Stiff, greenish gray, SILT, moist, with lenses of silty, fine to medium SAND, wet. Scattered partly decomposed woody fragments. Blow counts overstated.	S-13	12-15-18								
50 — - -		CL	Stiff, greenish gray, gray, and olive brown, CLAY, moist. Highly plastic. Piece of wire (chain link fencing) caught in tip; blow counts overstated.	S-14	8-9-14				· · · · · · · · · •				- 50
			Harder drilling below 51'.										
55 — - -		ML SM	Gravelly drill action below ~54'. Dense, gray, silty, fine to medium SAND, wet, over light brown, non-plastic SILT, wet, over gray, slightly sandy, SILT (plastic), moist. Stiff bluish gray CLAY, moist, in tip.	S-15	12-15-20		•••				.		- 55
-		SP	Dense to very dense, gray, clean, fine to medium SAND, wet, with silty, fine SAND, moist (lenses). (ADVANCE OUTWASH)	S-16A 5-16B	23-50/6"							>>▲	
60 – NOTE	exp geo This and	lorat tech log of therefo	oper understanding of the nature of subsurfa ion log should be read in conjunction with the nical report. subsurface conditions applies only at the specified location and on ore may not necessarily be indicative of other times and/or locations dard Penetration Resistance (where applicable) was standardized t	e text o the date ir s.	f the		0 od indica		Water C ∟imit Natural W	ater Conter	nt	->>▲	-60
HW	AG	E OS	PROJECT EVERG 36th STREET & RIVERS CIENCES INC. EVERETT, WASHIN	IDE D					В	RING: H-32 E: 2 of 3			
					PROJ	ECT NO	: 20'	15-06	1	FIGURE		Α	-33

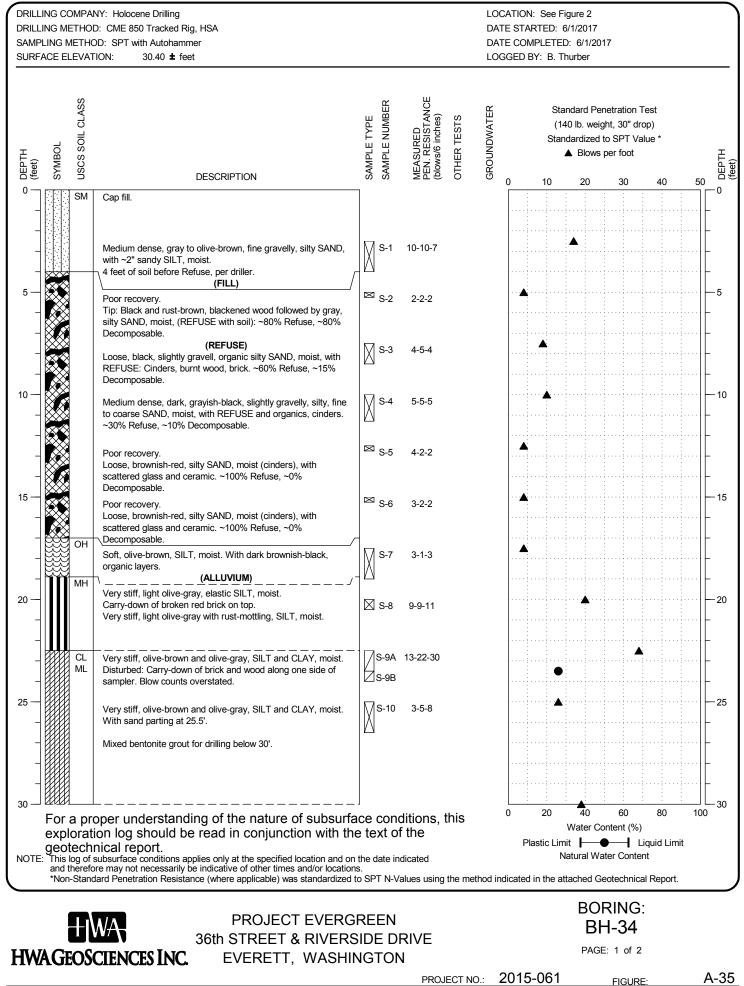
DRILLING COMPANY: Holocene Drilling DRILLING METHOD: CME 850 Tracked Rig, HSA SAMPLING METHOD: SPT with Autohammer		LOCATION: Se DATE STARTE DATE COMPLE	D: 5/24/2017 TED: 5/24/2			
SURFACE ELEVATION: 28.31 ± feet		LOGGED BY: I	B. Thurber			
SSVUECTORS SOIL OLASS (feet) HTTATE MBOL USCS SOIL OLASS MBOL DESCLIDION	SAMPLE TYPE SAMPLE NUMBER MEASURED PEN. RESISTANCE (blows/6 inches) OTHER TESTS	GROUNDWATER	Standard Pe (140 lb. weig Standardized A Blows	ght, 30" dr to SPT Va	op)	05 DEPTH (feet)
60 SM Very dense, gray, silty fine SAND, wet, grading to SILT, ML plastic (wet) and non-plastic (moist). 2x1/4" lenses of med	S-17 18-28-30					>> 4 -60
65 Hard, dark brown, with a few tan deformed laminae, organ						····· – ···· – >>▲──65
- SILT, moist, with lenses of gray, slightly silty, fine to coars - SAND, wet. Gravel in sand lens at 66.2'. (PRE-VASHON ALLUVIUM) - <tr< td=""><td></td><td></td><td></td><td></td><td></td><td>····· - ···· - >> - 70</td></tr<>						····· - ···· - >> - 70
 ML Hard, grayish brown, oxidized to olive-gray, fine sandy SIL moist, with lenses of partly decomposed woody organics, grading to (lower ~6") brown, organic SILT, moist, with len of woody organics. 			•			>>
A ML Dark brown and gray, sandy SILT, moist, over very dense gray, clean, fine to medium SAND, wet.	≥,					
75 Very dense, gray, slightly silty, interbedded, fine to mediur and fine to coarse SAND, wet. Dark brown, fine organics i <1mm lenses, in fine sand.						>>: A 75
Borehole terminated at 75.9' below ground surface. Ground water encountered at 17.5' during drilling. Borehole abandoned with bentonite grout, with chips in up ~3'.	oper					
 ⁹⁰ For a proper understanding of the nature of subsexploration log should be read in conjunction wit geotechnical report. NOTE: This log of subsurface conditions applies only at the specified location a and therefore may not necessarily be indicative of other times and/or lo *Non-Standard Penetration Resistance (where applicable) was standard 	th the text of the and on the date indicated ocations.		Water Construction Water Construction Constr	ater Conte	Liquid Limit Int	90 100
PROJECT EVE 36th STREET & RIVI	ERSIDE DRIVE		Bl	RING H-32 E: 3 of 3		
HWAGEOSCIENCES INC. EVERETT, WAS		o.: 2015-06	61	FIGUR	E:	A-33

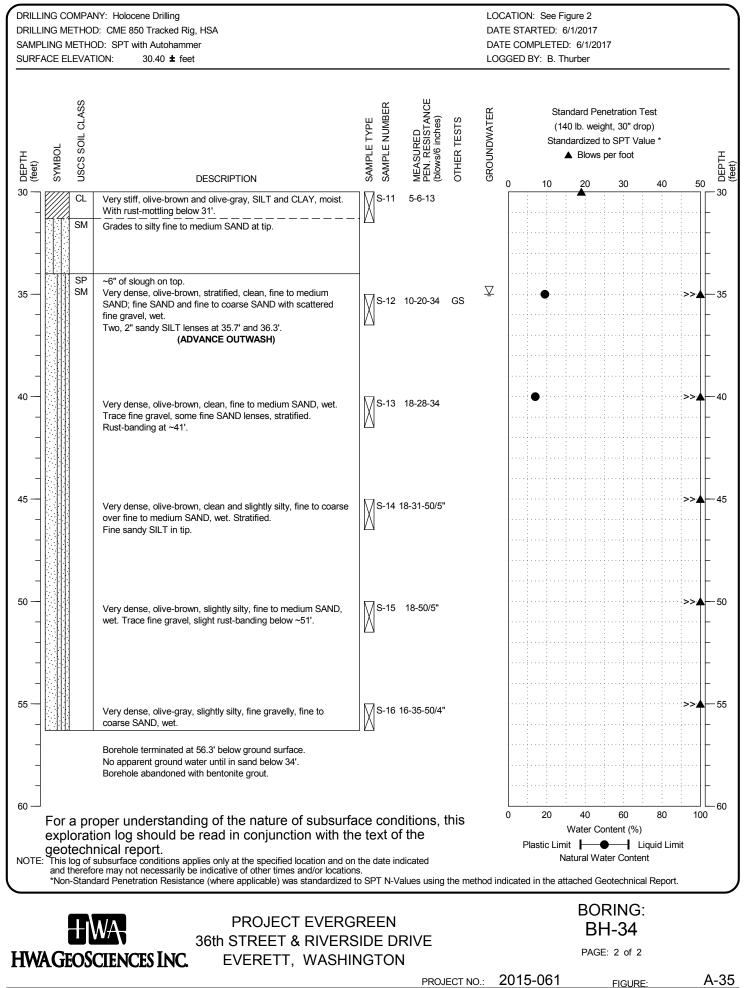


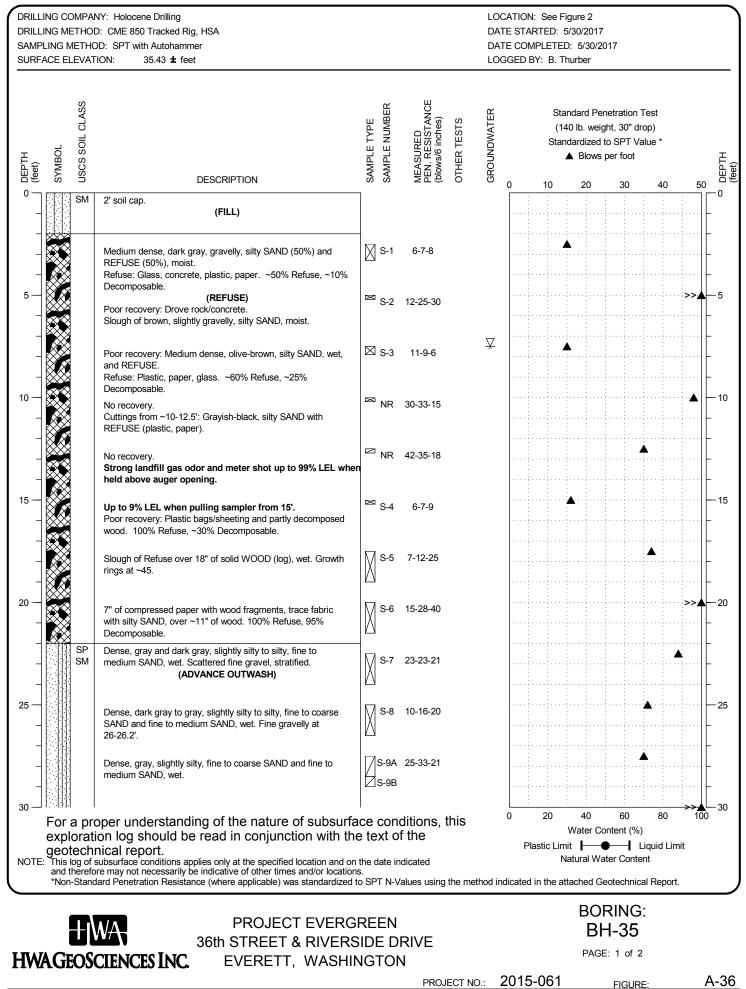
A-34

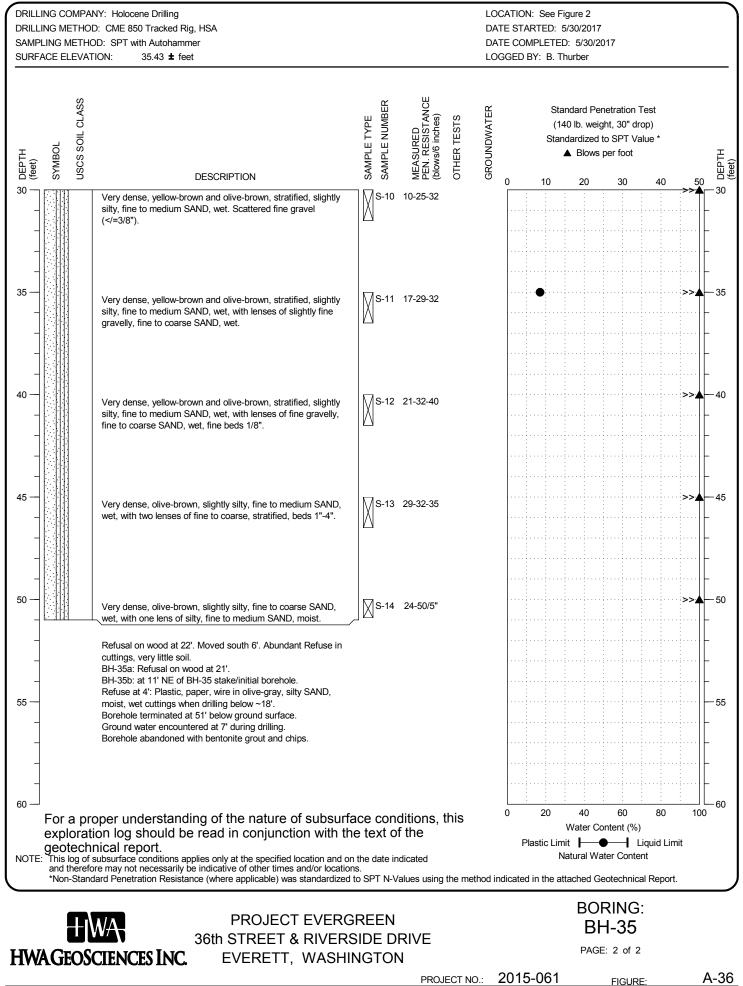


A-34









BORING 2015-061 EVERGREEN 2015-061 GPJ 8/17/17

A-36

APPENDIX B

GEOENGINEERS PREVIOUS INVESTIGATIONS

SOIL CLASSIFICATION CHART SYMBOLS TYPICAL MAJOR DIVISIONS GRAPH LETTER DESCRIPTIONS WELL-GRADED GRAVELS, GRAVEL 0 GW CLEAN 0 GRAVEL GRAVELS 2 AND 0 0 (LITTLE OR NO FINES) POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES GRAVELLY ို 0 q GP 0 SOILS 0 ____} COARSE SILTY GRAVELS, GRAVEL - SAND -SILT MIXTURES MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE GRAVELS WITH GM GRAINED FINES SOILS (APPRECIABLE AMOUNT OF FINES) CLAYEY GRAVELS, GRAVEL - SAND -CLAY MIXTURES GC WELL-GRADED SANDS, GRAVELLY SANDS SW CLEAN SANDS MORE THAN 50% RETAINED ON NO. 200 SIEVE SAND (LITTLE OR NO FINES) AND POORLY-GRADED SANDS, GRAVELLY SAND SANDY SP SOILS MORE THAN 50% SANDS WITH SM SILTY SANDS, SAND - SILT OF COARSE FRACTION PASSING NO. 4 SIEVE MIXTURES FINES (APPRECIABLE AMOUNT CLAYEY SANDS, SAND - CLAY MIXTURES SC OF FINES) INORGANIC SILTS, ROCK FLOUR, CLAYEY SILTS WITH SLIGHT PLASTICITY ML INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS SILTS CL LIQUID LIMIT LESS THAN 50 FINE AND CLAYS GRAINED SOILS ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY OL MORE THAN 50% PASSING NO. 200 SIEVE INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS SILTY SOILS MH SILTS LIQUID LIMIT GREATER THAN 50 INORGANIC CLAYS OF HIGH PLASTICITY CH AND CLAYS ORGANIC CLAYS AND SILTS OF MEDIUM TO HIGH PLASTICITY OH PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS HIGHLY ORGANIC SOILS PT _ NOTE: Multiple symbols are used to indicate borderline or dual soil classifications %F Sampler Symbol Descriptions AL CA 2.4-inch I.D. split barrel ĊР CS Standard Penetration Test (SPT) DS HA Shelby tube MC MD Piston OC **Organic content** ΡM **Direct-Push** PP SA Sieve analysis \mathbb{N} Bulk or grab ТΧ UC VS Blowcount is recorded for driven samplers as the number of blows required to advance sampler 12 inches (or distance noted). See exploration log for hammer weight NS and drop. SS Slight Sheen MS Moderate Sheen A "P" indicates sampler pushed using the weight of the **Heavy Sheen** HS drill ria. NT Not Tested

ADDITIONAL MATERIAL SYMBOLS

SYM	BOLS	TYPICAL			
GRAPH	LETTER	DESCRIPTIONS			
	сс	Cement Concrete			
	AC	Asphalt Concrete			
	CR	Crushed Rock/ Quarry Spalls			
	TS	Topsoil/ Forest Duff/Sod			

- Measured groundwater level in exploration, well, or piezometer
- Groundwater observed at time of exploration
- Perched water observed at time of exploration
- Measured free product in well or piezometer

Stratigraphic Contact

- Distinct contact between soil strata or geologic units
- Gradual change between soil strata or geologic units
- Approximate location of soil strata change within a geologic soil unit

Laboratory / Field Tests

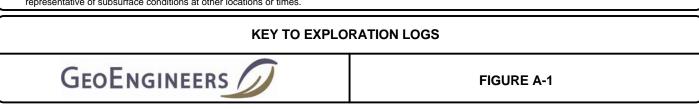
- Percent fines Atterberg limits Chemical analysis Laboratory compaction test Consolidation test **Direct shear** Hydrometer analysis Moisture content Moisture content and dry density
 - Permeability or hydraulic conductivity
 - Pocket penetrometer

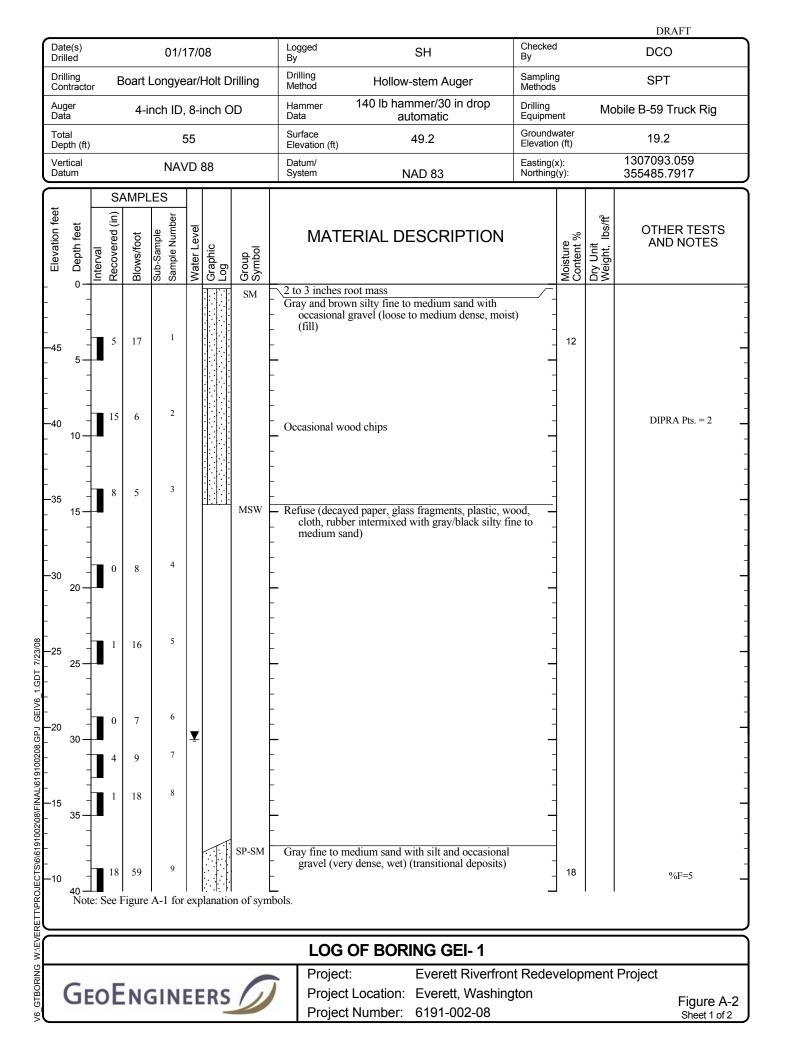
 - Triaxial compression
 - Unconfined compression
 - Vane shear

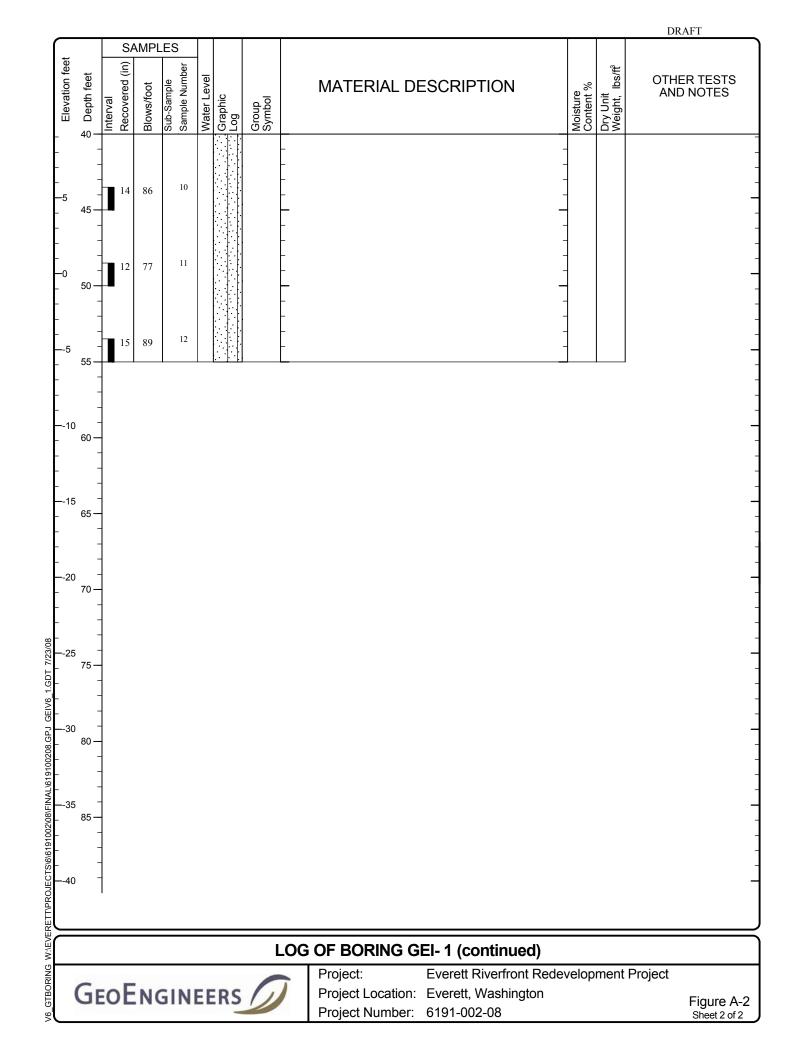
Sheen Classification

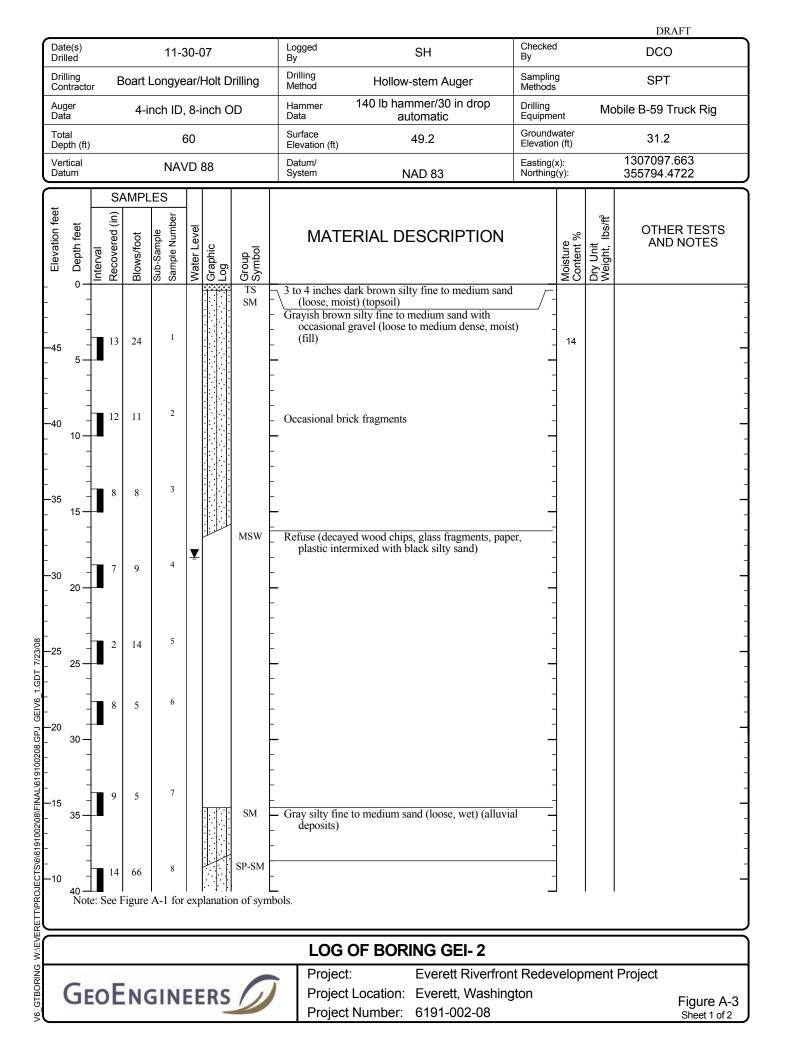
- No Visible Sheen

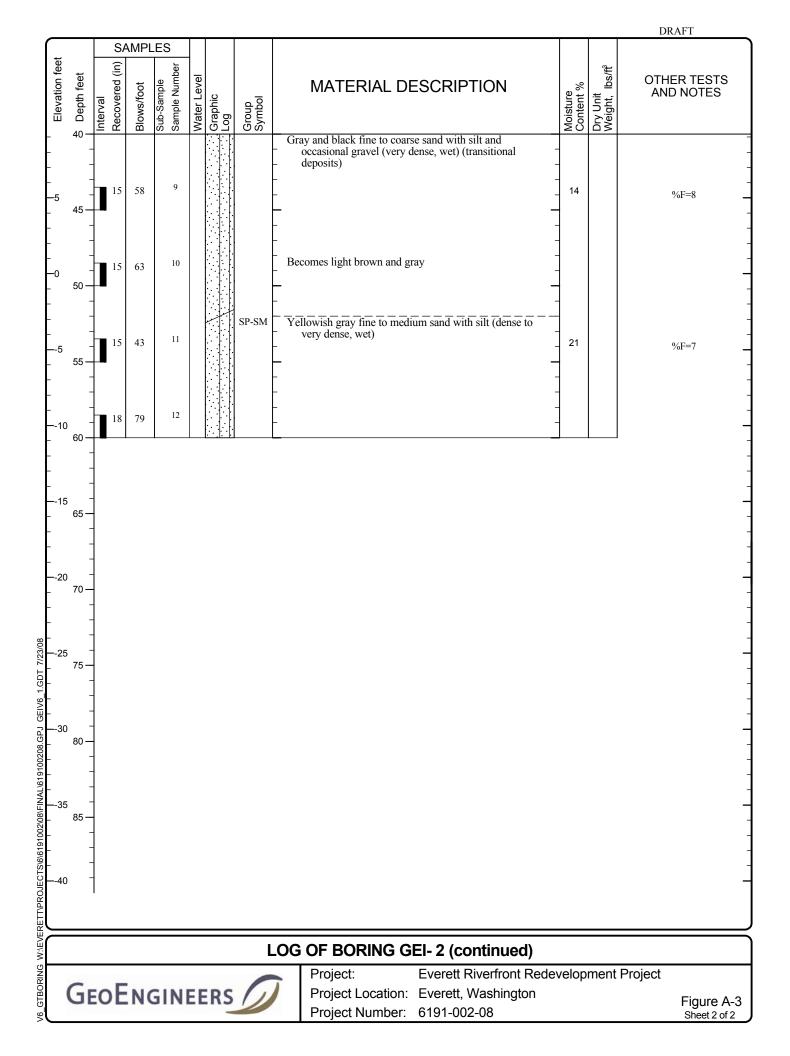
NOTE: The reader must refer to the discussion in the report text and the logs of explorations for a proper understanding of subsurface conditions. Descriptions on the logs apply only at the specific exploration locations and at the time the explorations were made; they are not warranted to be representative of subsurface conditions at other locations or times

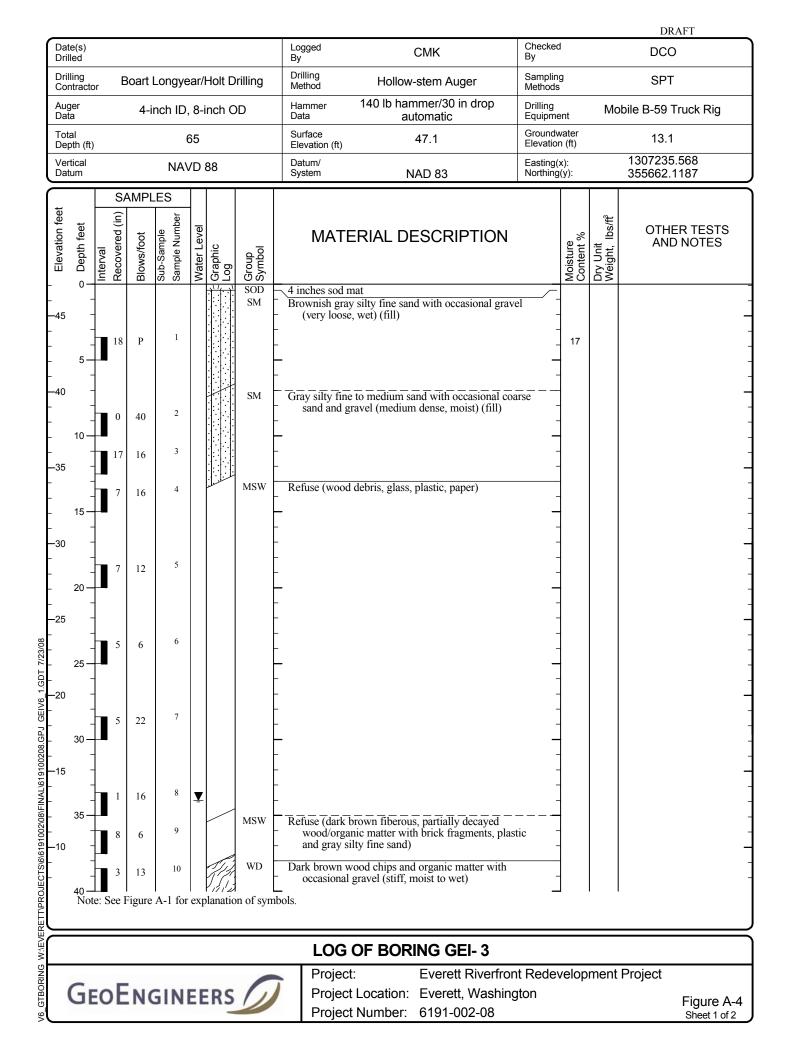


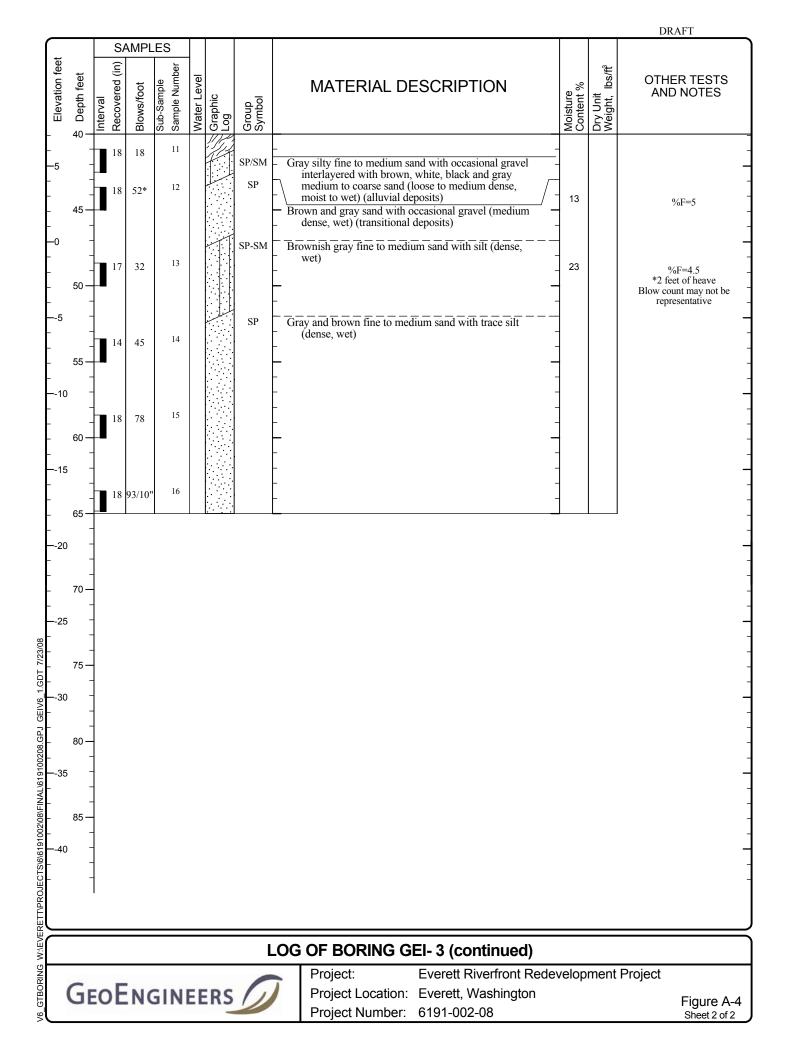


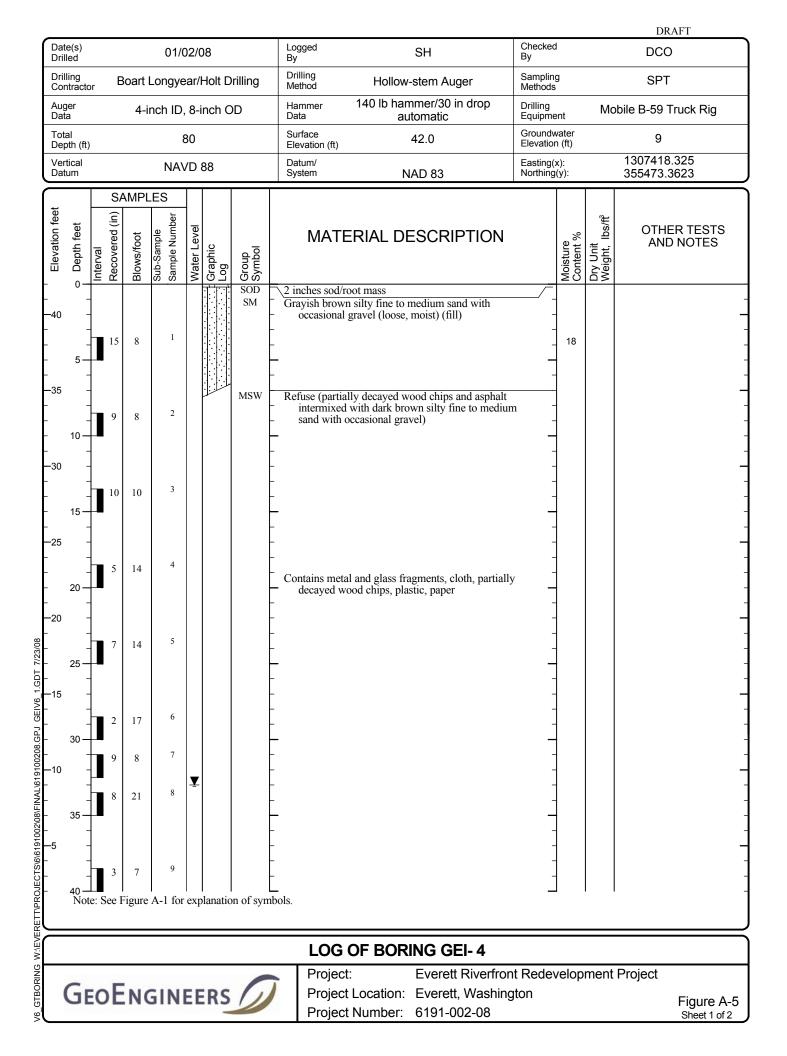


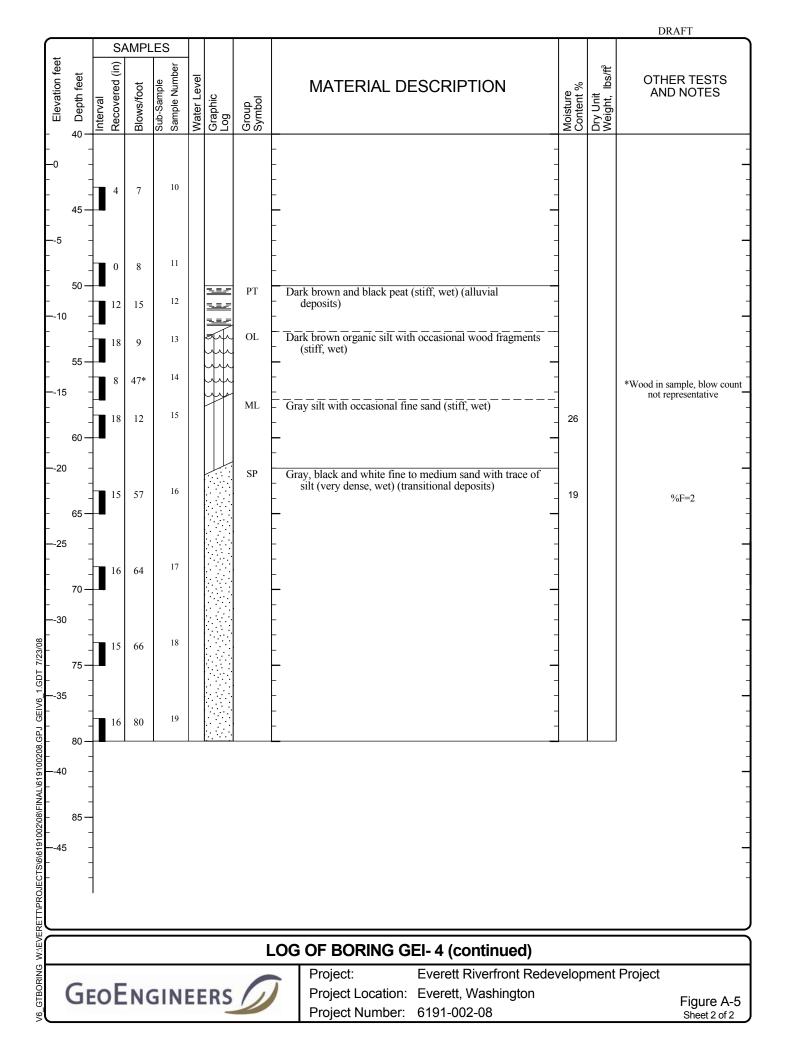


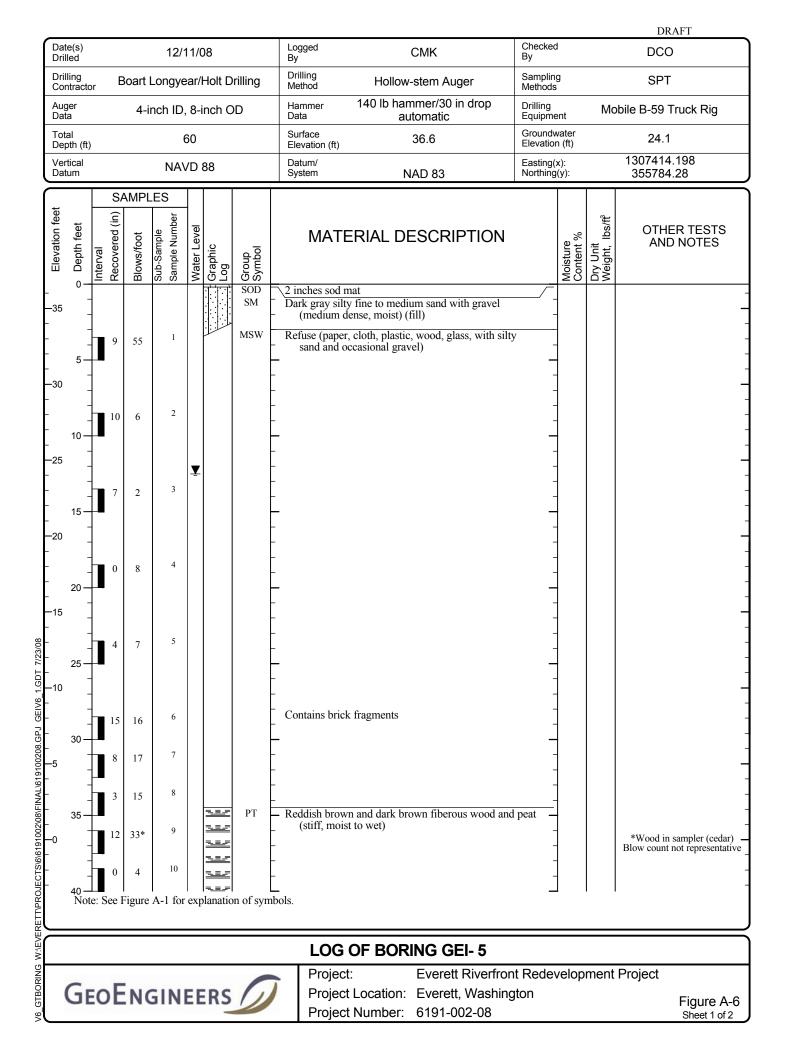




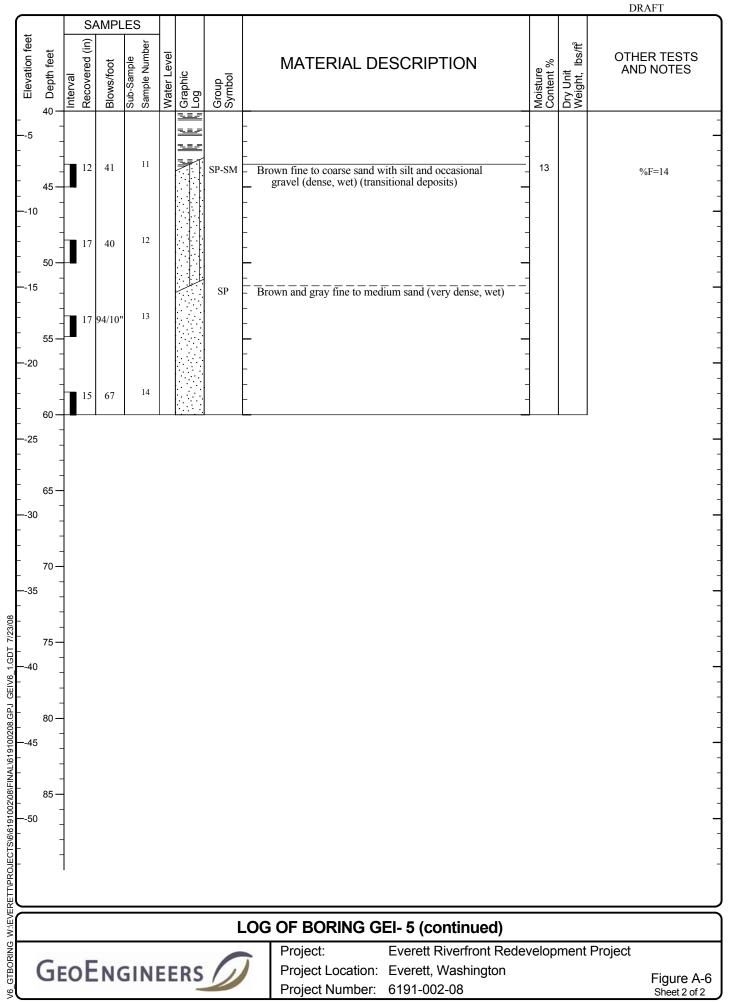


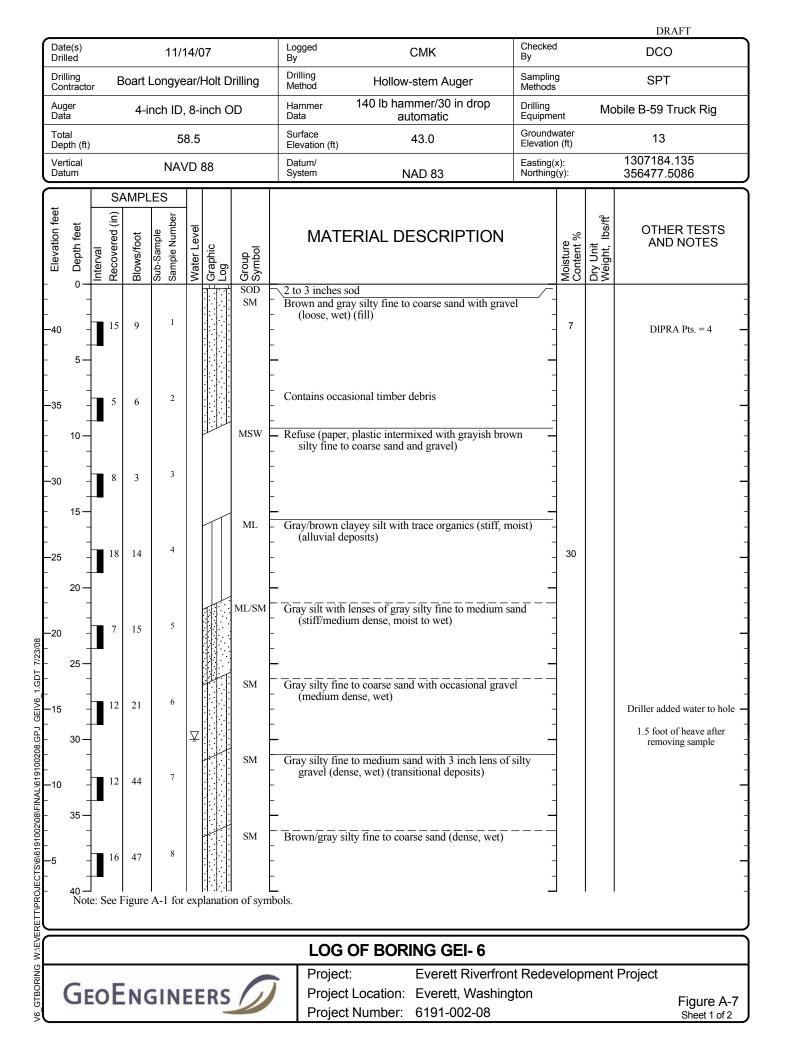


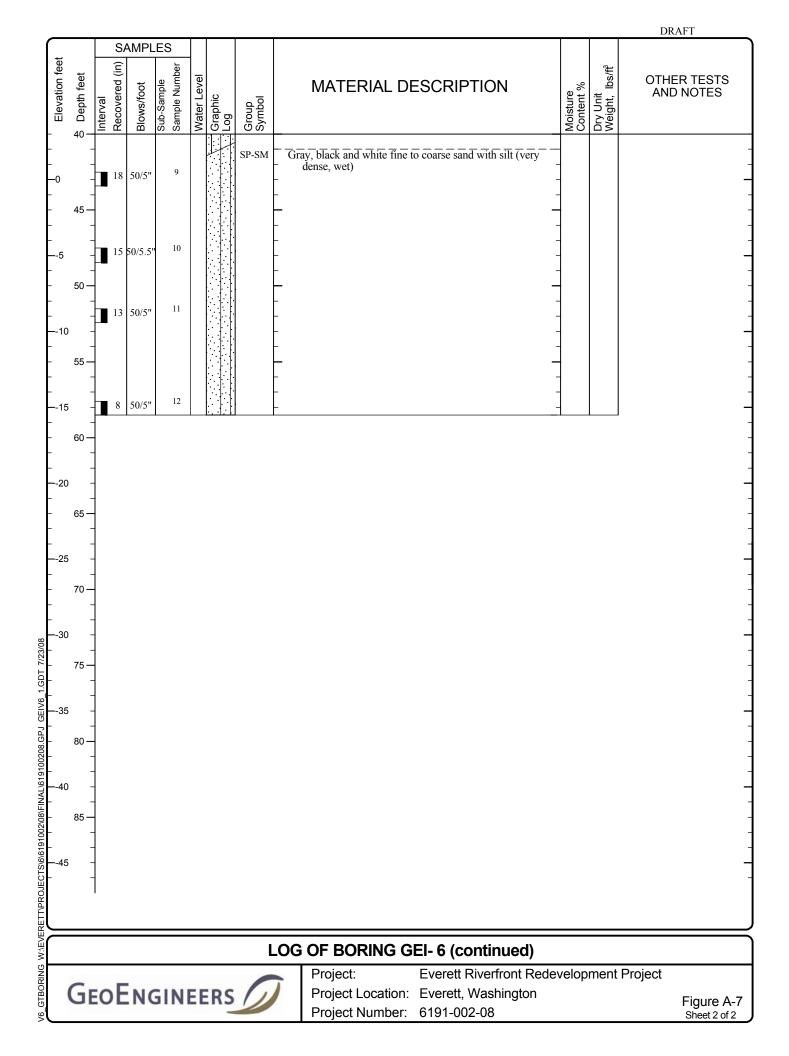


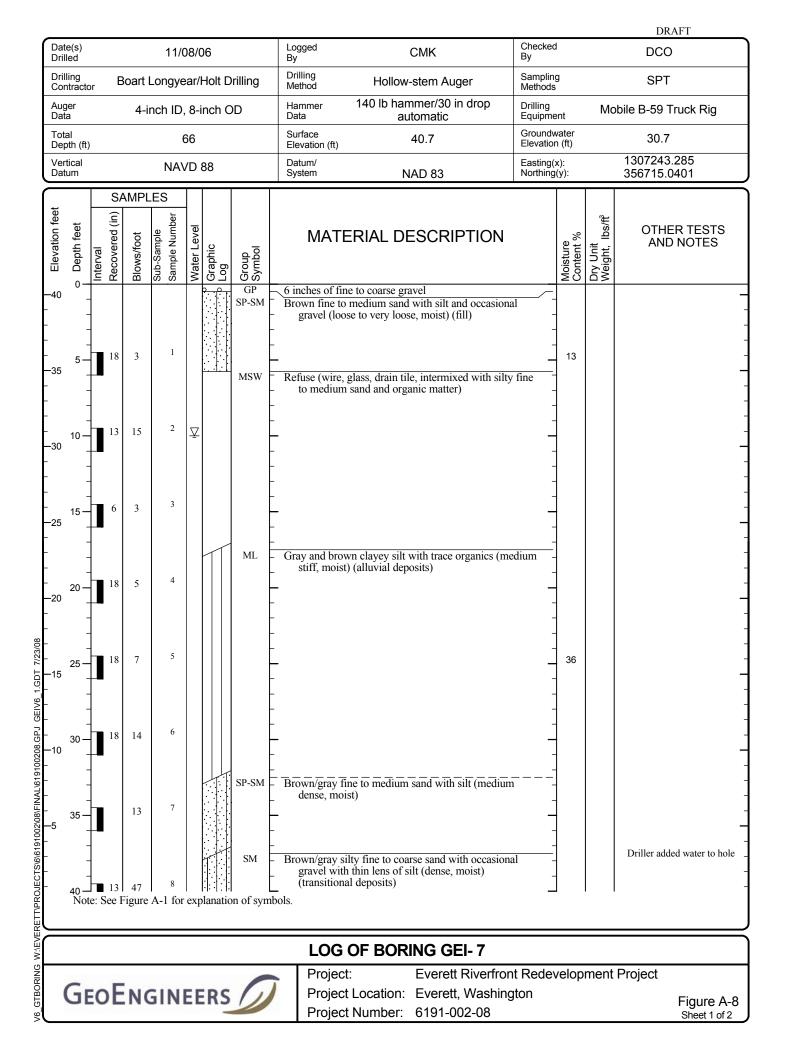




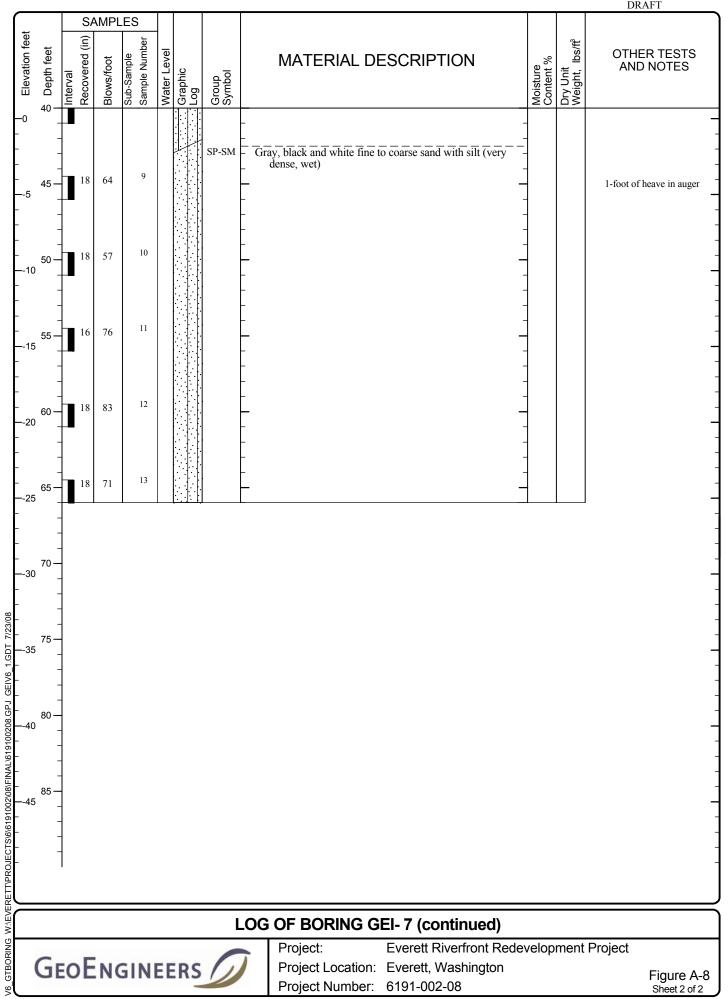


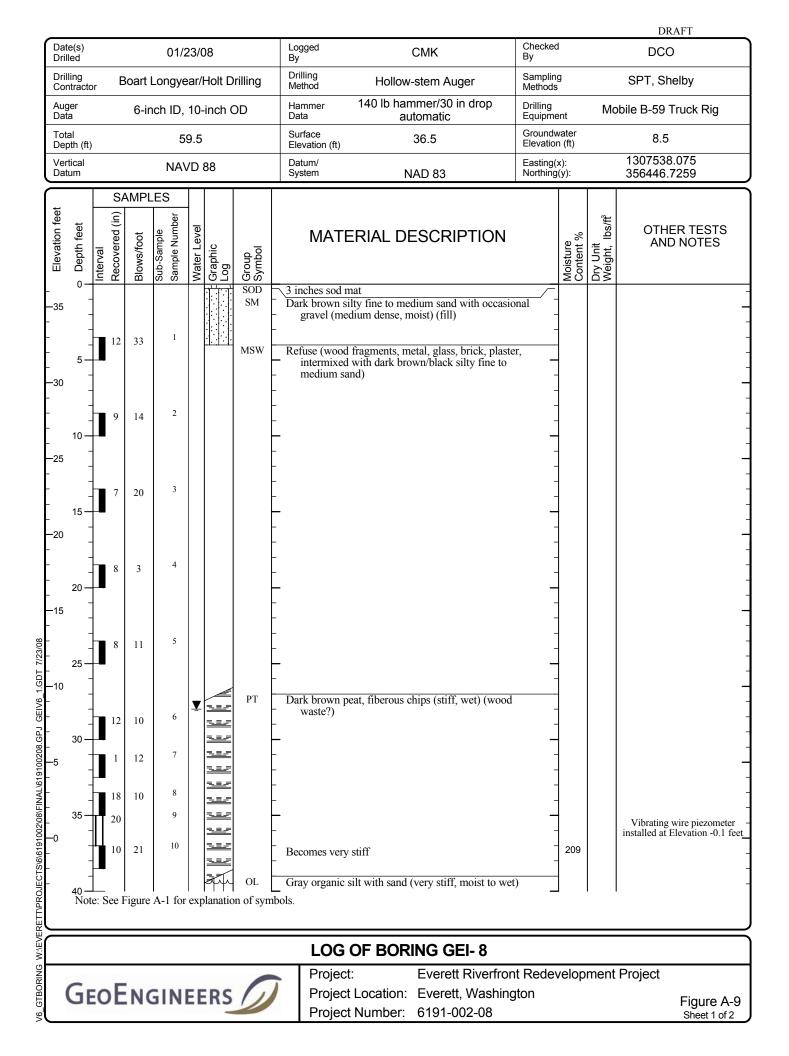


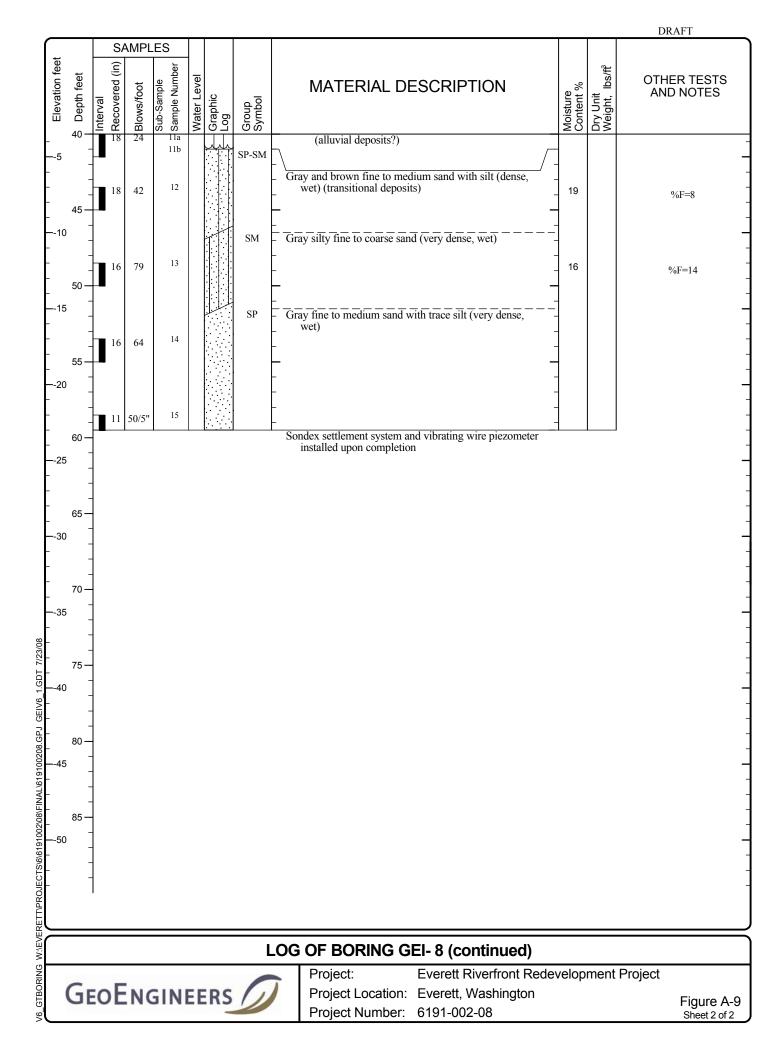


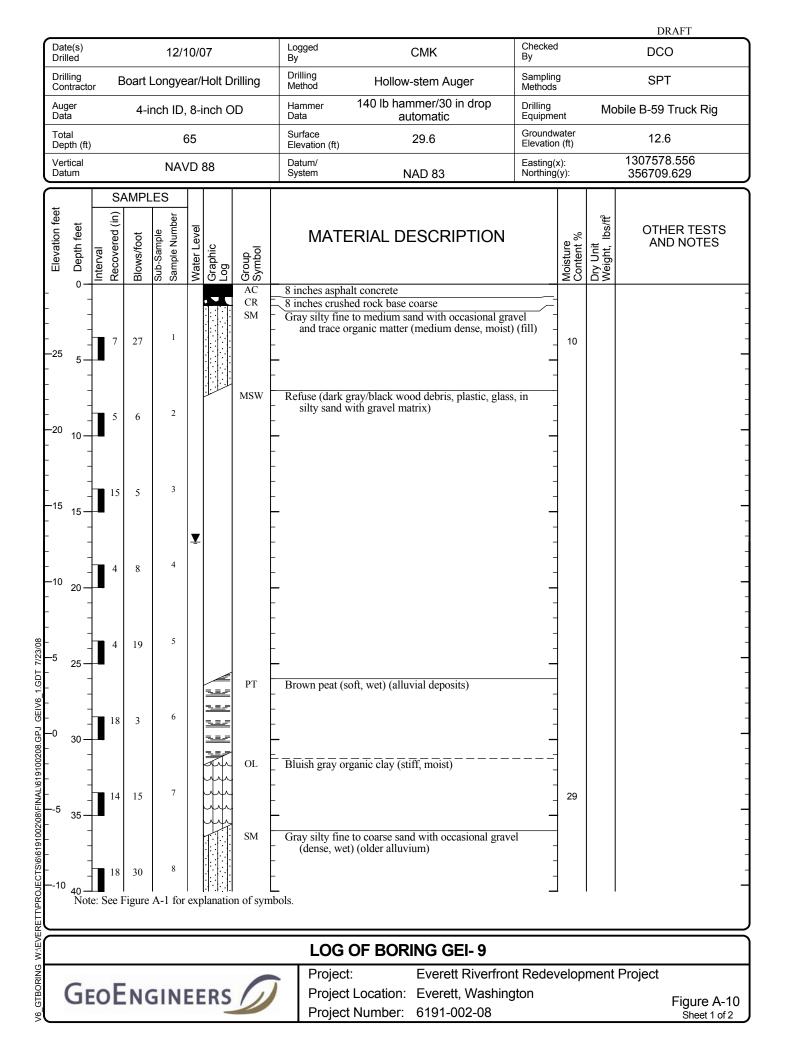


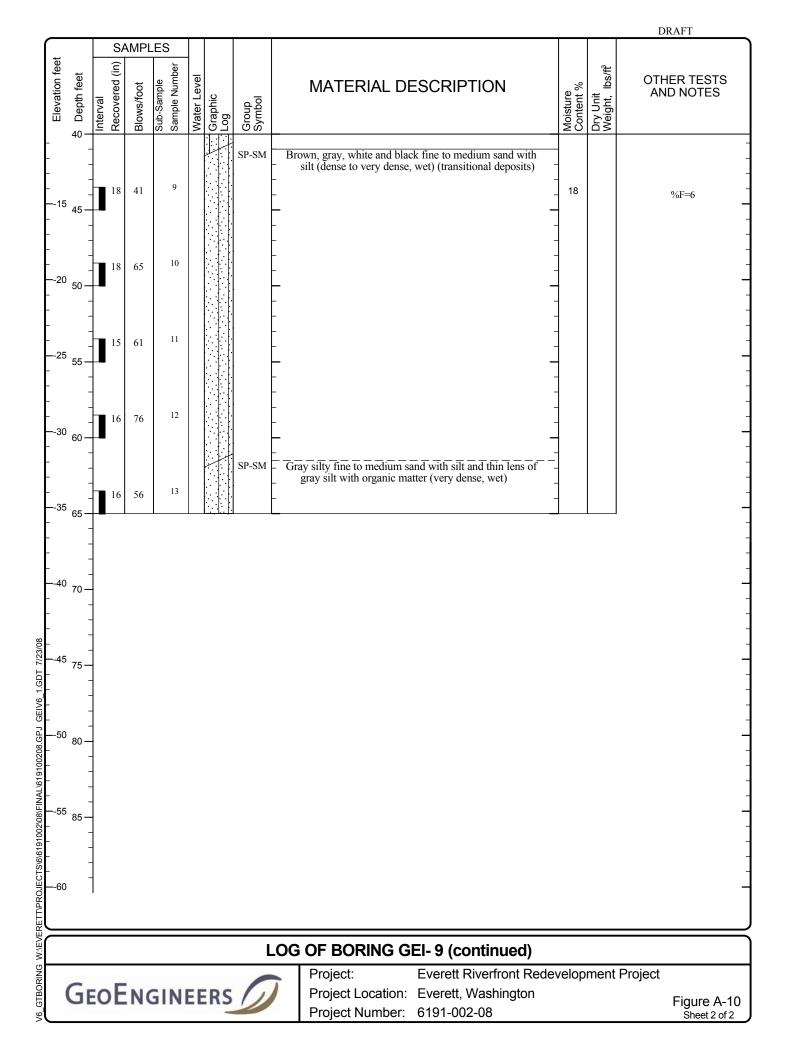
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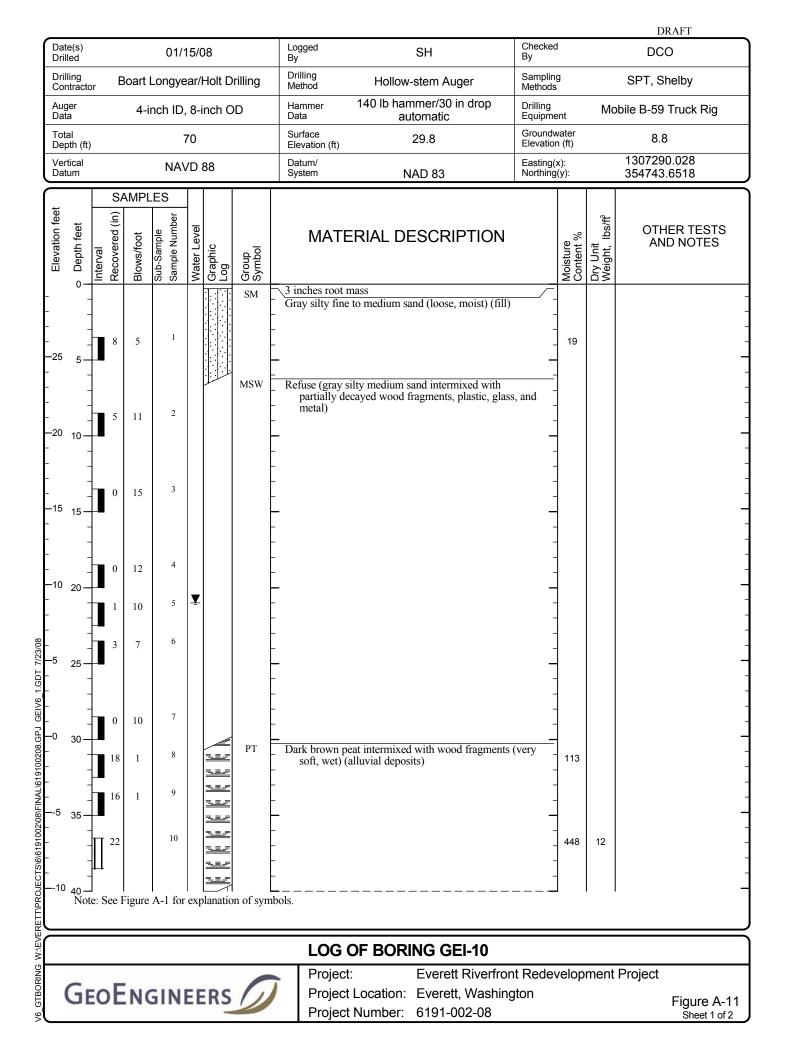




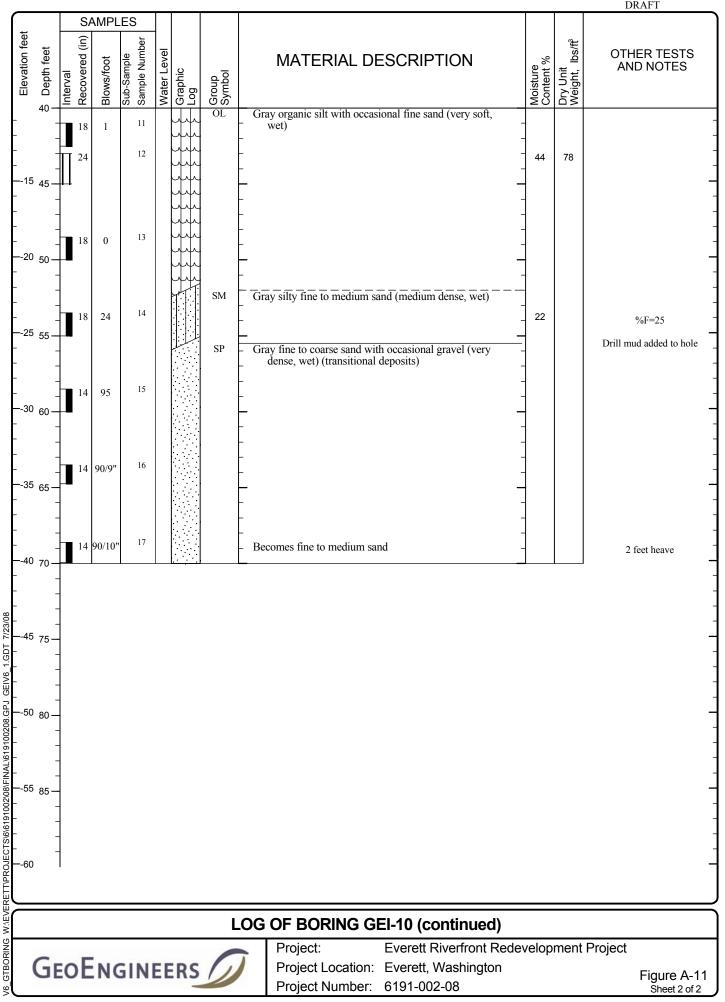


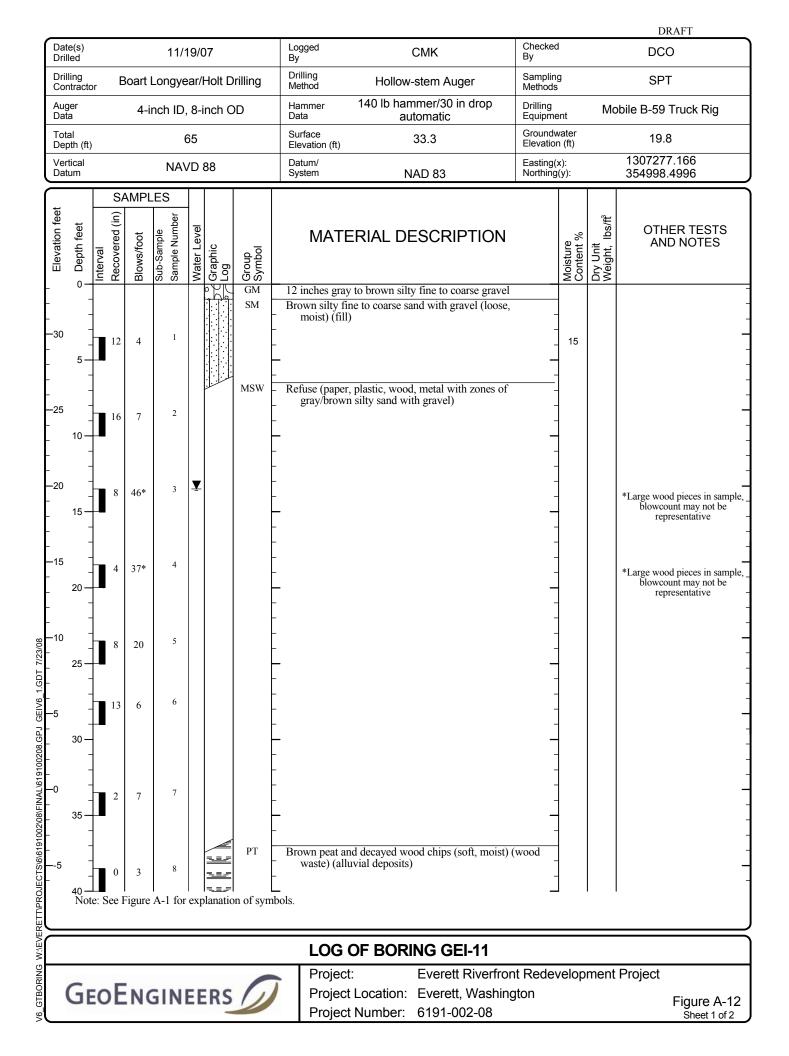




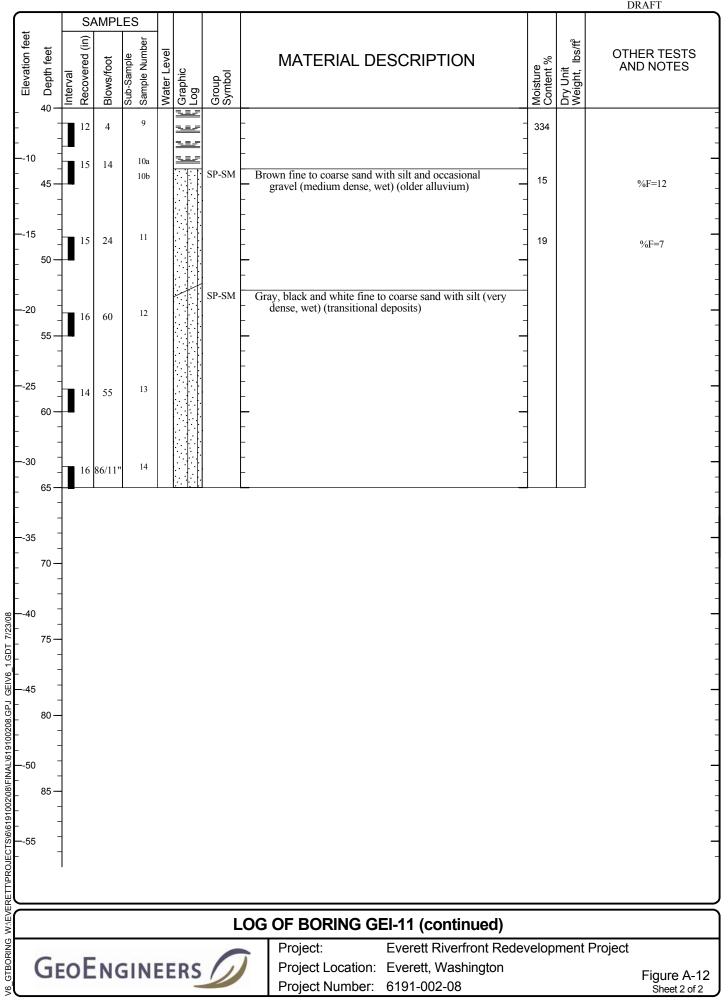


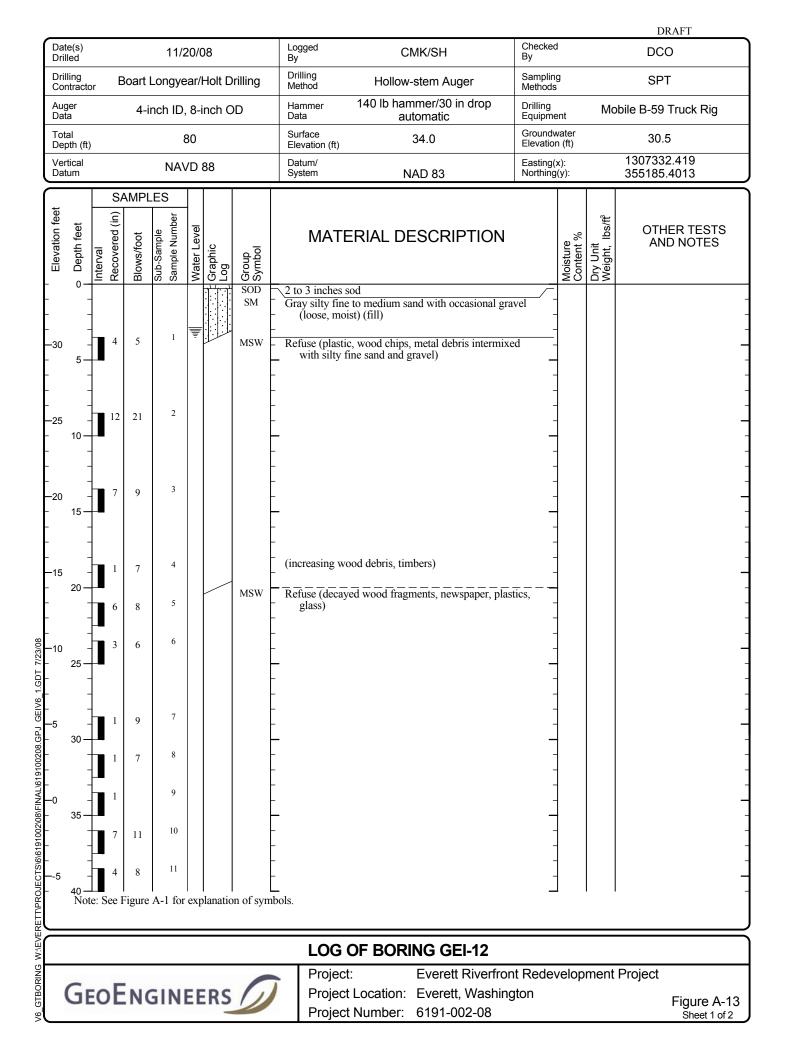


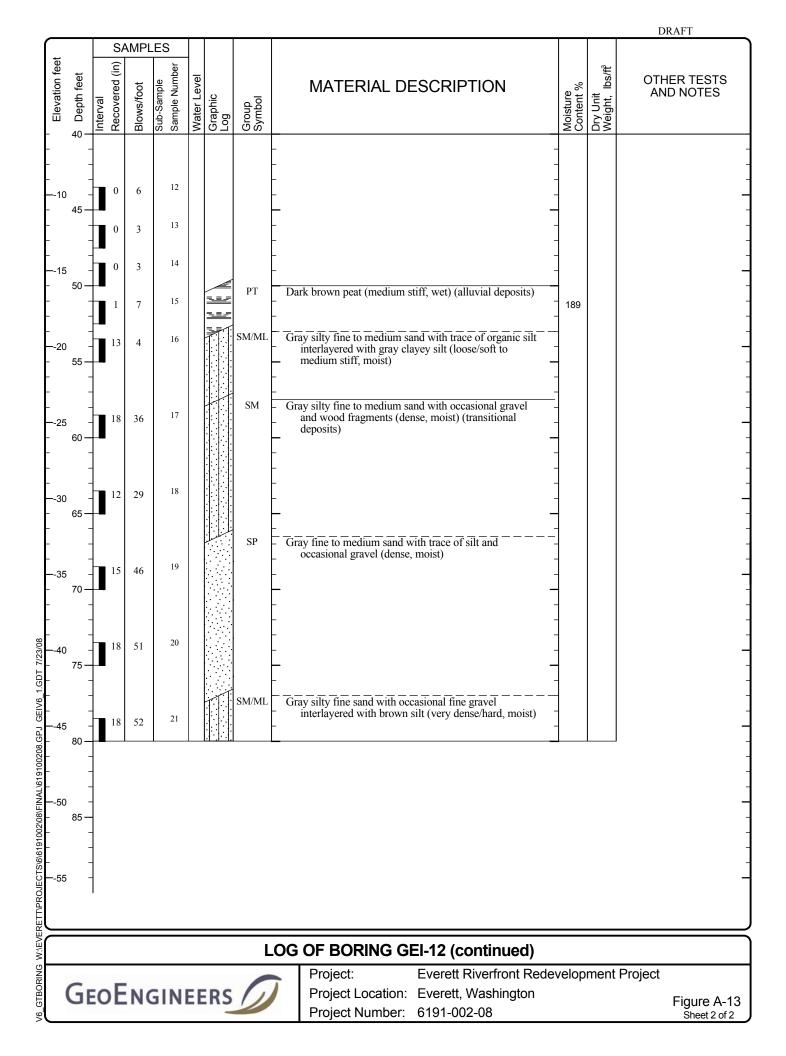


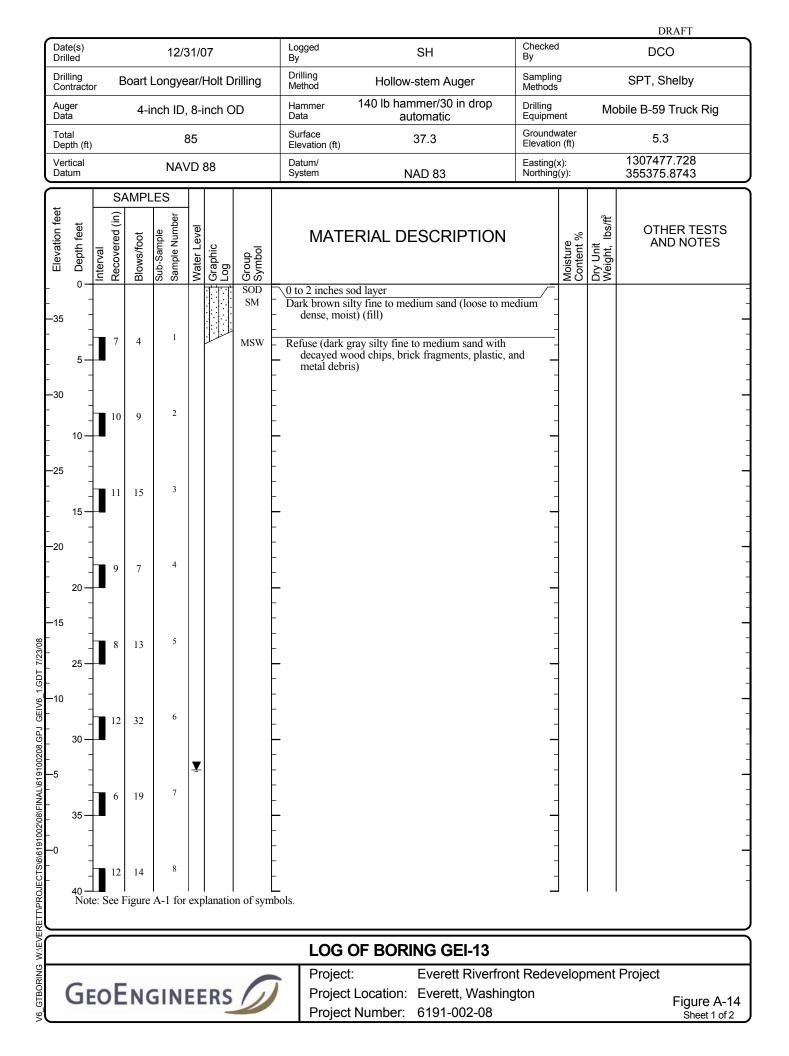


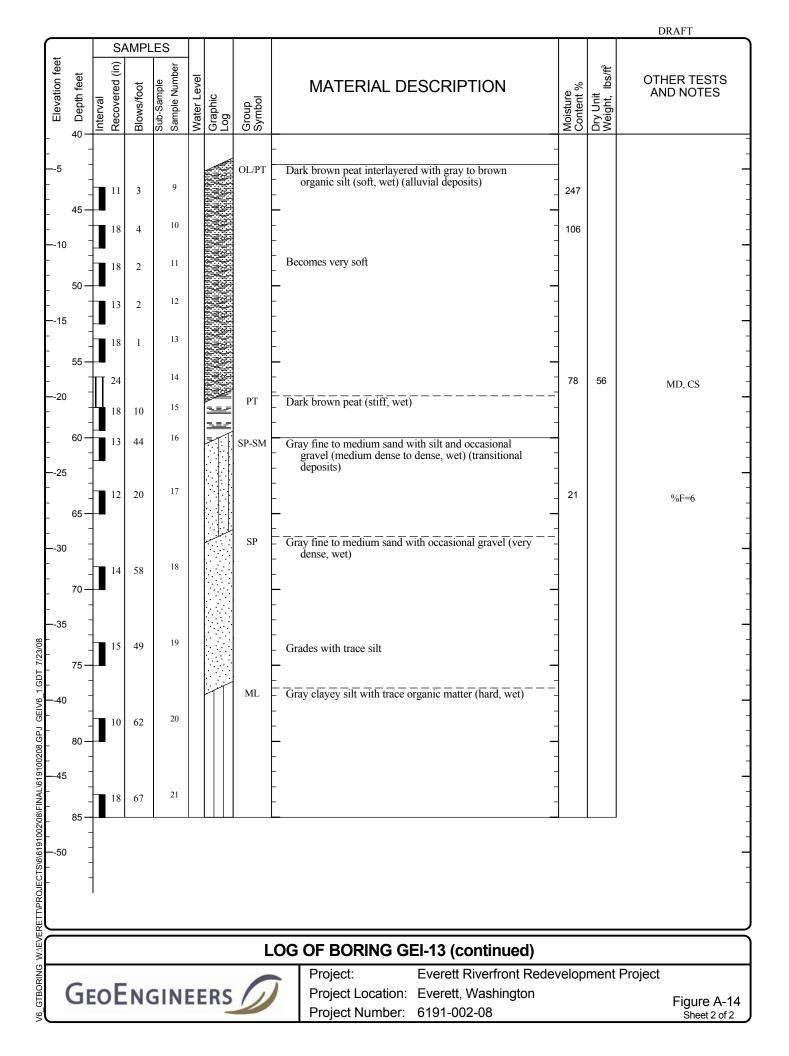
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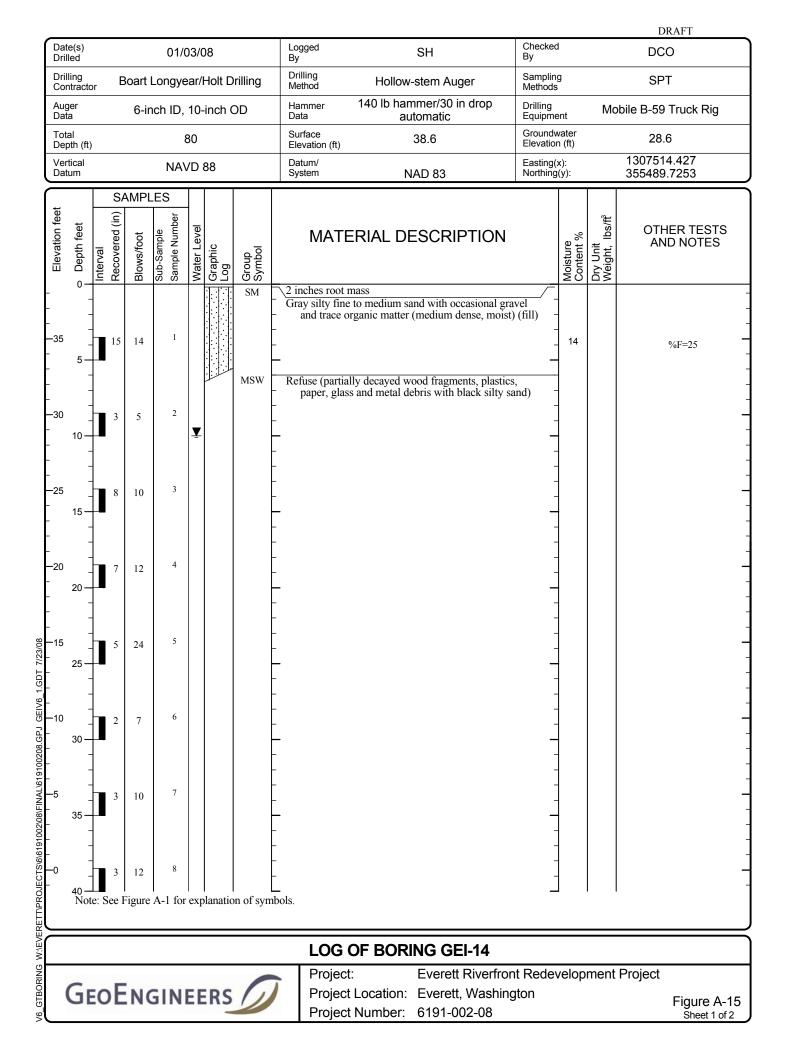


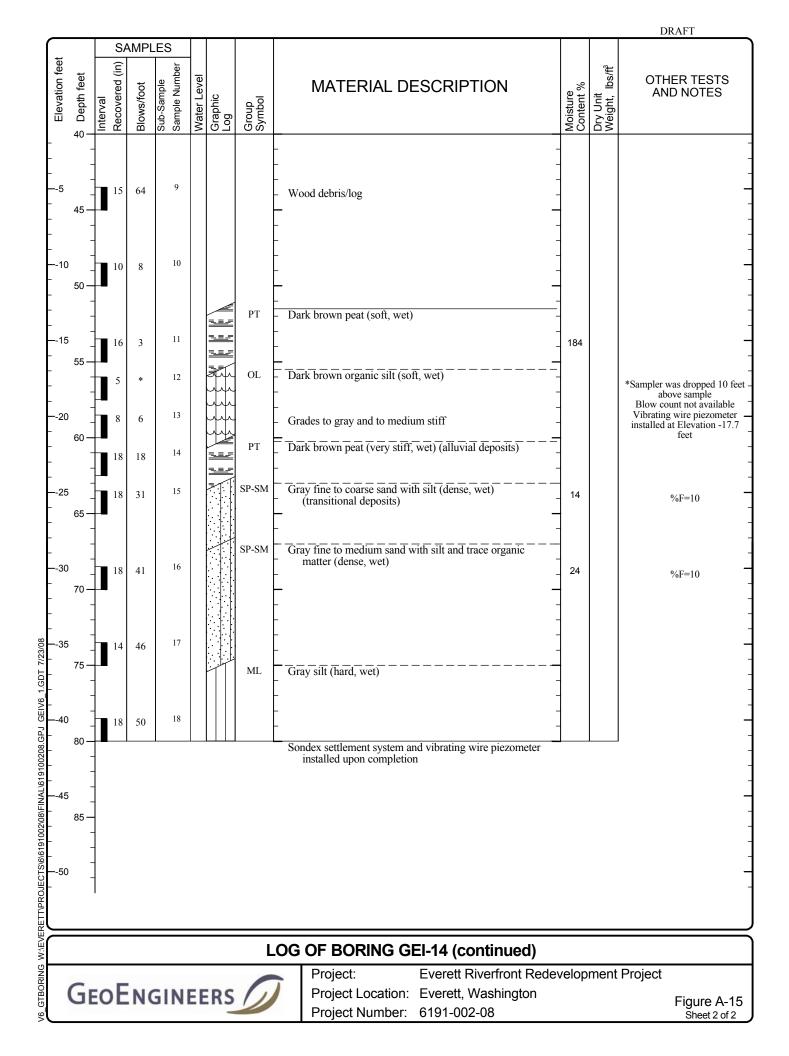


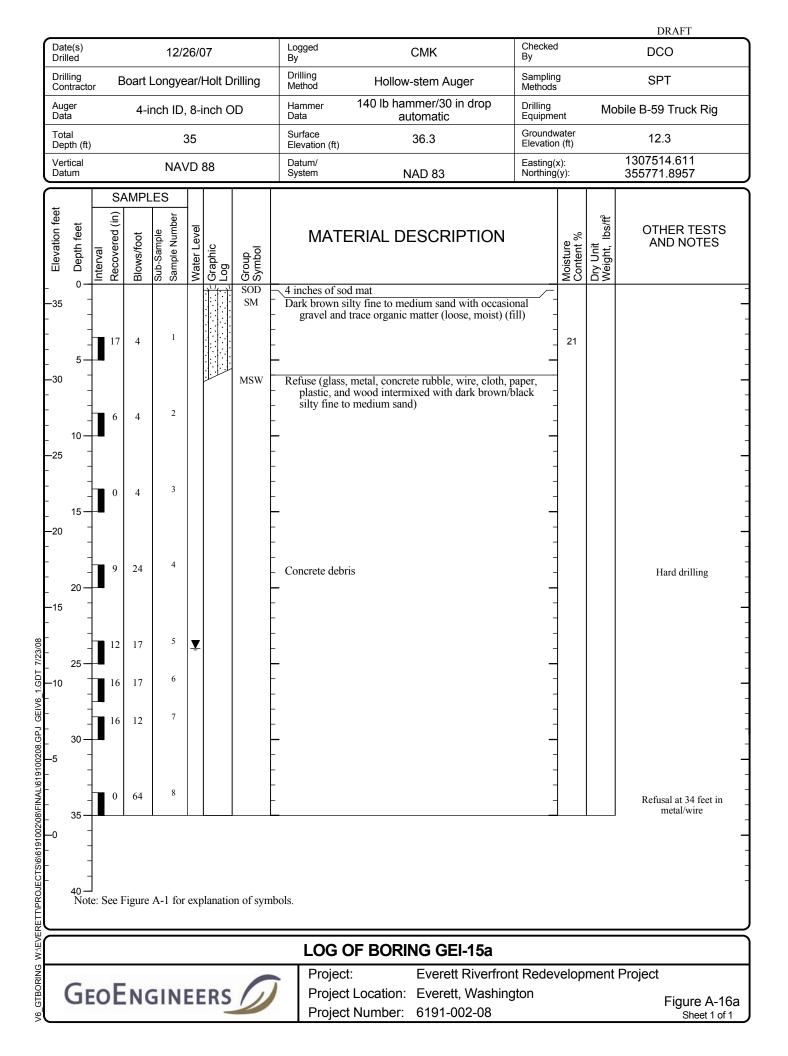


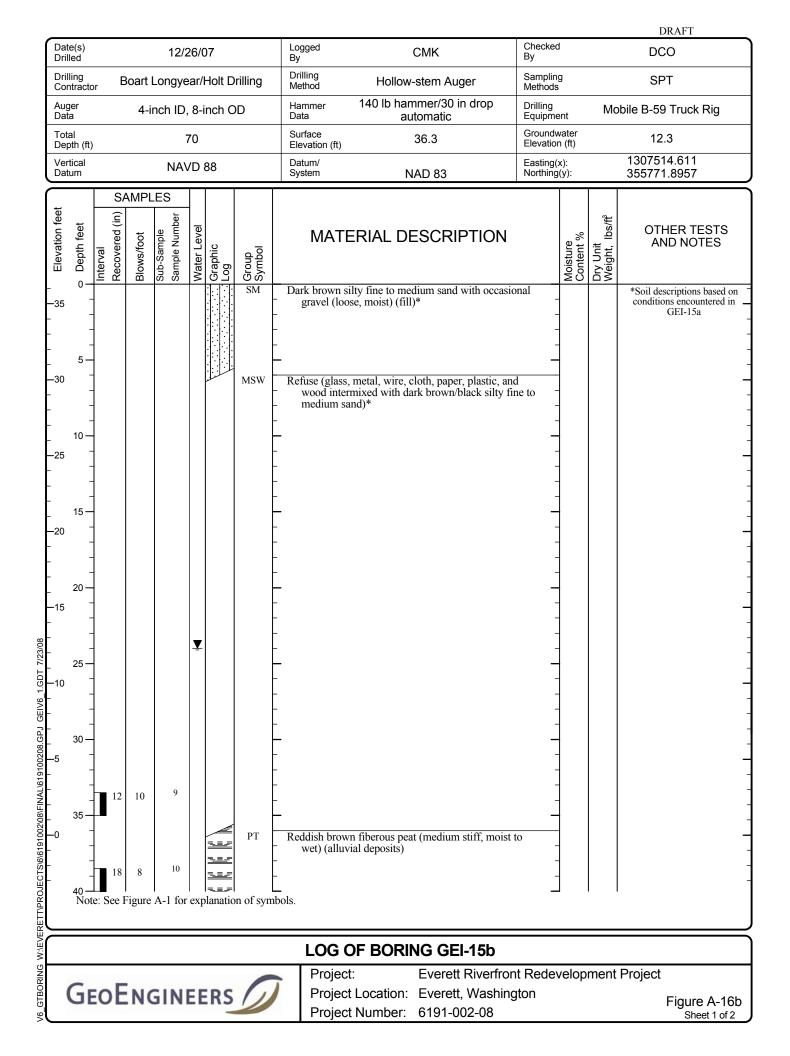


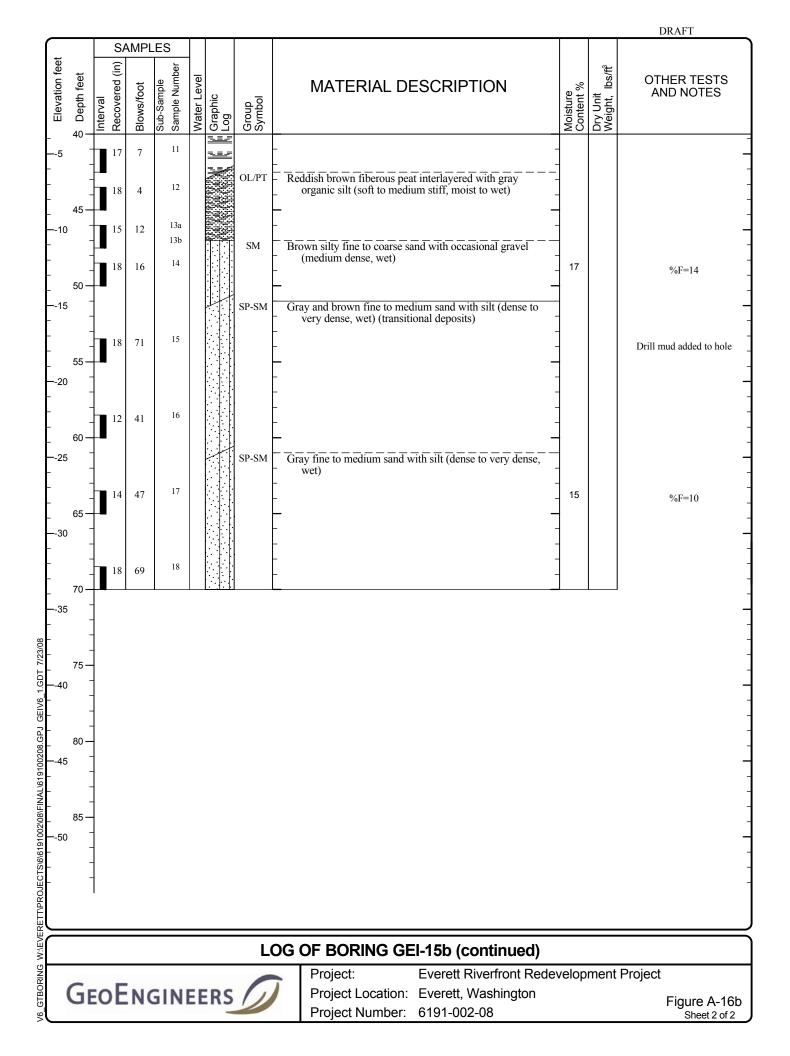


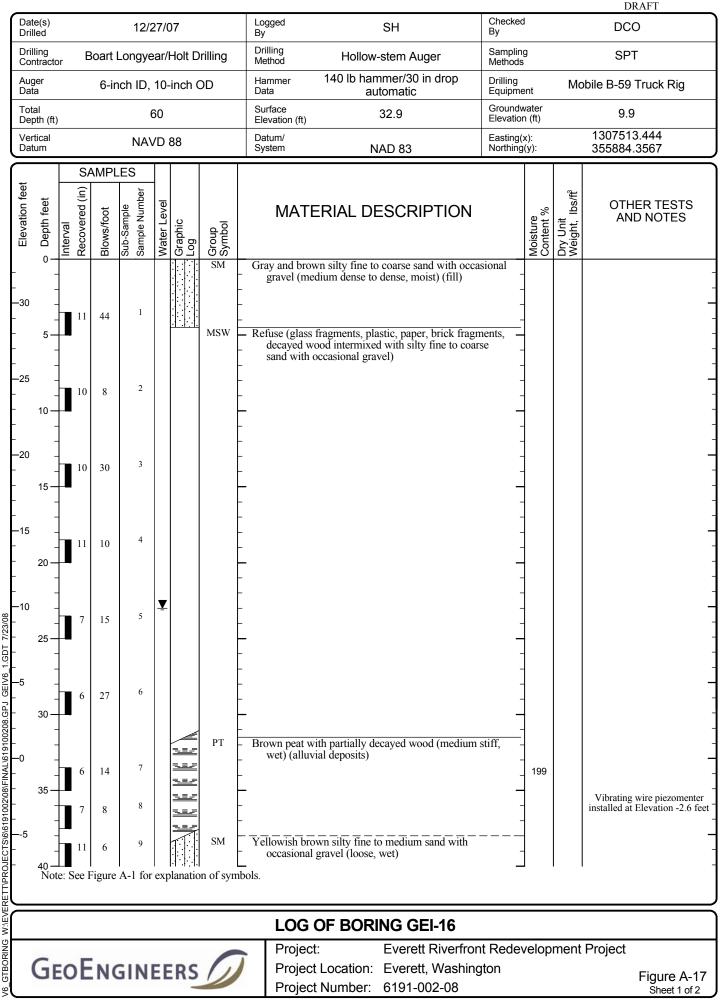




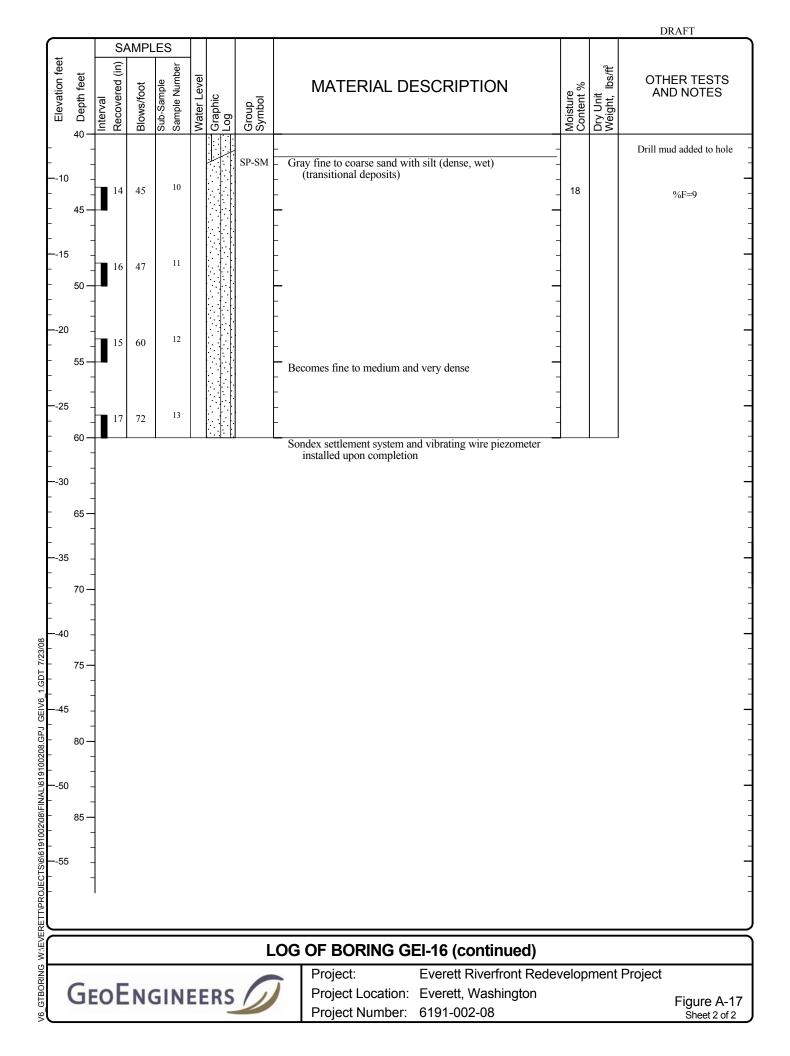


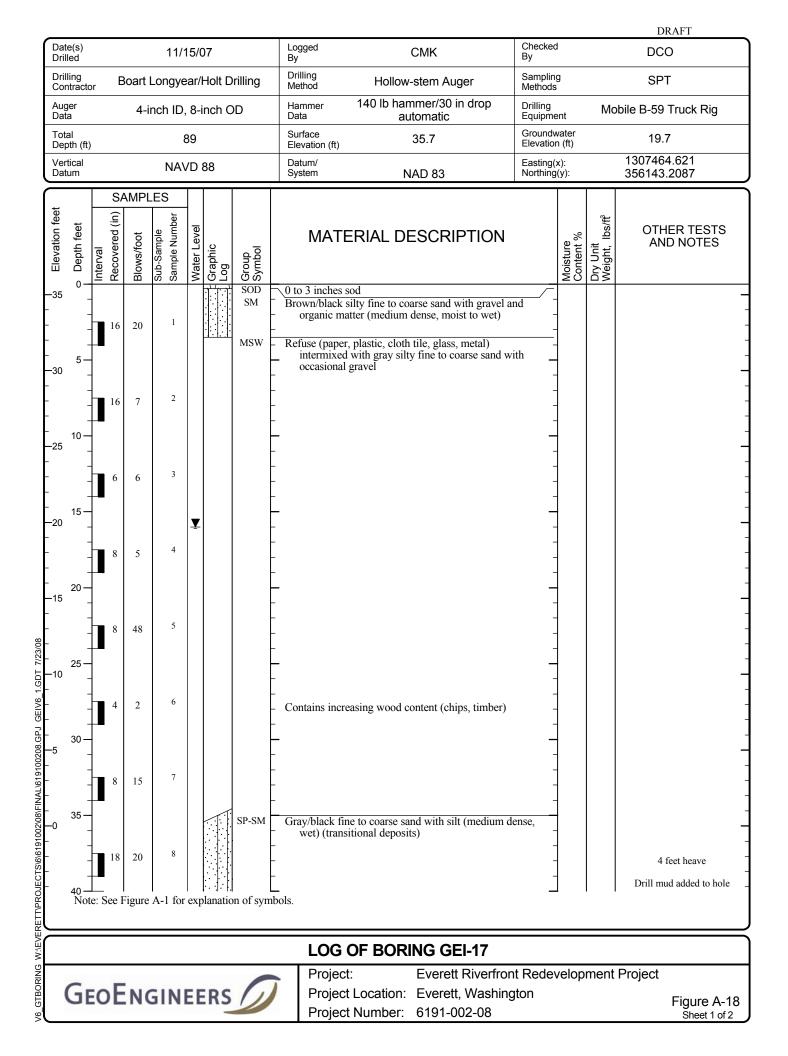




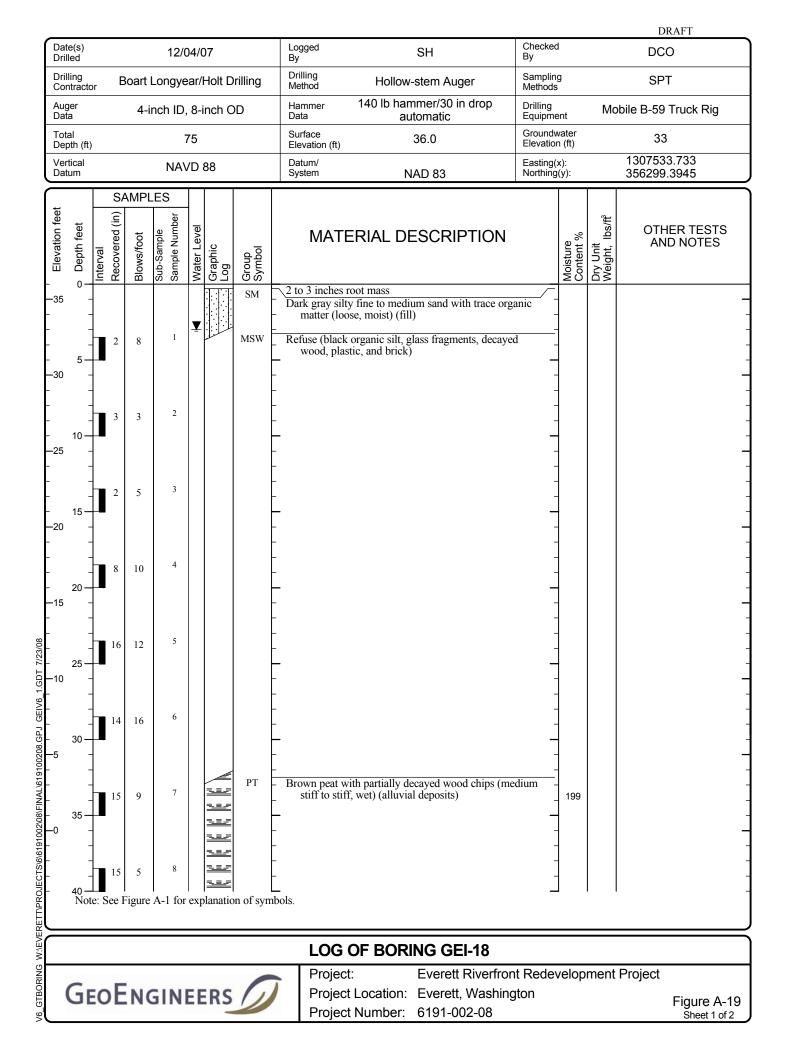


GTBORING 9

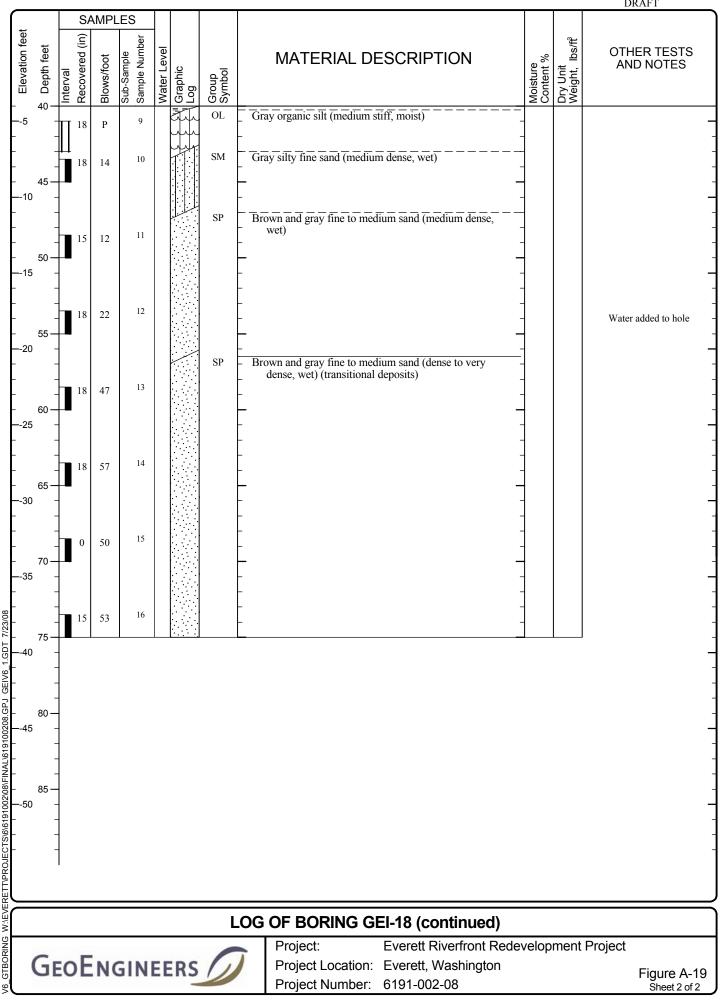


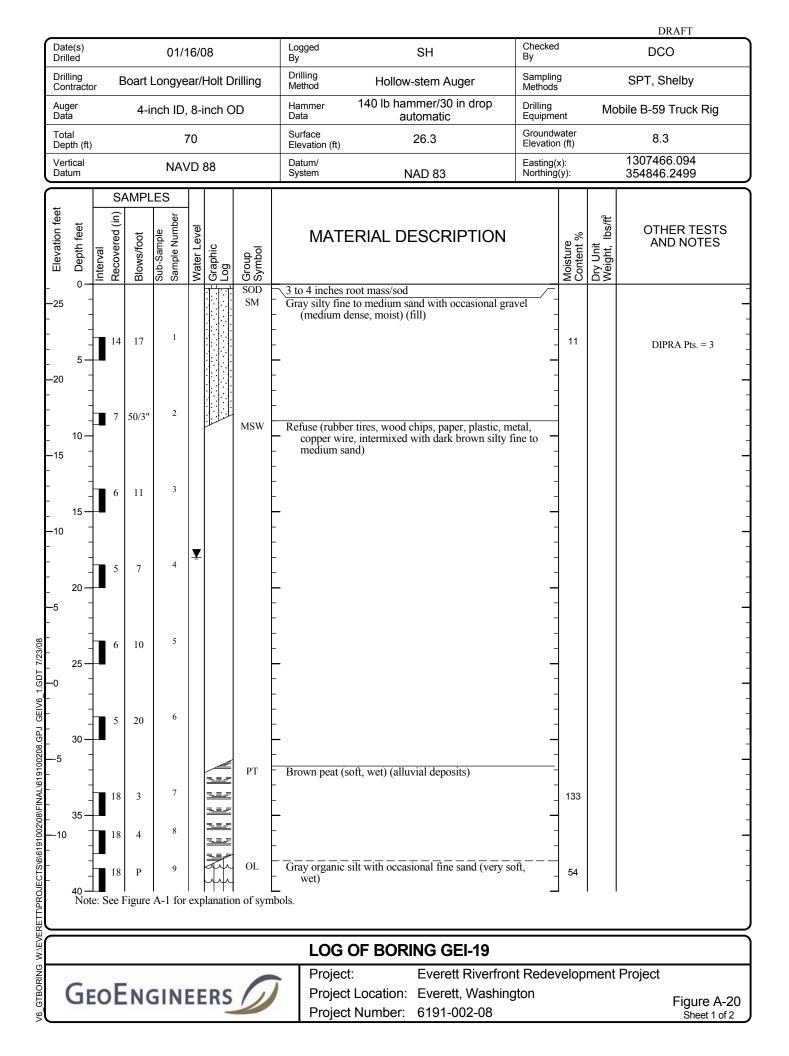


\subset			ç	AMP	IFS				1	[DRAFT
Elevation feet	+		Interval Recovered (in)		Sub-Sample Sample Number	Water Level Graphic Log	Group Symbol	MATERIAL DESCRIPTION	Moisture Content %	Dry Unit Weight, Ibs/ff	OTHER TESTS AND NOTES
5 - -		-			10			- - Grades to very dense	27		%F=6
- - 1		5 — -							-		Water added to hole
-	_	-	1	19	11		· · · · · · · · · · · · · · · · · · ·	-	-		1 foot heave Blowcount not representative -
1 -	5	0 	1 [′]		12				-		
- - 2	5: 20	- - 5 -		, 30				- - 			-
-		-	1	5 71	14			-	-		-
- 2	6 25	0	1	5 70	15		· • •		-		
- - 3	6: 50	- - 5 -						- - 	-		-
- - -		-	1	5 42	16a 16b		ОН	- - Brown gray organic clay (hard, moist)	33 27		- AL -
7/23/08	5 5 7	-		3 52	17		SP-SM	Gray fine to medium sand with silt (very dense, wet)	20		
GPJ GEIV6_1.GDT	0 8		1	3 73	18		ML	- - - Gray silt with fine sand (hard, moist)	-		
INAL/619100208	5	,	1	3 37	19		SP	Gray fine sand (dense, wet)	-		
	8 50	5 — - -	1	583/11.	5, 20		SP-SM	Gray fine to medium sand with silt (very dense, wet)	-		- - - - -
NG W:							L	OG OF BORING GEI-17 (continued) Project: Everett Riverfront Rede	velor	ment	Project
V6_GTBORING	0	ÈE	0	EN	GIN	EERS	0	Project Location: Everett, Washington Project Number: 6191-002-08		ment	Figure A-18 Sheet 2 of 2

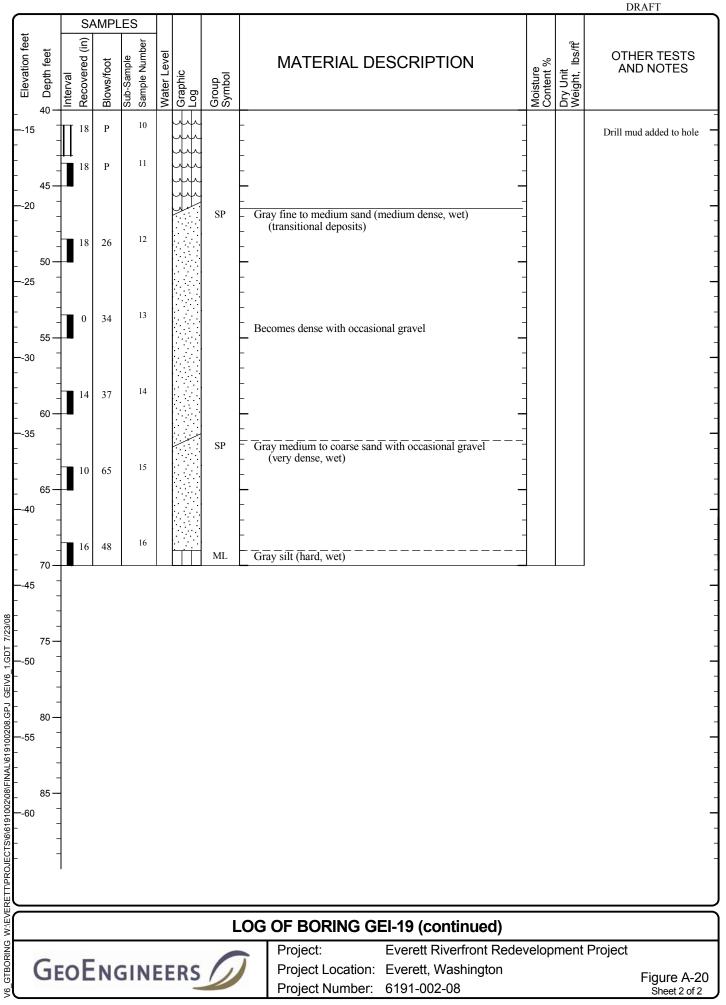


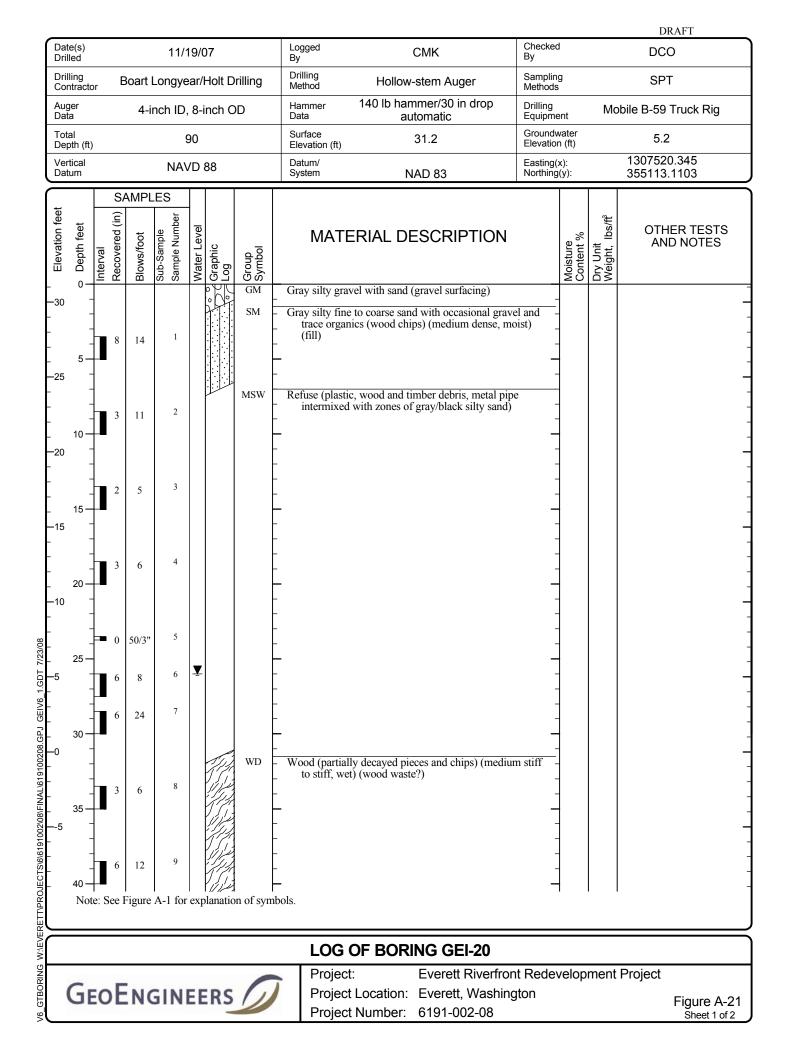
DRAFT



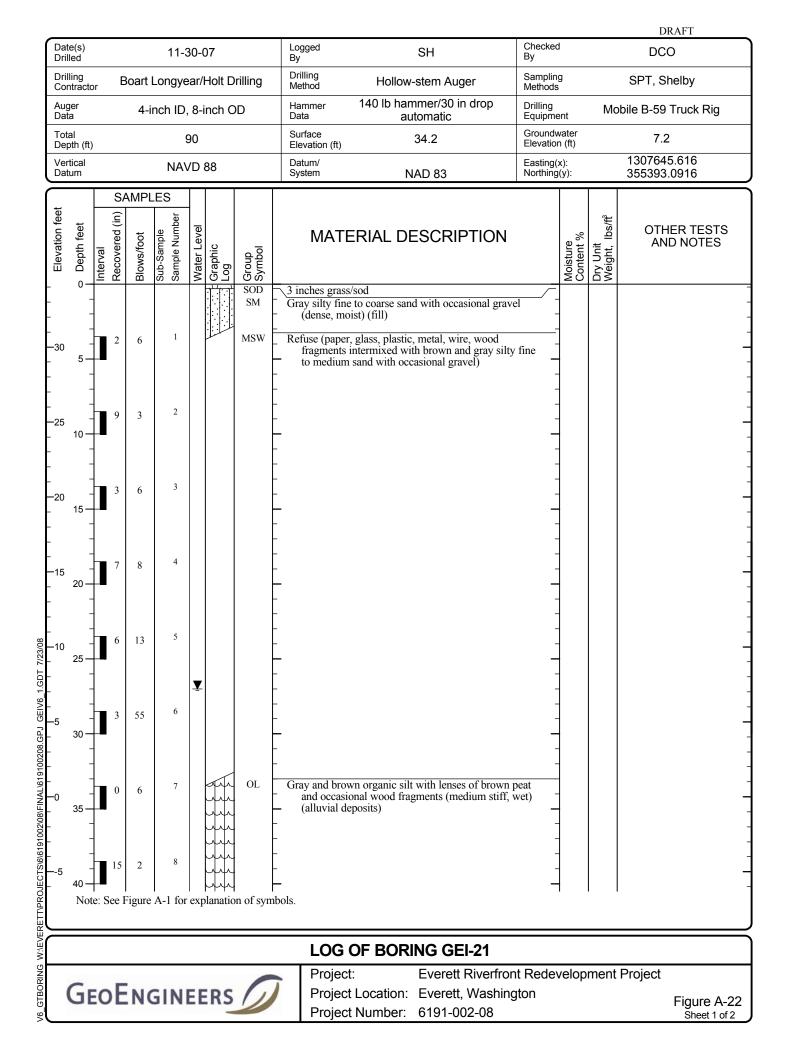




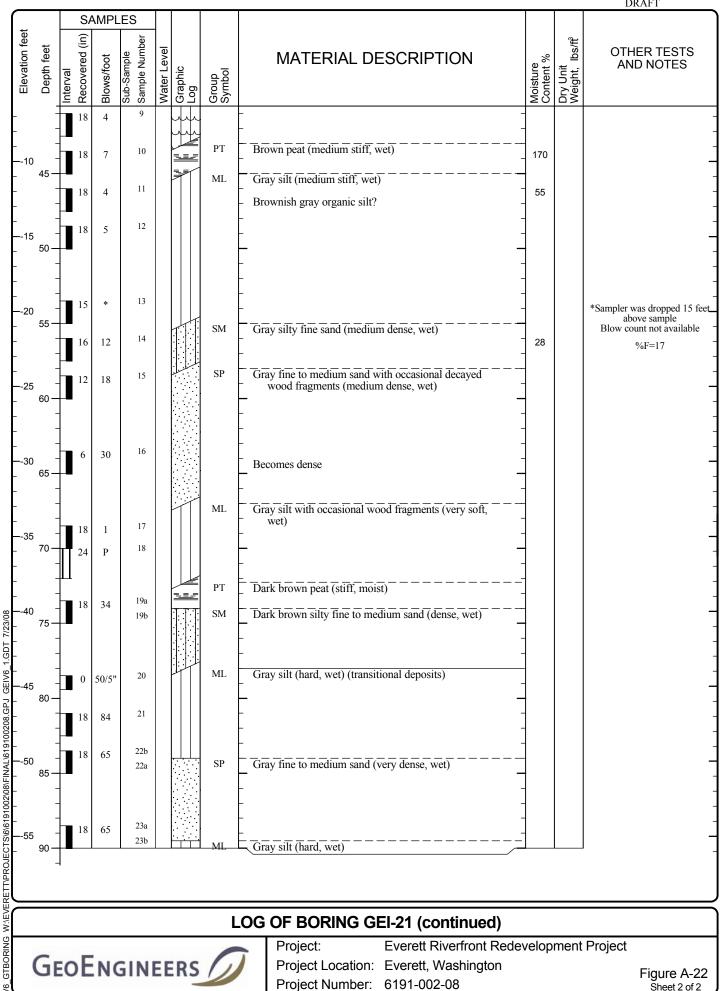


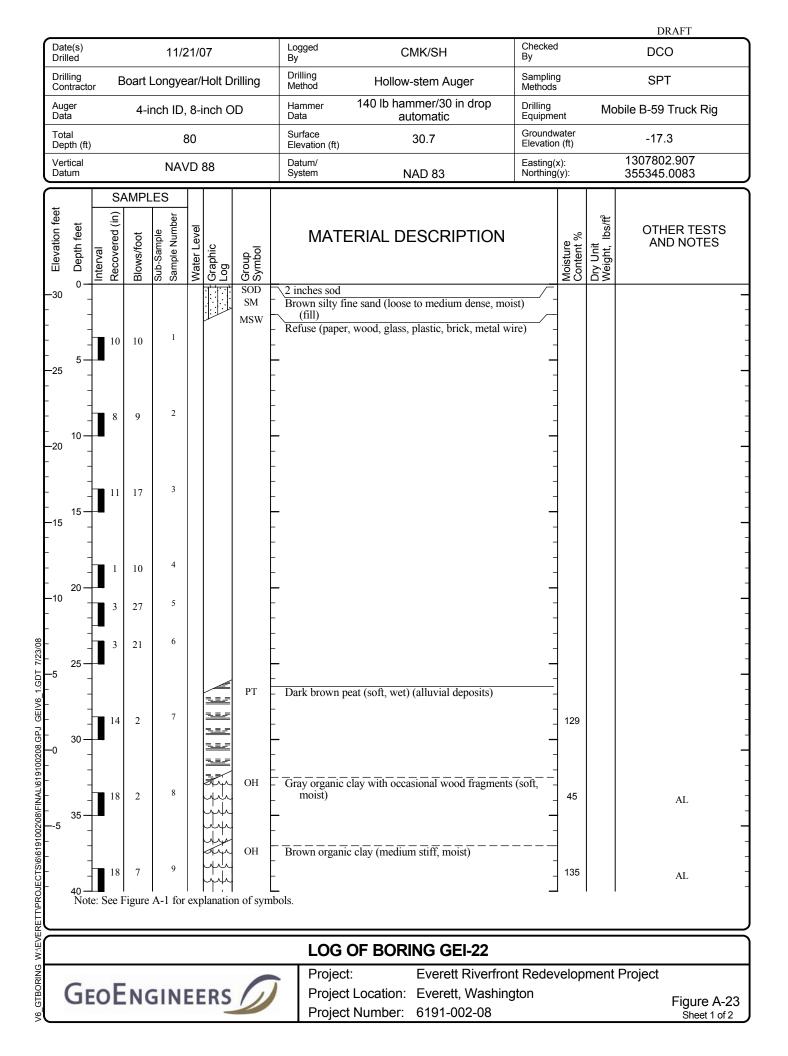


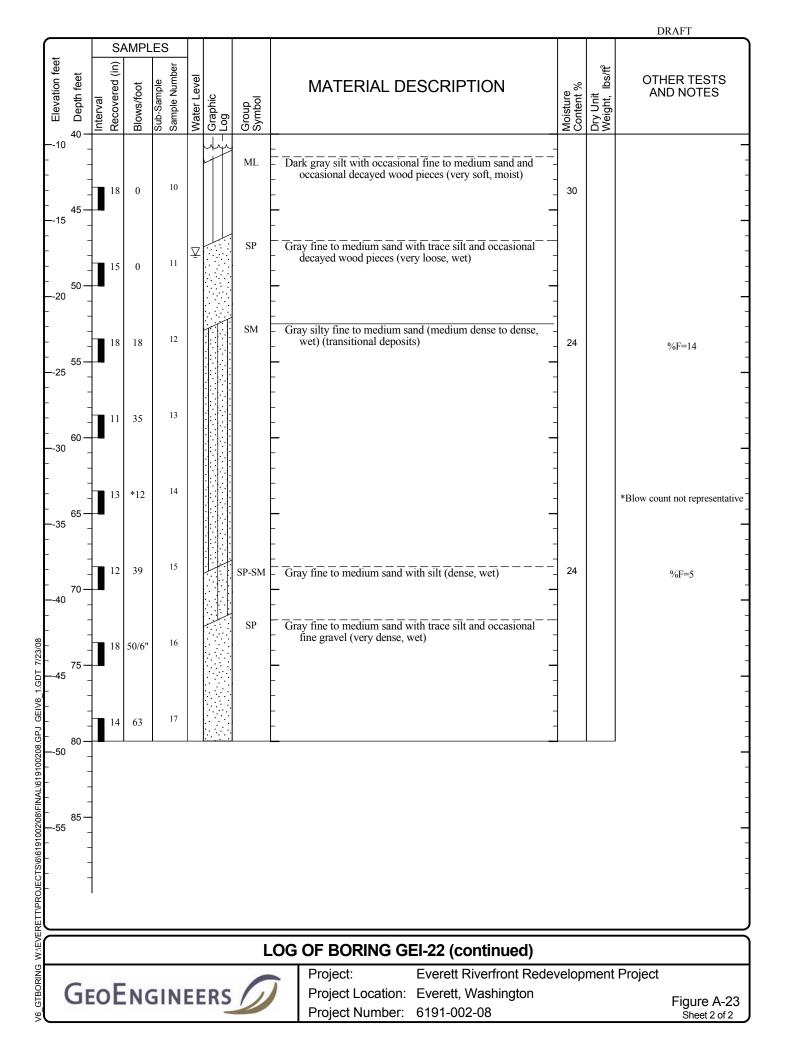
		9	AMP	IES	Τ						DRAFT
Elevation feet Depth feet		Interval Recovered (in)	-	Sub-Sample Sample Number	Water Level	Graphic Log	Group Symbol	MATERIAL DESCRIPTION	Moisture Content %	Dry Unit Weight, Ibs/ff	OTHER TESTS AND NOTES
10 - - - - - - - - - - - - - - - - - -		<pre>0 18 18 0 0 10 12 12 12 11 11 11 11 11 11 11 11 11 11</pre>	2 16 25 37	10 11 12 13 14 15 16			OL SP-SM	Gray organic silt with occasional wood chips (very soft to soft, wet) (alluvial deposits) Gray fine to medium sand with silt (medium dense to dense, wet) (transitional deposits)	46		%F=9
35 - - - 70 40 -		12		17a 17b 18			OL SP/SM	Gray organic silt interlayered with brown peat (medium stiff to stiff, moist to wet) Gray fine to coarse sand with silt, with layers of silty fine sand (dense, wet)	23		-
45 - - - 80 50 -		■ 18 ■ 16		19 20			SM	- - - - - Gray silty fine to medium sand (very dense, wet) -	17		%F=18
- 75 45 - 80 50 - 85 - 85 55 - 90			59	21							-
								OG OF BORING GEI-20 (continued)			
	E	o	ĒN	GIN	EE	RS	0	Project: Everett Riverfront Rede Project Location: Everett, Washington Project Number: 6191-002-08	velop	ment I	Project Figure A-21 Sheet 2 of 2

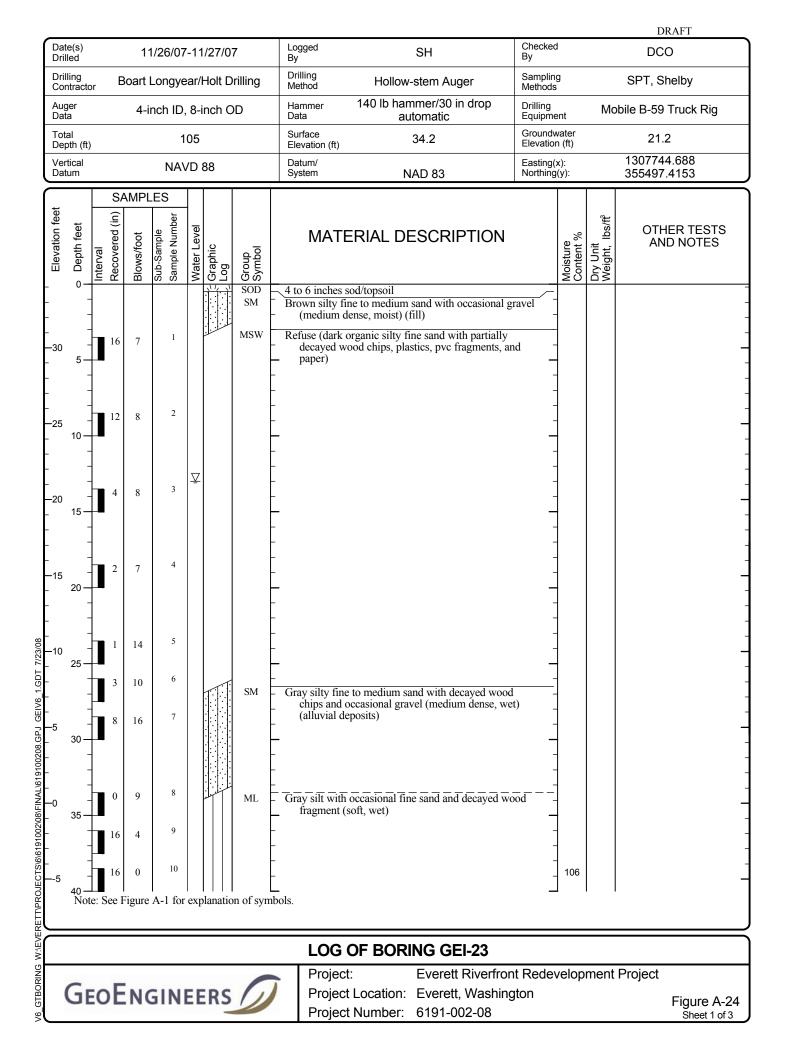


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DIC	Π.	τ.	1





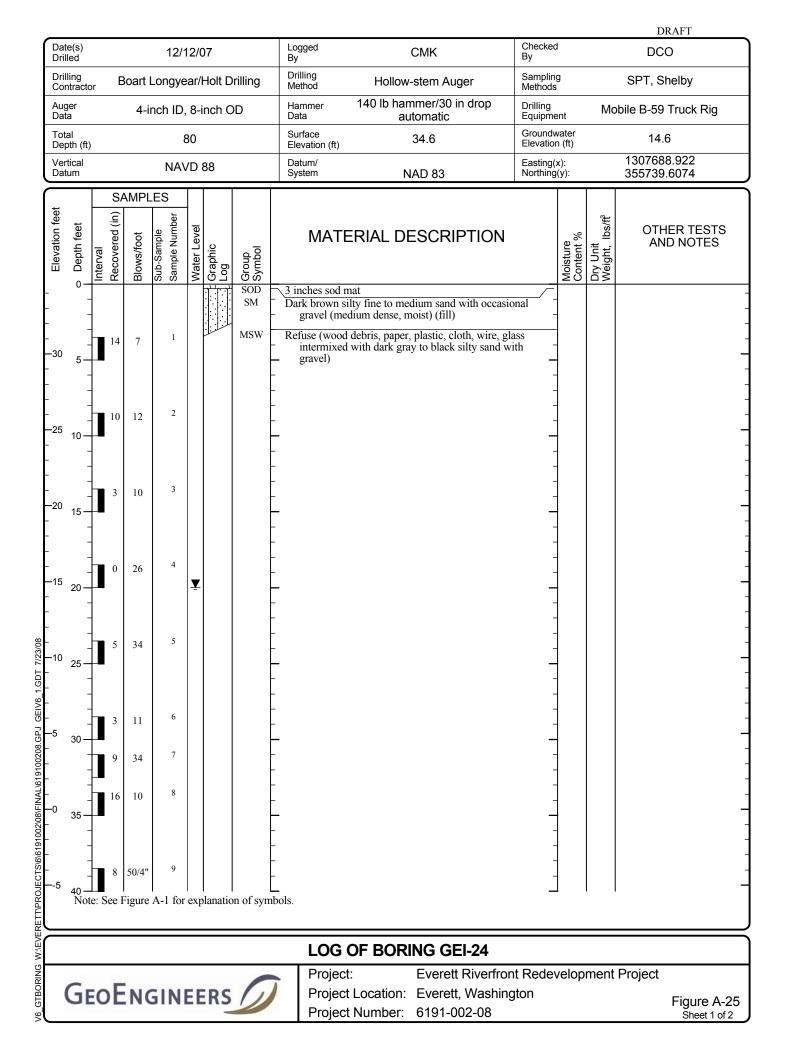


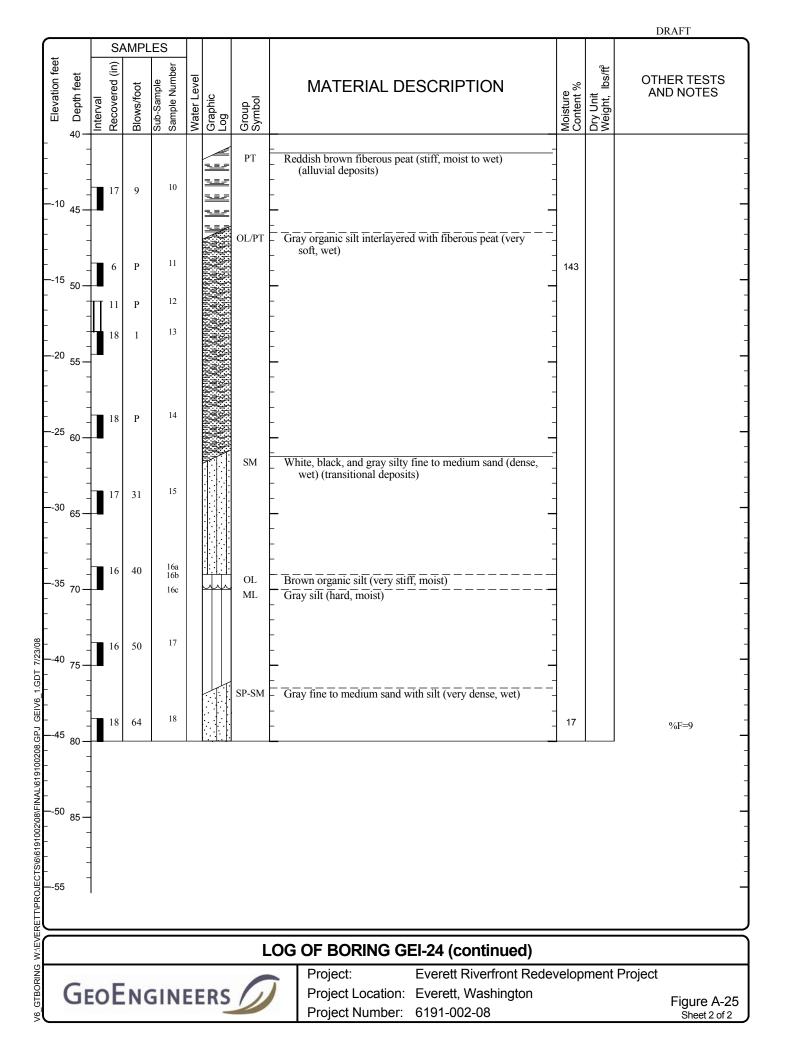


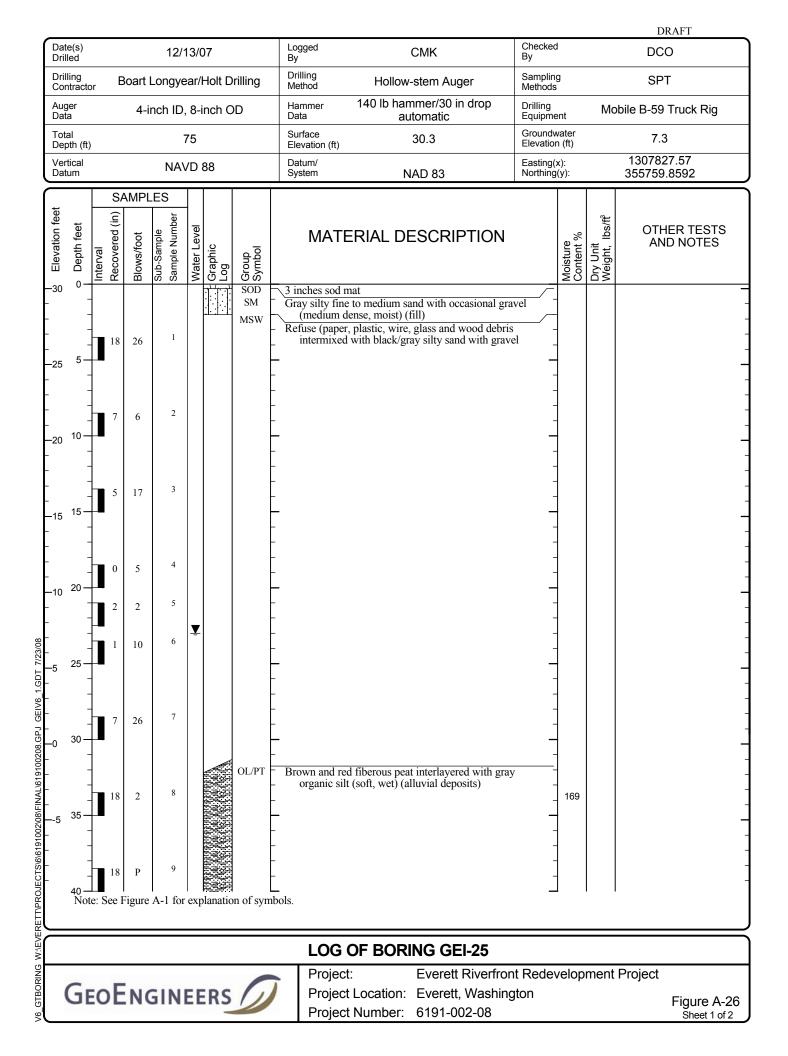
DD	۸	D	т
$D\Lambda$	73	л.	1

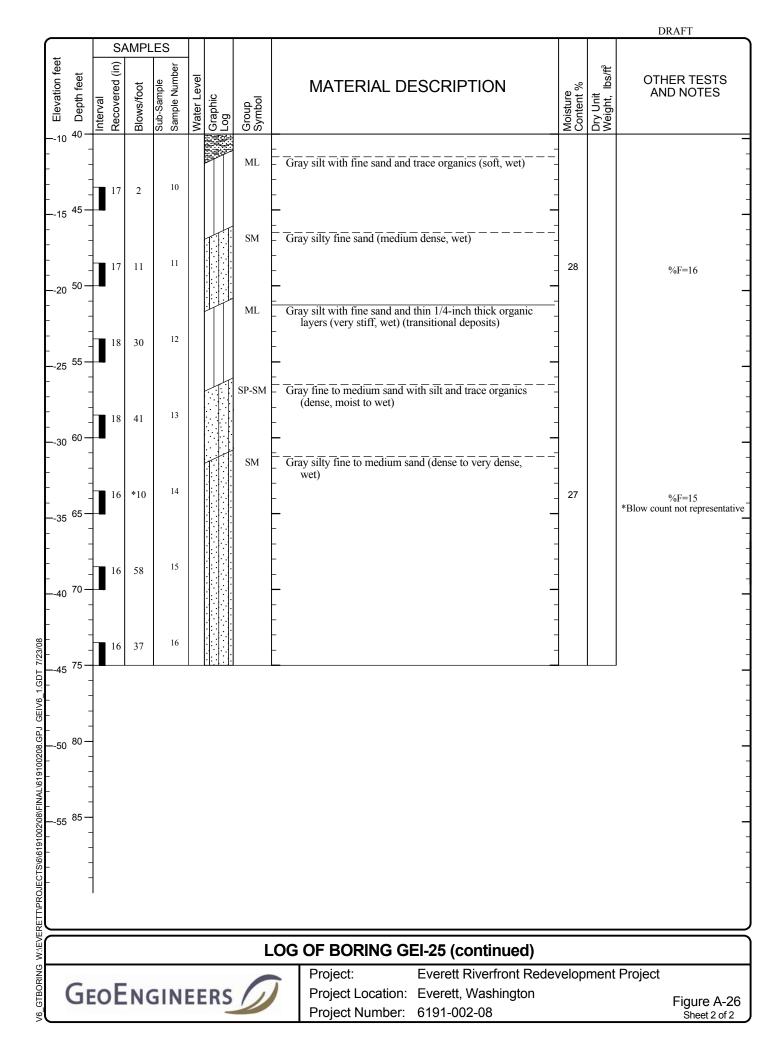
											1		DRAFT
et				MPI		-							
	· Deptriteet	Interval	Recovered (in)	Blows/foot	Sub-Sample	Sample Number	Water Level	Graphic Log	Group Symbol	MATERIAL DESCRIPTION	Moisture Content %	Dry Unit Weight, Ibs/ff	OTHER TESTS AND NOTES
_ 4	- 0									Grades to very soft			-
- - —-10			18	5	1	11			РТ	Brown peat with occasional partially decayed wood chips (soft to medium stiff, wet)	386		-
_ 4 _ _	5 -		18	2	1	12		1	ML	Gray silt with decayed wood fragments (very soft, wet)			-
- 	- - 0 -		18	0	1	13					-		-
-	1 1		18	0	1	14							-
- 20 - 5 -	- 5 — -	Π	24	Р	1	15			SM	Gray silty fine sand (medium dense, wet)			
- - 25 - 6	- - 0 -		3	21	1	16					-		- - -
- - 30 - 6	- - 5 -		14	30	1	17			SP	Gray fine to medium sand with trace silt (medium dense to dense, wet)	-		- - - -
- - 35 - 7	- - 0 -		10	14	1	18					-		- - -
- - 40 7			18	3	1	19			ML	Gray silt with occasional fine to medium sand and occasional wood fragments (soft, wet)	-		- - -
-	5 -		24	Р	2	20							-
- 45 - 8	- - 0 -		14	5	2	21			SM	Brown to gray silty fine to medium sand (loose, wet)	-		-
- - 	- - 5		18	44	2	22			SP/OL	 Brown organic silt interlayered with gray fine to medium sand (dense/hard, moist) 	-		- - - -
45 8	-		8	*22	2	23			SP-SM	- Gray fine to medium sand with silt and occasional	-		*Blow count not representative_
									L	OG OF BORING GEI-23 (continued)			
C	Ē	0	E	N	GII	NE	EE	RS	0	Project: Everett Riverfront Rede Project Location: Everett, Washington Project Number: 6191-002-08	velop	ment	Project Figure A-24 Sheet 2 of 3

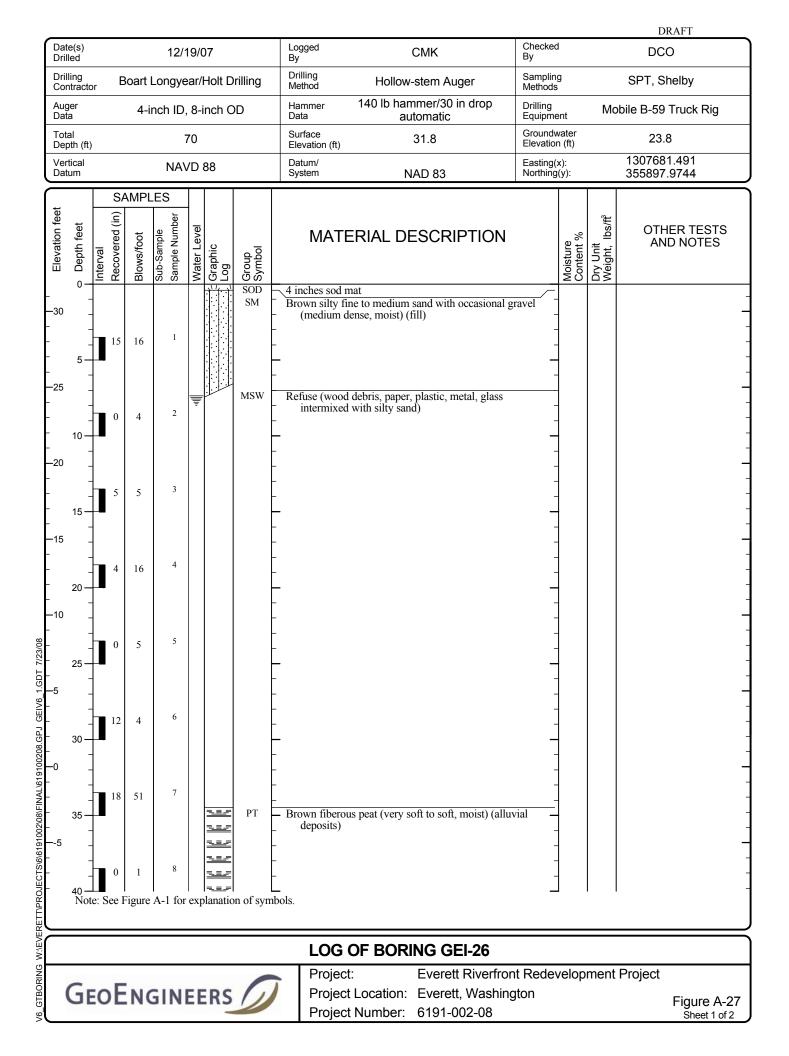
-						1				RAFT	
	ິຮ Depth feet ∣		Recovered (in)	Blows/foot	Sub-Sample Sample Sample Number	Water Level	Graphic Log	Group Symbol	MATERIAL DESCRIPTION Moisture Content % Veight, lbs/rf	HER TESTS ID NOTES	
- - - 60	90 - - - 95 -		14	38	24			SP	- Gray fine to coarse sand with trace silt and occasional - gravel (dense, wet)	- - - -	
- - 65 - 1	- - - 00 -		16	39	25			ML	Gray silt with fine sand (hard, wet)	-	
- - - 70			15	49	26			ML	- Gray silt with 2 inch layer of gray fine to medium sand - (hard, moist)		
- -	05 — - -		1							- - -	
75 - 1 - -	10 — - -										
—-80 _ 1 - -	- 15 - -									-	
- 	- 20 — -									-	
- 90 - 1	_ 25 — _										
-	- - 30 — -									- - -	
95 - 1 - - - - - - - - - - - - - - - - - -	- - 35 —									- - -	
- - - 105	-									-	
								L	.OG OF BORING GEI-23 (continued)		
	GEOENGINEERS Project: Everett Riverfront Redevelopment Project Project Location: Everett, Washington Project Number: 6191-002-08										

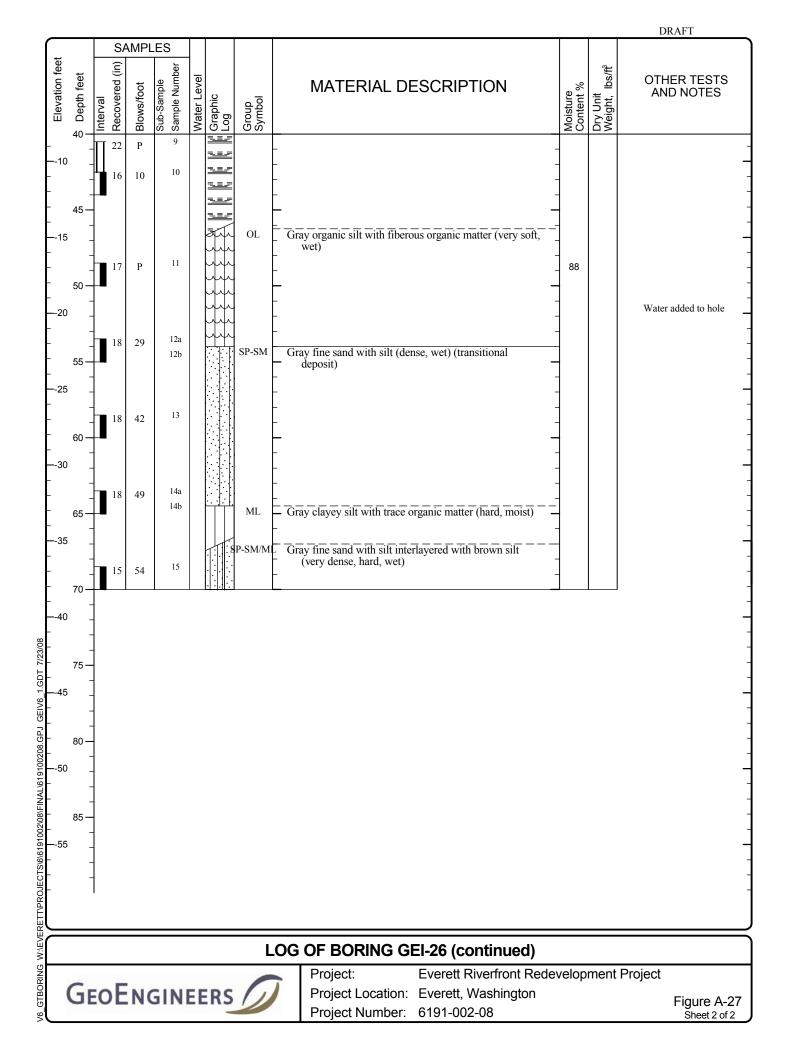


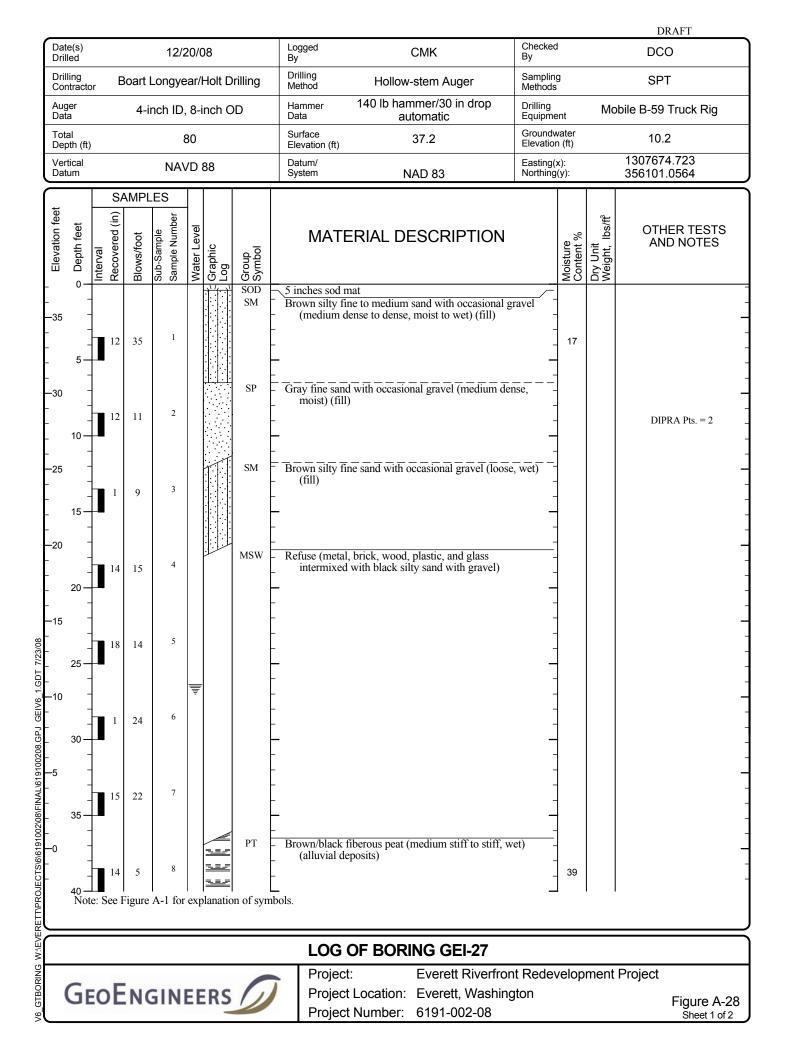


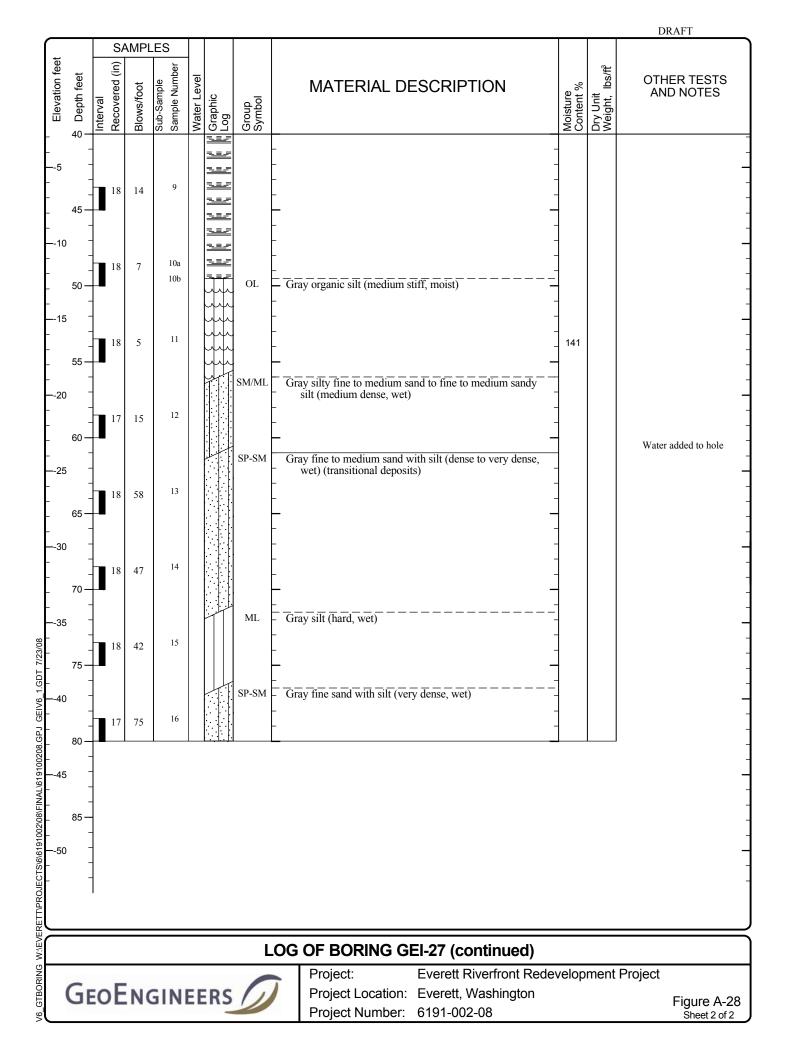


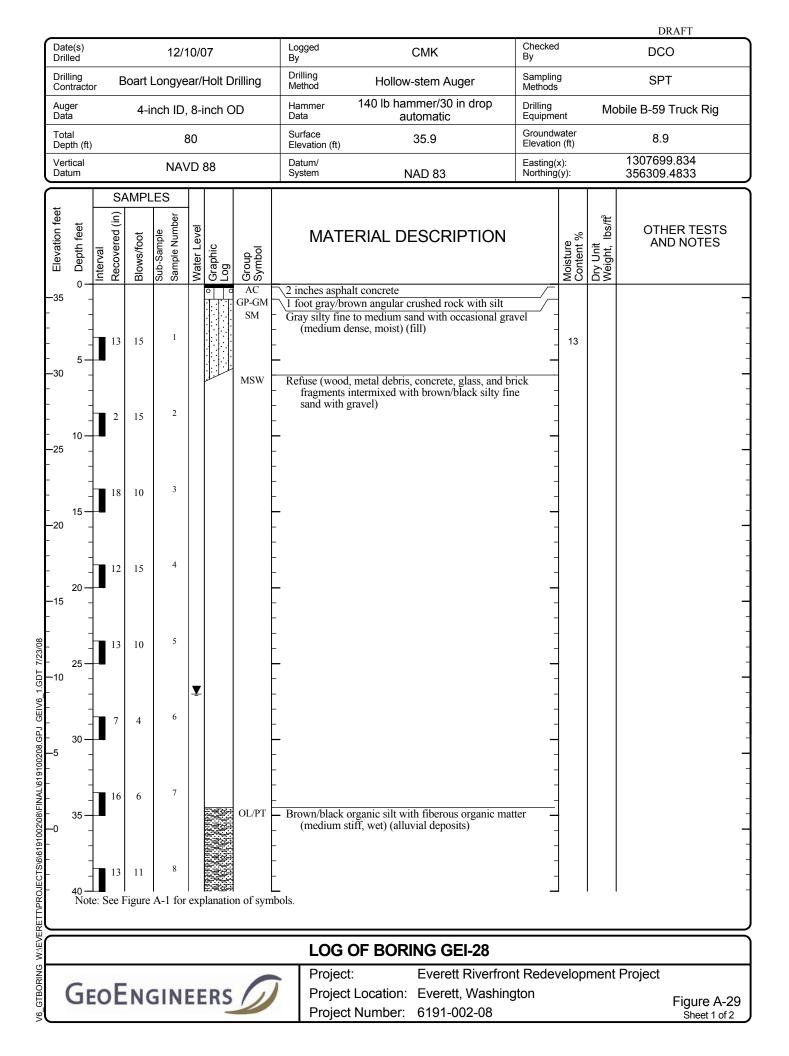


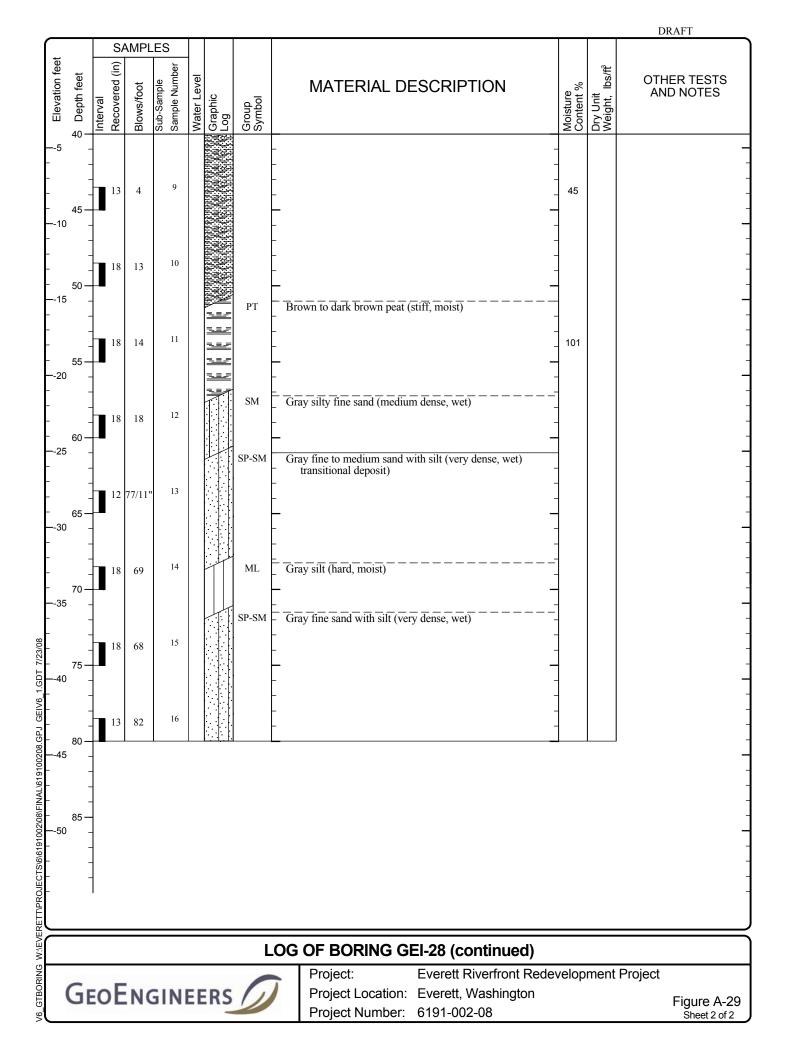


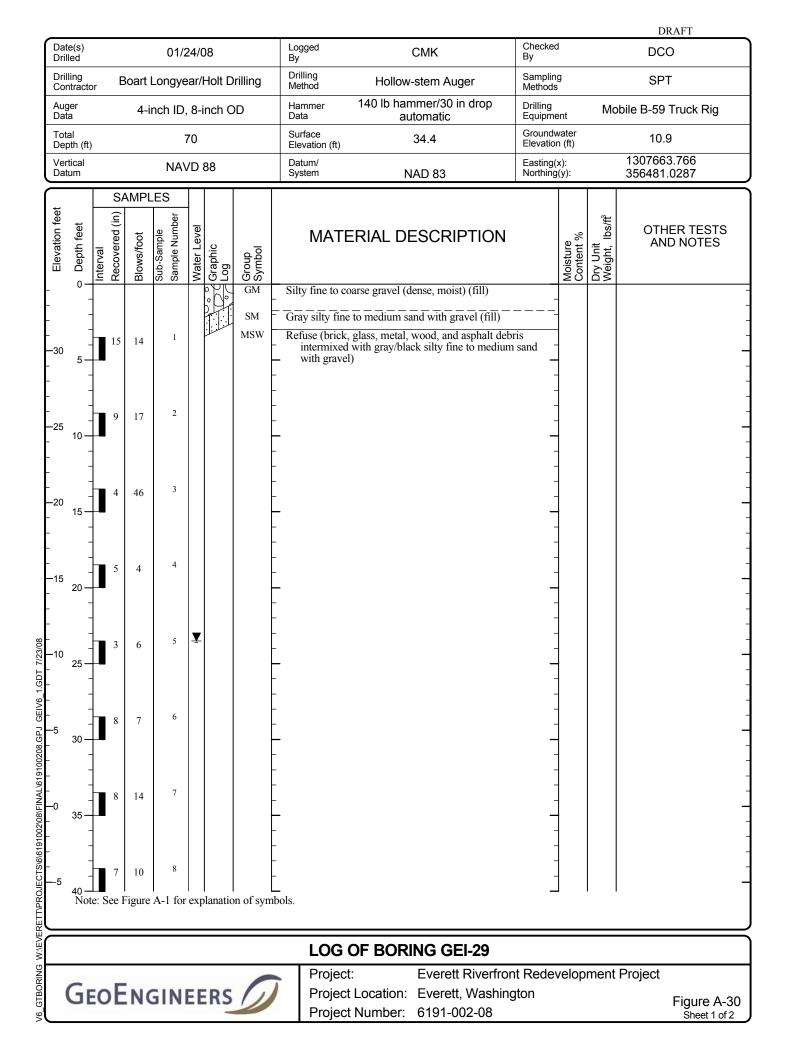


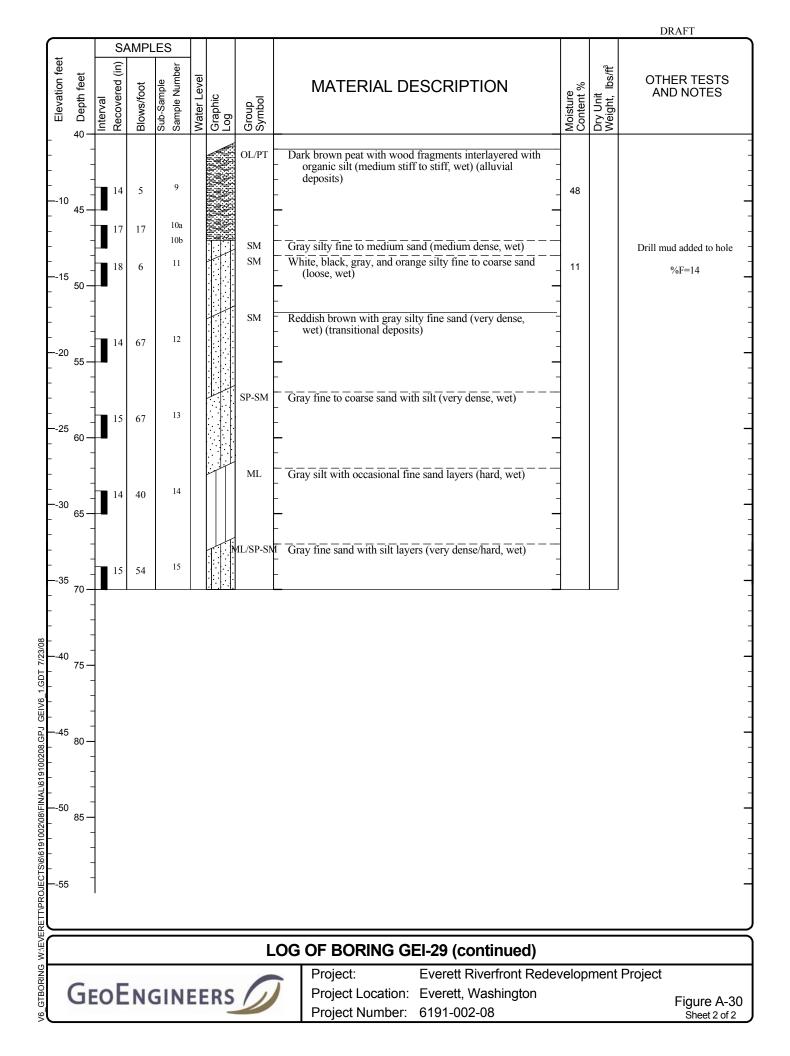


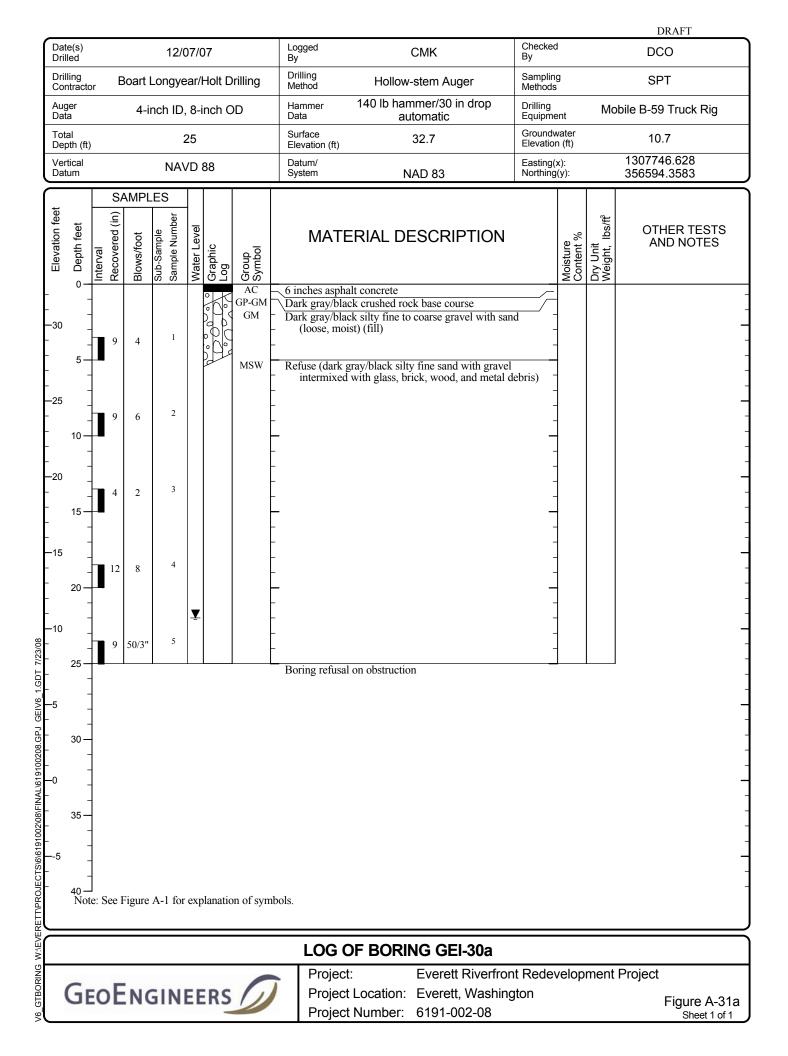


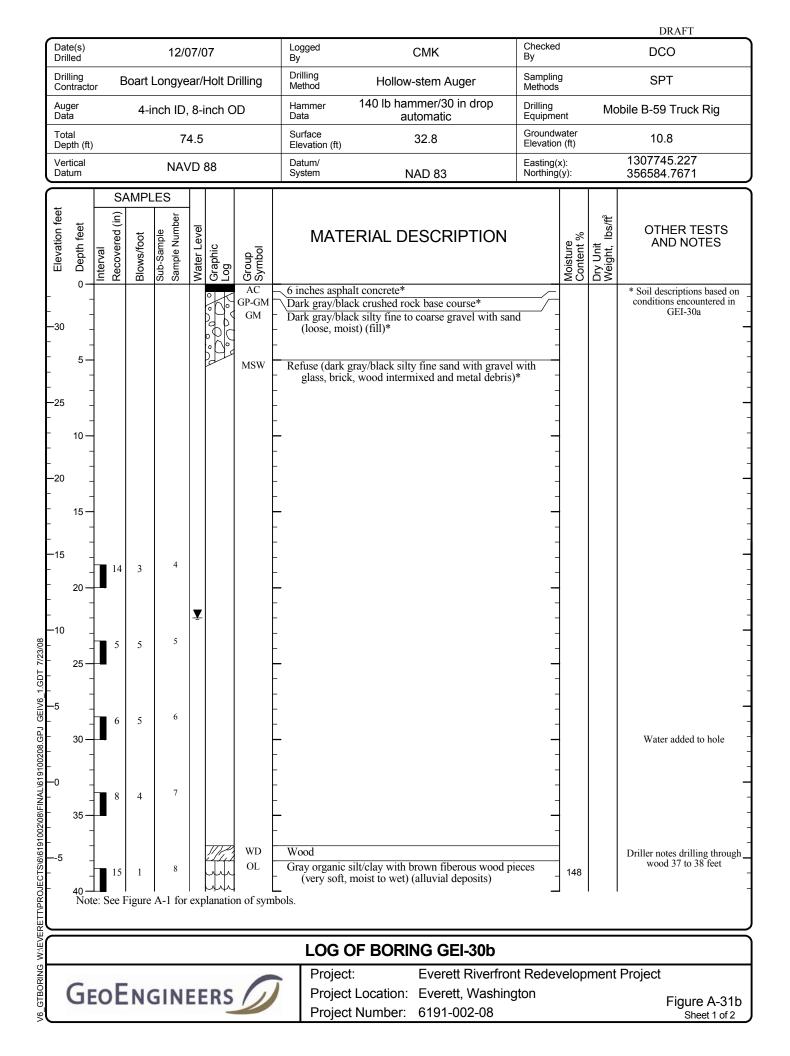


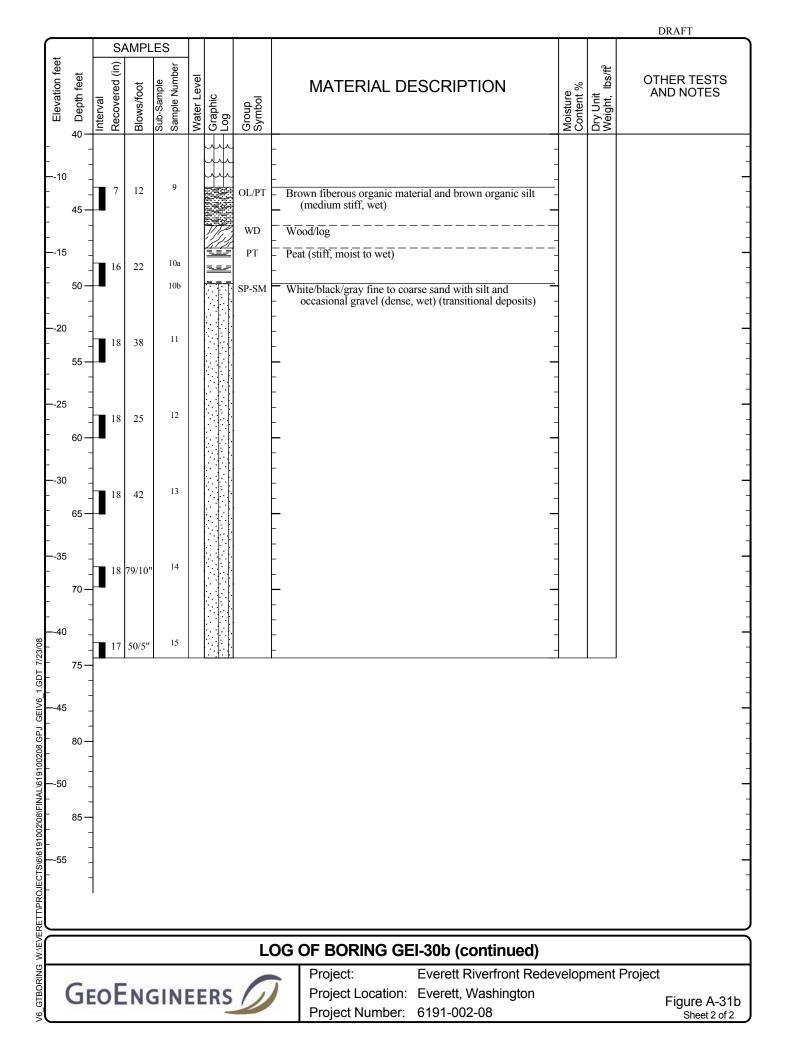


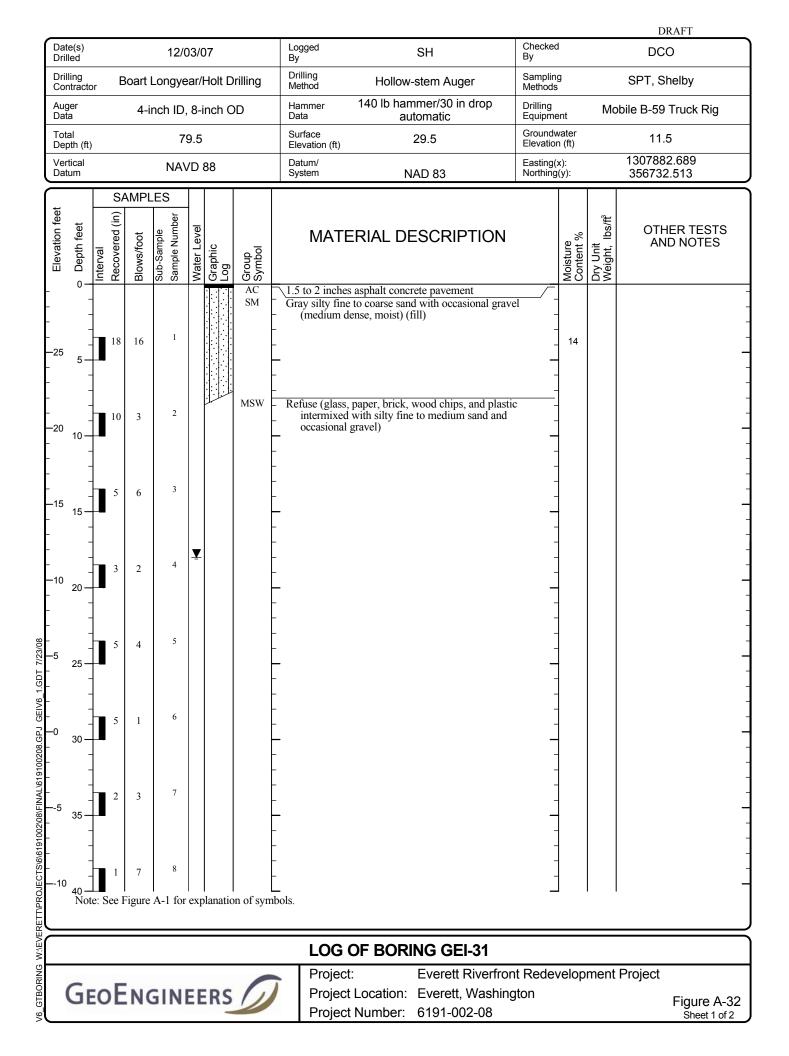


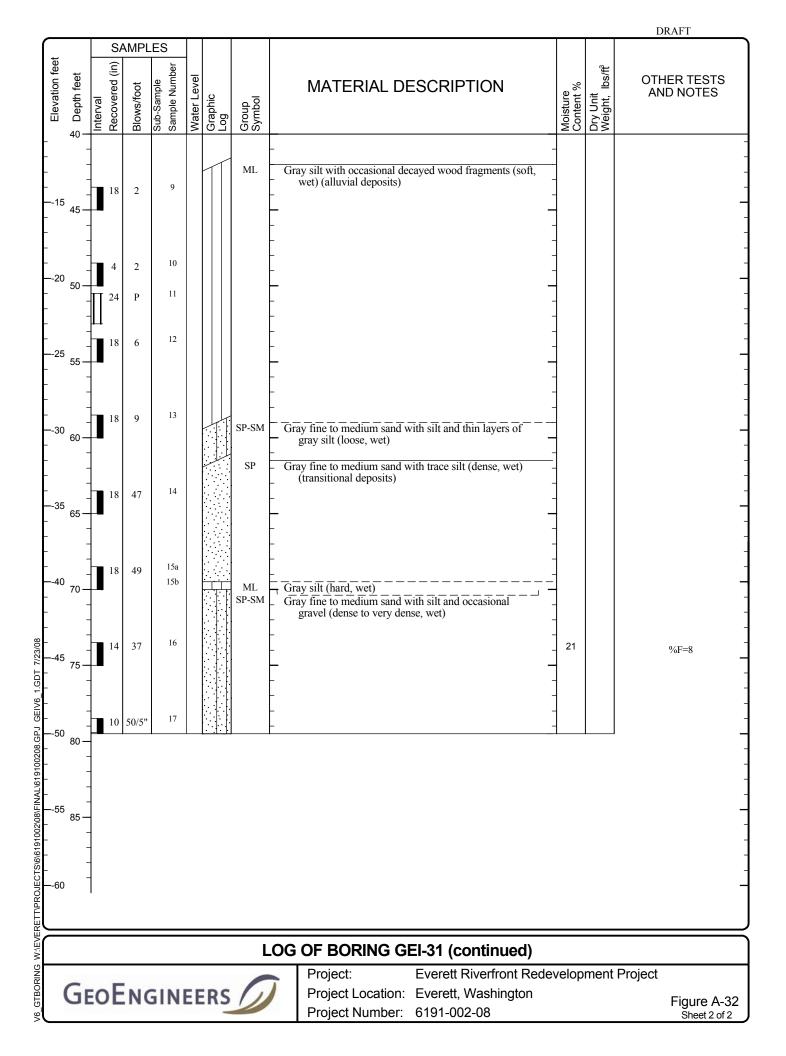


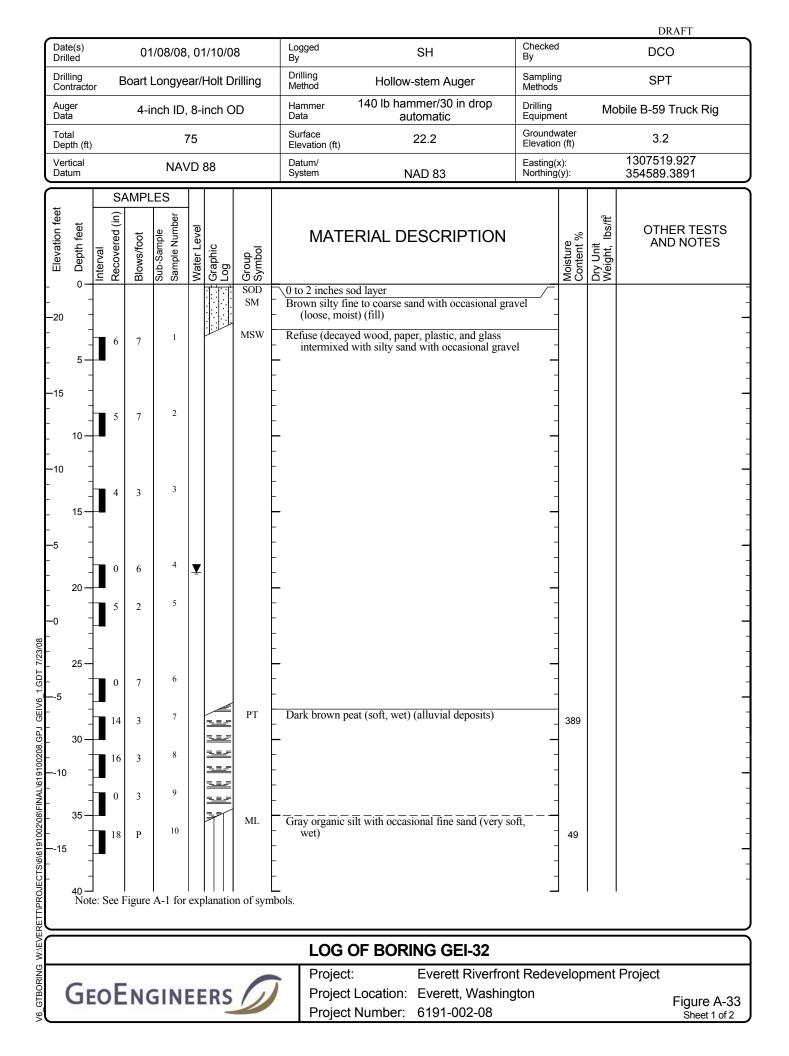


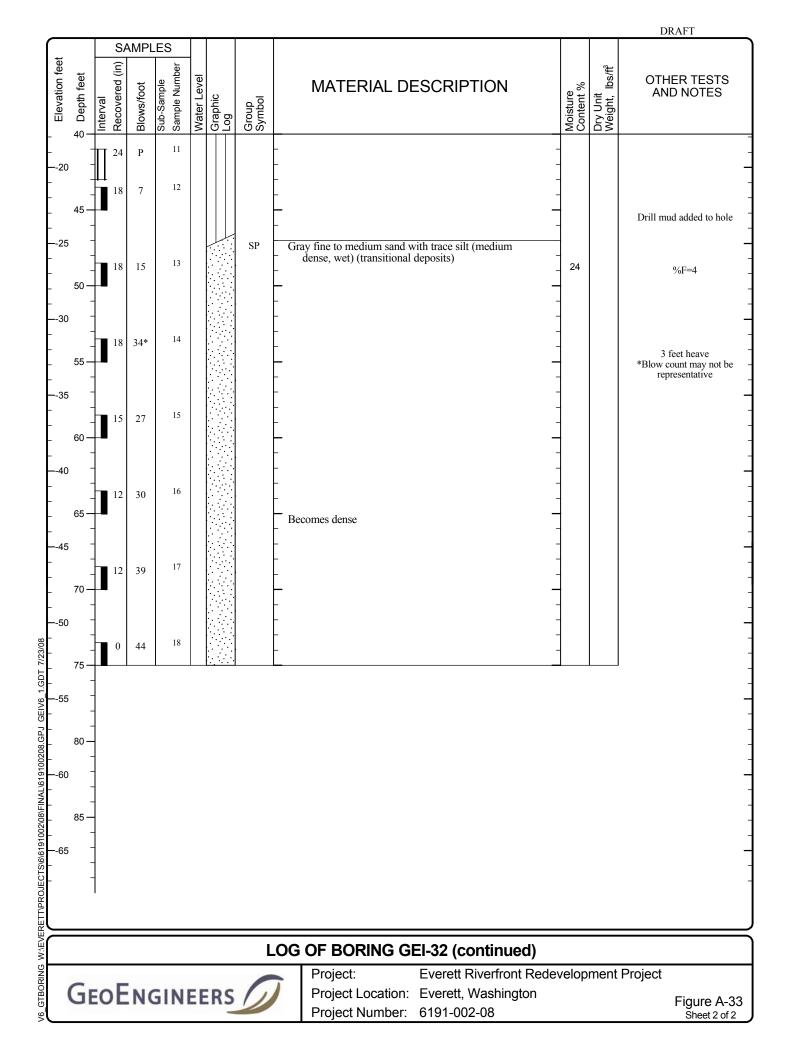


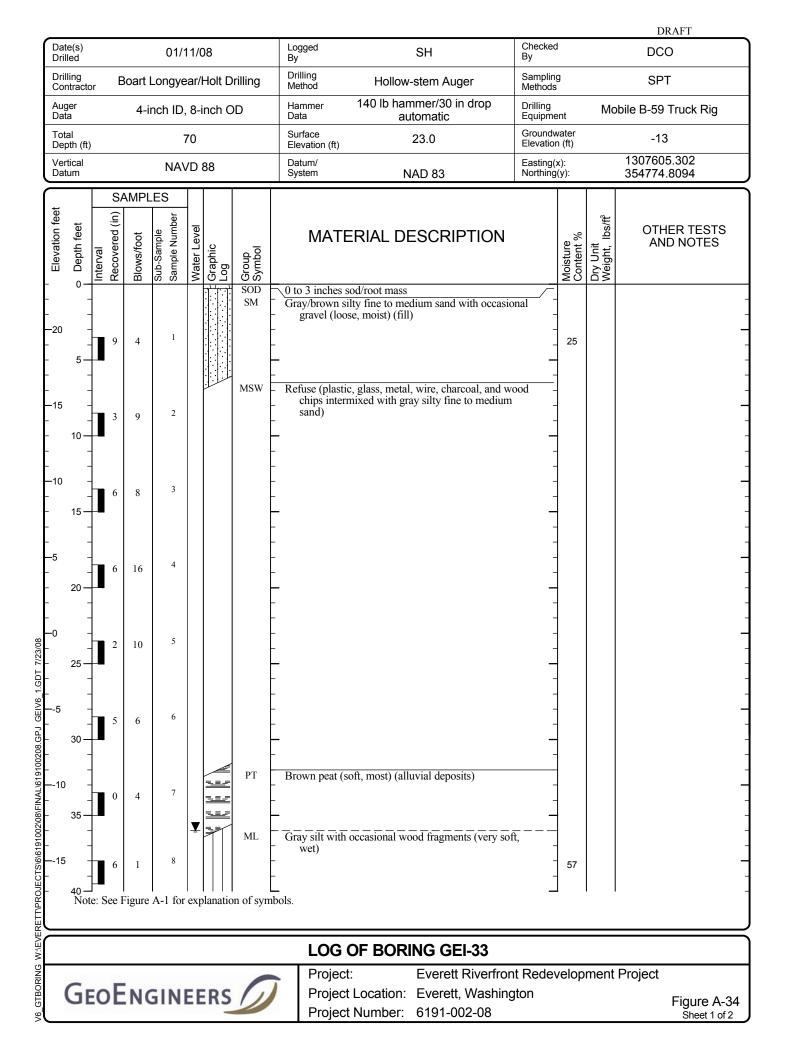


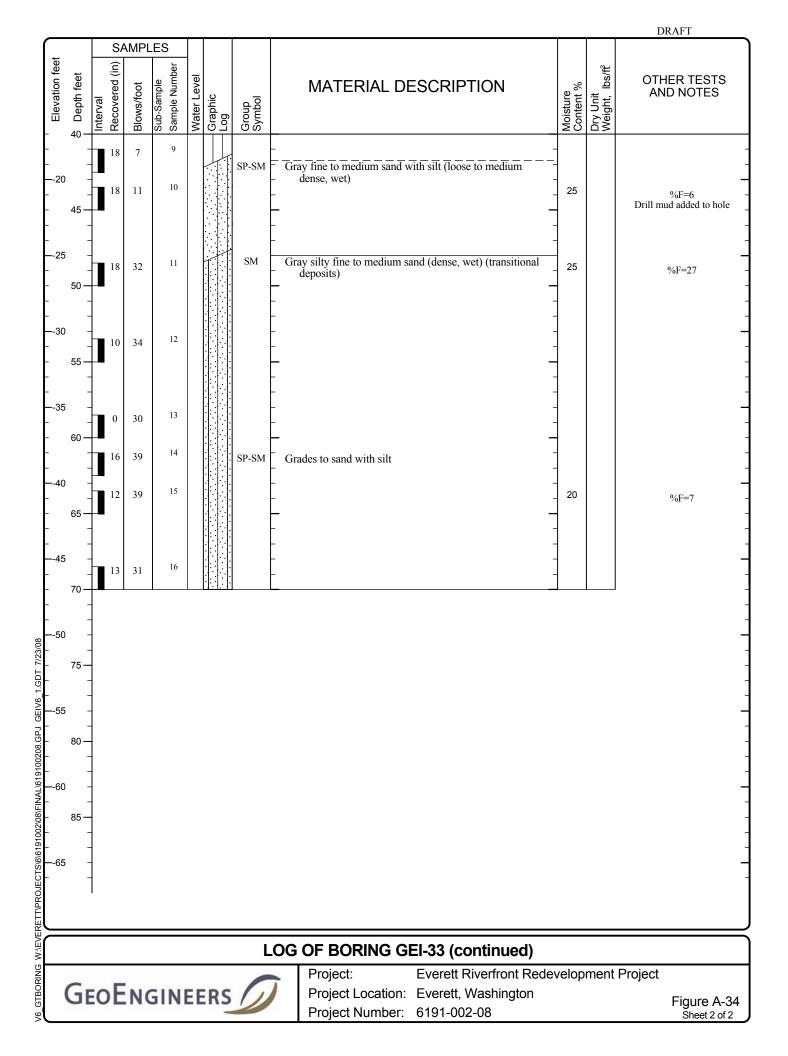


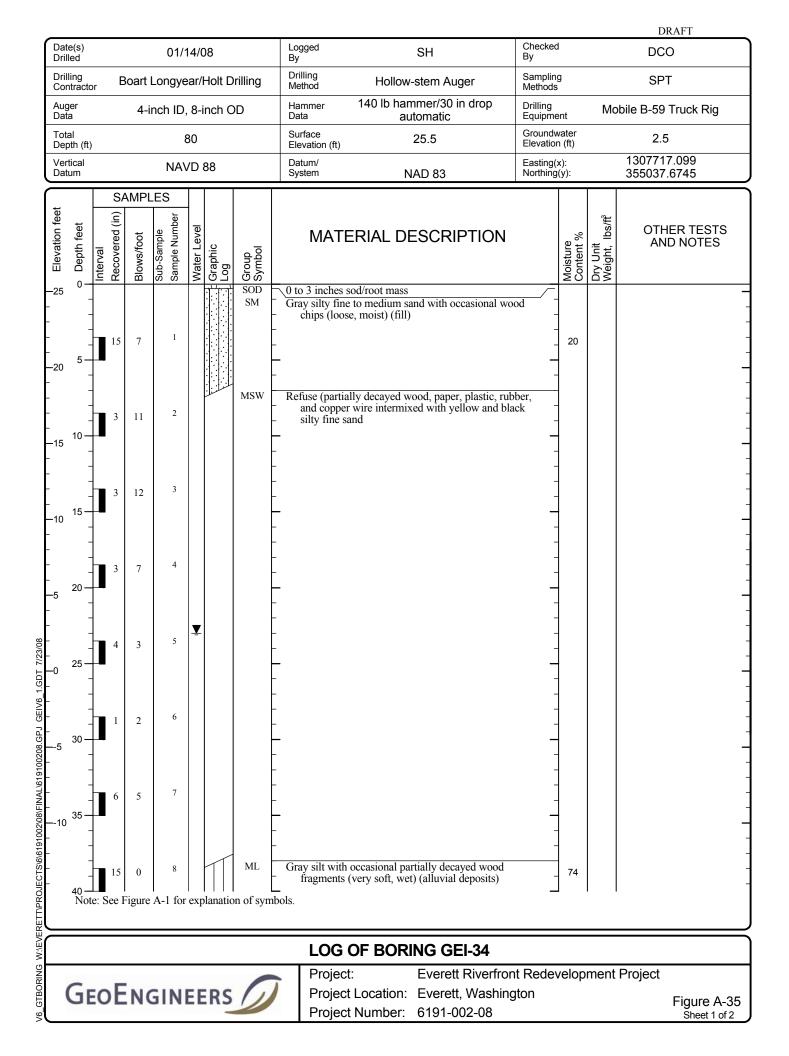


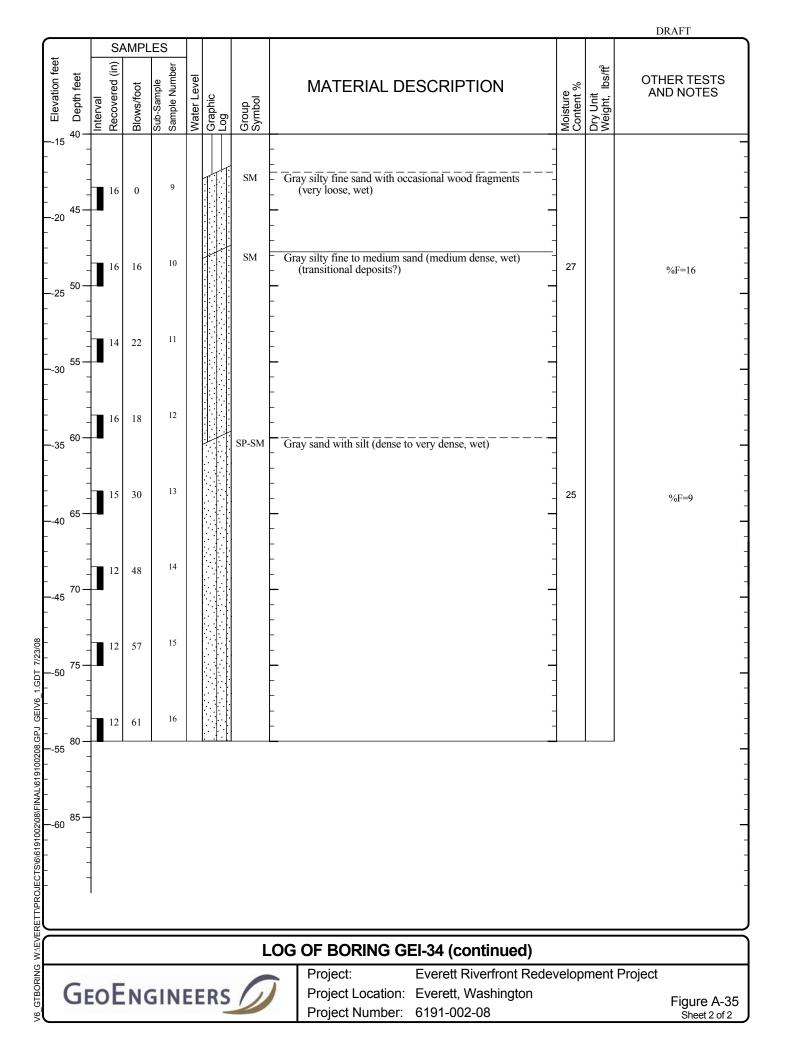


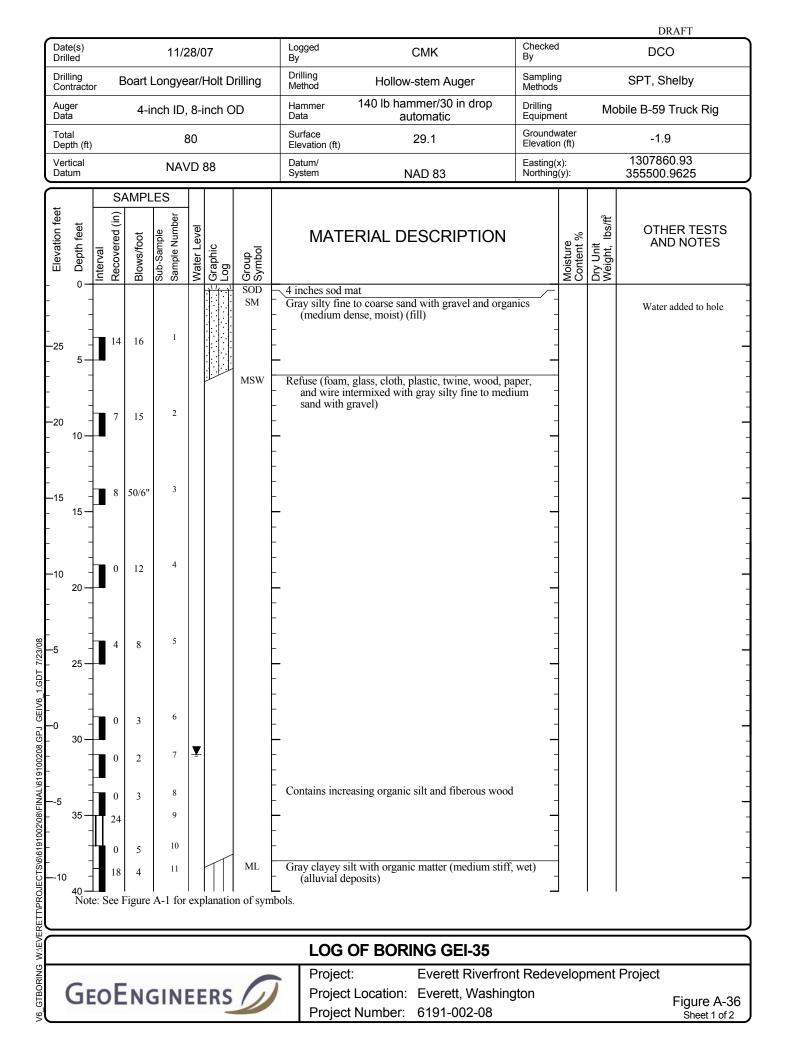


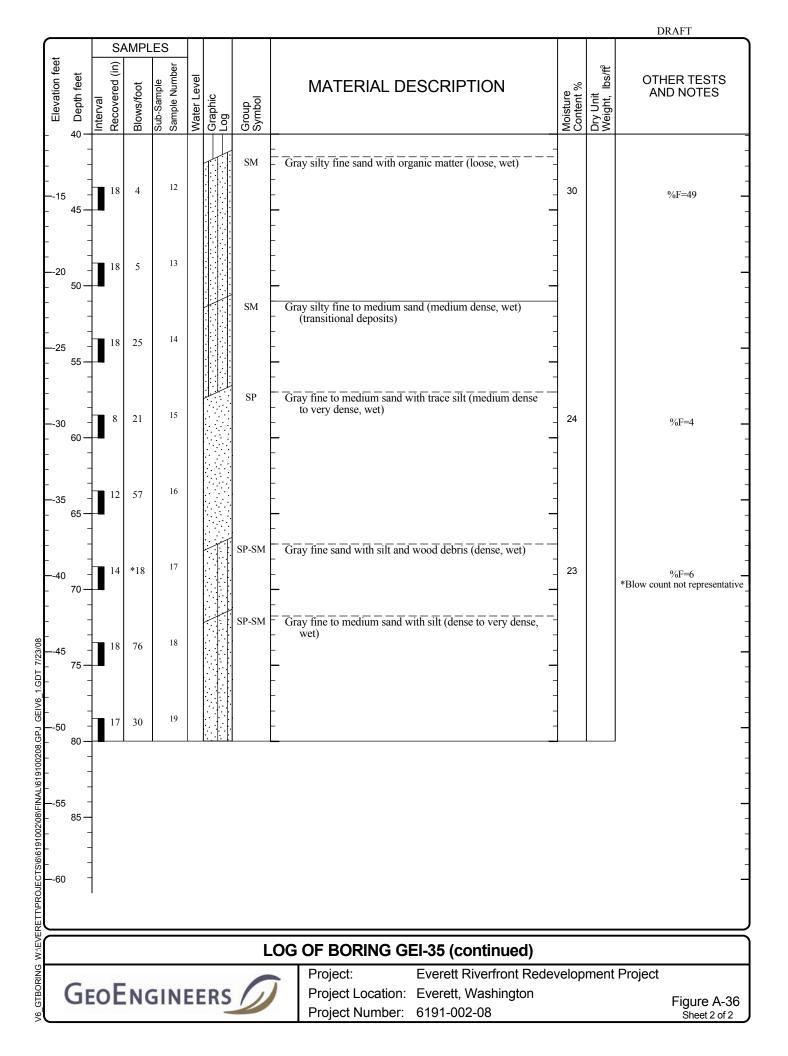


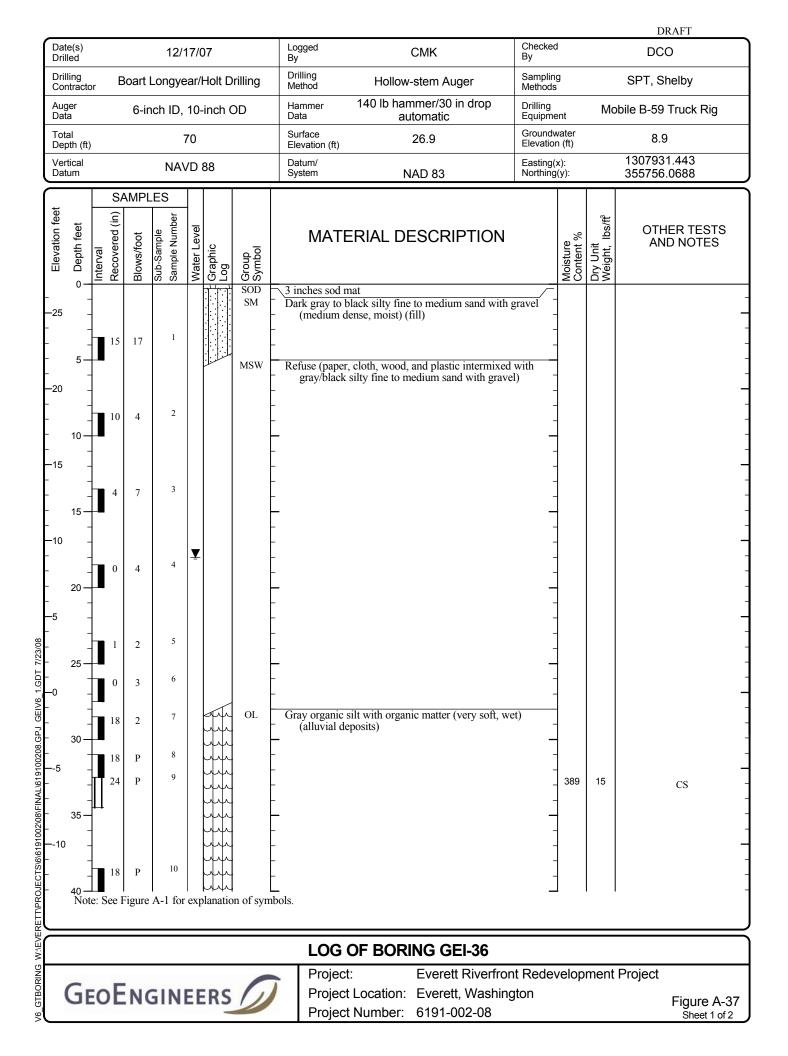


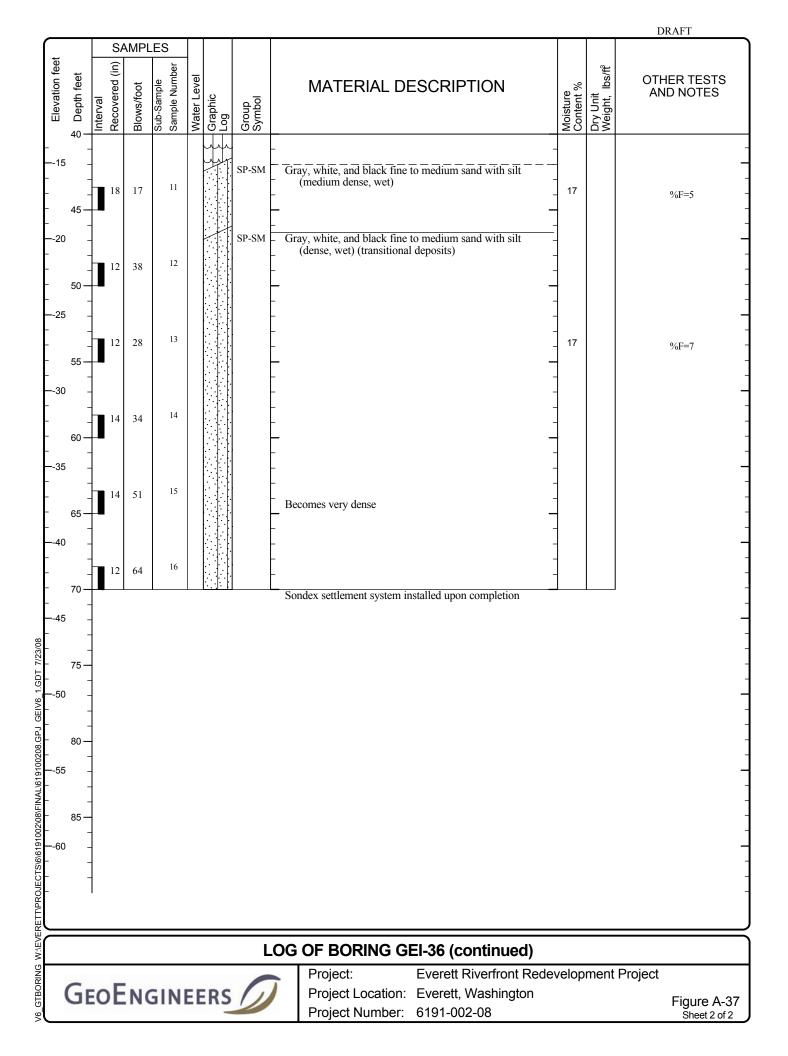


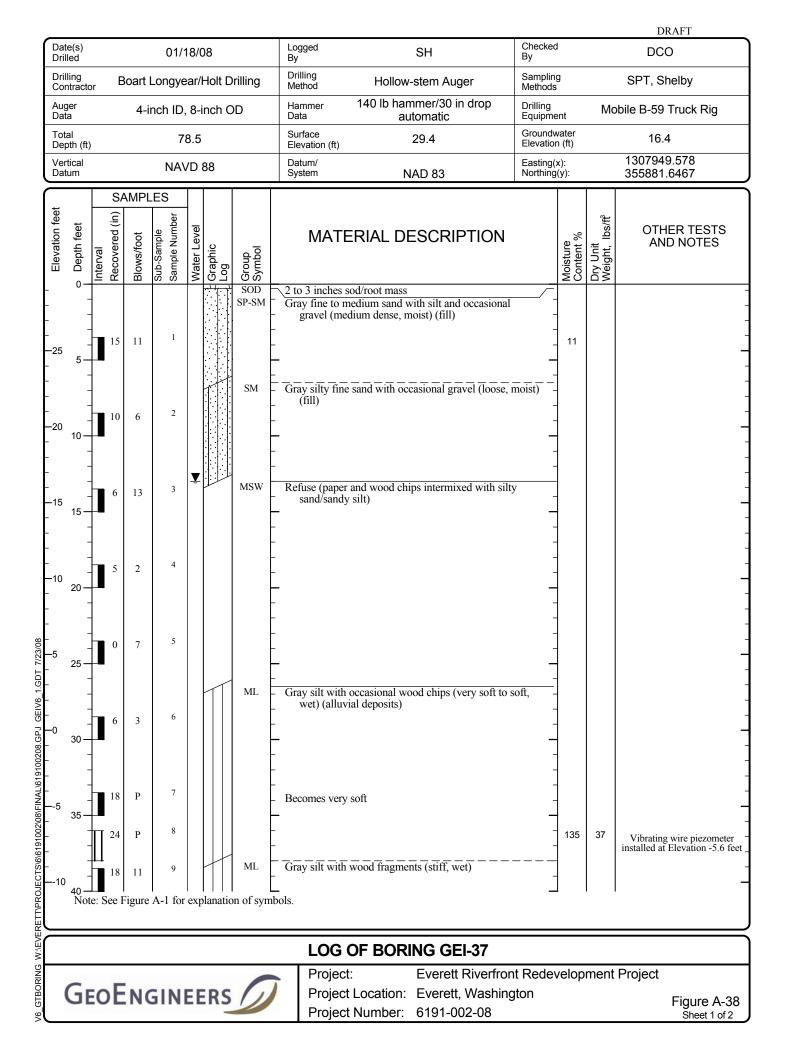


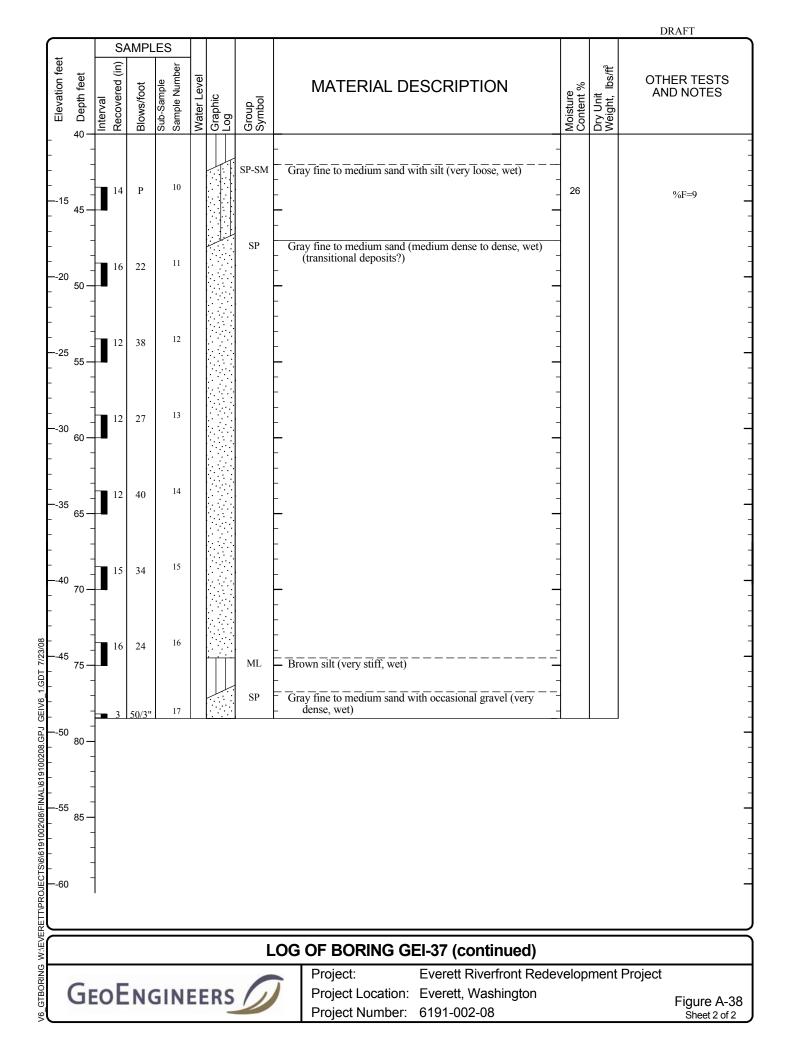


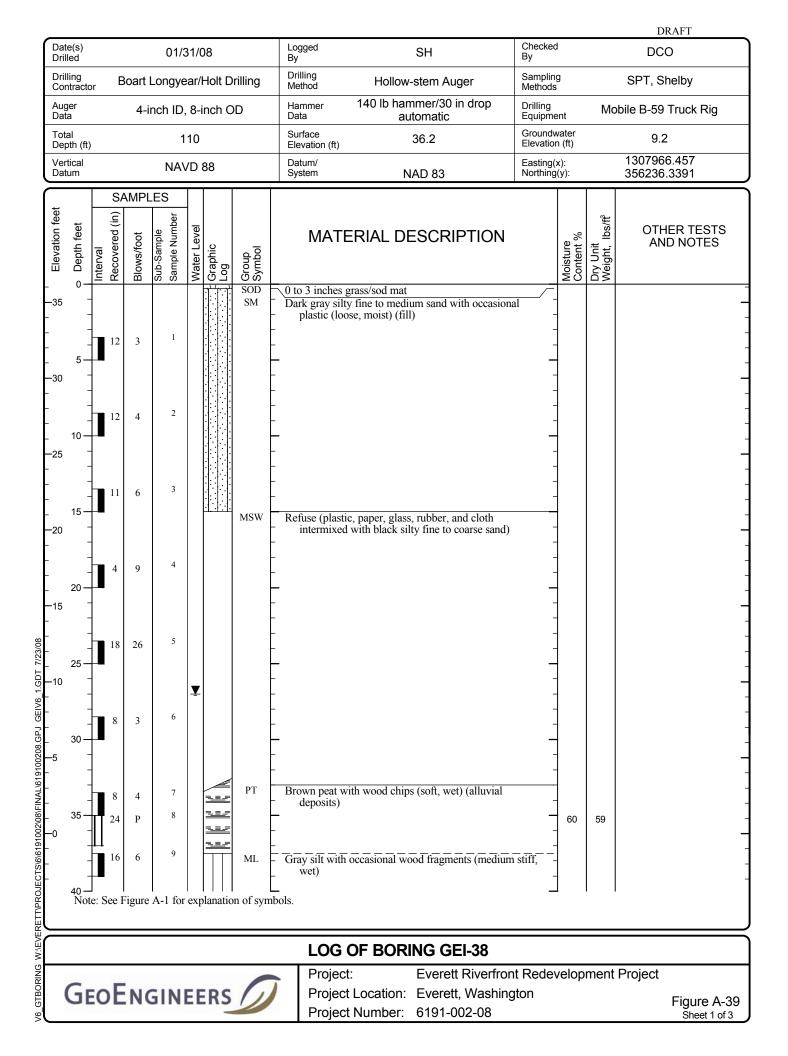


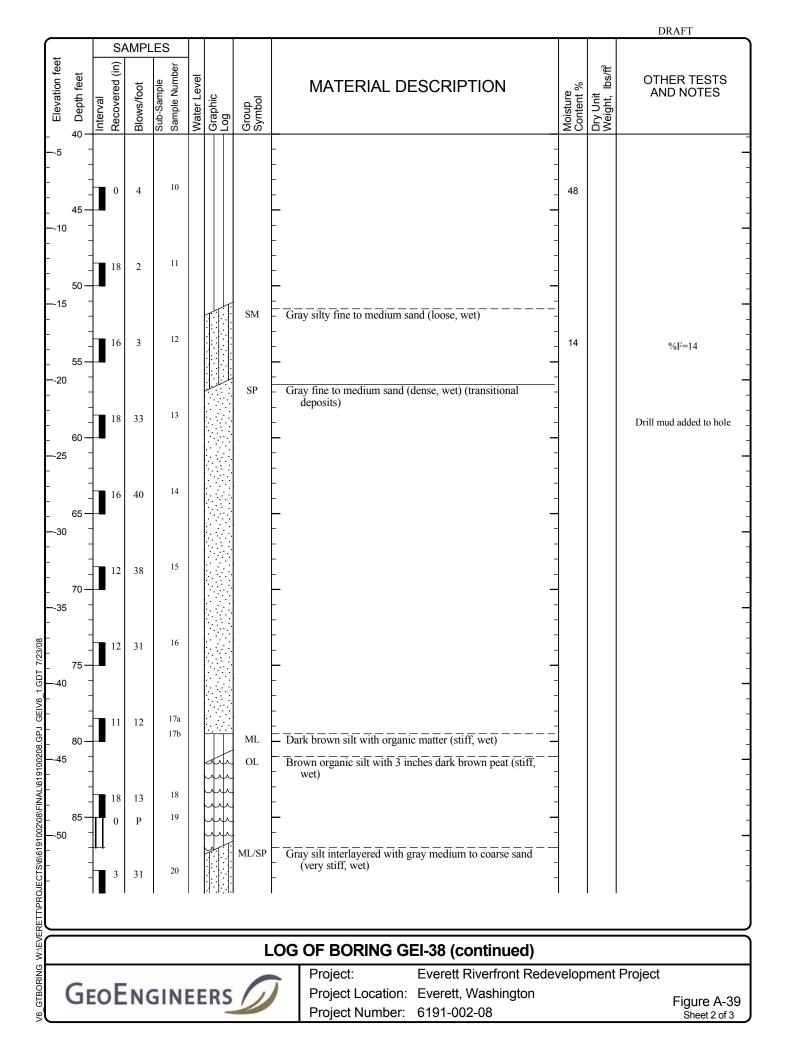




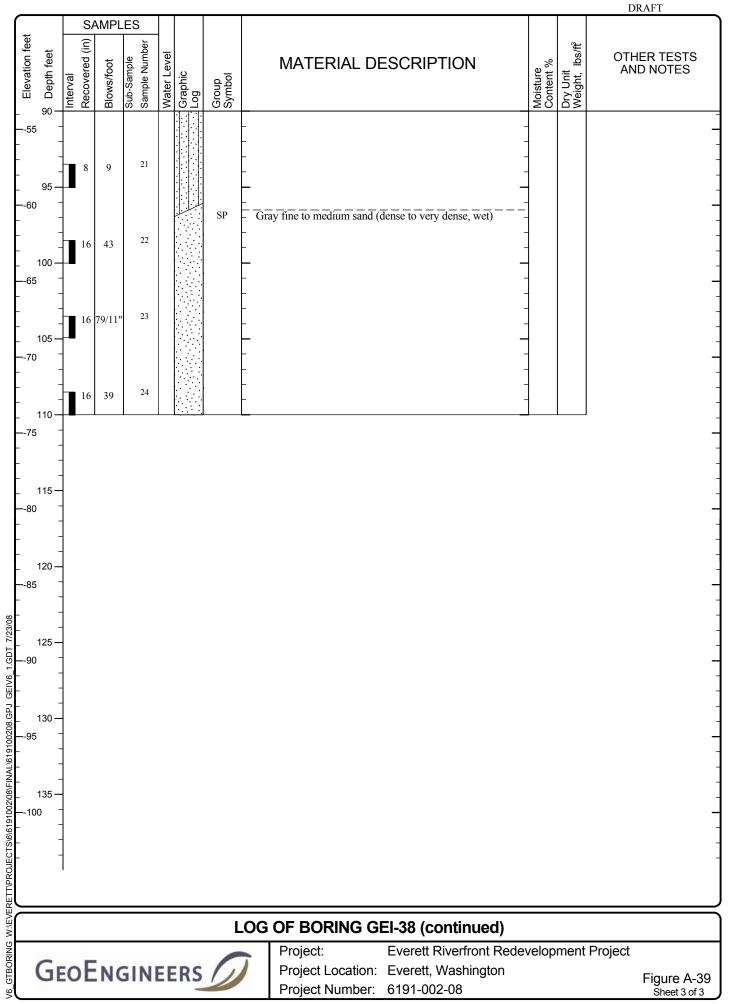


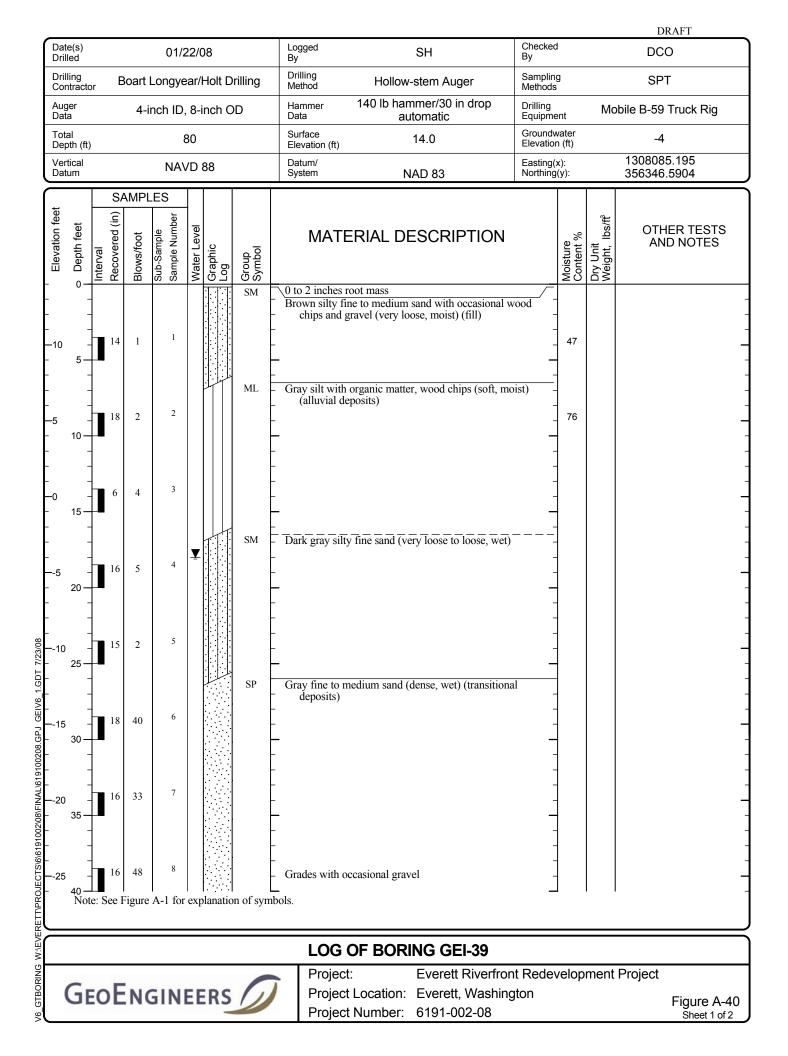


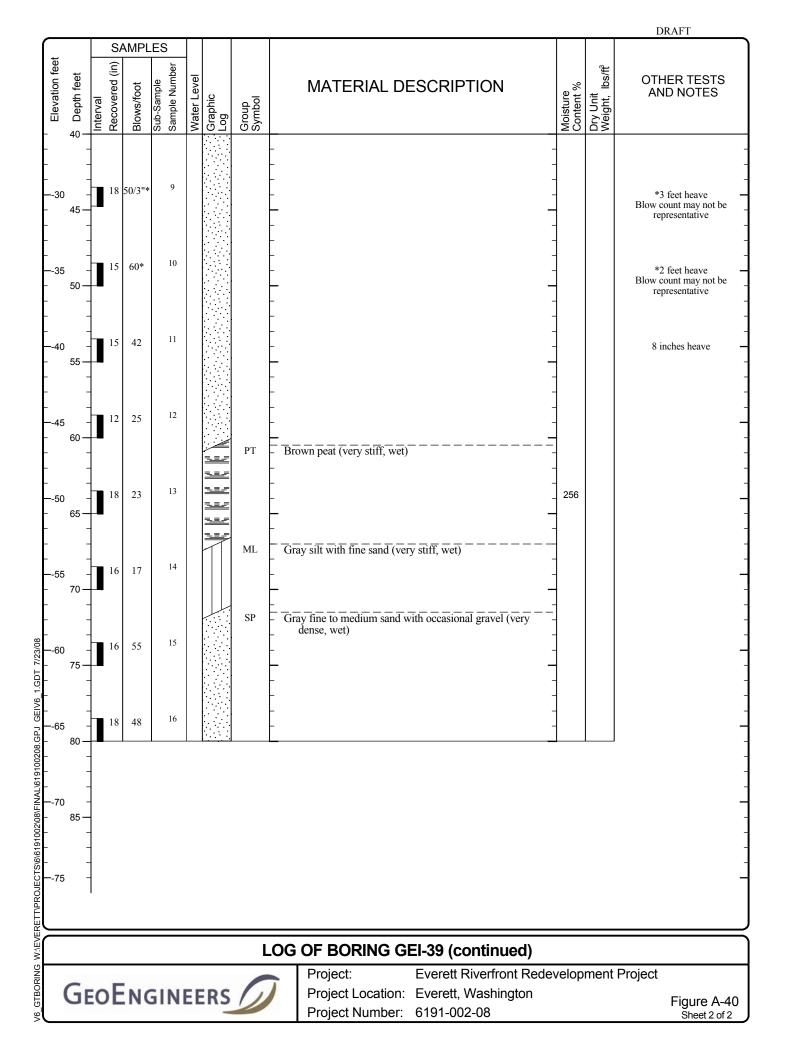


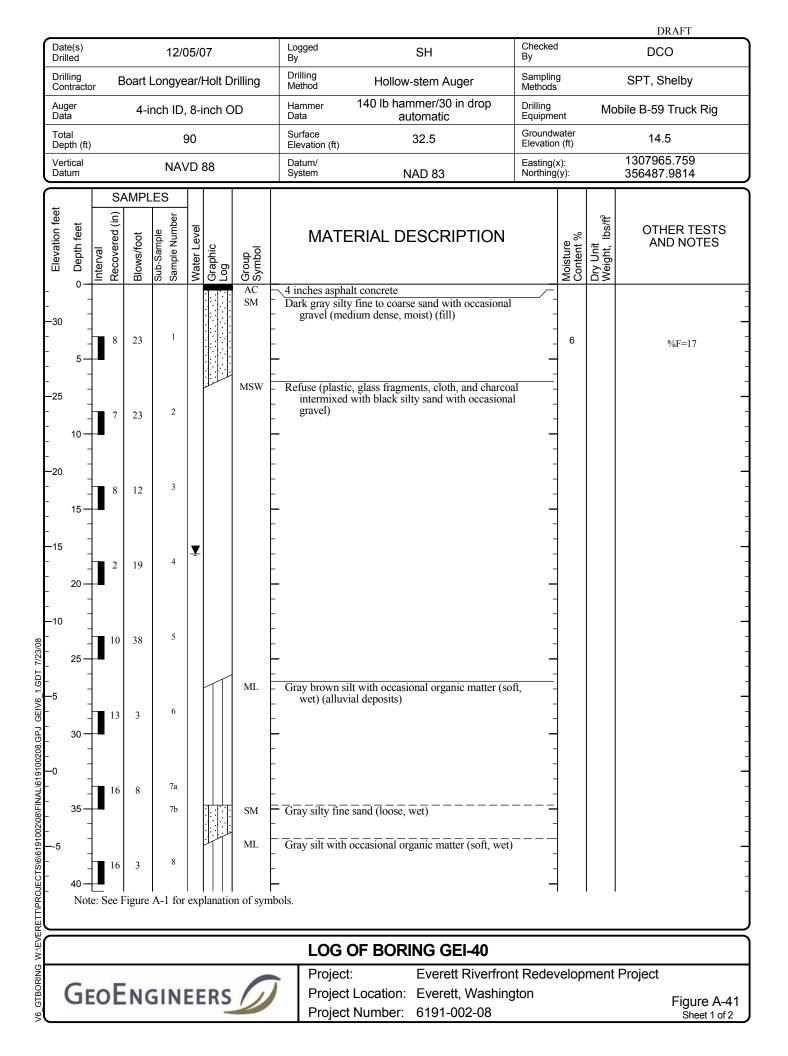


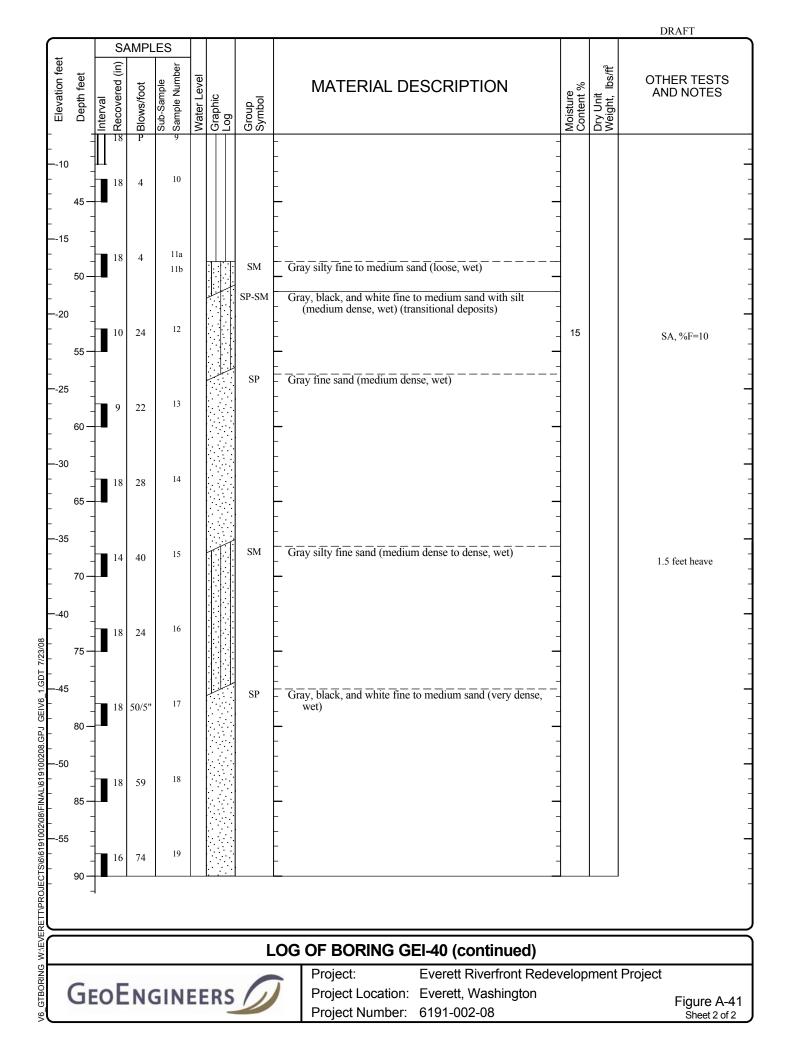


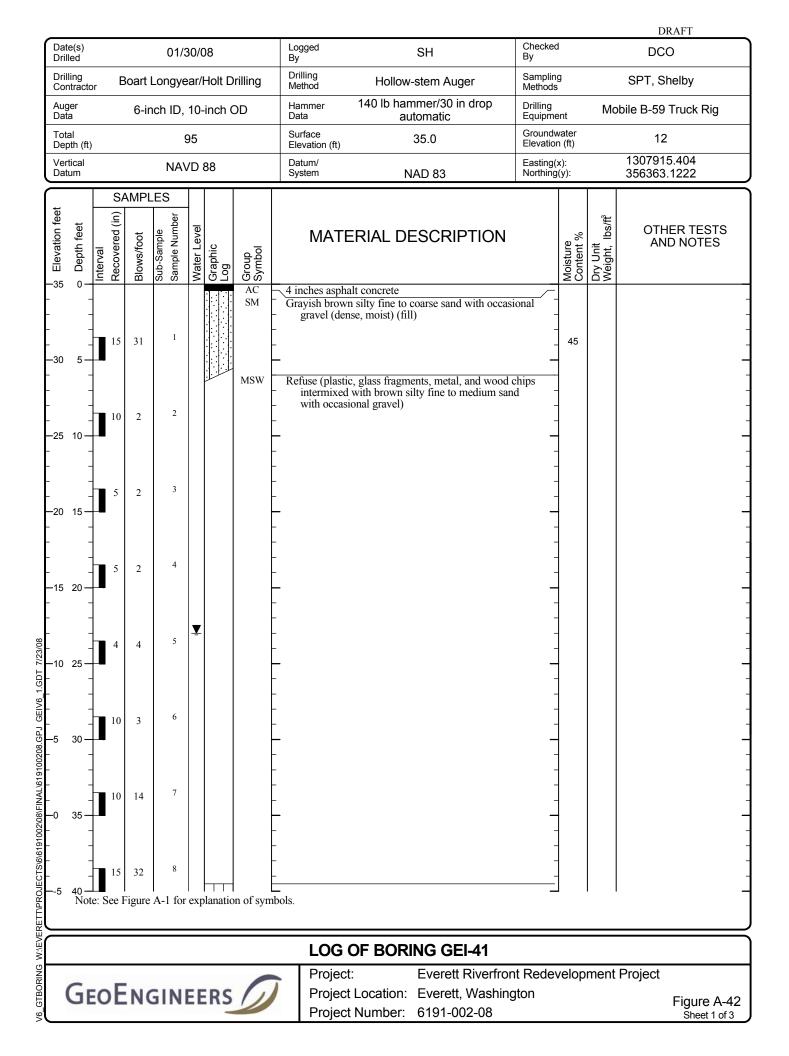












											DRAFT	
Elevation feet	Interval Recovered (in)	Blows/foot	Sub-Sample Sample Number	Water Level	Graphic Log	Group Symbol	MATERIAL DESCRIPTION		Moisture Content %	Dry Unit Weight, Ibs/ff	OTHER TESTS AND NOTES	
						ML WD	Gray silt (medium stiff, wet) (fill?) Red partially decayed wood chips (wet) (wood waste?)					
 10 45 	4	13	9			WD	Gray silt with occasional wood chips (soft, wet) (fill?)	- - - -			-	
	8	3	10				-		79			
15 50	18	4	11			ML	Gray sandy silt (soft, wet) (alluvial deposits)				Vibrating wire piezomenter intalled at elevation -16.0 feet	
		P	12				-	-				
20 55 	16	Р	13			SP	Gray fine sand (very loose, wet)				Drill mud added to hole	
25 60 	10	18	14					-			-	
 	16	15	15a 15b			PT	- - - Brown peat (stiff, wet)	- - - -	40 120		-	
	18	15	16a 16b		<u>=</u> :: : :	SM	 Dark gray silty fine to medium sand with organic silt (medium dense, wet) 					
	3	13	17				-	- - -			-	
	16	60	18			SP	 Gray fine to medium sand with trace silt (very dense, wet) (transitional deposits) 	-			-	
EVERETTAPROJECTSI61619100208.614100208.647	12	50/6"	19				Grades with occasional gravel	-			-	
PROJECTS(6)(6)(1)	16	93	20				-	-				
WIEVE	LOG OF BORING GEI-41 (continued)											
SING	GEOENGINEERS Project: Everett Riverfront Redevelopment Project Project Location: Everett, Washington Figure A-42											
9 ×	Project Number: 6191-002-08 Sheet 2 of 3											

