



## **Oakland Bay Study**

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**A dye and modeling study in an enclosed estuary with a high degree of refluxing**

May 2004

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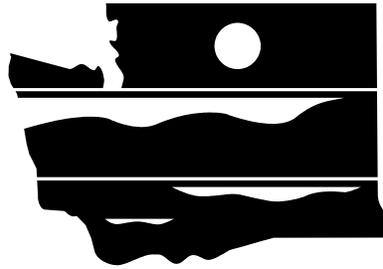
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WASHINGTON STATE  
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E C O L O G Y

## Oakland Bay Study

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### A dye and modeling study in an enclosed estuary with a high degree of refluxing

*by*  
*S. L. Albertson*

Environmental Assessment Program  
Olympia, Washington 98504-7710

May 2004

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## Abstract

The city of Shelton is growing in population and is considering the possibility of increasing their wastewater discharge via an outfall that straddles two (sanitary) lines, which define a shellfish closure zone in Oakland Bay and Hammersley Inlet.

The Washington State Department of Ecology was approached to develop a model to help the Washington State Department of Health evaluate conditions at these sanitary lines, in response to Shelton's needs for possible wastewater treatment plant expansion.

We developed the Hammersley Oakland Bay Oceanographic (HOB0) model based on the Environmental Fluid Dynamics Code (EFDC), a three-dimensional hydrodynamic computer primitive equation model (Hamrick, 1992, 1996). HOB0 is driven by real data acquired at its boundaries. The air-sea boundary conditions were generally provided from the nearby airport, Sanderson Field. The open sea boundary conditions were recorded near Libby Point as a part of the overall study. We used the HOB0 model to simulate various discharge scenarios and to determine consequences at the two sanitary lines during periods specified by the Washington State Department of Health.

Ecology also participated in a dye study to determine the far-field dilution factor at the sanitary lines at a specific time, April 2003, and to validate the model.

Model results show that extending the diffuser horizontally across Hammersley Inlet can be very effective in controlling the far-field dilution at the sanitary line, although releasing effluent further north toward Munson and away from Eagle Point can cause problems on the east end of the Oakland Bay sanitary line. Holding back effluent at slack tide is another very effective method for controlling initial dilution at the sanitary lines. Controlling the vertical plume trapping depth is not very effective as a control method since there is a substantial amount of vertical mixing in this estuary, and the plume gets mixed anyway by the time it gets to either sanitary line.

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# Executive Summary

The Washington State Department of Ecology was approached to develop a computer model of Hammersley Inlet and Oakland Bay for the City of Shelton.

The driving force for this work was a desire by the City of Shelton, Port of Shelton, Washington State Department of Corrections, and other regional partners to possibly expand the city's wastewater treatment plant (WWTP) output into Oakland Bay and Hammersley Inlet without compromising shellfish harvest areas defined by two sanitary lines fore and aft of its discharge diffuser. The model was used to run and test various discharge scenarios of the WWTP.

The Washington State Department of Health (WDOH) maintains these sanitary lines, which are important to many commercial and tribal aquaculture interests in the area. The adverse condition for far-field dilution at the upstream sanitary line (Oakland Bay) is the flood tide after ebb, such that the sum of effluent accumulated during the ebb as well as low slack tide advects over the line shortly after the onset of the flood. Conversely, the adverse condition for far-field dilution at the downstream sanitary line in Hammersley Inlet is the ebb following a flood tide.

Oakland Bay is connected to the remainder of South Puget Sound by a long narrow entrance channel with a high degree of tidal-refluxing. Refluxing in an estuary refers to the quantity of outflowing water in a mixing zone that returns into the estuary over many tidal cycles. Refluxing occurs throughout the model domain, and there is ample justification for using dye to study circulation whereas drogues and drifters would likely be stranded on the jagged shoreline. To complement the understanding of a dye study instigated by the Federal Drug Administration (FDA), the Washington State Department of Ecology created a three-dimensional hydrodynamic model of the embayment using a sigma-coordinate primitive equation model called the Environmental Fluid Dynamics Code, or EFDC (Hamrick, 1992, 1996). We show that the model can be tuned to recreate the dye experiment, and then we use it to investigate various proposed discharge scenarios.

Model results show that the initial dilution of effluent released at the WDOH effluent criteria of 46,300 fecal coliform bacteria units per 100/ml (fcbs) would satisfactorily dilute to the Class A water quality standard of 14 fcbs crossing the sanitary lines when discharge is set at 2.6 million gallons per day (mgd). At 6.7 mgd, the satisfactory dilution did not occur. At intermediate levels between 4 and 4.6 mgd, success was dependent on diffuser configuration and discharge timing relative to the tide.

The model results also indicate that extending the diffuser horizontally across Hammersley Inlet can be very effective in controlling the far-field dilution at the sanitary line, although releasing effluent further north toward Munson and away from Eagle Point can cause problems on the east end of the Oakland Bay sanitary line. Holding back effluent at slack tide is another very effective method for controlling initial dilution at the sanitary lines. Controlling the vertical plume trapping depth is not very effective as a control method since there is a substantial amount of vertical mixing in this estuary, and the plume gets mixed anyway by the time it gets to either sanitary line.

This study does not recommend a decision on the future of discharge in the Oakland Bay system, but can be an aid to WDOH and the Regional Wastewater Task Force in making that decision on a timely and scientific basis. Ecology is planning an additional study based on the Clean Water Act-based 303(d) list that identifies impaired segments and prioritizes them for development of Total Maximum Daily Load (TMDL) studies.

# Introduction

In 2002 the Food and Drug Administration (FDA) offered to send a team of specialists to Washington State to help the Washington State Department of Health (WDOH) conduct dye work at wastewater treatment plant (WWTP) outfalls. Two WWTPs were selected by WDOH for assessment: Hartstene Pointe and Shelton. This assessment was to be conducted during wintertime conditions, in order to assess higher discharge flows from these plants.

1. The Hartstene Pointe WWTP releases effluent in batch discharges, and was selected in order to compare measured plume dilutions with predicted Cornell Mixing Zone Models (CORMIX) model dilutions. These results were considered essential in determining the impact of various potential outfall configurations on nearby geoduck resources.
2. The Shelton WWTP was selected for assessment due to a proposal to expand the plant and discharge into Oakland Bay, which could impact its shellfish industry. The WDOH will not allow the existing shellfish closure zone for the Shelton WWTP outfall to expand in future flows. Therefore it was essential to measure effluent dilutions on the adverse tidal conditions that could have the most impact to the shellfish growing area. These results were then used to develop and calibrate a predictive three-dimensional computer model for the Shelton WWTP into these waters.

The Washington Department of Ecology (Ecology), in addition to being the agency responsible for administering National Pollution Discharge Elimination System (NPDES) permits, maintains an oceanographic capability in its Environmental Assessment Program at Olympia. This program already had a functional 3D model of Puget Sound called the South Puget Sound Synthesis Area Model and in it had noted a high degree of refluxing in Hammersley Inlet, adjacent to Oakland Bay. Refluxing in an estuary refers to the quantity of outflowing water in a mixing zone that returns into the estuary over many tidal cycles. Despite a large barotropic (pressure-driven) tidal exchange, tidally-averaged residual (or net) outflows were very weak and seasonal. Weak residual flow can imply a high degree of refluxing.

WDOH asked that Ecology apply the EFDC model to the Oakland Bay – Hammersley Inlet site. The City of Shelton partially funded the work described here. There was also interest in this project from the shellfish industry and the Squaxin Tribe. During this work, we consulted with all of these parties and worked interactively with the Cosmopolitan Engineering Group of Tacoma (Cosmopolitan), the consultant for the City of Shelton.

In order to frame further understanding of water circulation in this system, we summarize a series of motivating questions in Table 1.

Table 1. Five central question sets pertinent to flushing contaminants in Oakland Bay seaward:

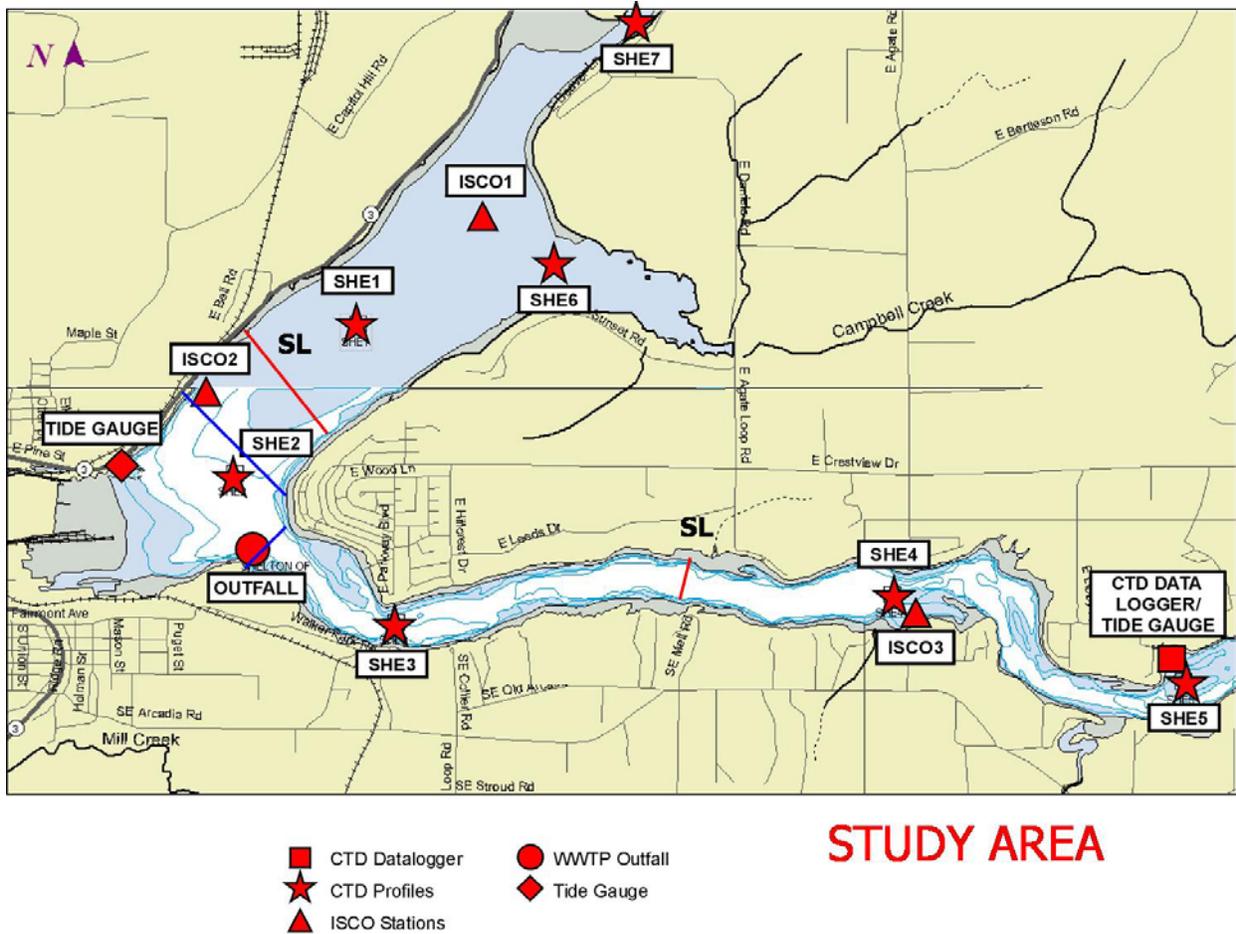
1. What is the initial dilution (near and far-field) of outfall effluent at two sanitary lines that enclose this discharge and demark a shellfish harvest closure zone? Are there exceedances of simulated fecal coliform bacteria tracers under specific discharge scenarios?
2. Once a tracer is diluted throughout the bay, how long will it take to flush out (steady-state solution)?
3. Where does the tracer go after leaving Hammersley Inlet seaward? (We can only use the complete South Puget Sound Synthesis Area Model to address this; no data were collected seaward of Libby Point in this study.)
4. How clean is the incoming water from Puget Sound?
5. What about other seasons? What is the significance of doing this work in April?

Questions 1 and 2 were addressed by the actual dye release experiment, but only for the specific conditions of mid-April 2003 and at specific locations where data were recorded. We can use the model, once validated against these real data, to address questions 3-5 and to provide a more comprehensive view (filling in data gaps along an entire sanitary line that would be impractical to gather sufficient data across). The South Puget Sound Synthesis Area Model and its larger-scale counterpart at the University of Washington, Puget Sound Regional Synthesis Model (PRISM), lacked the small-scale resolution necessary to address the questions surrounding the Shelton WWTP but would be required for questions 1, 2, and 5. WDOH wanted a resolution approaching 50 m in the vicinity of the outfall, hence the need for the Hammersley Oakland Bay Oceanographic (HOB0) model. Regarding question 4, we do not know how clean the receiving water is. We assumed that incoming water was perfectly clean and devoid of fecal coliform bacteria for these model runs. In order to make an assessment, we would need to have the full South Puget Sound Synthesis Area Model with all known fecal loadings and decay in South Puget Sound included.

## Physical Setting

Oakland Bay (Fig.1) is a broad shallow estuary connected by the narrow channel of Hammersley Inlet to outer Puget Sound. Creeks include Goldsborough, Johns, Cranberry, Shelton, Deer, Uncle Johns, Mill, Malaney, Campbell, and Creek A.

A volume of  $7.9 \times 10^7 \text{ m}^3$  of water enters Oakland Bay during a typical flood tide, which only has a volume of  $8.7 \times 10^7 \text{ m}^3$  (Oakland Bay and Hammersley Inlet combined are about  $14.8 \times 10^7 \text{ m}^3$ ). A simple tidal prism calculation results in a flushing time of slightly over a day. This is a very poor estimate because much of the same water returns (refluxes) on each subsequent flood tide, as we will demonstrate in this study.



## STUDY AREA

Figure 1. The Oakland Bay – Hammersley Inlet study area.

“SL” indicates a sanitary line location.

The outfall location is indicated with a circle, the sanitary lines (Oakland Bay and Hammersley Inlet) demarking the closure zones for shellfish harvesting are indicated as red lines on either side of the outfall, and ADCP transects are shown with blue lines.

ISCO samplers for dye collection are shown with triangles; boundary conditions were collected at 15-m intervals at the square, and a limited number of vertical CTD casts were collected at the stars.

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## Hydrology and Hydrodynamics

Hydrology of the Oakland Bay system is typical of midsize estuaries, with higher river and stream flows occurring in the late fall through late spring, and low flows occurring in the summer and early fall. From a hydrodynamic perspective, Oakland Bay is one of the more energetic estuaries in Puget Sound with two-meter tidal ranges and strong tidal currents in the navigational channel. During the high river flow portion of the year, the combination of energetic tidal mixing and fresh water flushing of the tributary rivers lessens the accumulation of material in the system and fortunately, this is deemed by WDOH the more likely time for an upset event at the WWTP to occur. However, if there were an incident in the summer or early fall, the lower river flow conditions into Oakland Bay would cause any introduced material to be retained for a longer time. For example, for an overflow event resulting in an effluent concentration of  $C_0$  in the estuary, the first-order, well-mixed rate-of-change would be:

$$dC/dt + C(Q/V_0) = 0 \quad (1)$$

where  $V_0$  = mean volume of the bay  
 $C$  = effluent concentration in the bay  
 $Q$  = freshwater inflow volume = net seaward transport (well-mixed)

The solution of this differential equation demonstrates that the concentration would be inversely proportional to the net seaward transport.

$$C(t) = C_0 e^{-t/\tau} \quad (2)$$

where  $\tau$  = flushing time of the estuary =  $V_0/Q$   
 $t$  = time  
 $C_0$  = initial concentration  
 $C(t)$  = concentration versus time

Thus in summer and early fall, when  $Q$  is lowest, there is a higher retention of effluent in the estuary. This time period was not chosen for the dye experiment because of the low risk of a release event at the Shelton plant. These equations address question 2 in Table 1.

From a hydrodynamic modeling perspective, Oakland Bay exhibits sufficient three-dimensional features to warrant a three-dimensional modeling approach. In the estuary proper, the combination of a narrow, deep navigation channel bordered by wider shallow areas requires both horizontal resolution to properly represent the channel and vertical resolution to represent the predominance of landward transport of saltier water along the channel and seaward transport over the shallow areas. Although during portions of the year, stratification may not be significant and a depth-averaged model might be appropriate, depth-averaged models are not capable of representing circulation associated with wind. For this reason, a minimum two-layer vertical resolution is desirable.

## Purpose and Scope of Project

This project was called for by regional partners (e.g., The City of Shelton, Port of Shelton, Washington Corrections Center) tasked with trying to evaluate the future wastewater collection, treatment, and discharge needs within the region. The population in this coastal area is expected to continue to increase, thereby increasing wastewater production from residential, commercial, and industrial sources. Ecology, through its NPDES permitting process, must approve new or expanded wastewater discharges. Typically, Ecology uses water quality models to evaluate the impacts of new or expanding discharges, and then sets NPDES permit limits reflecting minimum federal and state guidelines or more restrictive levels when they are needed to protect water quality standards in the receiving water.

The primary purpose of the HOBOM model is to address the farfield dilution cited under Table 1, question 1. However, this study will also address aspects of questions 2 and 3. The model could also be used predicatively for periods with wetter or drier conditions, with and without wind. Although the late summer period is not a likely time for a sewage event to occur, it is the worst time of the year for flushing in the system, due to the low river flow (buoyancy forcing), lower kinetic energy from calmer wind, and smaller average tidal exchanges near the equinox in September. We could also use the model to look at the effects of seasonal and inter-annual variability.

The resulting modeling analyses provide the City/County, WDOH, and Ecology with scientific information previously unavailable for evaluating impacts on fecal coliform bacteria counts in Oakland Bay and Hammersley Inlet. Given the model results and corresponding reliability, local and state officials should be able to use the modeling analyses to provide a scientific and technically-sound basis for altering the WWTP discharge. Additionally, the modeling results can be used to guide further water-quality model development to support Ecology's long-term goal of establishing a TMDL for the estuary.

## Scope and Approach for the Modeling Project

We selected the Environmental Fluid Dynamics Code (EFDC) model (Hamrick 1992, 1996) for this application. EFDC is an open-source, public domain, surface water modeling system incorporating fully integrated hydrodynamic, water quality, and sediment-contaminant simulation capabilities. EFDC is very versatile and can be used for 1, 2, or 3-dimensional simulation of rivers, lakes, estuaries, coastal regions, and wetlands. EFDC can be used with a water quality component that is based on water quality kinetics from the Chesapeake Bay Water Quality model or CE-QUAL-ICM (Cercio and Cole, 1994).

The justification for three-dimensional modeling stems from vertical stratification evident in Ecology's historical monitoring data in Oakland Bay (station OAK004), although less is known about lateral gradients. We would require a minimum of two layers to characterize this stratification, but four were chosen to allow for surface and bottom boundary layers.

Calibration and validation of the EFDC model was achieved using available information, plus data obtained during the Rhodamine dye release study performed by the FDA, Cosmopolitan, WDOH, and Ecology in mid April of 2003. Residence time was established with a variety of methods and compared with that of the model.

Future uses of the HOBOT model may include evaluating potential alternatives to the existing outfall structure, if modeling indicated that redesign or relocation could enhance mixing in the receiving waters. Results of one such tactic, a diversion of effluent at slack tide, is included in this report.



Figure 2. The Rhodamine dye was visible at the surface around low slack on 15 April 2003. The divers discover that there is no end cap on the diffuser.

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# Methods

## Overall Synopsis

Ecology created the HOBO model by coupling the open-source, public domain EFDC source and executable code, with specific input (.INP) files we created that relate to its specific bathymetry, shoreline, river, and stream configuration. In April 2003, we experimentally determined the flushing characteristics of Oakland Bay by three experimental methods (Cosmopolitan, 2003a): repeated ADCP transects, paired-tide gauges, and most directly with a Rhodamine dye release (Fig. 2).

FDA injected dye at the outfall of the WWTP for one full tidal cycle (24.8 h) starting at 0505 on 15 April 2003. We measured its dilution during this period at several fixed points via ISCO samplers fixed on moorings that took a sample every hour for subsequent analysis by a laboratory fluorometer (Fig. 1). We then validated the calibrated model, driven by real boundary data, against these experimental results before solving for the dilution factors for the various plant operation proposals.

Fecal coliform bacteria are indicative of organisms from the intestinal tract of humans and other animals and are used as an indicator bacteria as a measure of public health (Thomann and Mueller, 1987). In the present model, fecal coliform bacteria are interchangeable with dye and do not interact with other state variables (no die-off). These bacteria are the same as a completely conservative dye tracer, but with units that are significant to fecal bacteria levels in real situations. The critical level, established by WDOH, for longer duration simulations is 14 fecal coliform units per 100 ml. Future TMDL work will address a realistic die-off rate and other sources of fecal coliform bacteria.

## Model Development

HOBO is a three-dimensional, hydrodynamic, sigma-coordinate model similar to the Princeton Ocean Model that solves the primitive equations for a curvilinear grid of water cells that each have uniform properties throughout (i.e., a finite resolution). The model solves the Newtonian laws of fluid mechanics. Boundary conditions for the model were provided by using real observational data. Solutions are found by time-integrating the equations of motion (finite-differencing). Details are available in Hamrick, 1992.

The location of the CTD that acquired the boundary conditions of temperature and salinity determined the seaward boundary of HOBO. Effluent advects passed this location on a typical flood tide, which makes measured versus model dye results diverge with time since some dye is regrettably lost over each tidal cycle in the model.

The general procedure for the applying the EFDC model to the Oakland Bay system followed a sequence of steps beginning with model set-up or configuration. Model configuration involved the construction of a horizontal grid of the waterbody (Fig. 3) and interpolation of bathymetric

data to the grid, construction of EFDC input files, and compilation of the source code with appropriate parameter specification of array dimensions. The EFDC input files include the master input file, `efdc.inp`; files specifying the grid and bathymetry, `cell.inp`, `celllt.inp`, `dxdy.inp`, `lxly.inp`; an atmospheric forcing file, `aser.inp`; an inflow-outflow file, `qser.inp`; salinity boundary condition files, `sser.inp`; an initial salinity file, `salt.inp`; temperature boundary condition files `tser.inp`; an initial temperature file, `temp.inp`; and dye initial conditions (DYE.INP) and time series inputs (DSER.INP) (see Appendices A and B).

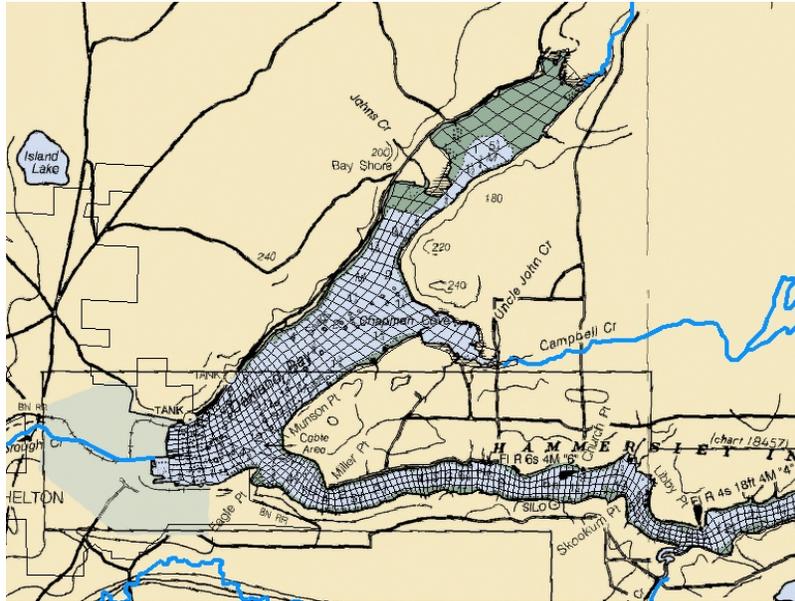


Figure 3. HOBOWAY model grid.

The outfall location from Shelton's WWTP was measured with differential GPS to be  $47^{\circ} 12.537' N \times 123^{\circ} 04.398' W$ . This is closest to South Puget Sound Synthesis Area Model cell I=54, J=14 and HOBOWAY cell I=19, J=11.

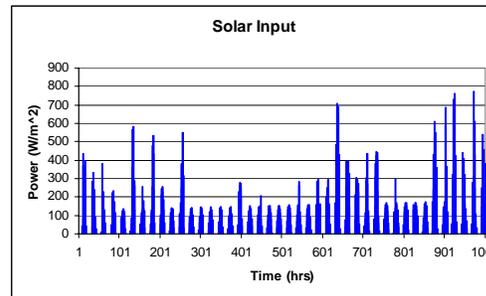
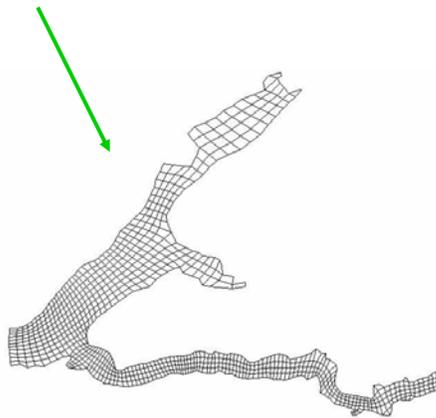
## Data Observations to Support Modeling

Numerous agencies have collected data that pertain to the Oakland Bay system. These information sources have been drawn upon extensively to set up, calibrate, and validate the 3-D HOBOT model (Fig. 4). We describe uses of specific data in detail in the subsequent sections of this report where they are applied. An overview of the data sources and types is as follows:

- National Oceanic and Atmospheric Administration – digital bathymetry and tide data
- National Weather Service – atmospheric data including observations of wind speed, wind direction, barometric pressure, air temperature, rainfall, and cloud cover from the automated station at Sanderson Field (Shelton Airport Automated Surface Observing System (ASOS))
- U.S. Geological Survey – daily river flow data for river data used to adjust small stream data
- Pacific Shellfish Growers and Cosmopolitan Engineering – ungaged stream data

### HOBOT forcings:

river flows & temperatures



sunlight, dry bulb air T,  
relative humidity, wind speed  
and direction, precipitation

(open BCs) - T, S, tidal forcing

Figure 4. HOBOT forcing functions.

## Model Configuration for the Oakland Bay System

The horizontal grid constructed for the HOB0 uses curvilinear horizontal grid cells and was constructed using the GEFDC grid generation code based on an orthogonal mapping procedure (Ryskin and Leal, 1983). The horizontal grid was constructed using NOAA Chart 18457, published in 1997, for the main estuarine regions. The horizontal cell distribution was sketched on the NOAA chart and printed copies of the tributary shoreline plots. Horizontal cell vertices along land-water boundaries were digitized and processed by GEFDC to create the complete grid. Figure 3 shows the horizontal grid.

## Model Calibration and Validation

After the model was spatially defined and gridded, and the input and run conditions were set, we conducted initial runs to evaluate its performance. These preliminary results led to refinements that allowed HOB0 to produce results more comparable to the experimental data sets we collected. The HOB0 model was calibrated to National Ocean Service tide gauge and validated against temperature, salinity, and dye data collected in support of its development.

## Discharge Dilution Analysis

We applied the calibrated and validated hydrodynamic model to evaluate initial dilution (mixing and transport) of the Shelton WWTP effluents. Our dilution calculations quantify the degree of mixing and transport of the effluent in the Oakland Bay system. For each model run, we introduced a tracer having a concentration of 46,300 fecal coliform bacteria colonies per 100 ml into the plant effluent. The Rhodamine dye we used in the mid-April experiment mimicked this concentration with a 2.954-ppm average concentration of dye. Since both tracers are conservative, all model fecal coliform bacteria results scale proportionally to the dye study. We used the hydrodynamic and transport model to simulate the distribution of the tracer in the system for a two-month period encompassing March and April 2003. HOB0 was spun-up for 45 days from March 1 to April 15 before the simulated dye releases began. Model runs terminated at day 60, which was at the end of April.

## Near-field Dilution

Cosmopolitan Engineering performed the near-field dilution analysis. We transferred dilution data from this analysis for input into HOB0 to calculate far-field dilution. Near-field dilution refers to the mixing and diffusion of the effluents in the immediate vicinity of the plant discharge (30-60 meters). Cosmopolitan ran these analyses with the Lagrangian formulation used in the Updated Merge (UM) two-dimensional diffuser formulation of EPA's PLUMES model (Baumgartner, Frick, and Roberts, 1994). Results were summarized in a Cosmopolitan Technical Memorandum (Cosmopolitan, 2003b).

The outfall discharge is located in a high-energy region of the inlet where tidal mixing dominates the dilution process. The three configurations used for the near-field dilution consisted of the original as-designed diffuser, the as-found condition with the end-cap missing and acting like a

single port discharge, and finally an extended diffuser design. The extended diffuser design simply spreads effluent over one additional column of HOB0 cells in the horizontal.

Table 2 illustrates inflows that correspond to the near-field dilutions over a tidal cycle calculated, with a specific simulation (2.6 mgd, ebb/flood condition with no end cap) listed in Table 3. The initial and final outfall flows are set to zero. Because this is an upset event, we do not need to concern ourselves with adding zero-concentration outfall flows before and after the upset event. We hold the effluent concentration constant at 46,300 fecal coliform bacteria per 100 ml, vary the flow rate, and layer distribution according to the near-field dilution and plume trapping depths calculated by PLUMES.

Table 2. Outfall flow, Q, for each of the sigma depth layers of the HOB0 model over the tidal cycle. At each time-step during the injection, flow across four layers sum to 0.1138 m<sup>3</sup>/s (2.6 mgd). Outfall effluent concentration, C, is constant at 46,300 fecal coliform units per 100 ml. The total injection rate is Q \* C.

depth day	layer1 (deep)	layer2	layer3	layer4 (shallow)	tide
45.2113	0.	0.	0.	0.	
45.2114	0.	0.0569	0.0569	0.	high
45.2844	0.	0.0569	0.0569	0.	
45.2845	0.	0.0569	0.0569	0.	
45.3573	0.	0.0569	0.0569	0.	
45.3574	0.	0.0569	0.0569	0.	ebb
45.4302	0.	0.0569	0.0569	0.	
45.4303	0.	0.	0.0569	0.0569	
45.5031	0.	0.	0.0569	0.0569	
45.5032	0.	0.	0.	0.1138	low
45.5676	0.	0.	0.	0.1138	
45.5677	0.	0.	0.0569	0.0569	
45.6235	0.	0.	0.0569	0.0569	
45.6236	0.	0.0569	0.0569	0.	flood
45.6794	0.	0.0569	0.0569	0.	
45.6795	0.	0.0569	0.0569	0.	
45.7353	0.	0.0569	0.0569	0.	
45.7354	0.	0.0569	0.0569	0.	high
45.7531	0.	0.0569	0.0569	0.	
45.7532	0.	0.	0.	0.	
∞	0.	0.	0.	0.	

## Far-field Dilution

The far-field dilution of the WWTP discharge was determined in HOB0 after initializing it with the near-field dilution calculated in PLUMES (Cosmopolitan, 2003b). We ran most of the aforementioned diffuser configurations for three distinct discharge flow rates, 2.6 mgd, 4.0 mgd, and 6.7 mgd. Adverse condition simulations were made for both the Hammersley Inlet (flood-ebb) and Oakland Bay (ebb-flood) sanitary lines. Conditions for the 15 original HOB0 model runs are summarized in Table 3.

Table 3. Conditions for the 15 original HOB0 model runs.

Run	Flow rate	Tide sequence	Diffuser configuration
1	2.6 mgd = 0.1138 m <sup>3</sup> /s	ebb/flood	no end cap
2	2.6 mgd = 0.1138 m <sup>3</sup> /s	ebb/flood	diffuser
3	4.0 mgd = 0.1750 m <sup>3</sup> /s	ebb/flood	diffuser
4	6.7 mgd = 0.2931 m <sup>3</sup> /s	ebb/flood	diffuser
5	2.6 mgd = 0.1138 m <sup>3</sup> /s	ebb/flood	extended diffuser
6	4.0 mgd = 0.1750 m <sup>3</sup> /s	ebb/flood	extended diffuser
7	6.7 mgd = 0.2931 m <sup>3</sup> /s	ebb/flood	extended diffuser
8	2.6 mgd = 0.1138 m <sup>3</sup> /s	flood/ebb	diffuser
9	4.0 mgd = 0.1750 m <sup>3</sup> /s	flood/ebb	diffuser
10	6.7 mgd = 0.2931 m <sup>3</sup> /s	flood/ebb	diffuser
11	2.6 mgd = 0.1138 m <sup>3</sup> /s	flood/ebb	extended diffuser
12	4.0 mgd = 0.1750 m <sup>3</sup> /s	flood/ebb	extended diffuser
13	6.7 mgd = 0.2931 m <sup>3</sup> /s	flood/ebb	extended diffuser
14	2.6 mgd = 0.0.1138 m <sup>3</sup> /s	flood/ebb	no end cap
15	2.6 mgd = 0.0.1138 m <sup>3</sup> /s	ebb/flood/ebb	no end cap

# Results

## Model Calibration and Validation

Our initial model results (Fig. 5) demonstrated acceptable heat and salt flux performance when tidal and wind forcing were turned off, and river flow and open boundary conditions were gradually increased to tune the grid and stabilize it. We then applied a radiative-separative condition for tidal forcing at the open eastern boundary in Hammersley Inlet. By iteratively adjusting the amplitudes and phase angles for five primary tidal constituents (M2, S2, N2, K1, and O1), we achieved a calibrated known tidal time series at an interior point (Fig. 6).

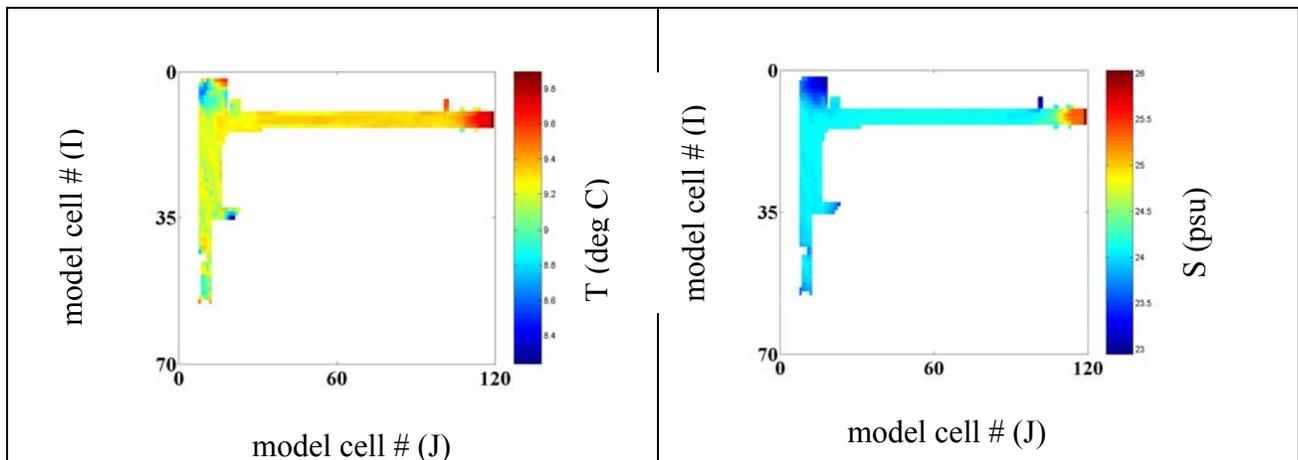


Figure 5. Surface-layer temperature and salinity on computational plane prior to adding tides in HOB0.

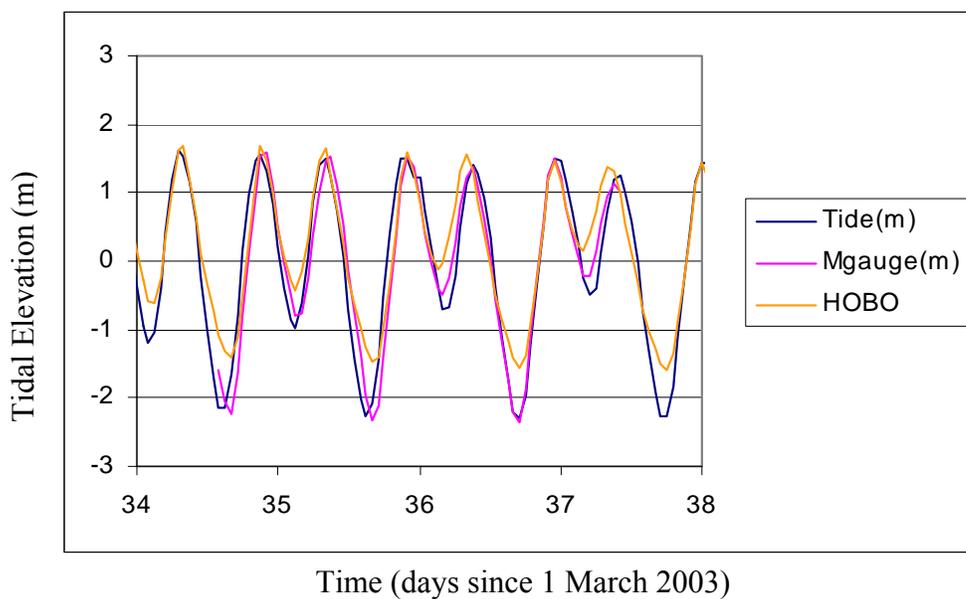


Figure 6. Calibration results for HOBO model tidal elevations.

Shown is the HOBO model output (orange) achieved after mathematical iterations against a fully-submerged pressure transducer tide-gauge (red) at an interior point. NOAA tide predictions (blue) are included for comparison.

Tidal parameters for HOBO are included in Appendix A (EFDC.INP, card 17).

Validation can be defined as the application of the calibrated model to simulate an entirely different set of prototype conditions with model performance satisfying criteria similar to those specified for calibration. Validation results between the hydrodynamic model and the dye data from the FDA injection of Rhodamine are shown in Figure 7. For the most part, the general trends between HOBO and the experiment are the same during the initial phase, which is our primary interest. The widening gap between HOBO and the experimental data is due to the fact that HOBO loses some dye out of its open boundary during each tidal cycle. Future use of HOBO (e.g., the upcoming TMDL study at Ecology) could remedy this problem by extending the model boundary closer to Pickering Passage and collecting boundary condition data (temperature and salinity) at that location (beyond the point at which injected dye advects on the ebb tide). Large log booms noted in the area (Fig. 7d) might account for the measured dye spike at the Oakland Bay mussel raft ISCO sampler due to retention of surface water (Fig. 7b, HOBO day 46.5).

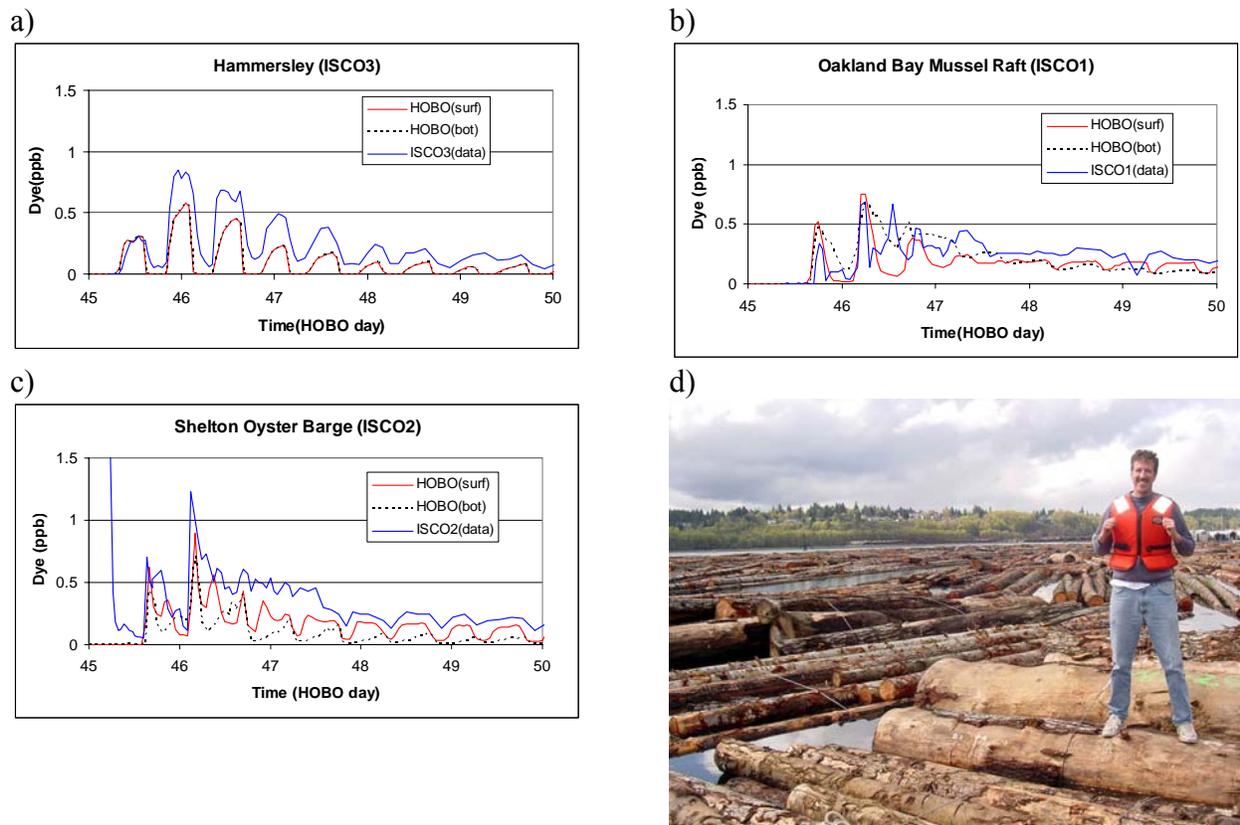


Figure 7. HOBO model duplication of experimental results in a) Hammersley Inlet (ISCO3), b) Oakland Bay at the mussel raft (ISCO1), and c) Oakland Bay at the Shelton oyster barge (ISCO2). Red represents HOBO's near-surface sigma-layer, blue represents experimental surface data. The black dashed line represents HOBO's near-bottom sigma-layer to give some idea of vertical variability. A picture of the log boom on the day of the experiment is in d).

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## Residence Time Estimation

As an overall indication of how well the model is performing, we seek to compare flushing (residence) time estimates between HOBOT and several experimental methods at our disposal. We can make the best estimate of residence time for a specific date from the data collected with the dye study.

### The Dye Study

Regarding question 2 in Table 1: Every ebb tide removed approximately 8% of the well-mixed dye (and water) from the east end of Hammersley Inlet, but every flood tide returned 92% of it. The dilution factor was such that after four days, half of the dye still remained. In addition to accurately determining the flushing characteristics of the bay for this specific time period, we also used these data to validate HOBOT.

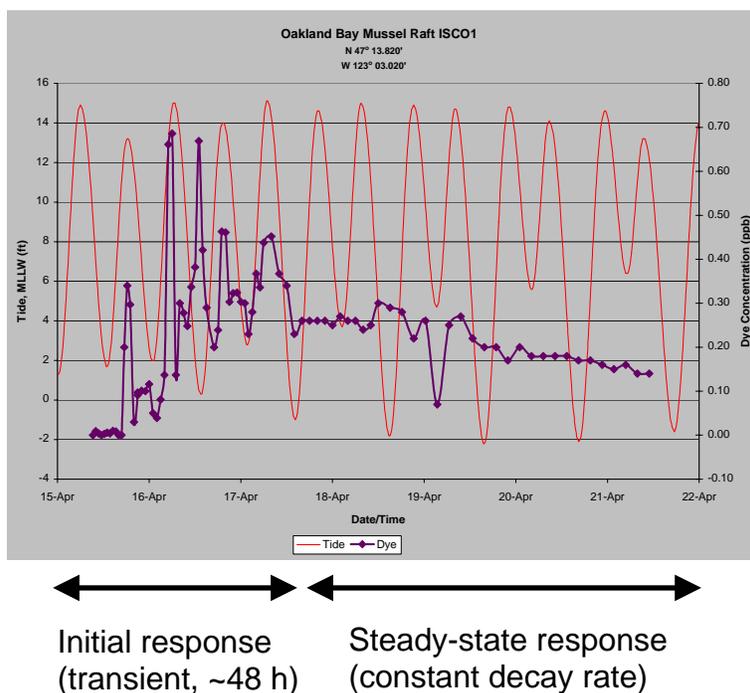


Figure 8. Time series of measured dye concentration at the Oakland Bay mussel raft (Fig. 1) on 15 Apr 2004. The concentration of effluent (bold) during the initial response phase of release is highly dependent on the initial conditions of the tide (light). During the steady-state phase, the response exhibits first-order (exponential) decay.

We need to make distinctions between the initial near-field (first 0.5 hr), initial far-field (1-48 hr), and steady-state (after 48 hr) phases of dilution (Fig. 8). Using EPA PLUMES software, Cosmopolitan Engineering determined the initial near-field dilution and trapping depth of the plume. HOBOS resolution was still too coarse in the vertical at four sigma layers to do this precisely. To determine the far-field dilution (e.g., at the sanitary lines or an ISCO sampler), the near-field results from PLUMES had to be entered into the appropriate grid cells in HOBOS, and these changed with tidal stage. In the experiment we can clearly see the chaotic initial phase during the first 48 hours where the dye concentration at a fixed point in Oakland Bay is sensitive to the initial conditions (i.e., what tide phase the dye is released).

Determination of the steady-state phase (question 2 in Table 1) is possible because the total dye injected over the 24.8-h injection period used in the experiment was high enough to saturate the bay with an observable concentration of dye. If the total dye input were uniformly dissolved throughout the volume of the bay, the dye concentration would be:

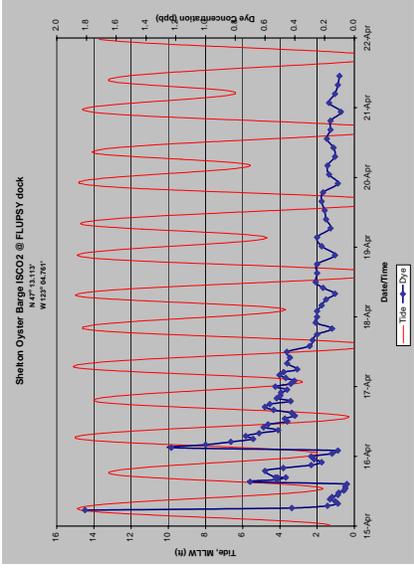
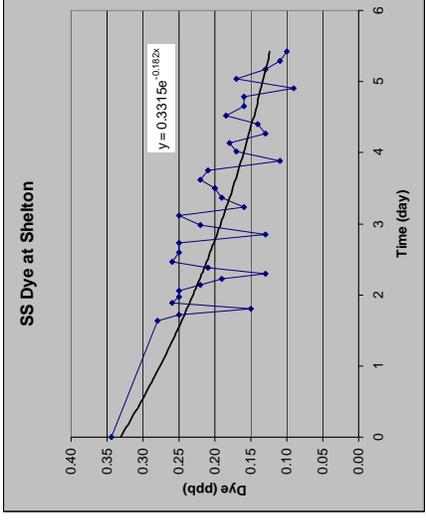
$$\frac{(2.954 \text{ ppm})(9,728 \text{ m}^3 / \text{day})(24.84 \text{ h})}{(24 \text{ h} / \text{day})(8.7 \times 10^7 \text{ m}^3)} = 0.34 \text{ ppb}$$

This initial value of 0.34 ppb plots agreeably along a best-fit, first-order exponential decay curve ( $C(t) = C_0 e^{-t/\tau}$ ) of dye concentration at the two ISCO locations within Oakland Bay (Fig. 9). The solution of the time constant,  $\tau$  (e-folding time), for these curves are 5.5 days ( $1/0.182 \text{ d}^{-1}$ ) and 6 days ( $1/0.165 \text{ d}^{-1}$ ), which agree well with each other because Oakland Bay is well-mixed over this timescale. Solving for how long it would take to flush out half the injected contamination ( $t_{50}$ ) in April 2003, we get about four days. We add a day to this result to account for the time lost between the end of the dye injection and start of the constant decay rate period. Perhaps we should add even more time because decay starts even before the end of the injection period. Therefore, we estimate from this approach that it takes over five days to flush out half of the total dye injected at the time of the experiment.

**How long does it take to flush half the dye seaward?  
Dye Study: ~4 days**

**Shelton Oyster Barge:**

**Dye =  $0.3315 e^{-0.182 t}$   
For 0.5 dye,  $t = 3.8d$**



**Oakland Bay Raft:**

**Dye =  $0.3615 e^{-0.165 t}$   
For 0.5 dye,  $t = 4.2d$**

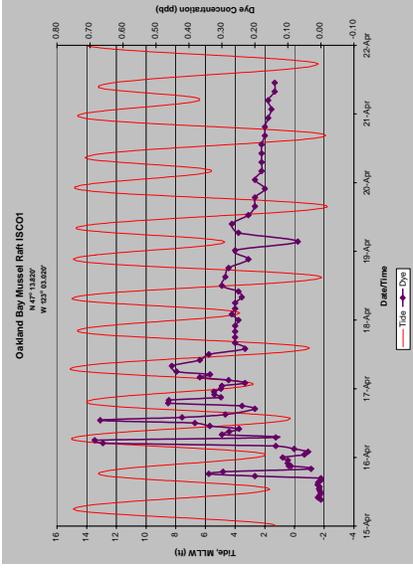
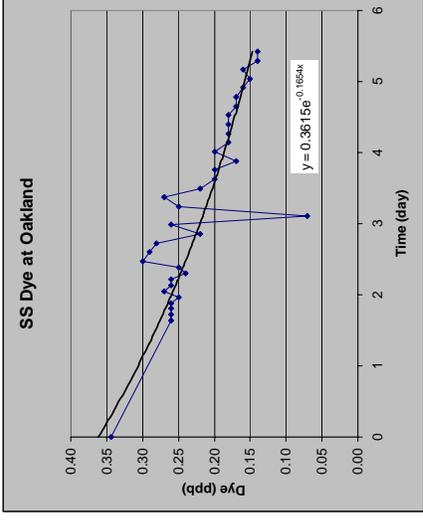


Figure 9. Dye concentration results from two stations in Oakland Bay used to estimate flushing times. Dye (bold) and tides (light) versus time as in Figure 8.

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## Twin Tide Gauges

We also used the two tide gauge stations (Fig. 1) to make an estimation of flushing, assuming that the absolute pressure readings and timing information is correct at both stations. By assuming that Hammersley Inlet can be idealized as a perfect straight channel with a uniform friction factor, we can solve a simplified hydrodynamic equation by directly integrating it with Euler's method (e.g., within Excel in VBA after Steve Chapra (Chapra, 2000).

$$\partial u / \partial t = -g \partial \eta / \partial x + (f / 8h) \bar{u} \|\bar{u}\| \quad (3)$$

where  $u$  = along-channel velocity  
 $x$  = along-channel distance  
 $h$  = mean channel depth,  $f$  = friction factor  
 $\eta$  = surface elevation due to tide  
 $g$  = gravity,  $t$  = time

Hence, we take a time series of sea-surface elevation changes ( $\eta$ ) over the channel's length ( $x$ ) and estimate acceleration, velocity, and displacement of a particle released at the outfall with an initial velocity of 0 m/s (Fig. 10). We can calculate the average channel depth,  $h$ , but must determine the friction factor  $f$ . We iterated  $f$  until we achieved velocities similar to those measured with our ADCP in Hammersley Inlet (Fig. 1).

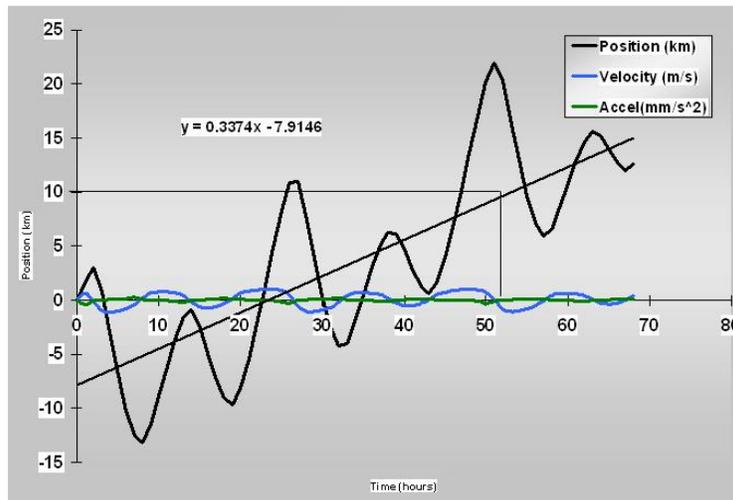


Figure 10. Approximate solution for a perfect channel with friction.  
 $f = 0.025$  N-s/m (kg/s),  $h = 6.8$  m,  $dx$  (distance between gauges) = 8.1488 km).

It takes over 50 hours (flushing time = two days) for a “drifter” released at the outfall location at time zero to make it 10 km past the model boundary. Not surprisingly, this method under predicts the flushing time as compared with the dye release approach, because the real channel is not ideal (straight and with uniform friction).

## Acoustic Doppler Current Profiler (ACDP) Velocity

We repeated two ADCP transects (Fig. 1) every 1.5 hours over the tidal cycle and published the results in a recent data report (Cosmopolitan, 2003a). It is possible to calculate the approximate mean residual flow from these data and get an idea of the net flushing time of the estuary (Fig. 11).

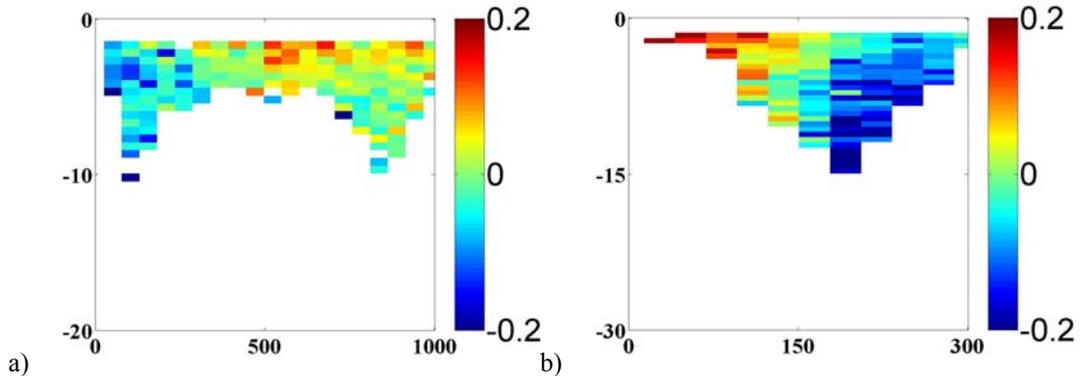


Figure 11. Residual flow as calculated by averaging eight ADCP transects on 16 April 2003 across a) Oakland Bay and b) Hammersley Inlet.

The residual exchange flow was highest in Oakland Bay at about 5,000 m<sup>3</sup>/s and lowest at the western end of Hammersley Inlet (750 m<sup>3</sup>/s). This translates into a flushing time of (750 m<sup>3</sup>/s) / (8.7 x 10<sup>7</sup> m<sup>3</sup>), which is 116,000 s or about 1.3 day, 2.6 day for half the volume.

This method under-predicts the flushing time due to a too vigorous exchange flow. This is likely because the amount of refluxing changes throughout the inlet by location. In a vertical estuary (net flow out at the surface, net flow in at depth), exchange flow generally increases from the head of the estuary toward the mouth. As rivers enter Oakland Bay on its seaward journey, the strength of the two-layered, upstream-downstream net transport is approximately proportional to the local salinity gradient.

The lowered exchange flow in Hammersley Inlet is due to the active mixing and refluxing in the inlet and is associated with a weaker salinity gradient. A similar phenomenon occurs in Tacoma Narrows where the residual mean flow is about ten percent that of the Nisqually Reach. The ADCP data, crude as the 8-cycle averaging over 13h is, shows more residual flow on our landward Oakland Bay transect and less on our seaward Hammersley Inlet transect (backwards for a vertical estuary). It is reasonable to expect that ADCP transects further east in Hammersley Inlet would produce an even smaller residual flow getting closer to the 4-5 day flushing time observed with the dye injections. This worthwhile estimate is closer to the simplistic tidal prism calculation and the tide gauge approach but once again is not as realistic as the dye value. A month-long, bottom-mounted deployment would yield better results.

## Model-Derived Net Flow

The velocity outputs from a north-to-south transect in Hammersley Inlet at the sanitary line (I=58) over four depth layers can be tidally-averaged over a lunar day. We calculated the time it takes to flush half the volume from Oakland Bay as a moving average (Fig. 12). We use the average of net surface outflow, and the near-bottom inflow. Note the presence of the fortnightly neap/spring cycle due to this short 25-hour averaging period. The residence time predicted on 15 April 2004 was about five days, which is in very good agreement with the dye experiment. The slight decrease in the trend line may be indicative of drier weather as the season proceeds.

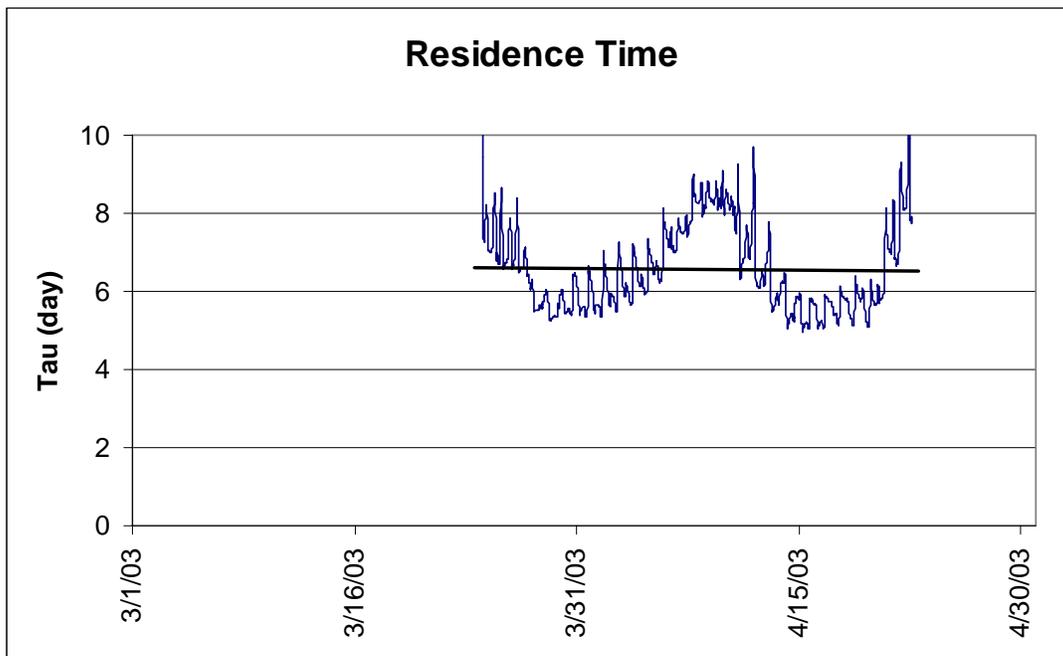


Figure 12. The calculated net-flow residence times from HOB0 output across Hammersley Inlet (I=58). Results are for below the depth-of-no-motion using a 24.8-h moving average. The trend line is a linear best fit.

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## Model Output

The distinguishing characteristic of each simulation in Table 3 is the variation in discharge rate and timing of that discharge. The outfall discharge rate,  $Q$ , changes with time and which cell or cells it affects (see Table 2). Outfall effluent concentration,  $C$ , is constant at 46,300 fecal coliform units per 100 ml so that the total injection rate is  $Q * C$ .

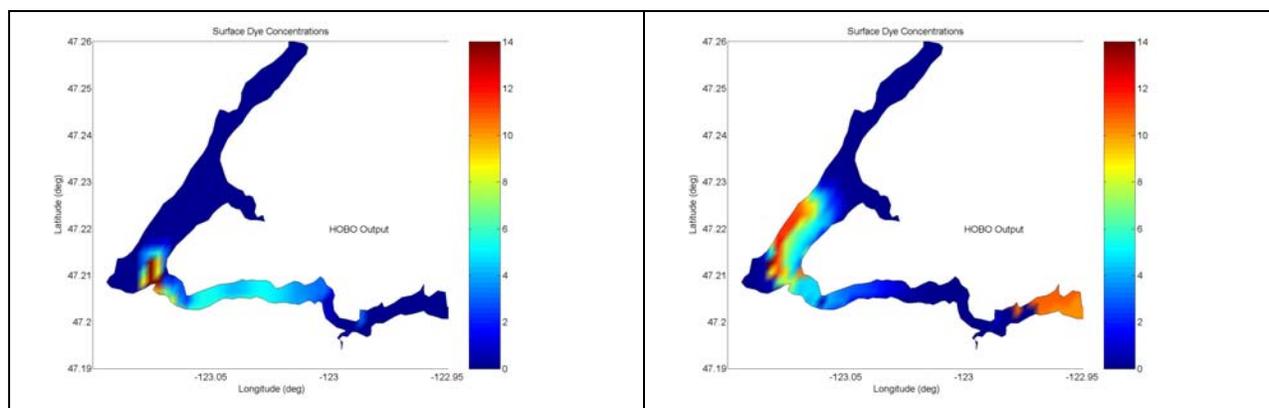


Figure 13. Plan view of the adverse condition (ebb tide after flood) for the Oakland Bay sanitary line from HOB0 run approximating actual dye-release conditions (run 15): a) time = 45.479 days, b) time = 45.531 days.

In the model output for the 15 runs, effluent generally works its way to west of the center line in Oakland Bay on the flood tide (Fig. 13b). There is, however, the potential for a large concentration on the east side of the Oakland Bay sanitary line if the release point for the effluent is too far north across Hammersley Inlet (Fig. 13a).

The location of ISCO sampler(s) used to validate HOB0 were often canted off to the side because of logistics. The location of the ISCO nearest the Oakland Bay sanitary line (ISCO2) was west of the most concentrated part of the plume (Fig. 14) but closer to the outfall than the line.

In general, the Oakland Bay sanitary line was the more critical location (Fig. 14) where there was more likely to be an exceedance of the 14 fecal coliform bacteria / 100 ml criteria than along the well-mixed sanitary line in Hammersley Inlet, which could probably be moved closer to the outfall. Repositioning the sanitary lines would call for interactive HOB0 runs to resolve the optimal location.

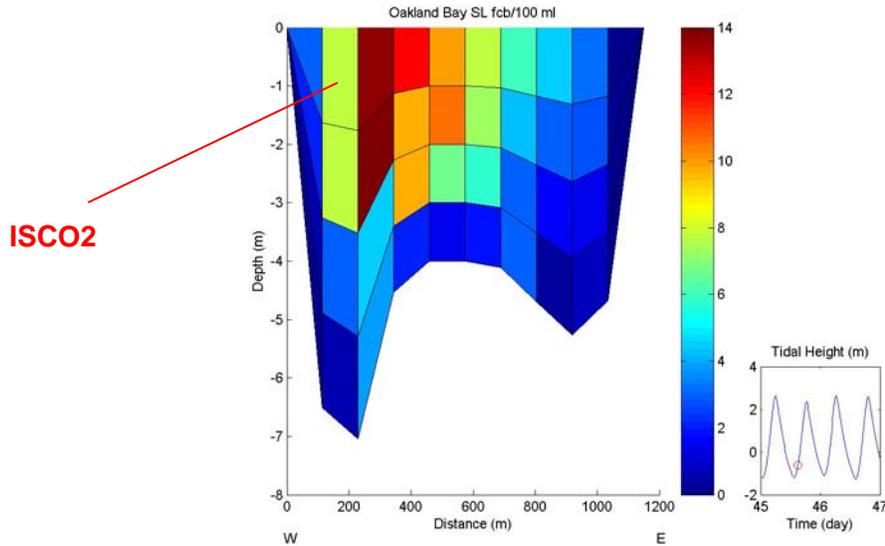
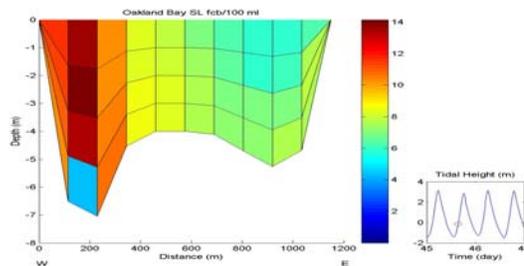


Figure 14. HOB0 simulated maximum dye concentrations across the Oakland Bay sanitary line from run 15. Note that the ISCO2 sampler is positioned west of the peak concentrations, although it is closer to the outfall than the sanitary line being modeled (see Fig. 1).



```

time(2162) =      45.645830 (3:30PM)
  11.61  13.53  10.27  8.35  7.77  6.89  6.24  5.86  5.80  6.39
  11.38  14.15  10.36  8.39  8.12  7.44  6.60  5.87  6.06  7.62
  10.94  13.39  10.48  8.46  8.28  7.58  6.93  7.02  6.92  7.80
  10.38  4.35  10.72  8.54  8.39  7.70  7.11  7.12  7.73  7.69

```

Figure 15. Fecal bacteria counts (# organisms per 100 ml) per cell for run 6 (4.0 mgd for 13 hours; ebb then flood with extended diffuser). This is the time step containing the peak concentration for the Oakland Bay sanitary line during this simulation.

The tabular results and graphic mappings of tracer concentrations for all 15 runs are included in Appendix C and are summarized in Table 4.

Table 4. Maximum fecal coliform concentrations for all 16 HOBO model runs at both sanitary lines.

Run	Oakland Bay		Hammersley Inlet	
	Total number exceedances	Maximum value (colonies per 100 ml)	Total number exceedances	Maximum value (colonies per 100 ml)
1 - 2.6 mgd no end cap, e/f	0	13	0	7
2 - 2.6 mgd diffuser, e/f	0	13	0	6
3 - 4.0 mgd diffuser, e/f	6	<b>19</b>	0	9
4 - 6.7 mgd diffuser, e/f	<b>17</b>	<b>33</b>	<b>25</b>	<b>16</b>
5 - 2.6 mgd x-diffuser, e/f	0	9	0	6
6 - 4.0 mgd x-diffuser, e/f	1	14	0	9
7 - 6.7 mgd x-diffuser, e/f	8	<b>24</b>	<b>25</b>	<b>16</b>
8 - 2.6 mgd diffuser, f/e	0	10	0	9
9 - 4.0 mgd diffuser, f/e	3	<b>15</b>	0	13
10 - 6.7 mgd diffuser, f/e	<b>15</b>	<b>26</b>	<b>29</b>	<b>22</b>
11 - 2.6 mgd x-diffuser, f/e	0	8	0	9
12 - 4.0 mgd x-diffuser, f/e	0	13	0	13
13 - 6.7 mgd x-diffuser, f/e	11	<b>21</b>	<b>29</b>	<b>22</b>
14 - 2.6 mgd no end cap, f/e	0	12	0	9
15 - 1&14 combined, e/f/e	0	13	0	9
16 - 4.6 mgd diffuser, e/f (90-m hold at low slack)	3	<b>15</b>	n/a	n/a

Runs 1-14 were made with a 13-h dye injection. Run 15 was made with conditions as close as possible to those of the actual experiment using a 24.84-h dye, to compare model output with field experimental results (e.g., Fig. 7).

After completing the original 15 runs, a 16<sup>th</sup> run (Table 5) was made to test the effects of holding back effluent at low slack tide to decrease concentrations at the Oakland Bay sanitary line. We also used a slightly higher effluent flow rate of 4.6 mgd than run 3. All the effluent held back during the 1.5-h slack tide was re-released during the remaining injection period.

Results for the 16<sup>th</sup> run showed fewer exceedances at the Oakland Bay sanitary line than in run 3. HOBO showed a maximum concentration of 15 fcbs/ 100 ml at the Oakland Bay line. As shown in Figure 2, effluent discharged during the low slack water period is poorly diluted compared to all other tidal stages. It is this poorly-diluted pool of effluent that is transported across the sanitary line at high concentrations as shown in Figure 13.

Table 5. Near-field outfall flow model input for run 16 at 4.6 mgd. Note that effluent is diverted at low slack tide and pumped out at a higher flow rate during the remainder of the tidal cycle to make up the difference. Outfall effluent concentration is again fixed at 46,300 fecal coliform units per 100 ml.

depth day	layer1 (deep)	layer2	layer3	layer4 (shallow)	tide
45.2113	0.	0.	0.	0.	
45.2114	0.	0.0654	0.0654	0.	high
45.2844	0.	0.0654	0.0654	0.	
45.2845	0.	0.0654	0.0654	0.	
45.3573	0.	0.0654	0.0654	0.	
45.3574	0.	0.0654	0.0654	0.	ebb
45.4302	0.	0.0654	0.0654	0.	
45.4303	0.	0.	0.0654	0.0654	
45.5031	0.	0.	0.0654	0.0654	
45.5032	0.	0.	0.	0.0	low
45.5676	0.	0.	0.	0.0	
45.5677	0.	0.	0.0818	0.0818	
45.6235	0.	0.	0.0818	0.0818	
45.6236	0.	0.0818	0.0818	0.	flood
45.6794	0.	0.0818	0.0818	0.	
45.6795	0.	0.0818	0.0818	0.	
45.7353	0.	0.0818	0.0818	0.	
45.7354	0.	0.0818	0.0818	0.	high
45.7531	0.	0.0818	0.0818	0.	
45.7532	0.	0.	0.	0.	
∞	0.	0.	0.	0.	

## Conclusions

The present 2.6 mgd flow rate was generally acceptable in all HOB0 simulations using it, although fecal coliform bacteria values were approaching 13 organisms per 100 ml at the Oakland Bay sanitary line (Table 4; runs 1, 2, and 15). The 6.7 mgd flow rate has multiple exceedances of the standard at both sanitary lines. The initial dilution can be greatly aided either by extending the diffuser horizontally across the inlet (runs 5-7 and 11-13), or by holding back injection at slack tide so that the tidal mixing can have more effect on it when released later (run 16). Extending the diffuser across the channel works well to a point; if it is extended too far from Eagle Point toward Munson Point, there is a danger of obtaining high counts on the east side of the Oakland Bay sanitary line (see Fig. 13a). Effluent injected too far north will potentially wrap around with poor dilution on to the east side of the Oakland Bay sanitary line. The sanitary line on Hammersley Inlet could possibly be moved further west.

Residence time estimates varied from a low of two days (tide gauge method and a straight channel) to five days (dye release and HOB0 model). The least accurate method we used was the tide gauge method because the real channel is more complex than the rectangular geometry assumed for its solution of two days. The most accurate method was the dye release experiment, but it is only valid for the date of testing. The ADCP method was intermediate and would have been more accurate if multiple bottom-mounted instruments had been deployed for a longer period of time.

Conclusions regarding the WWTP's alternatives are not made here, but this report will be given to WDOH for their interpretation and recommendations. There are several considerations regarding the interpretation of these results. Oakland Bay has high refluxing, low flushing, and a naturally high retention rate. We still do not know how tainted the incoming replacement waters are with other fecal sources from elsewhere in South Puget Sound, so we have assumed none exist. We also do not know Oakland Bay's behavior in other seasons or years. In winter, there is more discharge potential but the estuary also flushes faster due to wind and rain action from storms. In late summer, the estuary flushing is particularly slow but there is also little chance for discharge. If there were an "accident" in September, anything that goes in the water would likely remain for a very long time.

A probabilistic or Monte Carlo type investigation of the system with HOB0 could be undertaken. It appears that effluent from the Oakland Bay system would eventually head out Dana Passage (Fig. 16), but we need to verify this South Puget Sound Synthesis Area Model result experimentally. Some investigators have suggested that inlets to the west of Hope Island discharge to northern Case Inlet via Pickering Passage instead (Ebbesmeyer, 1997). We see a continued need for a comprehensive regional model (South Puget Sound Synthesis Area Model ) to address large-scale management issues for the entire South Puget Sound basin.

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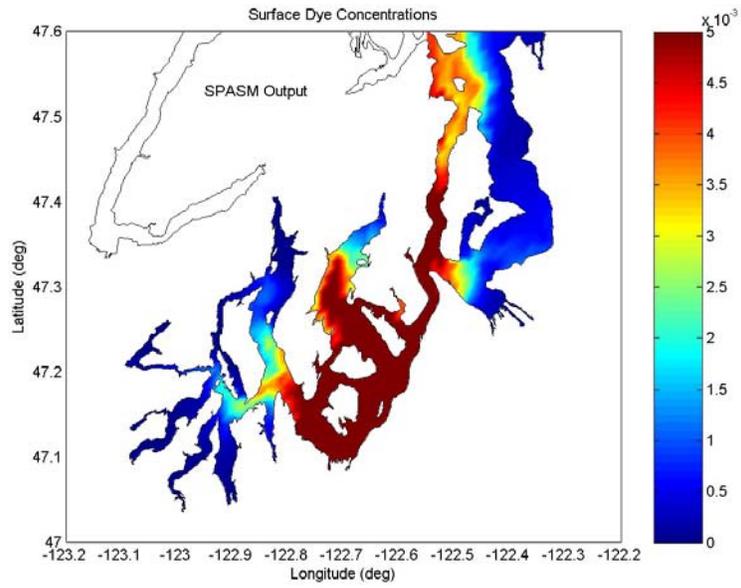


Figure 16. Apparent fate and transport of Oakland Bay effluent seaward according to the South Puget Sound Synthesis Area Model.

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## References

- Baumgartner, D.J., W.E. Frick, and P.J.W. Roberts, 1994. Dilution Models for Effluent Discharges (Third Edition). U.S. Environmental Protection Agency, Pacific Ocean Systems Branch, Newport, OR, EPA/600/R-94/086.
- Cerco, C.F. and T.M. Cole, 1994. Three-Dimensional Eutrophication Model of Chesapeake Bay. Volume I: Main Report. Technical Report EL-94-4. U.S. Army Corps of Engineers, Waterways Experiment Station, Vicksburg, MS. May 1994.
- Chapra, S., 2000. Personal communication. Tufts University, Boston, MA.
- Cosmopolitan, 2003a. Hammersley Inlet and Oakland Bay Fixed Station Dye Data. Technical memorandum from Bill Fox, Cosmopolitan Engineering Group of Tacoma, to the Shelton Area Regional Wastewater Task Force. 13 June 2003.
- Cosmopolitan, 2003b. Memorandum from Bill Fox, Cosmopolitan Engineering Group of Tacoma, to Skip Albertson, Washington State Department of Ecology. 7 September 2003.
- Ebbesmeyer, C., 1997. Personal communication. Evans Hamilton, Inc., Seattle, WA.
- Hamrick, J.M., 1992. A Three-Dimensional Environmental Fluid Dynamics Computer Code: Theoretical and Computational Aspects. The College of William and Mary, Virginia Institute of Marine Science. Special Report 317.
- Hamrick, J.M., 1996. A User's Manual for the Environmental Fluid Dynamics Computer Code (EFDC). The College of William and Mary, Virginia Institute of Marine Science. Special Report 331.
- Ryskin, G. and L.G. Leal, 1983. Orthogonal mapping. *J. Comp. Phys.*, 50, 71-100.
- Thomann, R.V. and J.A. Mueller, 1987. Principles of Surface Water Quality Modeling and Control. Harper and Row Publishers, New York, NY

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## **Appendix A**

### **Model input file for Oakland Bay (EFDC.INP)**

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*****
*
* WELCOME TO THE ENVIRONMENTAL FLUID DYNAMICS COMPUTER CODE SERIES
* DEVELOPED BY JOHN M. HAMRICK.
*
* THIS IS THE MASTER INPUT FILE efdc.inp, AND SHOULD BE USED WITH THE
* 15 AUGUST 1998 OR LATER VERSION OF efdc.f DIRECTLY RELEASED BY DEVELOPER
*
* THIS FILE IS SELF DOCUMENTED WITH DEFINITIONS AND GUIDENCE FOR EACH
* INPUT VARIABLE CONTAINED IN ITS CARD IMAGE SECTION. REFER TO USERS MAN
* AVAILABLE FROM DEVELOPER AT ham@visi.net FOR ADDITIONAL DOCUMENTATION
*
*****

```

```

C1 TITLE FOR RUN
C
  TITLE OR IDENTIFIER FOR THIS INPUT FILE AND RUN
C
C1 (LIMIT TO 80 CHARACTERS LENGTH)
  'HOBO - Hammersley Oakland Bay Oceanographic model'
-----

```

```

C2 RESTART, GENERAL CONTROL AND AND DIAGNOSTIC SWITCHES
C
  ISRESTI: 1 FOR READING INITIAL CONDITIONS FROM FILE restart.inp
            -1 AS ABOVE BUT ADJUST FOR CHANGING BOTTOM ELEVATION
            2 INTIALIZES A KC LAYER RUN FROM A KC/2 LAYER RUN FOR KC.GE.4
            10 FOR READING IC'S FROM restart.inp WRITTEN BEFORE 8 SEPT 92
  ISRESTO:-1 FOR WRITING RESTART FILE restart.out AT END OF RUN
            N INTEGER.GE.0 FOR WRITING restart.out EVERY N REF TIME PERIODS
  ISRESTR: 1 FOR WRITING RESIDUAL TRANSPORT FILE restran.out
  ISLOG:   1 FOR WRITING LOG FILE efdc.log
  ISPAR:   0 FOR EXECUTION OF CODE ON A SINGLE PROCESSOR MACHINE
            1 FOR PARALLEL EXECUTION, PARALLELIZING PRIMARILY OVER LAYERS
            2 FOR PARALLEL EXECUTION, PARALLELIZING PRIMARILY OVER
              NDM HORIZONTAL GRID SUBDOMAINS, SEE CARD CARD C9
  ISDIVEX: 1 FOR WRITING EXTERNAL MODE DIVERGENCE TO SCREEN
  ISNEGH:  1 FOR SEARCHING FOR NEGATIVE DEPTHS AND WRITING TO SCREEN
  ISMMC:   1 FOR WRITING MIN AND MAX VALUES OF SALT AND DYE
            CONCENTRATION TO SCREEN
  ISBAL:   1 FOR ACTIVATING MASS, MOMENTUM AND ENERGY BALANCES AND
            WRITING RESULTS TO FILE bal.out
  ISHP:    1 FOR CALLING HP 9000 S700 VERSIONS OF CERTAIN SUBROUTINES
  ISHOW:   1 TO SHOW PUV&S ON SCREEN, SEE INSTRUCTIONS FOR FILE show.inp
C
C2 ISRESTI ISRESTO ISRESTR ISPAR ISLOG ISDIVEX ISNEGH ISMMC ISBAL ISHP ISHOW
   0        -1         0         0         2         0         2         0         0         0         1
-----

```

```

C3 EXTERNAL MODE SOLUTION OPTION PARAMETERS AND SWITCHES
C
  RP:      OVER RELAXATION PARAMETER
  RSQM:    TRAGET SQUARE RESIDUAL OF ITERATIVE SOLUTION SCHEME
  ITERM:   MAXIMUN NUMBER OF ITERARTIONS
  IRVEC:   0 STANDARD RED-BLACK SOR SOLUTION
            1 MORE VECTORIZABLE RED-BLACK SOR (FOR RESEARCH PURPOSES)
            2 RED-BLACK ORDERED CONJUGATE GRADIENT SOLUTION
            3 REDUCED SYSTEM R-B CONJUGATE GRADIENT SOLUTION
            9 NON-DRYING CON GRADIENT SOLUTION WITH MAXIMUM DIAGNOSTICS
  RPAJ:    RELAXATION PARAMETER FOR AUXILLARY POTENTIAL ADJUSTME

```

OF THE MEAN MASS TRANSPORT ADVECTION FIELD  
(FOR RESEARCH PURPOSES)  
RSQMADJ: TRAGET SQUARED RESIDUAL ERROR FOR ADJUSTMENT  
(FOR RESEARCH PURPOSES)  
ITRMADJ: MAXIMUM ITERARTIONS FOR ADJUSTMENT(FOR RESEARCH PURPOSES)  
ITERHPM: MAXIMUM ITERATIONS FOR STRONGLY NONLINER DRYING AND WETTING  
SCHEME (ISDRY=3 OR OR 4) ITERHPM.LE.4  
IDRYCK: ITERATIONS PER DRYING CHECK (ISDRY.GE.1) 2.LE.IDRYCK.LE.20  
ISDSOLV: 1 TO WRITE DIAGNOSTICS FILES FOR EXTERNAL MODE SOLVER  
FILT: FILTER COEFFICIENT FOR 3 TIME LEVEL EXPLICIT ( 0.0625 )  
C 1.E-3  
C3 RP RSQM ITERM IRVEC RPA DJ RSQMADJ ITRMADJ ITERHPM IDRYCK ISDSOLV FILT  
1.8 1.E-5 20000 9 1.8 1.E-16 1500 0 20 0 0.0625

C4 LONGTERM MASS TRANSPORT INTEGRATION ONLY SWITCHES

C  
ISLTMT: 1 FOR LONG-TERM MASS TRANSPORT ONLY (FOR RESEARCH PURPOSES)  
ISSSMT: 0 WRITES MEAN MASS TRANSPORT TO restran.out AFTER EACH  
AVERAGING PERIOD (FOR RESEARCH PURPOSES)  
1 WRITES MEAN MASS TRANSPORT TO restran.out AFTER LAST  
AVERAGING PERIOD (FOR RESEARCH PURPOSES)  
ISLTMTS: 0 ASSUMES LONG-TERM TRANSPORT SOLUTION IS TRANSIENT  
(FOR RESEARCH PURPOSES)  
1 ASSUMES LONG-TERM TRANSPORT SOLUTION IS ITERATED TOWARD  
STEADY STATE (FOR RESEARCH PURPOSES)  
ISIA: 1 FOR IMPLICIT LONG-TERM ADVECTION INTEGRATION FOR ZEBRA  
VERTICAL LINE R-B SOR (FOR RESEARCH PURPOSES)  
RPIA: RELAXATION PARAMETER FOR ZEBRA SOR(FOR RESEARCH PURPOSES)  
RSQMIA: TRAGET RESIDUAL ERROR FOR ZEBRA SOR (FOR RESEARCH PURPOSES)  
ITRMIA: MAXIMUM ITERATIONS FOR ZEBRA SOR (FOR RESEARCH PURPOSES)

C  
C4 ISLTMT ISSSMT ISLTMTS ISIA RPIA RSQMIA ITRMIA  
0 1 0 0 1.8 1.E-10 100

C5 MOMENTUM ADVEC AND HORIZ DIFF SWITCHES AND MISC SWITCHES

C  
ISCDMA: 1 FOR CENTRAL DIFFERENCE MOMENTUM ADVECTION  
0 FOR UPWIND DIFFERENCE MOMENTUM ADVECTION  
2 FOR EXPERIMENTAL UPWIND DIFF MOM ADV (FOR RESEACH PURPOSES)  
ISHDMF: 1 TO ACTIVE HORIZONTAL MOMENTUM DIFFUSION  
ISDISP: 1 CALCULATE MEAN HORIZONTAL SHEAR DISPERSION TENSOR OVER LAST  
MEAN MASS TRANSPORT AVERAGING PERIOD  
ISWASP: 4 or 5 TO WRITE FILES FOR WASP4 or WASP5 MODEL LINKAGE  
ISDRY: GREATER THAN 0 TO ACTIVE WETTING & DRYING OF SHALLOW AREAS  
1 CONSTANT WETTING DEPTH SPECIFIED BY HWET ON CARD 11  
WITH NONLINEAR ITERATIONS SPECIFIED BY ITERHPM ON CARD C3  
2 VARIABLE WETTING DEPTH CALCULATED INTERNALLY IN CODE  
WITH NONLINEAR ITERATIONS SPECIFIED BY ITERHPM ON CARD C3  
11 SAME AS 1, WITHOUT NONLINEAR ITERATION  
12 SAME AS 2, WITHOUT NONLINEAR ITERATION  
3 DIFFUSION WAVE APPROX, CONSTANT WETTING DEPTH (NOT ACTIVE)  
4 DIFFUSION WAVE APPROX, VARIABLE WETTING DEPTH (NOT ACTIVE)  
ISQQ: 1 TO USE STANDARD TURBULENT INTENSITY ADVECTION SCHEME  
ISRLID: 1 TO RUN IN RIGID LID MODE (NO FREE SURFACE)  
ISVEG: 1 TO IMPLEMENT VEGETATION RESISTANCE  
2 IMPLEMENT WITH DIAGNOSTICS TO FILE cbot.log  
ISVEGL: 1 TO INCLUDE LAMINAR FLOW OPTION IN VEGETATION RESISTANCE

```

ISITB: 1 FOR IMPLICIT BOTTOM & VEGETATION RESISTANCE IN EXTERNAL MODE
        FOR SINGLE LAYER APPLICATIONS (KC=1) ONLY
ISEVER: 1 TO DEFAULT TO EVERGLADES HYDRO SOLUTION OPTIONS
C
11
C5 ISCDMA ISHDMF ISDISP ISWASP ISDRY ISQQ ISRLID ISVEG ISVEGL ISITB ISEVER
    0      0      0      0      0      1      0      0      0      0      0

```

```

C6 DISSOLVED AND SUSPENDED CONSTITUENT TRANSPORT SWITCHES
C6 TURB INT=0,SAL=1,TEM=2,DYE=3,SFL=4,TOX=5,SED=6,SND=7,CWQ=8
C

```

```

ISTRAN: 1 OR GREATER TO ACTIVATE TRANSPORT
ISTOPT: NONZERO FOR TRANSPORT OPTIONS, SEE USERS MANUAL
ISCDCA: 0 FOR STANDARD DONOR CELL UPWIND DIFFERENCE ADVECTION
        1 FOR CENTRAL DIFFERENCE ADVECTION FOR THREE TIME LEVEL STEPS
        2 FOR EXPERIMENTAL UPWIND DIFFERENCE ADVECTION (FOR RESEARCH)
ISADAC: 1 TO ACTIVATE ANTI-NUMERICAL DIFFUSION CORRECTION TO
        STANDARD DONOR CELL SCHEME
ISFCT: 1 TO ADD FLUX LIMITING TO ANTI-NUMERICAL DIFFUSION CORRECTION
ISPLIT: 1 TO OPERATOR SPLIT HORIZONTAL AND VERTICAL ADVECTION
        (FOR RESEARCH PURPOSES)
ISADAH: 1 TO ACTIVATE ANTI-NUM DIFFUSION CORRECTION TO HORIZONTAL
        SPLIT ADVECTION STANDARD DONOR CELL SCHEME (FOR RESEARCH)
ISADAV: 1 TO ACTIVATE ANTI-NUM DIFFUSION CORRECTION TO VERTICAL
        SPLIT ADVECTION STANDARD DONOR CELL SCHEME (FOR RESEARCH)
ISCI: 1 TO READ CONCENTRATION FROM FILE restart.inp
ISCO: 1 TO WRITE CONCENTRATION TO FILE restart.out
C

```

```

C6 ISTRAN ISTOPT ISCDCA ISADAC ISFCT ISPLIT ISADAH ISADAV ISCI ISCO
    1      0      0      0      0      0      0      0      0      0      0      !turb 0
    1      1      0      1      1      0      0      0      1      1      1      !sal 1
    1      1      0      1      1      0      0      0      1      1      1      !tem 2
    1      1      0      1      1      0      0      0      1      1      1      !dye 3
    0      0      0      1      1      0      0      0      0      0      0      !sfl 4
    0      0      0      1      1      0      0      0      0      0      0      !tox 5
    0      0      0      1      1      0      0      0      0      0      0      !sed 6
    0      0      0      1      1      0      0      0      0      0      0      !snd 7
    0      0      0      1      1      0      0      0      0      0      0      !cwq 8

```

```

C7 TIME-RELATED INTEGER PARAMETERS
C

```

```

NTC: NUMBER OF REFERENCE TIME PERIODS IN RUN
NTSPTC: NUMBER OF TIME STEPS PER REFERENCE TIME PERIOD
NLTC: NUMBER OF LINEARIZED REFERENCE TIME PERIODS
NLTC: NUMBER OF TRANSITION REF TIME PERIODS TO FULLY NONLINEAR
NTCPP: NUMBER OF REFERENCE TIME PERIODS BETWEEN FULL PRINTED OUTPUT
        TO FILE efdc.out
NTSTBC: NUMBER OF REFERENCE TIME PERIODS BETWEEN TWO TIME LEVEL
        TRAPEZOIDAL CORRECTION TIME STEP
NTCNB: NUMBER OF REFERENCE TIME PERIODS WITH NO BUOYANCY FORCING
NTCVB: NUMBER OF REF TIME PERIODS WITH VARIABLE BUOYANCY FORCING
NTCMMT: NUMBER OF NUMBER OF REF TIME TO AVERAGE OVER TO OBTAIN
        RESIDUAL OR MEAN MASS TRANSPORT VARIABLES
NFLTMT: USE 1 (FOR RESEARCH PURPOSES)
NDRYSTP: MIN NO. OF TIME STEPS A CELL REMAINS DRY AFTER INTIAL DYRING
C

```

```

C7 NTC NTSPTC NLTC NTTC NTCPP NTSTBC NTCNB NTCVB NTSMMT NFLTMT NDRYSTP
    30  8640    0    0    30    4    0    0    29    1    16

```

C8 TIME-RELATED REAL PARAMETERS

C

TCON: CONVERSION MULTIPLIER TO CHANGE TBEGIN TO SECONDS  
 TBEGIN: TIME ORIGIN OF RUN  
 TREF: REFERENCE TIME PERIOD IN SEC (ie 44714.16s or 86400s)  
 CORIOLIS: CONSTANT CORIOLIS PARAMETER IN 1/SEC  
 ISCORV: 1 TO READ VARIABLE CORIOLIS COEFFICIENT FROM lxly.inp FILE  
 ISCCA: WRITE DIAGNOSTICS FOR MAX CORIOLIS-CURV ACCEL TO FILEefdc.log  
 ISCFL: 1 WRITE DIAGNOSTICS OF MAX THEORETICAL TIME STEP TO cfl.out  
 GT 1 TIME STEP ONLY AT INTERVAL ISCFL FOR ENTIRE RUN  
 ISCFLM: 1 TO MAP LOCATIONS OF MAX TIME STEPS OVER ENTIRE RUN

C

1247.125/1186.125/1171.1250/1096.125

C8	TCON	TBEGIN	TREF	CORIOLIS	ISCORV	ISCCA	ISCFL	ISCFLM
	86400.	23.125	86400.	1.1E-4	0	0	0	0

C9 SPACE-RELATED AND SMOOTHING PARAMETERS

C

KC: NUMBER OF VERTICAL LAYER  
 IC: NUMBER OF CELLS IN I DIRECTION  
 JC: NUMBER OF CELLS IN J DIRECTION  
 LC: NUMBER OF ACTIVE CELLS IN HORIZONTAL + 2  
 LVC: NUMBER OF VARIABLE SIZE HORIZONTAL CELLS  
 ISCO: 1 FOR CURVILINEAR-ORTHOGONAL GRID (LVC=LC-2)  
 NDM: NUMBER OF DOMAINS FOR HORIZONTAL DOMAIN DECOMPOSITION  
 ( NDM=1, FOR MODEL EXECUTION ON A SINGLE PROCESSOR SYSTEM OR  
 NDM=MM\*NCPUS, WHERE MM IS AN INTEGER AND NCPUS IS THE NUMBER  
 OF AVAILABLE CPU'S FOR MODEL EXECUTION ON A PARALLEL  
 MULTIPLE PROCESSOR SYSTEM )  
 LDW: NUMBER OF WATER CELLS PER DOMAIN  
 ( LDW=(LC-2)/NDM, FOR MULTIPLE VECTOR PROCESSORS, LDW MUST BE  
 AN INTEGER MULTIPLE OF THE VECTOR LENGTH OR STRIDE NVEC  
 THUS CONSTRAINING LC-2 TO BE AN INTEGER MULTIPLE OF NVEC )  
 ISMASK: 1 FOR MASKING WATER CELL TO LAND OR ADDING THIN BARRIERS  
 USING INFORMATION IN FILE mask.inp  
 ISPGNS: 1 FOR IMPLEMENTING A PERIODIC GRID IN COMP N-S DIRECTION OR  
 CONNECTING ARBITRARY CELLS USING INFO IN FILE mappgns.inp  
 NSHMAX: NUMBER OF DEPTH SMOOTHING PASSES  
 NSBMAX: NUMBER OF INITIAL SALINITY FIELD SMOOTHING PASSES  
 WSMH: DEPTH SMOOTHING WEIGHT  
 WSMB: SALINITY SMOOTHING WEIGHT

C

C9	KC	IC	JC	LC	LVC	ISCO	NDM	LDW	ISMASK	ISPGNS	NSHMX	NSBMX	WSMH	WSMB
	4	120	70	868	866	1	1	866	0	0	1	1	0.0625	0.0625

C10 LAYER THICKNESS IN VERTICAL

C

THICKNESS OF EACH VERTICAL LAYER, 1 = BOTTOM  
 LAYER THICKNESSES MUST SUM TO 1.0

C

C10	LAYER NUMBER	DIMENSIONLESS LAYER THICKNESS
	1	0.25
	1	0.25
	1	0.25
	1	0.25

-----  
 C11 GRID, ROUGHNESS AND DEPTH PARAMETERS

C  
 DX: CARTESIAN CELL LENGTH IN X OR I DIRECTION  
 DY: CARTESIAN CELL LENGTH IN Y OR J DIRECTION  
 DXYCVT: MULTIPLY DX AND DY BY TO OBTAIN METERS  
 IMD: GREATER THAN 0 TO READ MODDXDY.INP FILE  
 ZBRADJ: LOG BDRY LAYER CONST OR VARIABLE ROUGH HEIGHT ADJ IN METERS  
 ZBRCVRT: LOG BDRY LAYER VARIABLE ROUGHNESS HEIGHT CONVERT TO METERS  
 HMIN: MINIMUM DEPTH OF INPUTS DEPTHS IN METERS  
 HADJ: ADJUSTMENT TO DEPTH FIELD IN METERS  
 HCVRT: CONVERTS INPUT DEPTH FIELD TO METERS  
 HDRY: DEPTH AT WHICH CELL OR FLOW FACE BECOMES DRY  
 HWET: DEPTH AT WHICH CELL OR FLOW FACE BECOMES WET  
 BELADJ: ADJUSTMENT TO BOTTOM BED ELEVATION FIELD IN METERS  
 BELCVRT: CONVERTS INPUT BOTTOM BED ELEVATION FIELD TO METERS

C  
 C11 DX DY DXYCVT IMD ZBRADJ ZBRCVRT HMIN HADJ HCVT HDRY HWET BELADJ BELCVT  
 1. 1. 1. 0 0.005 0.0 0.5 0.2 1.0 0.11 0.16 -0.2 1.00

-----  
 C12 TURBULENT DIFFUSION PARAMETERS

C  
 AHO: CONSTANT HORIZONTAL MOMENTUM AND MASS DIFFUSIVITY M\*M/S  
 AHD: DIMENSIONLESS HORIZONTAL MOMENTUM DIFFUSIVITY  
 AVO: BACKGROUND, CONSTANT OR MOLECULAR KINEMATIC VISCOSITY M\*M/S  
 ABO: BACKGROUND, CONSTANT OR MOLECULAR DIFFUSIVITY M\*M/S  
 AVMN: MINIMUM KINEMATIC EDDY VISCOSITY M\*M/S  
 ABMN: MINIMUM EDDY DIFFUSIVITY M\*M/S  
 AVBCON: EQUALS ZERO FOR CONSTANT VERTICAL VISCOSITY AND DIFFUSIVITY  
 WHICH ARE SET EQUAL TO AVO AND ABO OTHERWISE SET TO 1.0  
 ISAVBMN: SET TO 1 TO ACTIVATE MIN VIS AND DIFF OF AVMN AND ABMN  
 ISFAVB: SET TO 1 OR 2 TO AVG OR SQRT FILTER AVV AND AVB  
 ISINWV: SET TO 1 TO ACTIVATE PARAMETERIZATION OF INTERNAL WAVE  
 GENERATED TURBULENCE

C  
 C12 AHO AHD AVO ABO AVMN ABMN AVBCON ISAVBMN ISFAVB ISINWV  
 0.0 0.0 1.E-6 1.E-8 1.E-6 1.E-8 1.0 0 1 0

-----  
 C13 TURBULENCE CLOSURE PARAMETERS

C  
 VKC: VON KARMAN CONSTANT  
 CTURB1: TURBULENT CONSTANT (UNIVERSAL)  
 CTURB2: TURBULENT CONSTANT (UNIVERSAL)  
 CTE1: TURBULENT CONSTANT (UNIVERSAL)  
 CTE2: TURBULENT CONSTANT (UNIVERSAL)  
 CTE3: TURBULENT CONSTANT (UNIVERSAL)  
 QQMIN: MINIMUM TURBULENT INTENSITY SQUARED  
 QQLMIN: MINIMUM TURBULENT INTENSITY SQUARED TIME MACRO-SCALE  
 DMLMIN: MINIMUM DIMENSIONLESS MACRO-SCALE

C  
 C13 VKC CTURB1 CTURB2 CTE1 CTE2 CTE3 QQMIN QQLMIN DMLMIN  
 0.4 16.6 10.1 1.8 1.33 0.53 1.E-8 1.E-12 1.E-4

-----  
 C14 TIDAL & ATMOSPHERIC FORCING, GROUND WATER AND SUBGRID CHANNEL PARAMETERS

C  
 MTIDE: NUMBER OF PERIOD (TIDAL) FORCING CONSTITUENTS  
 NWSER: NUMBER OF WIND TIME SERIES (0 SETS WIND TO ZERO)

NASER : NUMBER OF ATMOSPHERIC CONDITION TIME SERIES (0 SETS ALL ZERO)  
 ISGWI: 1 TO ACTIVATE SOIL MOISTURE BALANCE WITH DRYING AND WETTING  
 ISCHAN: 1 ACTIVATE SUBGRID CHANNEL MODEL AND READ MODCHAN.INP  
 ISWAVE 1 FOR WAVE CURRENT BOUNDARY LAYER REQUIRES FILE wave.inp  
 2 FOR WCBL AND WAVE INDUCED CURRENTS REQUIRES FILE wave.inp

C

C14	MTIDE	NWSER	NASER	ISGWI	ISCHAN	ISWAVE	ITIDASM
	5	1	1	0	0	0	0

C15 PERIODIC FORCING (TIDAL) CONSTITUENT SYMBOLS AND PERIODS

C

SYMBOL: FORCING SYMBOL (CHARACTER VARIABLE) FOR TIDES, THE NOS SYMBOL  
 PERIOD: FORCING PERIOD IN SECONDS

C

C15	SYMBOL	PERIOD
	'M2'	44714.1643936 1
	'S2'	43200.0000000 2
	'N2'	45570.0536814 3
	'K1'	86164.0907615 4
	'O1'	92949.6299931 5

C16 SURFACE ELEVATION OR PRESSURE BOUNDARY CONDITION PARAMETERS

C

NPBS: NUMBER OF SURFACE ELEVATION OR PRESSURE BOUNDARY CONDITIONS  
 CELLS ON SOUTH OPEN BOUNDARIES  
 NPBW: NUMBER OF SURFACE ELEVATION OR PRESSURE BOUNDARY CONDITIONS  
 CELLS ON WEST OPEN BOUNDARIES  
 NPBE: NUMBER OF SURFACE ELEVATION OR PRESSURE BOUNDARY CONDITIONS  
 CELLS ON EAST OPEN BOUNDARIES  
 NPBN: NUMBER OF SURFACE ELEVATION OR PRESSURE BOUNDARY CONDITIONS  
 CELLS ON NORTH OPEN BOUNDARIES  
 NPFOR: NUMBER OF HARMONIC FORCINGS  
 NPSE: NUMBER OF TIME SERIES FORCINGS  
 PDGINIT: ADD THIS CONSTANT ADJUSTMENT GLOBALLY TO THE SURFACE ELEVATION

C

C16	NPBS	NPBW	NPBE	NPBN	NPFOR	NPSE	PDGINIT
	0	0	4	0	1	0	0.0

C17 PERIODIC FORCING (TIDAL) SURF ELEV OR PRESSURE BOUNDARY COND. FORCINGS

C

NPFOR: FORCING NUMBER  
 SYMBOL: FORCING SYMBOL (FOR REFERENCE HERE ONLY)  
 AMPLITUDE: AMPLITUDE IN M (PRESSURE DIVIDED BY RHO\*G)  
 PHASE: FORCING PHASE RELATIVE TO TBEGIN IN SECONDS

C

C17	NPFOR	SYMBOL	AMPLITUDE	PHASE
	1	'M2'	1.60	14638.
	1	'S2'	0.65	21000.
	1	'N2'	0.35	27100.
	1	'K1'	0.80	22000.
	1	'O1'	0.61	41000.

C18 PERIODIC FORCING (TIDAL) SURF ELEV OR PRESSURE ON SOUTH OPEN BOUNDARIES

C

IPBS: I CELL INDEX OF BOUNDARY CELL  
 JPBS: J CELL INDEX OF BOUNDARY CELL  
 ISPBS: 1 FOR RADIATION-SEPARATION CONDITION

0 FOR ELEVATION SPECIFIED

NPFORS: APPLY HARMONIC FORCING NUMBER NPFORS  
NPSERS: APPLY TIME SERIES FORCING NUMBER NPSERS

C

C18 IPBS JPBS ISPBS NPFORS NPSERS

C19 PERIODIC FORCING (TIDAL) SURF ELEV OR PRESSURE ON WEST OPEN BOUNDARIES

C

IPBW: SEE CARD 19  
JPBW:  
ISPBW:  
NPFORW:  
NPSERW:

C

C19 IPBW JPBW ISPBW NPFORW NPSERW

C20 PERIODIC FORCING (TIDAL) SURF ELEV OR PRESSURE ON EAST OPEN BOUNDARIES

C

IPBE: SEE CARD 19  
JPBE:  
ISPBE:  
NPFORW:  
NPSERE:

C

C20	IPBE	JPBE	ISPBE	NPFORW	NPSERE
	119	10	0	1	0
	119	11	0	1	0
	119	12	0	1	0
	119	13	0	1	0

C21 PERIODIC FORCING (TIDAL) SURF ELEV OR PRESSURE ON NORTH OPEN BOUNDARIES

C

IPBN: SEE CARD 19  
JPBN:  
ISPBN:  
NPFORN:  
NPSERN:

C

C21 IPBN JPBW ISPBN NPFORN NPSERN

C22 SPECIFY NUM OF SEDIMENT AND TOXICS AND NUM OF CONCENTRATION TIME SERIES

C

NTOX: NUMBER OF TOXIC CONTAMINANTS (DEFAULT = 1)  
NSEED: NUMBER OF COHESIVE SEDIMENT SIZE CLASSES (DEFAULT = 1)  
NSND: NUMBER OF NON-COHESIVE SEDIMENT SIZE CLASSES (DEFAULT = 1)  
NSSER: NUMBER OF SALINITY TIME SERIES  
NTSER: NUMBER OF TEMPERATURE TIME SERIES  
NDSER: NUMBER OF DYE CONCENTRATION TIME SERIES  
NSFSER: NUMBER OF SHELLFISH LARVAE CONCENTRATION TIME SERIES  
NTXSER: NUMBER OF TOXIC CONTAMINANT CONCENTRATION TIME SERIES  
EACH TIME SERIES MUST HAVE DATA FOR NTOX TOXICICANTS  
NSDSER: NUMBER OF COHESIVE SEDIMENT CONCENTRATION TIME SERIES  
EACH TIME SERIES MUST HAVE DATA FOR NSED COHESIVE SEDIMENTS  
NSNSER: NUMBER OF NONCOHESIVE SEDIMENT CONCENTRATION TIME SERIES  
EACH TIME SERIES MUST HAVE DATA FOR NSND NON-COHESIVE SEDIMENTS  
ISDBAL: SET TO 1 FOR SEDIENT MASS BALANCE

C

C22	NTOX	NSED	NSND	NSSER	NTSER	NDSER	NSFSER	NTXSER	NSDSER	NSNSER	ISSBAL
	1	1	1	2	2	1	0	0	0	0	0

C23 VELOCITY, VOLUMN SOURCE/SINK, FLOW CONTROL, AND WITHDRAWAL/RETURN DATA  
C

NVBS: NUMBER OF VELOCITY BC'S ON SOUTH OPEN BOUNDARIES  
 NUBW: NUMBER OF VELOCITY BC'S ON WEST OPEN BOUNDARIES  
 NUBE: NUMBER OF VELOCITY BC'S ON EAST OPEN BOUNDARIES  
 NVBN: NUMBER OF VELOCITY BC'S ON NORTH OPEN BOUNDARIES  
 NQSIJ: NUMBER OF CONSTANT AND/OR TIME SERIES SPECIFIED SOURCE/SINK LOCATIONS (RIVER INFLOWS,ETC)  
 NQJPIJ: NUMBER OF CONSTANT AND/OR TIME SERIES SPECIFIED SOURCE LOCATIONS TREATED AS JETS/PLUMES  
 NQSER: NUMBER OF VOLUMN SOURCE/SINK TIME SERIES  
 NQCTL: NUMBER OF PRESSURE CONTROLLED WITHDRAWAL/RETURN PAIRS  
 NQCTLT: NUMBER OF PRESSURE CONTROLLED WITHDRAWAL/RETURN TABLES  
 NQWR: NUMBER OF CONSTANT OR TIME SERIES SPECIFIED WITHDRAWL/RETURN PAIRS  
 NQWRSR: NUMBER OF TIME SERIES SPECIFYING WITHDRAWL,RETURN AND CONCENTRATION RISE SERIES  
 ISDIQ: SET TO 1 TO WRITE DIAGNOSTIC FILE, diaq.out

C23	NVBS	NUBW	NUBE	NVBN	NQSIJ	NQJPIJ	NQSER	NQCTL	NQCTLT	NQWR	NQWRSR	ISDIQ
	0	0	0	0	12	0	12	0	0	0	0	0

C24 VOLUMETRIC SOURCE/SINK LOCATIONS, MAGNITUDES, AND CONCENTRATION SERIES  
C

IQS: I CELL INDEX OF VOLUME SOURCE/SINK  
 JQS: J CELL INDEX OF VOLUME SOURCE/SINK  
 QSSE: CONSTANT INFLOW/OUTFLOW RATE IN M\*M\*M/S  
 NQSMUL: MULTIPLIER SWITCH FOR CONSTANT AND TIME SERIES VOL S/S  
 = 0 MULT BY 1. FOR NORMAL IN/OUTFLOW (L\*L\*L/T)  
 = 1 MULT BY DY FOR LATERAL IN/OUTFLOW (L\*L/T) ON U FACE  
 = 2 MULT BY DX FOR LATERAL IN/OUTFLOW (L\*L/T) ON V FACE  
 = 3 MULT BY DX+DY FOR LATERAL IN/OUTFLOW (L\*L/T) ON U&V FACES  
 NQSMFF: IF NON ZERO ACCOUNT FOR VOL S/S MOMENTUM FLUX  
 = 1 MOMENTUM FLUX ON NEG U FACE  
 = 2 MOMENTUM FLUX ON NEG V FACE  
 = 3 MOMENTUM FLUX ON POS U FACE  
 = 4 MOMENTUM FLUX ON POS V FACE  
 NQSERQ: ID NUMBER OF ASSOCIATED VOLUMN FLOW TIME SERIES  
 NSSERQ: ID NUMBER OF ASSOCIATED SALINITY TIME SERIES  
 NTSERQ: ID NUMBER OF ASSOCIATED TEMPERATURE TIME SERIES  
 NDSERQ: ID NUMBER OF ASSOCIATED DYE CONC TIME SERIES  
 NSFSERQ: ID NUMBER OF ASSOCIATED SHELL FISH LARVAE RELEASE TIME SERIES  
 NTXSERQ: ID NUMBER OF ASSOCIATED TOXIC CONTAMINANT CONC TIME SERIES  
 NSDSERQ: ID NUMBER OF ASSOCIATED COHEASIVE SEDIMENT CONC TIME SERIES  
 NSNSERQ: ID NUMBER OF ASSOCIATED NONCOHEASIVE SED CONC TIME SERIES

C24	IQS	JQS	QSSE	NQSMUL	NQSMFF	NQSERQ	NS-	NT-	ND-	NSF-	NTX-	NSD-	NSN-	
	18	2	0.0	0	0	1	2	2	0	0	0	0	0	
GOLDSBOROUGH CREEK														
	11	44	0.0	0	0	2	2	2	0	0	0	0	0	JOHNS
CREEK														
	8	55	0.0	0	0	3	2	2	0	0	0	0	0	CRANBERRY
CREEK														

CREEK	8	2	0.0	0	0	4	2	2	0	0	0	0	0	SHELTON
CREEK	12	55	0.0	0	0	5	2	2	0	0	0	0	0	DEER
JOHNS CREEK	22	34	0.0	0	0	6	2	2	0	0	0	0	0	UNCLE
CREEK	102	10	0.0	0	0	7	2	2	0	0	0	0	0	MILL
CREEK	12	48	0.0	0	0	8	2	2	0	0	0	0	0	MALANEY
CREEK	23	33	0.0	0	0	9	2	2	0	0	0	0	0	CAMPBELL
	20	33	0.0	0	0	10	2	2	0	0	0	0	0	CREEK A
	19	11	0.0	0	0	11	0	0	1	0	0	0	0	OUTFALL
	19	12	0.0	0	0	12	0	0	1	0	0	0	0	X-OUTFALL

C25 TIME CONSTANT INFLOW CONCENTRATIONS FOR TIME CONSTANT VOLUMETRIC SOURCES  
C

SAL: SALT CONCENTRATION CORRESPONDING TO INFLOW ABOVE  
 TEM: TEMPERATURE CORRESPONDING TO INFLOW ABOVE  
 DYE: DYE CONCENTRATION CORRESPONDING TO INFLOW ABOVE  
 SFL: SHELL FISH LARVAE CONCENTRATION CORRESPONDING TO INFLOW ABOVE  
 TOX: NTOX TOXIC CONTAMINANT CONCENTRATIONS CORRESPONDING TO  
 INFLOW ABOVE WRITTEN AS TOXC(N), N=1,NTOX A SINGLE DEFAULT  
 VALUE IS REQUIRED EVEN IF TOXIC TRANSPORT IS NOT ACTIVE

C

C25	SAL	TEM	DYE	SFL	TOX1-20
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.
0.	10.	0.	0.	0.	0.

C26 TIME CONSTANT INFLOW CONCENTRATIONS FOR TIME CONSTANT VOLUMETRIC SOURCES  
C

SED: NSED COHESIVE SEDIMENT CONCENTRATIONS CORRESPONDING TO  
 INFLOW ABOVE WRITTEN AS SEDC(N), N=1,NSED. I.E., THE FIRST  
 NSED VALUES ARE COHESIVE A SINGLE DEFAULT VALUE IS REQUIRED  
 EVEN IF COHESIVE SEDIMENT TRANSPORT IS INACTIVE  
 SND: NSND NON-COHESIVE SEDIMENT CONCENTRATIONS CORRESPONDING TO  
 INFLOW ABOVE WRITTEN AS SND(N), N=1,NSND. I.E., THE LAST  
 NSND VALUES ARE NON-COHESIVE. A SINGLE DEFAULT VALUE IS  
 REQUIRED EVEN IF NON-COHESIVE SEDIMENT TRANSPORT IS INACTIVE

C

C26	SED1	SND1
0.	0.	
0.	0.	
0.	0.	
0.	0.	
0.	0.	

0. 0.  
0. 0.  
0. 0.  
0. 0.  
0. 0.  
0. 0.  
0. 0.

-----  
C27 JET/PLUME SOURCE LOCATIONS, GEOMETRY AND ENTRAINMENT PARAMETERS

C

ID: ID COUNTER FOR JET/PLUME  
ICAL: 1 ACTIVE, 0 BYPASS  
IQJP: I CELL INDEX OF JET/PLUME  
JQJP: J CELL INDEX OF JET/PLUME  
KQJP: K CELL INDEX OF JET/PLUME (DEFAULT, QJET=0 OR JET COMP DIVERGES)  
XJET: LOCAL EAST JET LOCATION RELATIVE TO DISCHARGE CELL CENTER (M)  
YJET: LOCAL NORTH JET LOCATION RELATIVE TO DISCHARGE CELL CENTER (M)  
ZJET: ELEVATION OF DISCHARGE (M)  
PHJET: VERTICAL JET ANGLE POSITIVE FROM HORIZONTAL (DEGREES)  
THJET: HORIZONTAL JET ANGLE POS COUNTER CLOCKWISE FROM EAST (DEGREES)  
DJET: DIAMETER OF DISCHARGE PORT (M)  
CFRD: ADJUSTMENT FACTOR FOR FROUDE NUMBER  
DJPER: ENTRAINMENT ERROR CRITERIA

C

C27 ID ICAL IQJP JQJP KQJP XJET YJET ZJET PHJET THJET DJET CFRD DJPER

-----  
C28 JET/PLUME SOLUTION CONTROL AND OUTPUT CONTROL PARAMETERS

C

ID: ID COUNTER FOR JET/PLUME  
NJEL: MAXIMUM NUMBER OF ELEMENTS ALONG JET/PLUME LENGTH  
NJPMX: MAXIMUM NUMBER OF ITERATIONS  
ISENT: 0 USE MAXIMUM OF SHEAR AND FORCED ENTRAINMENT  
1 USE SUM OF SHEAR AND FORCED ENTRAINMENT  
ISTJP: 0 STOP AT SPECIFIED NUMBER OF ELEMENTS  
1 STOP WHEN CENTERLINE PENETRATES BOTTOM OR SURFACE  
2 STOP WITH BOUNDARY PENETRATES BOTTOM OR SURFACE  
NUDJP: FREQUENCY FOR UPDATING JET/PLUME (NUMBER OF TIME STEPS)  
IOJP: 1 FOR FULL ASCII, 2 FOR COMPACT ASCII OUTPUT AT EACH UPDATE  
3 FOR FULL AND COMPACT ASCII OUTPUT, 4 FOR BINARY OUTPUT  
IPJP: NUMBER OF SPATIAL PRINT/SAVE POINT IN VERTICAL  
ISDJP: 1 WRITE DIAGNOSTIS TO jplog\_\_.out

C

C28 ID NJEL NJPMX ISENT ISTJP NUDJP IOJP IPJP ISDJP

-----  
C29 JET/PLUME SOURCE PARAMETERS AND DISCHARGE/CONCENTRATION SERIES IDS

C

ID: ID COUNTER FOR JET/PLUME  
QQJP: CONSTANT JET/PLUME FLOW RATE IN M\*M\*M/S  
NQSERJP: ID NUMBER OF ASSOCIATED VOLUMN FLOW TIME SERIES  
NSSERJP: ID NUMBER OF ASSOCIATED SALINITY TIME SERIES  
NTSERJP: ID NUMBER OF ASSOCIATED TEMPERATURE TIME SERIES  
NDSERJP: ID NUMBER OF ASSOCIATED DYE CONC TIME SERIES  
NSFSERJP: ID NUMBER OF ASSOCIATED SHELL FISH LARVAE RELEASE TIME SERIES  
NTXSERJP: ID NUMBER OF ASSOCIATED TOXIC CONTAMINANT CONC TIME SERIES  
NSDSERJP: ID NUMBER OF ASSOCIATED COHEASIVE SEDIMENT CONC TIME SERIES  
NSNSERJP: ID NUMBER OF ASSOCIATED NONCOHEASIVE SED CONC TIME SERIES

C

10

C29 ID QQJP NQSERJP NS- NT- ND- NSF- NTX- NSD- NSN-

C30 TIME CONSTANT INFLOW CONCENTRATIONS FOR TIME CONSTANT JET/PLUME SOURCES  
C

SAL: SALT CONCENTRATION CORRESPONDING TO INFLOW ABOVE  
TEM: TEMPERATURE CORRESPONDING TO INFLOW ABOVE  
DYE: DYE CONCENTRATION CORRESPONDING TO INFLOW ABOVE  
SFL: SHELL FISH LARVAE CONCENTRATION CORRESPONDING TO INFLOW ABOVE  
TOX: NTOX TOXIC CONTAMINANT CONCENTRATIONS CORRESPONDING TO  
INFLOW ABOVE WRITTEN AS TOXC(N), N=1,NTOX A SINGLE DEFAULT  
VALUE IS REQUIRED EVEN IF TOXIC TRANSPORT IS NOT ACTIVE

C

C30 SAL TEM DYE SFL TOX1-20

C31 TIME CONSTANT INFLOW CONCENTRATIONS FOR TIME CONSTANT JET/PLUME SOURCES  
C

SED: NSED COHESIVE SEDIMENT CONCENTRATIONS CORRESPONDING TO  
INFLOW ABOVE WRITTEN AS SEDC(N), N=1,NSED. I.E., THE FIRST  
NSED VALUES ARE COHESIVE A SINGLE DEFAULT VALUE IS REQUIRED  
EVEN IF COHESIVE SEDIMENT TRANSPORT IS INACTIVE  
SND: NSND NON-COHESIVE SEDIMENT CONCENTRATIONS CORRESPONDING TO  
INFLOW ABOVE WRITTEN AS SND(N), N=1,NSND. I.E., THE LAST  
NSND VALUES ARE NON-COHESIVE. A SINGLE DEFAULT VALUE IS  
REQUIRED EVEN IF NON-COHESIVE SEDIMENT TRANSPORT IS INACTIVE

C

C31 SED1 SND1 SND2 SND3

C32 SURFACE ELEV OR PRESSURE DEPENDENT FLOW INFORMATION

C

IQCTLU: I INDEX OF UPSTREAM OR WITHDRAWAL CELL  
JQCTLU: J INDEX OF UPSTREAM OR WITHDRAWAL CELL  
IQCTLD: I INDEX OF DOWNSTREAM OR RETURN CELL  
JQCTLD: J INDEX OF DOWNSTREAM OR RETURN CELL  
NQCTYP: FLOW CONTROL TYPE  
= 0 HYDRAULIC STRUCTURE: INSTANT FLOW DRIVEN BY ELEVATION  
OR PRESSURE DIFFERENCE TABLE  
= 1 ACCELERATING FLOW THROUGH TIDAL INLET  
NQCTLQ: ID NUMBER OF CONTROL CHARACTERIZATION TABLE  
NQCMUL: MULTIPLIER SWITCH FOR FLOWS FROM UPSTREAM CELL  
= 0 MULT BY 1. FOR CONTROL TABLE IN (L\*L\*L/T)  
= 1 MULT BY DY FOR CONTROL TABLE IN (L\*L/T) ON U FACE  
= 2 MULT BY DX FOR CONTROL TABLE IN (L\*L/T) ON V FACE  
= 3 MULT BY DX+DY FOR CONTROL TABLE IN (L\*L/T) ON U&V FACES  
NQCMFU: IF NON ZERO ACCOUNT FOR FLOW MOMENTUM FLUX IN UPSTREAM CELL  
= 1 MOMENTUM FLUX ON NEG U FACE  
= 2 MOMENTUM FLUX ON NEG V FACE  
= 3 MOMENTUM FLUX ON POS U FACE  
= 4 MOMENTUM FLUX ON POS V FACE  
NQCMFD: IF NON ZERO ACCOUNT FOR FLOW MOMENTUM FLUX IN DOWNSTREAM CELL  
= 1 MOMENTUM FLUX ON NEG U FACE  
= 2 MOMENTUM FLUX ON NEG V FACE  
= 3 MOMENTUM FLUX ON POS U FACE  
= 4 MOMENTUM FLUX ON POS V FACE  
BQCMFU: UPSTREAM MOMENTUM FLUX WIDTH (M)  
BQCMFD: DOWNSTREAM MOMENTUM FLUX WIDTH (M)

C

C32 IQCTLU JQCTLU IQCTLD JQCTLD NQCTYP NQCTLQ NQCMUL NQC\_U NQC\_D BQC\_U BQC\_D

-----  
C33 FLOW WITHDRAWAL, HEAT OR MATERIAL ADDITION, AND RETURN DATA

C

IWRU: I INDEX OF UPSTREAM OR WITHDRAWAL CELL  
JWRU: J INDEX OF UPSTREAM OR WITHDRAWAL CELL  
KWRU: K INDEX OF UPSTREAM OR WITHDRAWAL LAYER  
IWRD: I INDEX OF DOWNSTREAM OR RETURN CELL  
JWRD: J INDEX OF DOWNSTREAM OR RETURN CELL  
KWRD: J INDEX OF DOWNSTREAM OR RETURN LAYER  
QWRE: CONSTANT VOLUME FLOW RATE FROM WITHDRAWAL TO RETURN  
NQWRSERQ: ID NUMBER OF ASSOCIATED VOLUMN WITHDRAWAL-RETURN FLOW AND  
CONCENTRATION RISE TIME SERIES  
NQWRMFU: IF NON ZERO ACCOUNT FOR WITHDRAWAL FLOW MOMENTUM FLUX  
= 1 MOMENTUM FLUX ON NEG U FACE  
= 2 MOMENTUM FLUX ON NEG V FACE  
= 3 MOMENTUM FLUX ON POS U FACE  
= 4 MOMENTUM FLUX ON POS V FACE  
NQWRMFD: IF NON ZERO ACCOUNT FOR RETURN FLOW MOMENTUM FLUX  
= 1 MOMENTUM FLUX ON NEG U FACE  
= 2 MOMENTUM FLUX ON NEG V FACE  
= 3 MOMENTUM FLUX ON POS U FACE  
= 4 MOMENTUM FLUX ON POS V FACE  
BQWRMFU: UPSTREAM MOMENTUM FLUX WIDTH (M)  
BQWRMFD: UPSTREAM MOMENTUM FLUX WIDTH (M)

C

23.1

C33 IWRU JWRU KWRU IWRD JCWRD KWRD QWRE NQW\_RQ NQWR\_U NQWR\_D BQWR\_U BQWR\_D

-----  
C34 TIME CONSTANT WITHDRAWAL AND RETURN CONCENTRATION RISES

C

SAL: SALTINITY RISE  
TEM: TEMPERATURE RISE  
DYE: DYE CONCENTRATION RISE  
SFL: SHELLFISH LARVAE CONCENTRATION RISE  
TOX#: NTOX TOXIC CONTAMINANT CONCENTRATION RISES

C

C34 SALT TEMP DYEC SFLC TOX1

-----  
C35 TIME CONSTANT WITHDRAWAL AND RETURN CONCENTRATION RISES

C

SED#: NSEDC COHESIVE SEDIMENT CONCENTRATION RISE  
SND#: NSEDN NONCOHESIVE SEDIMENT CONCENTRATION RISE

C

C35 SED1 SND1 SND2

-----  
C36 SEDIMENT INITIALIZATION AND WATER COLUMN/BED REPRESENTATION OPTIONS

C

DATA REQUIRED EVEN IF ISTRAN(6) AND ISTRAN(7) ARE 0

C

ISEDINT: 0 FOR CONSTANT INITIAL CONDITIONS  
1 FOR SPATIALLY VARIABLE WATER COLUMN INITIAL CONDITIONS  
FROM sedw.inp AND sndw.inp  
2 FOR SPATIALLY VARIABLE BED INITIAL CONDITIONS  
FROM sedb.inp AND sndb.inp  
3 FOR SPATIALLY VARIABLE WATER COL AND BED INITIAL CONDITIONS  
ISEDINT: 0 FOR SPATIALLY VARYING BED INITIAL CONDITIONS IN MASS/AREA  
1 FOR SPATIALLY VARYING BED INITIAL CONDITIONS IN MASS FRACTION  
OF TOTAL SEDIMENT MASS (REQUIRES BED LAYER THICKNESS  
FILE bedlay.inp)

ISEDWC: 0 COHESIVE SED WC/BED EXCHANGE BASED ON BOTTOM LAYER CONDITIONS  
 1 COHESIVE SED WC/BED EXCHANGE BASED ON WAVE/CURRENT/SEDIMENT  
 BOUNDARY LAYERS EMBEDDED IN BOTTOM LAYER  
 ISMUD: 1 INCLUDE COHESIVE FLUID MUD VISCOUS EFFECTS USING EFDC  
 FUNCTION CSEDVIS(SED,T)  
 ISNDWC: 0 NONCOH SED WC/BED EXCHANGE BASED ON BOTTOM LAYER CONDITIONS  
 1 NONCOH SED WC/BED EXCHANGE BASED ON WAVE/CURRENT/SEDIMENT  
 BOUNDARY LAYERS EMBEDDED IN BOTTOM LAYER  
 ISEDVW: 0 FOR CONSTANT OR SIMPLE CONCENTRATION DEPENDENT  
 COHESIVE SEDIMENT SETTLING VELOCITY  
 >1 CONCENTRATION AND/OR SHEAR/TURBULENCE DEPENDENT COHESIVE  
 SEDIMENT SETTLING VELOCITY. VALUE INDICATES OPTION TO BE USED  
 IN EFDC FUNCTION CSEDSET(SED,SHEAR,ISEDVWC)  
 1 HUANG AND METHA - LAKE OKEECHOBEE  
 2 SHRESTA AND ORLOB - FOR KRONES SAN FRANCISCO BAY DATA  
 3 ZIEGLER AND NESBIT - FRESH WATER  
 ISNDVW: 0 USE CONSTANT SPECIFIED NONCHOESIVE SED SETTLING VELOCITIES  
 OR CALCULATE FOR CLASS DIAMETER IS SPECIFIED VALUE IS NEG  
 >1 FOLLOW OPTION 0 PROCEDURE BUT APPLY HINDERED SETTLING  
 CORRECTION. VALUE INDICATES OPTION TO BE USED WITH EFDC  
 FUNCTION CSNDSET(SND,SDEN,ISNDVW) VALUE OF ISNDVW INDICATES  
 EXPONENTIAL IN CORRECT (1-SDEN(NS)\*SND(NS)\*\*ISNDVW  
 KB: MAXIMUM NUMBER OF BED LAYERS (EXCLUDING ACTIVE LAYER)  
 ISEDAL: 1 TO ACTIVATE STATIONARY COHESIVE MUD ACTIVE LAYER  
 ISNDAL: 1 TO ACTIVATE NONCOHESIVE ARMORING LAYER ACTIVE LAYER

C

3

C36	ISEDINT	ISEDBINT	ISEDWC	ISMUD	ISNDWC	ISEDVW	ISNDVW	KB	ISEDAL	ISNDAL
	0	0	0	0	0	0	0	1	0	0

C37 BED MECHANICAL PROPERTIES PARAMETER SET 1

C DATA REQUIRED EVEN IF ISTRAN(6) AND ISTRAN(7) ARE 0

C

IBMECH: 0 TIME INVARIANT CONSTANT BED MECHANICAL PROPERITES  
 1 SIMPLE CONSOLIDATION CALCULATION WITH CONSTANT COEFFICIENTS  
 2 SIMPLE CONSOLIDATION WITH VARIABLE COEFFICIENTS DETERMINED  
 EFDC FUNCTIONS CSEDCON1,2,3(IBMECH)  
 3 COMPLEX CONSOLIDATION WITH VARIABLE COEFFICIENTS DETERMINED  
 EFDC FUNCTIONS CSEDCON1,2,3(IBMECH). IBMECH > 0 SETS THE  
 C38 PARAMETER ISEDBINT=1 AND REQUIRES INITIAL CONDITIONS  
 FILES bedlay.inp, bedbdn.inp and bedddn.in  
 IMORPH: 0 CONSTANT BED MORPHOLOGY (IBMECH=0, ONLY)  
 1 ACTIVE BED MORPHOLOGY: NO WATER ENTRAIN/EXPULSION EFFECTS  
 2 ACTIVE BED MORPHOLOGY: WITH WATER ENTRAIN/EXPULSION EFFECTS  
 HBEDMAX: TOP BED LAYER THICKNESS (M) AT WHICH NEW LAYER IS ADDED OR IF  
 KBT(I,J)=KB, NEW LAYER ADDED AND LOWEST TWO LAYERS COMBINED  
 BEDPORC: CONSTANT BED POROSITY (IBMECH=0, OR NSED=0)  
 ALSO USED AS POROSITY OF DEPOSITIN NONCOHESIVE SEDIMENT  
 SEDMDMX: MAXIMUM FLUID MUD COHESIVE SEDIMENT CONCENTRATION (mg/l)  
 SEDMDMN: MINIMUM FLUID MUD COHESIVE SEDIMENT CONCENTRATION (mg/l)  
 SEDVDRD: VOID RATIO OF DEPOSITING COHESIVE SEDIMENT  
 SEDVDRM: MINIMUM COHESIVE SEDIMENT BED VOID RATIO (IBMECH > 0)  
 SEDVDRT: BED CONSOLODATION RATED CONSTANT (1/SEC) (IBMECH = 1,2)

C

C37	IBMECH	IMORPH	HBEDMAX	BEDPORC	SEDMDMX	SEDMDMN	SEDVDRD	SEDVDRM	SEDVDRT
	0	0	2.0	0.5	1.1E5	4000.	20.	4.	1.E-5

-----  
 C38 BED MECHANICAL PROPERTIES PARAMETER SET 2  
 C DATA REQUIRED EVEN IF ISTRAN(6) AND ISTRAN(7) ARE 0  
 C

BMECH1: BED MECHANICS FUNCTION COEFFICIENT  
 BMECH2: BED MECHANICS FUNCTION COEFFICIENT  
 BMECH3: BED MECHANICS FUNCTION COEFFICIENT  
 BMECH4: BED MECHANICS FUNCTION COEFFICIENT  
 BMECH5: BED MECHANICS FUNCTION COEFFICIENT  
 BMECH6: BED MECHANICS FUNCTION COEFFICIENT

C  
 C38 BMECH1 BMECH2 BMECH3 BMECH4 BMECH5 BMECH6  
 0.0 0. 0. 0. 0. 0.

-----  
 C39 COHESIVE SEDIMENT PARAMETER SET 1 REPEAT DATA LINE NSED TIMES  
 C DATA REQUIRED EVEN IF ISTRAN(6) AND ISTRAN(7) ARE 0  
 C

SEDO: CONSTANT INITIAL COHESIVE SEDIMENT CONC IN WATER COLUMN  
 (mg/liter=gm/m\*\*3)  
 SEDBO: CONSTANT INITIAL COHESIVE SEDIMENT IN BED PER UNIT AREA  
 (gm/sq meter) IE LCM THICKNESS BED WITH SSG=2.5 AND  
 N=.6,.5 GIVES SEDBO 1.E4, 1.25E4  
 SDEN: SEDIMENT SPEC VOLUME (IE 1/2.25E6 m\*\*3/gm)  
 SSG: SEDIMENT SPECIFIC GRAVITY  
 WSEDO: CONSTANT OR REFERENCE SEDIMENT SETTLING VELOCITY  
 IN FORMULA WSED=WSEDO\*( (SED/SEDSN)\*\*SEXP )  
 SEDSN: NORMALIZING SEDIMENT CONC (COHESIVE SED TRANSPORT) (gm/m\*\*3)  
 SEXP: EXPONENTIAL (COHESIVE SED TRANSPORT)  
 TAUD: BOUNDARY STRESS BELOW WHICH DEPOSITION TAKES PLACE ACCORDING  
 TO (TAUD-TAU)/TAUD

ISEDSOR: 1 TO CORRECT BOTTOM LAYER CONCENTRATION TO NEAR BED CONC

C  
 C39 SEDO SEDBO SDEN SSG WSEDO SEDSN SEXP TAUD ISEDSOR  
 0.0 1.0E4 1.E-12 2.25 0.0 1.0 0. 1.E+6 0

-----  
 C40 COHESIVE SEDIMENT PARAMETER SET 2 REPEAT DATA LINE NSED TIMES  
 C DATA REQUIRED EVEN IT ISTRAN(6) AND ISTRAN(7) ARE 0  
 C

IWRSP: 0 USE RESUSPENSION RATE AND CRITICAL STRESS BASED ON PARAMETERS  
 ON THIS DATA LINE  
 >1 USE BED PROPERTIES DEPENDEDNT RESUSPENSION RATE AND CRITICAL  
 STRESS GIVEN BY EFDC FUNCTIONS CSEDRESS,CSEDTAUS,CSEDTAUB  
 FUNCTION ARGUMENSTS ARE (BDENBED,IWRSP)  
 1 HWANG AND METHA - LAKE OKEECHOBEE  
 WRSP0: REF SURFACE EROSION RATE IN FORMULA  
 WRSP=WRSP0\*( ((TAU-TAUR)/TAUN)\*\*TEX ) (gm/m\*\*2-sec)  
 TAUR: BOUNDARY STRESS ABOVE WHICH SURFACE EROSION OCCURS (m/s)\*\*2  
 TAUN: NORMALIZING STRESS (EQUAL TO TAUR FOR COHESIVE SED TRANS)  
 TEXP: EXPONENTIAL (COH SED)

C 0 0.01 4.E-4 4.E-4 1.  
 C40 IWRSP WRSP0 TAUR TAUN TEXP  
 0 0.00 1.E+6 1.E+6 1.

-----  
 C41 NONCOHESIVE SEDIMENT PARAMETER SET 1 REPEAT DATA LINE NSND TIMES  
 C DATA REQUIRED EVEN IT ISTRAN(6) AND ISTRAN(7) ARE 0  
 C

SNDO: CONSTANT INITIAL NONCOHESIVE SEDIMENT CONC IN WATER COLUMN

(mg/liter=gm/m\*\*3)  
 SNDBO: CONSTANT INITIAL NONCOHESIVE SEDIMENT IN BED PER UNIT AREA  
 (gm/sq meter) IE 1CM THICKNESS BED WITH SSG=2.5 AND  
 N=.6,.5 GIVES SNDBO 1.E4, 1.25E4  
 SDEN: SEDIMENT SPEC VOLUME (IE 1/2.65E6 m\*\*3/gm)  
 SSG: SEDIMENT SPECIFIC GRAVITY  
 SNDDIA: REPRESENTATIVE DIAMETER OF SEDIMENT CLASS  
 WSND0: CONSTANT OR REFERENCE SEDIMENT SETTLING VELOCITY  
 SNDN: MAX MASS/TOT VOLUME IN BED (NONCOHESIVE SED TRANS) (gm/m\*\*3)  
 SEXP: DIMENSIONLESS RESUSPENSION PARAMETER GAMMA ZERO  
 TAUD: DUNE BREAK POINT STRESS (m/s)\*\*2  
 ISNSCOR: 1 TO CORRECT BOTTOM LAYER CONCENTRATION TO NEAR BED CONC

C

C41	SNDO	SNDBO	SDEN	SSG	SNDDIA	WSND0	SNDN	SEXP	TAUD	ISNSCOR	
	0.0	1.E4	1.E-12	2.65	1.8E-4	0.00	1.E6	1.E-3	7.E-5	0	! fine sand

C42 NONCOHESIVE SEDIMENT PARAMETER SET 2 REPEAT DATA LINE NSND TIMES  
 C DATA REQUIRED EVEN IT ISTRAN(6) AND ISTRAN(7) ARE 0

C

ISNDEQ: >1 CALCULATE ABOVE BED REFERENCE NONCHOHESIVE SEDIMENT  
 EQUILIBRIUM CONCENTRATION USING EFDC FUNCTION  
 CSNDEQC(SNDDIA,SSG,WS,TAUR,TAUB,SIGPHI,SNDDMX,IOTP)  
 WHICH IMPLEMENT FORMULATIONS OF  
 1 GRACIA AND PARKER  
 2 SMITH AND MCLEAN  
 3 VAN RIJN

TAUR: CRITICAL SHIELDS STRESS IN (m/s)\*\*2 (ISNDEQ=2)  
 TAUN: EQUAL TO TAUR FOR NONCHOESIVE SED TRANS (ISNDEQ=2)  
 TEXP: CRITICAL SHIELDS PARAMETER (ISNDEQ=2)

C

C42	ISNDEQ	TAUR	TAUN	TEXP
	0	1.E+6	1.E+6	0.2

C43 TOXIC CONTAMINANT INITIAL CONDITIONS AND PARAMETERS  
 C USER MAY CHANGE UNITS OF WATER AND SED PHASE TOX CONCENTRATION  
 C AND PARTIATION COEFFICIENT ON C44 - C46 BUT CONSISTENT UNITS MUST  
 C MUST BE USED FOR MEANINGFUL RESULTS  
 C DATA REQUIRED EVEN IT ISTRAN(5) IS 0

C

NTOXN: TOXIC CONTAMINANT NUMBER ID (1 LINE OF DATA BY DEFAULT)  
 ITXINT: 0 FOR SPATIALLY CONSTANT WATER COL AND BED INITIAL CONDITIONS  
 1 FOR SPATIALLY VARIABLE WATER COLUMN INITIAL CONDITIONS  
 2 FOR SPATIALLY VARIABLE BED INITIAL CONDITIONS  
 3 FOR SPATIALLY VARIABLE WATER COL AND BED INITIAL CONDITION  
 ITXBDUT: SET TO 0 FOR CONST INITIAL BED GIVEN BY TOTAL TOX (ugm/litr)  
 SET TO 1 FOR CONST INITIAL BED GIVEN BY  
 SORBED MASS TOX/MASS SED(mg/kg)  
 TOXINTW: INIT WATER COLUMN TOT TOXIC VARIABLE CONCENTRATION (ugm/litr)  
 TOXINTB: INIT SED BED TOXIC CONC SEE ITXBDUT  
 RKTOXW: FIRST ORDER WATER COL DECAY RATE FOR TOX VARIABLE IN 1/SEC  
 TKTOXW: REF TEMP FOR 1ST ORDER WATER COL DECAY DEG C  
 RKTOXB: FIRST ORDER SED BED DECAY RATE FOR TOX VARIABLE IN 1/SEC  
 TKTOXB: REF TEMP FOR 1ST ORDER SED BED DECAY DEG C

C

C43	NTOXN	ITXINT	ITXBDUT	TOXINTW	TOXINTB	RKTOXW	TKTOXW	RKTOXB	TRTOXB	COMMENTS
	1	0	0	1.	1.	0.	0.	0.	0.	DUMMY

C44 ADDITIONAL TOXIC CONTAMINANT PARAMETERS

C DATA REQUIRED EVEN IT ISTRAN(5) IS 0

C

NTOXN: TOXIC CONTAMINANT NUMBER ID (1 LINE OF DATA BY DEFAULT)  
 VOLTOX: WATER SURFACE VOLITIALIZATION RATE MULTIPLIER (0. OR 1.)  
 RMOLTX: MOLECULAR WEIGHT FOR DETERMINING VOLATILIZATION RATE  
 RKTOXP: REFERENCE PHOTOLYSIS DECAY RATE 1/SEC  
 SKTOXP: REFERENCE SOLAR RADIATION FOR PHOTOLYSIS (WATTS/M\*\*2)  
 DIFTOX: DIFFUSION COEFF FOR TOXICANT IN SED BED PORE WATER (M\*\*2/S)

C

C44	NTOXN	VOLTOX	RMOLTX	RKTOXP	SKTOXP	DIFTOX	COMMENTS
	1	0.	0.	0.	0.	1.E-9	DUMMY

C45 TOXIC CONTAMINANT SEDIMENT INTERACTION PARAMETERS

C 2 LINES OF DATA REQUIRED EVEN IT ISTRAN(5) IS 0

C

NTOXC: TOXIC CONTAMINANT NUMBER ID. NSEDC+NSEDN LINES OF DATA  
 FOR EACH TOXIC CONTAMINANT (DEFAULT = 2)  
 NSEDN/NSNDN: FIRST NSED LINES COHESIVE, NEXT NSND LINES NON-COHESIVE.  
 REPEATED FOR EACH CONTAMINANT  
 ITXPARW: EQUAL 1 FOR SOLIDS DEPENDENT PARTITIONING (WC) GIVEN BY  
 TOXPARG=PARO\*(CSED\*\*CONPAR)  
 TOXPARG: WATER COLUMN PARO (ITXPARW=1) OR EQUIL TOX CON PART COEFF BETWEEN  
 EACH TOXIC IN WATER AND ASSOCIATED SEDIMENT PHASES (liters/mg)  
 CONPARW: EXPONENT IN TOXPARG=PARO\*(CSED\*\*CONPARW) IF ITXPARW=1  
 ITXPARG: EQUAL 1 FOR SOLIDS DEPENDENT PARTITIONING (BED)  
 TOXPARG: SEDIMENT BED PARO (ITXPARG=1) OR EQUIL TOX CON PART COEFF BETWEEN  
 EACH TOXIC IN WATER AND ASSOCIATED SEDIMENT PHASES (liters/mg)  
 CONPARG: EXPONENT IN TOXPARG=PARO\*(CSED\*\*CONPARG) IF ITXPARG=1

C

1 0.8770 -0.943 0.025

C45	NTOXC	NSEDN	ITXPARG	TOXPARG	CONPARW	ITXPARG	TOXPARG	CONPARG	COMMENTS
	1	1	0	1.	0.	0	1.	0.	DUMMY FOR NSED=1
	1	2	0	1.	0.	0	1.	0.	DUMMY FOR NSND=1

C46 BUOYANCY, TEMPERATURE, DYE DATA AND CONCENTRATION BC DATA

C

BSC: BUOYANCY INFLUENCE COEFFICIENT 0 TO 1, BSC=1. FOR REAL PHYSICS  
 TEMO: REFERENCE, INITIAL, EQUILIBRUM AND/OR ISOTHERMAL TEMP IN DEG C  
 HEQT: EQUILIBRUM TEMPERATURE TRANSFER COEFFICIENT M/SEC  
 RKDYE: FIRST ORDER DECAY RATE FOR DYE VARIABLE IN 1/SEC  
 NCBS: NUMBER OF CONCENTRATION BOUNDARY CONDITIONS ON SOUTH OPEN  
 BOUNDARIES  
 NCBW: NUMBER OF CONCENTRATION BOUNDARY CONDITIONS ON WEST OPEN  
 BOUNDARIES  
 NCBE: NUMBER OF CONCENTRATION BOUNDARY CONDITIONS ON EAST OPEN  
 BOUNDARIES  
 NCBN: NUMBER OF CONCENTRATION BOUNDARY CONDITIONS ON NORTH OPEN  
 BOUNDARIES

C

26.9

C46	BSC	TEMO	HEQT	RKDYE	NCBS	NCBW	NCBE	NCBN
	1.0	10.0	1.E-6	0.	0	0	4	0

C47 LOCATION OF CONC BC'S ON SOUTH BOUNDARIES

C

ICBS: I CELL INDEX  
 JCBS: J CELL INDEX

NTSCRS: NUMBER OF TIME STEPS TO RECOVER SPECIFIED VALUES ON CHANGE  
TO INFLOW FROM OUTFLOW  
NSSERS: SOUTH BOUNDARY CELL SALINITY TIME SERIES ID NUMBER  
NTSERS: SOUTH BOUNDARY CELL TEMPERATURE TIME SERIES ID NUMBER  
NDSERS: SOUTH BOUNDARY CELL DYE CONC TIME SERIES ID NUMBER  
NSFSERS: SOUTH BOUNDARY CELL SHELLFISH LARVAE TIME SERIES ID NUMBER  
NTXSERS: SOUTH BOUNDARY CELL TOXIC CONTAMINANT CONC TIME SERIES ID NUM.  
NSDSERS: SOUTH BOUNDARY CELL COHESIVE SED CONC TIME SERIES ID NUMBER  
NSNSERS: SOUTH BOUNDARY CELL NONCOHESIVE SED CONC TIME SERIES ID NUMBER

C

C47 IBBS JBBS NTSCRS NSSERS NTSERS NDSERS NSFSERS NTXSERS NSDSERS NSNSERS

-----  
C48 TIME CONSTANT BOTTOM CONC ON SOUTH CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING BOTTOM LAYER SALINITY  
TEM: ULTIMATE INFLOWING BOTTOM LAYER TEMPERATURE  
DYE: ULTIMATE INFLOWING BOTTOM LAYER DYE CONCENTRATION  
SFL: ULTIMATE INFLOWING BOTTOM LAYER SHELLFISH LARVAE CONCENTRAION  
TOX: NTOX ULTIMATE INFLOWING BOTTOM LAYER TOXIC CONTAMINANT  
CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C

C48 SAL TEM DYE SFL TOX1

-----  
C49 TIME CONSTANT BOTTOM CONC ON SOUTH CONC BOUNDARIES

C

SED: NSED ULTIMATE INFLOWING BOTTOM LAYER COHESIVE SEDIMENT  
CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
SND: NSND ULTIMATE INFLOWING BOTTOM LAYER NONCOHESIVE SEDIMENT  
CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C

C49 SED1 SND1 SND2 SND3

-----  
C50 TIME CONSTANT SURFACE CONC ON SOUTH CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING SURFAC LAYER SALINITY  
TEM: ULTIMATE INFLOWING SURFAC LAYER TEMPERATURE  
DYE: ULTIMATE INFLOWING SURFAC LAYER DYE CONCENTRATION  
SFL: ULTIMATE INFLOWING SURFAC LAYER SHELLFISH LARVAE CONCENTRAION  
TOX: NTOX ULTIMATE INFLOWING SURFAC LAYER TOXIC CONTAMINANT  
CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C

C50 SAL TEM DYE SFL TOX1

-----  
C51 TIME CONSTANT SURFACE CONC ON SOUTH CONC BOUNDARIES

C

SED: NSED ULTIMATE INFLOWING SURFAC LAYER COHESIVE SEDIMENT  
CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
SND: NSND ULTIMATE INFLOWING SURFAC LAYER NONCOHESIVE SEDIMENT  
CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C

C51 SED1 SND1 SND2 SND3

-----  
C52 LOCATION OF CONC BC'S ON WEST BOUNDARIES AND SERIES IDENTIFIERS

C

ICBW: I CELL INDEX  
JCBW: J CELL INDEX  
NTSCRW: NUMBER OF TIME STEPS TO RECOVER SPECIFIED VALUES ON CHANGE

TO INFLOW FROM OUTFLOW  
NSSERW: WEST BOUNDARY CELL SALINITY TIME SERIES ID NUMBER  
NTSERW: WEST BOUNDARY CELL TEMPERATURE TIME SERIES ID NUMBER  
NDSERW: WEST BOUNDARY CELL DYE CONC TIME SERIES ID NUMBER  
NSFSERW: WEST BOUNDARY CELL SHELLFISH LARVAE TIME SERIES ID NUMBER  
NTXSERW: WEST BOUNDARY CELL TOXIC CONTAMINANT CONC TIME SERIES ID NUM.  
NSDSERW: WEST BOUNDARY CELL COHESIVE SED CONC TIME SERIES ID NUMBER  
NSNSERW: WEST BOUNDARY CELL NONCOHESIVE SED CONC TIME SERIES ID NUMBER

C

C52 IBBW JBBW NTSCRW NSSERW NTSERW NDSERW NSFSERW NTXSERW NSDSERW NSNSERW

C53 TIME CONSTANT BOTTOM CONC ON WEST CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING BOTTOM LAYER SALINITY  
TEM: ULTIMATE INFLOWING BOTTOM LAYER TEMPERATURE  
DYE: ULTIMATE INFLOWING BOTTOM LAYER DYE CONCENTRATION  
SFL: ULTIMATE INFLOWING BOTTOM LAYER SHELLFISH LARVAE CONCENTRAION  
TOX: NTOX ULTIMATE INFLOWING BOTTOM LAYER TOXIC CONTAMINANT  
CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C

C53 SAL TEM DYE SFL TOX1

C54 TIME CONSTANT BOTTOM CONC ON WEST CONC BOUNDARIES

C

SED: NSED ULTIMATE INFLOWING BOTTOM LAYER COHESIVE SEDIMENT  
CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
SND: NSND ULTIMATE INFLOWING BOTTOM LAYER NONCOHESIVE SEDIMENT  
CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C

C54 SED1 SND1

C55 TIME CONSTANT SURFACE CONC ON WEST CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING SURFAC LAYER SALINITY  
TEM: ULTIMATE INFLOWING SURFAC LAYER TEMPERATURE  
DYE: ULTIMATE INFLOWING SURFAC LAYER DYE CONCENTRATION  
SFL: ULTIMATE INFLOWING SURFAC LAYER SHELLFISH LARVAE CONCENTRAION  
TOX: NTOX ULTIMATE INFLOWING SURFAC LAYER TOXIC CONTAMINANT  
CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C

C55 SAL TEM DYE SFL TOX1

C56 TIME CONSTANT SURFACE CONC ON WEST CONC BOUNDARIES

C

SED: NSED ULTIMATE INFLOWING SURFAC LAYER COHESIVE SEDIMENT  
CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
SND: NSND ULTIMATE INFLOWING SURFAC LAYER NONCOHESIVE SEDIMENT  
CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C

C56 SED1 SND1

C57 LOCATION OF CONC BC'S ON EAST BOUNDARIES AND SERIES IDENTIFIERS

C

ICBE: I CELL INDEX  
JCBE: J CELL INDEX  
NTSCRE: NUMBER OF TIME STEPS TO RECOVER SPECIFIED VALUES ON CHANGE  
TO INFLOW FROM OUTFLOW

NSSERE: EAST BOUNDARY CELL SALINITY TIME SERIES ID NUMBER  
 NTSERE: EAST BOUNDARY CELL TEMPERATURE TIME SERIES ID NUMBER  
 NDSERE: EAST BOUNDARY CELL DYE CONC TIME SERIES ID NUMBER  
 NSFSERE: EAST BOUNDARY CELL SHELLFISH LARVAE TIME SERIES ID NUMBER  
 NTXSERE: EAST BOUNDARY CELL TOXIC CONTAMINANT CONC TIME SERIES ID NUM.  
 NSDSERE: EAST BOUNDARY CELL COHESIVE SED CONC TIME SERIES ID NUMBER  
 NSNSERE: EAST BOUNDARY CELL NONCOHESIVE SED CONC TIME SERIES ID NUMBER

C

C57	IBBE	JBBE	NTSCRE	NSSERE	NTSERE	NDSERE	NSFSERE	NTXSERE	NSDSERE	NSNSERE
	119	10	24	1	1	0	0	0	0	0
	119	11	24	1	1	0	0	0	0	0
	119	12	24	1	1	0	0	0	0	0
	119	13	24	1	1	0	0	0	0	0

C58 TIME CONSTANT BOTTOM CONC ON EAST CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING BOTTOM LAYER SALINITY  
 TEM: ULTIMATE INFLOWING BOTTOM LAYER TEMPERATURE  
 DYE: ULTIMATE INFLOWING BOTTOM LAYER DYE CONCENTRATION  
 SFL: ULTIMATE INFLOWING BOTTOM LAYER SHELLFISH LARVAE CONCENTRAION  
 TOX: NTOX ULTIMATE INFLOWING BOTTOM LAYER TOXIC CONTAMINANT  
 CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C

C58	SAL	TEM	DYE	SFL	TOX1
	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.

C59 TIME CONSTANT BOTTOM CONC ON EAST CONC BOUNDARIES

C

SED: NSED ULTIMATE INFLOWING BOTTOM LAYER COHESIVE SEDIMENT  
 CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
 SND: NSND ULTIMATE INFLOWING BOTTOM LAYER NONCOHESIVE SEDIMENT  
 CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C

C59	SED1	SND1
	0.	0.
	0.	0.
	0.	0.
	0.	0.

C60 TIME CONSTANT SURFACE CONC ON EAST CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING SURFAC LAYER SALINITY  
 TEM: ULTIMATE INFLOWING SURFAC LAYER TEMPERATURE  
 DYE: ULTIMATE INFLOWING SURFAC LAYER DYE CONCENTRATION  
 SFL: ULTIMATE INFLOWING SURFAC LAYER SHELLFISH LARVAE CONCENTRAION  
 TOX: NTOX ULTIMATE INFLOWING SURFAC LAYER TOXIC CONTAMINANT  
 CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C

C60	SAL	TEM	DYE	SFL	TOX1
	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.

C61 TIME CONSTANT SURFACE CONC ON EAST CONC BOUNDARIES

C

SED: NSED ULTIMATE INFLOWING SURFAC LAYER COHESIVE SEDIMENT  
CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
SND: NSND ULTIMATE INFLOWING SURFAC LAYER NONCOHESIVE SEDIMENT  
CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C

C61 SED1 SND1  
0. 0. 0.  
0. 0. 0.  
0. 0. 0.  
0. 0. 0.

C62 LOCATION OF CONC BC'S ON NORTH BOUNDARIES AND SERIES IDENTIFIERS

C

ICBN: I CELL INDEX  
JCBN: J CELL INDEX  
NTSCRN: NUMBER OF TIME STEPS TO RECOVER SPECIFIED VALUES ON CHANGE  
TO INFLOW FROM OUTFLOW  
NSSERN: NORTH BOUNDARY CELL SALINITY TIME SERIES ID NUMBER  
NTSERN: NORTH BOUNDARY CELL TEMPERATURE TIME SERIES ID NUMBER  
NDSERN: NORTH BOUNDARY CELL DYE CONC TIME SERIES ID NUMBER  
NSFSERN: NORTH BOUNDARY CELL SHELLFISH LARVAE TIME SERIES ID NUMBER  
NTXSERN: NORTH BOUNDARY CELL TOXIC CONTAMINANT CONC TIME SERIES ID NUM.  
NSDSERN: NORTH BOUNDARY CELL COHESIVE SED CONC TIME SERIES ID NUMBER  
NSNSERN: NORTH BOUNDARY CELL NONCOHESIVE SED CONC TIME SERIES ID NUMBER

C

C62 IBBN JBBN NTSCRN NSSERN NTSERN NDSERN NSFSERN NTXSERN NSDSERN NSNSERN

C63 TIME CONSTANT BOTTOM CONC ON NORTH CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING BOTTOM LAYER SALINITY  
TEM: ULTIMATE INFLOWING BOTTOM LAYER TEMPERATURE  
DYE: ULTIMATE INFLOWING BOTTOM LAYER DYE CONCENTRATION  
SFL: ULTIMATE INFLOWING BOTTOM LAYER SHELLFISH LARVAE CONCENTRAION  
TOX: NTOX ULTIMATE INFLOWING BOTTOM LAYER TOXIC CONTAMINANT  
CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C

C63 SAL TEM DYE SFL TOX1-20

C64 TIME CONSTANT BOTTOM CONC ON NORTH CONC BOUNDARIES

C

SED: NSED ULTIMATE INFLOWING BOTTOM LAYER COHESIVE SEDIMENT  
CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
SND: NSND ULTIMATE INFLOWING BOTTOM LAYER NONCOHESIVE SEDIMENT  
CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C

C64 SED1 SED2 SND1 SND2 SND3

C65 TIME CONSTANT SURFACE CONC ON NORTH CONC BOUNDARIES

C

SAL: ULTIMATE INFLOWING SURFAC LAYER SALINITY  
TEM: ULTIMATE INFLOWING SURFAC LAYER TEMPERATURE  
DYE: ULTIMATE INFLOWING SURFAC LAYER DYE CONCENTRATION  
SFL: ULTIMATE INFLOWING SURFAC LAYER SHELLFISH LARVAE CONCENTRAION  
TOX: NTOX ULTIMATE INFLOWING SURFAC LAYER TOXIC CONTAMINANT  
CONCENTRATIONS NTOX VALUES TOX(N), N=1,NTOX

C  
C65 SAL TEM DYE SFL TOX1-20

-----  
C66 TIME CONSTANT SURFACE CONC ON NORTH CONC BOUNDARIES

C  
SED: NSED ULTIMATE INFLOWING SURFAC LAYER COHESIVE SEDIMENT  
CONCENTRAIONS FIRST NSED VALUES SED(N), N=1,NSND  
SND: NSND ULTIMATE INFLOWING SURFAC LAYER NONCOHESIVE SEDIMENT  
CONCENTRATIONS LAST NSND VALUES SND(N), N=1,NSND

C  
C66 SED1 SED2 SND1 SND2 SND3

-----  
C66a CONCENTRATION DATA ASSIMILATION

C  
NLCDA: NUMBER OF HORIZONTAL LOCATIONS FOR DATA ASSIMILATION  
TSCDA: WEIGHTING FACTOR, 0.-1., 1. = FULL ASSIMILATION  
ISCDA: 1 FOR CONCENTRATION DATA ASSIMILATION (NC=1.7 VALUES)

C  
C66A NLCDA TSCDA ISCDA  
0 0.5 0 0 0 0 0 0 0

-----  
C66B CONCENTRATION DATA ASSIMILATION

C  
ICDA: 1 FOR CONCENTRATION DATA ASSIMILATION  
JCDA: NUMBER OF HORIZONTAL LOCATIONS FOR DATA ASSIMILATION  
NS: WEIGHTING FACTOR, 0.-1., 1. = FULL ASSIMILATION

C  
C66B ICDA JCDA NS NT ND NSF NTX NSD NSN

-----  
C67 DRIFTER DATA (FIRST 4 PARAMETER FOR SUB DRIFER, SECOND 6 FOR SUB LAGRES)

C  
ISPD: 1 TO ACTIVE SIMULTANEOUS RELEASE AND LAGRANGIAN TRANSPORT OF  
NEUTRALLY BUOYANT PARTICLE DRIFTERS AT LOCATIONS INPUT ON C44  
NPD: NUMBER OF PARTICLE DIRIFERS  
NPDRT: TIME STEP AT WHICH PARTICLES ARE RELEASED  
NRPD: NUMBER OF TIME STEPS BETWEEN WRITING TO TRACKING FILE  
drifter.out  
ISLRPD: 1 TO ACTIVATE CALCULATION OF LAGRANGIAN MEAN VELOCITY OVER TIME  
INTERVAL TREF AND SPATIAL INTERVAL ILRPD1<I<ILRPD2,  
JLRPD1<J<JLRPD2, 1<K<KC, WITH MLRPDRT RELEASES. ANY AVERAGE  
OVER ALL RELEASE TIMES IS ALSO CALCULATED  
2 SAME BUT USES A HIGER ORDER TRAJECTORY INTEGRATION  
ILRPD1 WEST BOUNDARY OF REGION  
ILRPD2 EAST BOUNDARY OF REGION  
JLRPD1 NORTH BOUNDARY OF REGION  
JLRPD2 SOUTH BOUNDARY OF REGION  
MLRPDRT NUMBER OF RELEASE TIMES  
IPLRPD 1,2,3 WRITE FILES TO PLOT ALL,EVEN,ODD HORIZ LAG VEL VECTORS

C  
C67 ISPD NPD NPDRT NRPD ISLRPD ILRPD1 ILRPD2 JLRPD1 JLRPD2 MLRPDRT IPLRPD  
0 0 0 12 0 6 19 11 19 11 1

-----  
C68 INITIAL DRIFTER POSITIONS (FOR USE WITH SUB DRIFTER)

C  
RI: I CELL INDEX IN WHICH PARTICLE IS RELEASED IN  
RJ: J CELL INDEX IN WHICH PARTICLE IS RELEASED IN  
RK: K CELL INDEX IN WHICH PARTICLE IS RELEASED IN

C  
C68 RI RJ RK

-----  
C69 CONSTANTS FOR CARTESION GRID CELL CENTER LONGITUDE AND LATITUDE

C  
CDLON1: 6 CONSTANTS TO GIVE CELL CENTER LAT AND LON OR OTHER  
CDLON2: COORDINATES FOR CARTESIAN GRIDS USING THE FORMULAS  
CDLON3: DLON(L)=CDLON1+(CDLON2\*FLOAT(I)+CDLON3)/60.  
CDLAT1: DLAT(L)=CDLAT1+(CDLAT2\*FLOAT(J)+CDLAT3)/60.  
CDLAT2:  
CDLAT3:

C  
C69 CDLON1 CDLON2 CDLON3 CDLAT1 CDLAT2 CDLAT3  
0.0 0.0 0.0 0.0 0.0 0.0

-----  
C70 CONTROLS FOR WRITING ASCII OR BINARY DUMP FILES

C  
ISDUMP: GREATER THAN 0 TO ACTIVATE  
1 SCALED ASCII INTERGER (0<VAL<65535)  
2 SCALED 16BIT BINARY INTEGER (0<VAL<65535) OR (-32768<VAL<32767)  
3 UNSCALED ASCII FLOATING POINT  
4 UNSCALED BINARY FLOATING POINT  
ISADMP: GREATER THAN 0 TO APPEND EXISTING DUMP FILES  
NSDUMP: NUMBER OF TIME STEPS BETWEEN DUMPS  
TSDUMP: STARTING TIME FOR DUMPS (NO DUMPS BEFORE THIS TIME)  
TEDUMP: ENDING TIME FOR DUMPS (NO DUMPS AFTER THIS TIME)  
ISDMPP: GREATER THAN 0 FOR WATER SURFACE ELEVATION DUMP  
ISDMPU: GREATER THAN 0 FOR HORIZONTAL VELOCITY DUMP  
ISDMPW: GREATER THAN 0 FOR VERTICAL VELOCITY DUMP  
ISDMPT: GREATER THAN 0 FOR TRANSPORTED VARIABLE DUMPS  
IADJDMP: 0 FOR SCALED BINARY INTEGERS (0<VAL<65535)  
-32768 FOR SCALED BINARY INTEGERS (-32768<VAL<32767)

C  
C70 ISDUMP ISADMP NSDUMP TSDUMP TEDUMP ISDMPP ISDMPU ISDMPW ISDMPT IADJDMP  
0 0 288 0. 45. 0 0 0 1 -32768

-----  
C71 CONTROLS FOR HORIZONTAL PLANE SCALAR FIELD CONTOURING

C  
ISSPH: 1 TO WRITE FILE FOR SCALAR FIELD CONTOURING IN HORIZONTAL PLANE  
NPSPH: NUMBER OF WRITES PER REFERENCE TIME PERIOD  
ISRSPH: 1 TO WRITE FILE FOR RESIDUAL SALINITY PLOTTING IN  
HORIZONTAL  
ISPHXY: 0 DOES NOT WRITE I,J,X,Y IN \*\*\*cnh.out and r\*\*\*cnh.out FILES  
1 WRITES I,J ONLY IN \*\*\*cnh.out and r\*\*\*cnh.out FILES  
2 WRITES I,J,X,Y IN \*\*\*cnh.out and r\*\*\*cnh.out FILES  
DATA LINE REPEATS 7 TIMES FOR SAL,TEM,DYE,SFL,TOX,SED,SND

C  
C71 ISSPH NPSPH ISRSPH ISPHXY  
0 1 0 1 !SAL  
0 1 0 1 !TEM  
1 96 0 1 !DYE  
0 24 0 1 !SFL  
0 24 0 1 !TOX  
0 24 0 1 !SED  
0 24 0 1 !SND

-----  
C72 CONTROLS FOR HORIZONTAL SURFACE ELEVATION OR PRESSURE CONTOURING

C

ISPPH: 1 TO WRITE FILE FOR SURFACE ELEVATION OR PRESSURE CONTOURING  
IN HORIZONTAL PLANE  
NPPPH: NUMBER OF WRITES PER REFERENCE TIME PERIOD  
ISRPPH: 1 TO WRITE FILE FOR RESIDUAL SURFACE ELEVATION CONTOURING IN  
HORIZONTAL PLANE

C

C72 ISPPH NPPPH ISRPPH  
0 6 0

-----  
C73 CONTROLS FOR HORIZONTAL PLANE VELOCITY VECTOR PLOTTING

C

ISVPH: 1 TO WRITE FILE FOR VELOCITY PLOTTING IN HORIZONTAL PLANE  
NPVPH: NUMBER OF WRITES PER REFERENCE TIME PERIOD  
ISRVPH: 1 TO WRITE FILE FOR RESIDUAL VELOCITY PLOTTING IN  
HORIZONTAL PLANE

C

C73 ISVPH NPVPH ISRVPH  
1 96 0

-----  
C74 CONTROLS FOR VERTICAL PLANE SCALAR FIELD CONTOURING

C

ISECSPV: N AN INTEGER NUMBER OF VERTICAL SECTIONS (N.LE.9) TO WRITE  
N FILES FOR SCALAR FIELD CONTOURING  
NPSPV: NUMBER OF WRITES PER REFERENCE TIME PERIOD  
ISSPV: 1 TO ACTIVATE INSTANTANEOUS SCALAR FIELDS  
ISRSPV: 1 TO ACTIVATE FOR RESIDUAL SCALAR FIELDS  
ISHPLTV: 1 FOR VERTICAL PLANE PLOTTING FOR MSL DATUMS, ZERO OTHERWISE  
DATA LINE REPEATS 7 TIMES FOR SAL,TEM,DYE,SFL,TOX,SED,SND  
ISECSPV IS DETERMINED FOR ALL 7 VARIABLES BY VALUE ON FIRST DATA LINE

C

C74 ISECSPV NPSPV ISSPV ISRSPV ISHPLTV  
0 6 0 0 1 !SAL  
0 6 0 0 1 !TEM  
0 6 0 0 1 !DYE  
0 6 0 0 1 !SFL  
0 6 0 0 1 !TOX  
0 6 0 0 1 !SED  
0 6 0 0 1 !SND

-----  
C75 MORE CONTROLS FOR VERTICAL PLANE SCALAR FIELD CONTOURING

C

ISECSPV: SECTION NUMBER  
NIJSPV: NUMBER OF CELLS OR I,J PAIRS IN SECTION  
SEC ID: CHARACTER FORMAT SECTION TITLE

C

C75 ISECSPV NIJSPV SEC ID

-----  
C76 I,J LOCATIONS FOR VERTICAL PLANE SCALAR FIELD CONTOURING

C

ISECSPV: SECTION NUMBER  
ISPV: I CELL  
JSPV: J CELL

C

C76 ISECSPV ISPV JSPV

-----  
C77 CONTROLS FOR VERTICAL PLANE VELOCITY VECTOR PLOTTING

C

ISECVPV: N AN INTEGER NUMBER (N.LE.9) OF VERTICAL SECTIONS  
          TO WRITE N FILES FOR VELOCITY PLOTTING  
NPVPV:    NUMBER OF WRITES PER REFERENCE TIME PERIOD  
ISVPV:    1 TO ACTIVATE INSTANTANEOUS VELOCITY  
ISRSPV:    1 TO ACTIVATE FOR RESIDUAL VELOCITY

C

C77 ISECVPV   NPVPV   ISVPV   ISRSPV  
      0        6        0        0

-----  
C78 MORE CONTROLS FOR VERTICAL PLANE VELOCITY VECTOR PLOTTING

C

ISCEVPV:   SECTION NUMBER  
NIJVPV:    NUMBER IS CELLS OR I,J PAIRS IN SECTION  
ANGVPV:    CCW POSITIVE ANGLE FROM EAST TO SECTION NORMAL  
SEC ID:     CHARACTER FORMAT SECTION TITLE

C

C78 ISECVPV   NIJVPV   ANGVPV   SEC ID

-----  
C79 CONTROLS FOR VERTICAL PLANE VELOCITY PLOTTING

C

ISECVPV:   SECTION NUMBER (REFERENCE USE HERE)  
IVPV:       I CELL INDEX  
JVPV:       J CELL INDEX

C

C79 ISECVPV   IVPV       JVPV

-----  
C80 CONTROLS FOR 3D FIELD OUTPUT

C

IS3DO:    1 TO WRITE TO 3D ASCI INTEGER FORMAT FILES, JS3Dvar.LE.2        SEE  
          1 TO WRITE TO 3D ASCI FLOAT POINT FORMAT FILES, JS3Dvar.EQ.3 C57  
          2 TO WRITE TO 3D CHARACTER ARRAY FORMAT FILES (NOT ACTIVE)  
          3 TO WRITE TO 3D HDF IMAGE FORMAT FILES (NOT ACTIVE)  
          4 TO WRITE TO 3D HDF FLOATING POINT FORMAT FILES (NOT ACTIVE)  
ISR3DO:    SAME AS IS3DO EXCEPT FOR RESIDUAL VARIABLES  
NP3DO:     NUMBER OF WRITES PER LAST REF TIME PERIOD FOR INST VARIABLES  
KPC:       NUMBER OF UNSTRETCHED PHYSICAL VERTICAL LAYERS  
NWGG:      IF NWGG IS GREATER THAN ZERO, NWGG DEFINES THE NUMBER OF !2877  
          WATER CELLS IN CARTESIAN 3D GRAPHICS GRID OVERLAY OF THE  
          CURVILINEAR GRID. FOR NWGG>0 AND EFDC RUNS ON A CURVILINEAR  
          GRID, I3DMI,I3DMA,J3DMI,J3DMA REFER TO CELL INDICES ON THE  
          ON THE CARTESIAN GRAPHICS GRID OVERLAY DEFINED BY FILE  
          gcell.inp. THE FILE gcell.inp IS NOT USED BY EFDC, BUT BY  
          THE COMPANION GRID GENERATION CODE GEFDC.F. INFORMATION  
          DEFINING THE OVERLAY IS READ BY EFDC.F FROM THE FILE  
          gcellmp.inp. IF NWGG EQUALS 0, I3DMI,I3DMA,J3DMI,J3DMA REFER  
          TO INDICES ON THE EFDC GRID DEFINED BY cell.inp.  
          ACTIVATION OF THE REWRITE OPTION I3DRW=1 WRITES TO THE FULL  
          GRID DEFINED BY cell.inp AS IF cell.inp DEFINES A CARTESIAN  
          GRID. IF NWGG EQ 0 AND THE EFDC COMP GRID IS CO, THE REWRITE  
          OPTION IS NOT RECOMMENDED AND A POST PROCESSOR SHOULD BE USED  
          TO TRANSFER THE SHORT FORM, I3DRW=0, OUTPUT TO AN APPROPRIATE  
          FORMAT FOR VISUALIZATION. CONTACT DEVELOPER FOR MORE DETAILS  
I3DMI:     MINIMUM OR BEGINNING I INDEX FOR 3D ARRAY OUTPUT  
I3DMA:     MAXIMUM OR ENDING I INDEX FOR 3D ARRAY OUTPUT

J3DMI: MINIMUM OR BEGINNING J INDEX FOR 3D ARRAY OUTPUT  
 J3DMA: MAXIMUM OR ENDING J INDEX FOR 3D ARRAY OUTPUT  
 I3DRW: 0 FILES WRITTEN FOR ACTIVE CO WATER CELLS ONLY  
 1 REWRITE FILES TO CORRECT ORIENTATION DEFINED BY gcell.inp  
 AND gcellmp.inp FOR CO WITH NWGG.GT.0 OR BY cell.inp IF THE  
 COMPUTATIONAL GRID IS CARTESIAN AND NWGG.EQ.0  
 SELVMAX: MAXIMUM SURFACE ELEVATION FOR UNSTRETCHING (ABOVE MAX SELV )  
 BELVMIN: MINIMUM BOTTOM ELEVATION FOR UNSTRETCHING (BELOW MIN BELV)

C

C80	IS3DO	ISR3DO	NP3DO	KPC	NWGG	I3DMI	I3DMA	J3DMI	J3DMA	I3DRW	SELVMAX	BELVMIN
	0	0	0	1	0	1	120	1	70	0	15.0	-315.

C81 OUTPUT ACTIVATION AND SCALES FOR 3D FIELD OUTPUT

C

VARIABLE: DUMMY VARIABLE ID (DO NOT CHANGE ORDER)  
 IS3(VARID): 1 TO ACTIVATE THIS VARIABLES  
 JS3(VARID): 0 FOR NO SCALING OF THIS VARIABLE  
 1 FOR AUTO SCALING OF THIS VARIABLE OVER RANGE 0<VAL<255  
 AUTO SCALES FOR EACH FRAME OUTPUT IN FILES out3d.dia AND  
 rout3d.dia OUTPUT IN I4 FORMAT  
 2 FOR SCALING SPECIFIED IN NEXT TWO COLUMNS WITH OUTPUT  
 DEFINED OVER RANGE 0<VAL<255 AND WRITTEN IN I4 FORMAT  
 3 FOR MULTIPLIER SCALING BY MAX SCALE VALUE WITH OUTPUT  
 WRITTEN IN F7.1 FORMAT (IS3DO AND ISR3DO MUST BE 1)

C

C81	VARIABLE	IS3D(VARID)	JS3D(VARID)	MAX SCALE VALUE	MIN SCALE VALUE
	'U VEL'	1	3	100.0	-1.0
	'V VEL'	1	3	100.0	-1.0
	'W VEL'	0	0	1000.0	-1.0E-3
	'SALINITY'	1	3	1.0	0.0
	'TEMP'	1	3	1.0	10.0
	'DYE'	0	0	1000.0	0.0
	'COH SED'	1	3	1000.0	0.0
	'NCH SED'	1	3	1000.0	0.0
	'TOX CON'	1	3	1000.0	0.0

C82 INPLACE HARMONIC ANALYSIS PARAMETERS

C

ISLSHA: 1 FOR IN PLACE LEAST SQUARES HARMONIC ANALYSIS  
 MLLSHA: NUMBER OF LOCATIONS FOR LSHA  
 NTCLSHA: LENGTH OF LSHA IN INTEGER NUMBER OF REFERENCE TIME PERIODS  
 ISLSTR: 1 FOR TREND REMOVAL  
 ISHTA : 1 FOR SINGLE TREF PERIOD SURFACE ELEV ANALYSIS

C

C82	ISLSHA	MLLSHA	NTCLSHA	ISLSTR	ISHTA
	1	2	29	0	0

C83 HARMONIC ANALYSIS LOCATIONS AND SWITCHES

C

ILLSHA: I CELL INDEX  
 JLLSHA: J CELL INDEX  
 LSHAP: 1 FOR ANALYSIS OF SURFACE ELEVATION  
 LSHAB: 1 FOR ANALYSIS OF SALINITY  
 LSHAUE: 1 FOR ANALYSIS OF EXTERNAL MODE HORIZONTAL VELOCITY  
 LSHAU: 1 FOR ANALYSIS OF HORIZONTAL VELOCITY IN EVERY LAYER  
 CLSL: LOCATION AS A CHARACTER VARIABLE

C

C83	ILLSHA	JLLSHA	LSHAP	LSHAB	LSHAUE	LSHAU	CLSL		
	8	6	1	0	0	0	'Flupsy	'	! 1
	108	11	1	0	0	0	'SHE5	'	! 2

C84 CONTROLS FOR WRITING TO TIME SERIES FILES

C

ISTMSR: 1 OR 2 TO WRITE TIME SERIES OF SURF ELEV, VELOCITY, NET  
INTERNAL AND EXTERNAL MODE VOLUME SOURCE-SINKS, AND  
CONCENTRATION VARIABLES, 2 APPENDS EXISTING TIME SERIES FILES

MLTMSR: NUMBER HORIZONTAL LOCATIONS TO WRITE TIME SERIES OF SURF ELEV,  
VELOCITY, AND CONCENTRATION VARIABLES, MAXIMUM LOCATIONS = 9

NBTMSR: TIME STEP TO BEGIN WRITING TO TIME SERIES FILES

NSTMSR: TIME STEP TO STOP WRITING TO TIME SERIES FILES

NWTSER: WRITE INTERVAL FOR WRITING TO TIME SERIES FILES

NTSSTSP: NUMBER OF TIME SERIES START-STOP SCENARIOS, 1 OR GREATER

TCTMSR: UNIT CONVERSION FOR TIME SERIES TIME. FOR SECONDS, MINUTES,  
HOURS,DAYS USE 1.0, 60.0, 3600.0, 86400.0 RESPECTIVELY

IDUM: 2 DUMMY INTEGER VARIABLES REQUIRED, BOTH = 0

C		13				1728			
C84	ISTMSR	MLTMSR	NBTMSR	NSTMSR	NWTMSR	NTSSTSP	TCTMSR	IDUM	IDUM
	1	28	1	2000000	90	1	86400.	0	0

C85 CONTROLS FOR WRITING TO TIME SERIES FILES

C

ITSSS: START-STOP SCENARIO NUMBER 1.GE.ISSS.LE.NTSSTSP

MTSSTSP: NUMBER OF STOP-START PAIRS FOR SCENARIO ISSS

C85	ITSSS	MTSSTSP	
	1	1	!FULL SAVE

C86 CONTROLS FOR WRITING TO TIME SERIES FILES

C

ITSSS: START-STOP SCENARIO NUMBER 1.GE.ISSS.LE.NTSSTSP

MTSSS: NUMBER OF STOP-START PAIRS FOR SCENARIO ISSS

TSSTRT: STARTING TIME FOR SCENARIO ITSSS, SAVE INTERVAL MTSSS

TSSTOP: STOPPING TIME FOR SCENARIO ITSSS, SAVE INTERVAL MTSSS

C			-1000.		
C86	ISSS	MTSSS	TSSTRT	TSSTOP	USER COMMENT
	1	1	-1000.	10000.	! FULL SAVE

C87 CONTROLS FOR WRITING TO TIME SERIES FILES

C

ILTS: I CELL INDEX

JLTS: J CELL INDEX

NTSSSS: WRITE SCENARIO FOR THIS LOCATION

MTSP: 1 FOR TIME SERIES OF SURFACE ELEVATION

MTSC: 1 FOR TIME SERIES OF TRANSPORTED CONCENTRATION VARIABLES

MTSA: 1 FOR TIME SERIES OF EDDY VISCOSITY AND DIFFUSIVITY

MTSUE: 1 FOR TIME SERIES OF EXTERNAL MODE HORIZONTAL VELOCITY

MTSUT: 1 FOR TIME SERIES OF EXTERNAL MODE HORIZONTAL TRANSPORT

MTSU: 1 FOR TIME SERIES OF HORIZONTAL VELOCITY IN EVERY LAYER

MTSQE: 1 FOR TIME SERIES OF NET EXTERNAL MODE VOLUME SOURCE/SINK

MTSQ: 1 FOR TIME SERIES OF NET EXTERNAL MODE VOLUME SOURCE/SINK

CLTS: LOCATION AS A CHARACTER VARIABLE

C87	ILTS	JLTS	NTSSSS	MTSP	MTSC	MTSA	MTSUE	MTSUT	MTSU	MTSQE	MTSQ	CLTS
	108	11	1	1	1	0	0	0	0	0	0	'She5

58	13	1	1	1	0	0	0	0	0	0	'HSLNor1'	2
58	12	1	1	1	0	0	0	0	0	0	'HSL2'	3
58	11	1	1	1	0	0	0	0	0	0	'HSL3'	4
58	10	1	1	1	0	0	0	0	0	0	'HSLSou4'	5
83	11	1	1	1	0	0	0	0	0	0	'She4I3'	6
33	11	1	1	1	0	0	0	0	0	0	'She3'	7
19	11	1	1	1	1	1	1	1	1	1	'Outfall'	8
14	14	1	1	1	0	0	0	0	0	0	'She2'	9
8	6	1	1	1	0	0	0	0	0	0	'Flupsy'	10
8	16	1	1	1	0	0	0	0	0	0	'OSLWest'	11
9	16	1	1	1	0	0	0	0	0	0	'ISCO2'	12
10	16	1	1	1	0	0	0	0	0	0	'OSL3'	13
11	16	1	1	1	0	0	0	0	0	0	'OSL4'	14
12	16	1	1	1	0	0	0	0	0	0	'OSL5'	15
13	16	1	1	1	0	0	0	0	0	0	'OSL6'	16
14	16	1	1	1	0	0	0	0	0	0	'OSL7'	17
15	16	1	1	1	0	0	0	0	0	0	'OSL8'	18
16	16	1	1	1	0	0	0	0	0	0	'OSL9'	19
17	16	1	1	1	0	0	0	0	0	0	'OSLEast'	20
12	27	1	1	1	0	0	0	0	0	0	'She1'	21
15	34	1	1	1	0	0	0	0	0	0	'She6'	22
10	35	1	1	1	0	0	0	0	0	0	'ISCO1'	23
12	27	1	1	1	0	0	0	0	0	0	'She7'	24
119	13	1	1	1	0	0	0	0	0	0	'BLNorth'	25
119	12	1	1	1	0	0	0	0	0	0	'BL12'	26
119	11	1	1	1	0	0	0	0	0	0	'BL11'	27
119	10	1	1	1	0	0	0	0	0	0	'BLSouth'	28

C88 CONTROLS FOR EXTRACTING INSTANTANEOUS VERTICAL SCALAR FIELD PROFILES

C

ISVSFP: 1 FOR EXTRACTING INSTANTANEOUS VERTICAL FIELD PROFILES  
MDVSFP: MAXIMUM NUMBER OF DEPTHS FOR SAMPLING VALUES  
MLVSFP: NUMBER OF HORIZONTAL SPACE-TIME LOCATION PAIRS TO BE SAMPLED  
TMVSFP: MULTIPLIER TO CONVERT SAMPLING TIMES TO SECONDS  
TAVSFP: ADDITIVE ADJUSTMENT TO SAMPLING TIME BEFORE CONVERSION TO SEC

C

200max 1600max

C88 ISVSFP MDVSFP MLVSFP TMVSFP TAVSFP  
0 0 0 86400. 0.0

C89 SAMPLING DEPTHS FOR EXTRACTING INST VERTICAL SCALAR FIELD PROFILES

C

MMDVSFP: Mth SAMPLING DEPTH  
DMSFP: SAMPLING DEPTH BELOW SURFACE, IN METERS

C

C89 MMDVSFP DMVSFP

C90 HORIZONTAL SPACE-TIME LOCATIONS FOR SAMPLING

C

MMLVSFP: Mth SPACE TIME SAMPLING LOCATION  
TIMVSFP: SAMPLING TIME  
IVSFP: I HORIZONTAL LOCATON INDEX  
JVSFP: J HORIZONTAL LOCATON INDEX

C

C90 MMLVSFP TIMVSFP IVSFP JVSFP

\*\*\*\*\*

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## **Appendix B**

### **Computational plane for Oakland Bay system (CELL.INP)**

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## Appendix C

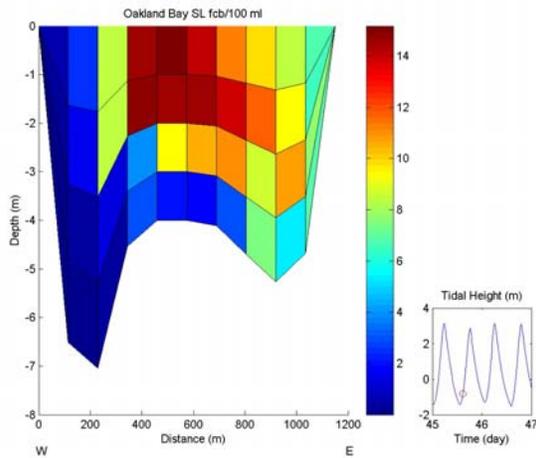
### Model run output

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**Tabular and select graphic output from all time-steps at which an exceedance occurred on the *Oakland Bay* sanitary line:**

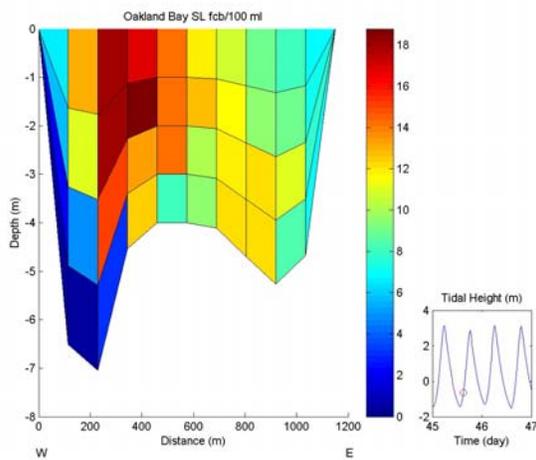
Run3: 4.0 mgd for 13 hours; ebb then flood with diffuser:

---



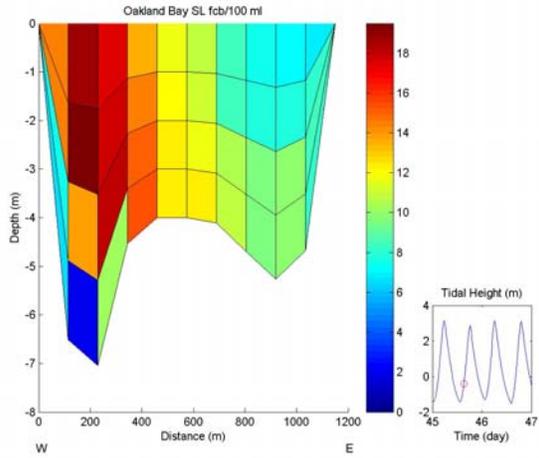
time(2159) = 45.614580

0.52	2.40	8.41	14.66	15.17	14.10	11.34	9.78	8.47	6.77
0.28	1.65	8.34	14.77	14.48	14.49	13.94	11.62	9.42	6.52
0.08	0.33	1.31	3.86	9.28	10.64	10.96	8.66	10.73	7.68
0.05	0.07	0.46	3.06	2.04	1.81	3.07	7.38	5.23	6.87



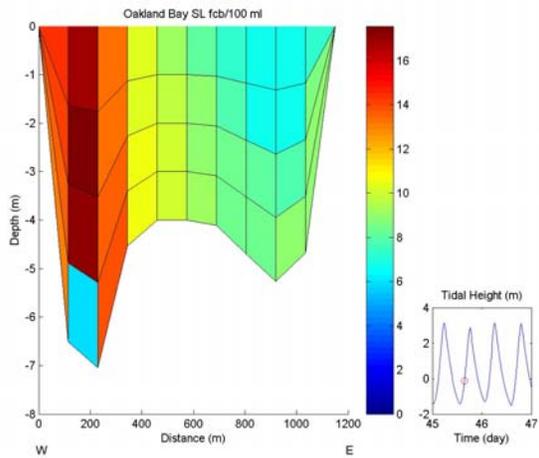
time(2160) = 45.625000 (3:00PM)

6.72	13.17	18.11	16.96	14.18	11.88	10.58	8.97	8.00	7.01
5.86	10.90	17.96	18.77	14.31	12.81	11.61	9.68	8.93	7.18
2.28	4.95	15.11	13.23	14.27	10.14	12.00	12.17	10.89	7.73
0.45	0.39	3.07	12.61	7.95	9.46	12.19	12.05	8.39	6.85



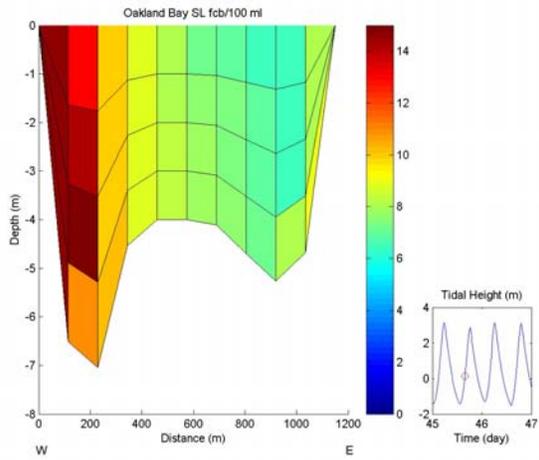
time(2161) = 45.635420

14.41	18.75	17.56	13.64	11.73	10.68	8.31	7.46	7.02	6.89
14.59	19.49	17.94	14.59	11.95	11.19	8.36	7.90	7.73	8.01
7.33	13.81	17.98	14.94	12.22	11.84	10.53	9.48	9.96	8.43
6.66	1.99	10.26	15.27	12.37	12.25	10.82	9.65	10.01	8.03

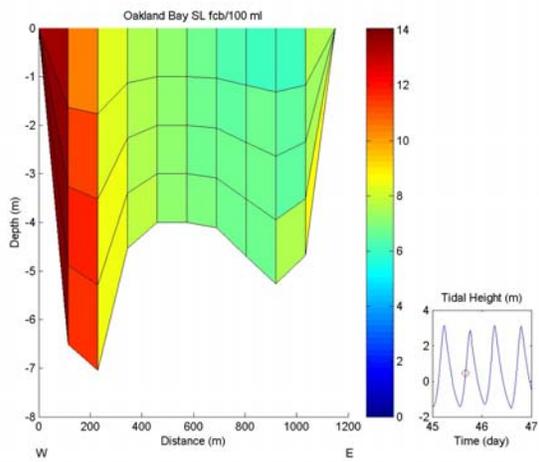


time(2162) = 45.645830 (3:30PM)

14.38	16.81	13.30	10.32	9.32	8.11	7.18	6.68	6.66	7.34
14.15	17.56	13.43	10.41	9.77	8.86	7.71	6.62	6.69	8.60
13.70	17.16	13.61	10.53	9.97	9.04	8.23	8.12	7.89	8.87
13.11	5.92	13.92	10.65	10.11	9.19	8.48	8.25	9.05	8.99



```
time(2163) = 45.656250
14.74 12.96 9.94 8.73 8.07 7.25 6.82 6.52 6.45 8.07
14.72 14.21 9.93 8.75 8.16 7.70 7.33 6.79 6.44 7.98
14.67 14.99 9.98 8.79 8.24 7.80 7.48 6.95 6.45 8.03
14.61 10.81 10.12 8.83 8.32 7.88 7.59 7.05 8.07 8.95
```



```
time(2164) = 45.666670
13.60 10.38 8.35 7.58 7.05 6.67 6.35 6.08 6.12 7.52
13.73 11.29 8.34 7.64 7.07 6.74 6.54 6.64 6.52 7.41
13.90 11.86 8.38 7.69 7.10 6.80 6.61 6.68 6.59 7.68
14.07 11.48 8.48 7.75 7.15 6.86 6.67 6.76 7.58 8.59
```

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## Run4: 6.7 mgd for 13 hours; ebb then flood with diffuser:

---

time(2158) = 45.604170  
0.04 0.18 1.39 5.47 10.03 13.36 15.03 15.09 13.95 11.07  
0.03 0.08 0.60 2.20 5.67 9.60 14.04 15.82 14.81 10.63  
0.04 0.04 0.19 0.84 1.95 4.63 9.88 12.96 14.90 10.29  
0.05 0.05 0.10 0.71 1.19 2.19 2.87 3.29 5.01 10.01

time(2159) = 45.614580  
0.89 4.07 14.20 24.61 25.39 23.55 19.00 16.41 14.24 11.44  
0.47 2.80 14.08 24.80 24.11 24.18 23.15 19.32 15.77 11.01  
0.13 0.55 2.19 6.46 15.57 17.70 18.19 14.35 17.72 12.80  
0.08 0.11 0.78 5.13 3.40 3.00 5.10 12.25 8.63 11.43

time(2160) = 45.625000 (3:00PM)  
11.33 22.17 30.40 28.45 23.79 19.97 17.78 15.10 13.43 11.78  
9.87 18.33 30.15 31.43 24.00 21.46 19.46 16.28 15.05 12.06  
3.84 8.28 25.34 22.14 23.92 16.86 20.06 20.30 18.16 12.98  
0.75 0.65 5.14 21.10 13.27 15.76 20.36 20.08 13.84 11.38

time(2161) = 45.635420  
24.23 31.51 29.49 22.87 19.66 17.91 13.94 12.52 11.76 11.54  
24.58 32.75 30.12 24.49 20.03 18.77 14.02 13.23 12.94 13.41  
12.19 23.14 30.19 25.08 20.48 19.85 17.65 15.86 16.62 14.10  
11.09 3.34 17.20 25.62 20.73 20.53 18.15 16.12 16.57 13.31

time(2162) = 45.645830 (3:30PM)  
24.15 28.21 22.30 17.29 15.59 13.57 12.03 11.19 11.15 12.29  
23.75 29.46 22.52 17.45 16.36 14.83 12.92 11.06 11.16 14.35  
22.96 28.74 22.82 17.64 16.69 15.13 13.76 13.55 13.13 14.74  
21.95 9.90 23.34 17.85 16.93 15.39 14.17 13.76 15.08 14.92

time(2163) = 45.656250  
24.70 21.73 16.66 14.63 13.52 12.15 11.43 10.93 10.80 13.46  
24.67 23.79 16.63 14.66 13.66 12.87 12.26 11.37 10.77 13.30  
24.60 25.10 16.72 14.72 13.79 13.05 12.52 11.63 10.78 13.36  
24.49 18.09 16.95 14.78 13.92 13.17 12.70 11.78 13.46 14.87

time(2164) = 45.666670  
22.82 17.41 14.00 12.69 11.80 11.18 10.63 10.19 10.24 12.55  
23.04 18.93 13.98 12.79 11.83 11.29 10.95 11.10 10.90 12.35  
23.31 19.88 14.05 12.87 11.89 11.39 11.07 11.18 11.02 12.82  
23.59 19.23 14.22 12.97 11.97 11.49 11.16 11.30 12.65 14.26

time(2165) = 45.677080  
18.60 12.86 10.05 8.96 8.27 7.87 7.69 7.69 8.09 11.27  
19.04 14.91 10.06 9.21 8.47 7.93 7.53 7.47 8.05 11.28  
19.53 16.57 10.15 9.31 8.56 8.07 7.84 7.91 8.38 12.52  
19.99 17.13 10.32 9.43 8.67 8.21 7.97 7.98 8.41 13.27

time(2166) = 45.687500  
12.79 9.01 6.67 6.00 5.68 5.47 5.35 5.33 5.53 10.28  
16.27 9.34 5.58 5.45 4.95 4.78 4.86 4.99 5.35 10.35  
16.62 10.97 5.62 5.51 5.04 5.02 4.93 4.78 5.03 10.45  
16.99 11.40 5.73 5.58 5.12 5.07 4.97 4.84 5.08 12.11

time(2183) = 45.864580  
14.78 12.33 11.00 10.77 11.06 11.69 11.51 10.83 9.67 7.75  
14.69 11.00 11.46 11.25 10.80 11.42 12.19 10.59 8.14 5.43  
10.60 5.01 8.21 7.68 7.28 7.01 6.80 5.42 4.87 4.52  
5.92 5.08 5.78 5.24 4.85 4.79 4.80 4.29 4.96 4.54

time(2184) = 45.875000  
14.75 12.45 10.99 10.59 10.99 11.66 11.59 11.04 10.06 8.21  
14.77 11.85 12.23 12.20 11.55 12.16 12.36 11.07 8.93 5.87  
11.43 5.01 9.09 9.11 8.22 7.62 7.42 6.13 5.58 4.85  
6.33 5.24 6.03 5.76 5.17 4.94 5.00 4.29 5.48 4.88

time(2187) = 45.906250  
9.56 9.33 8.71 8.56 8.86 9.42 10.50 11.10 10.82 9.95  
11.83 12.03 12.58 10.71 9.58 10.25 10.50 12.42 10.94 7.73  
13.36 5.44 11.29 11.99 13.39 14.34 12.38 10.94 8.60 5.73  
8.11 5.75 5.67 7.54 7.07 6.58 7.16 5.32 5.88 5.69

time(2188) = 45.916670  
7.39 7.58 7.29 7.19 7.51 8.10 9.16 10.31 10.59 10.16  
9.06 12.06 10.78 8.91 8.23 9.07 9.19 12.08 11.25 8.72  
12.69 5.60 12.23 12.64 13.68 14.33 13.03 12.83 10.04 6.13  
8.58 5.98 5.99 8.41 7.88 7.41 7.97 6.33 5.45 5.74

time(2189) = 45.927080  
5.33 6.13 6.02 5.96 6.22 6.77 7.75 9.02 9.85 9.84  
6.66 11.22 8.83 7.05 7.07 7.66 8.45 12.28 11.30 9.54  
12.19 5.66 11.47 11.91 12.60 14.26 12.52 13.86 10.44 6.94  
8.99 6.24 6.25 9.21 8.55 8.15 8.63 7.46 4.80 5.04

time(2190) = 45.937500  
4.07 5.20 5.16 5.03 5.16 5.58 6.46 7.62 8.77 9.01  
5.59 9.75 7.50 5.88 6.08 6.55 7.45 11.73 11.00 9.37  
11.86 5.75 10.31 10.30 11.63 13.12 11.91 14.29 10.41 7.51  
9.33 6.51 6.68 9.62 8.97 8.58 9.15 8.21 4.91 4.81

time(2191) = 45.947920  
3.61 4.73 4.73 4.44 4.43 4.69 5.38 6.29 7.55 7.93  
5.67 8.69 6.78 5.64 5.74 5.98 6.65 10.91 10.50 8.61  
11.43 5.80 9.34 9.07 11.40 11.95 11.86 14.33 10.08 7.99  
9.57 6.76 7.05 9.71 9.24 8.64 9.49 8.65 5.18 4.83

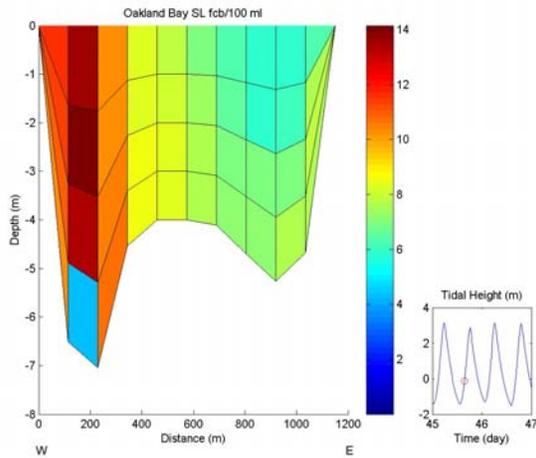
```

time(2192) =      45.958330
  3.40  4.38  4.44  4.11  4.00  4.13  4.59  5.21  6.38  6.88
  5.16  8.32  6.70  5.59  5.56  5.72  6.17  9.60  9.50  7.64
 10.67  5.80  8.58  8.53 10.60 11.00 12.60 14.13  9.66  8.36
  9.75  6.99  7.23  9.59  9.47  8.54  9.80  8.84  5.44  5.23

```

**Run6: 4.0 mgd for 13 hours; ebb then flood with extended diffuser:**

---



```

time(2162) =      45.645830 (3:30PM)
 11.61 13.53 10.27  8.35  7.77  6.89  6.24  5.86  5.80  6.39
 11.38 14.15 10.36  8.39  8.12  7.44  6.60  5.87  6.06  7.62
 10.94 13.39 10.48  8.46  8.28  7.58  6.93  7.02  6.92  7.80
 10.38  4.35 10.72  8.54  8.39  7.70  7.11  7.12  7.73  7.69

```

**Run7: 6.7 mgd for 13 hours; ebb then flood with extended diffuser:**

---

```

time(2159) =      45.614580
  0.47  2.20  8.01 14.70 16.31 16.22 13.75 12.45 11.25  9.26
  0.26  1.53  7.93 14.78 15.59 16.75 17.89 16.26 13.73  8.90
  0.08  0.33  1.35  3.93 10.02 12.05 13.06 11.33 15.49 11.38
  0.05  0.07  0.50  3.13  2.30  2.12  3.61  9.41  7.19 10.09

```

```

time(2160) =      45.625000 (3:00PM)
  6.99 14.02 20.22 20.85 18.42 16.05 14.58 12.67 11.24  9.84
  6.01 11.44 19.97 22.51 18.67 17.74 15.82 13.90 13.22 10.02
  2.35  5.30 16.61 15.06 18.35 12.40 15.87 16.71 16.14 11.29
  0.46  0.41  3.33 14.18  9.40 11.55 16.00 16.39 11.66 10.00

```

```

time(2161) =      45.635420
 17.25 23.28 22.35 17.91 15.85 14.82 11.79 10.75 10.14  9.82
 17.29 23.27 22.80 18.97 16.14 15.60 11.94 11.48 11.42 11.59
  8.35 16.54 22.80 19.34 16.45 16.51 15.19 13.99 14.78 12.35
  7.56  2.30 12.42 19.68 16.60 17.04 15.58 14.04 13.95 11.46

```

time(2162) = 45.645830 (3:30PM)

19.47	22.67	17.18	13.99	13.03	11.56	10.46	9.81	9.69	10.62
19.10	23.69	17.31	14.04	13.61	12.47	11.05	9.82	10.09	12.64
18.38	22.39	17.52	14.15	13.86	12.70	11.62	11.76	11.49	12.91
17.47	7.30	17.90	14.29	14.04	12.90	11.91	11.93	12.83	12.70

time(2163) = 45.656250

21.65	18.20	13.52	12.72	11.88	10.91	10.35	9.82	9.57	11.82
21.59	19.61	13.44	12.75	11.94	11.30	10.97	10.21	9.58	11.82
21.49	20.32	13.47	12.77	12.01	11.39	11.14	10.42	9.58	11.81
21.35	13.59	13.61	12.78	12.08	11.45	11.26	10.54	11.74	12.74

time(2164) = 45.666670

21.19	16.35	12.95	11.86	11.11	10.58	10.07	9.55	9.39	11.17
21.34	17.09	12.94	11.90	11.13	10.65	10.32	10.37	10.18	11.09
21.53	17.35	12.99	11.95	11.17	10.72	10.41	10.44	10.28	11.40
21.73	15.52	13.12	12.01	11.22	10.78	10.48	10.53	11.54	12.35

time(2165) = 45.677080

19.16	14.08	10.50	9.53	8.89	8.40	8.01	7.72	7.66	10.20
19.39	14.61	10.36	9.27	8.42	7.60	7.20	6.95	7.27	10.21
19.68	15.30	10.37	9.29	8.46	7.69	7.40	7.29	7.70	11.16
19.97	15.62	10.47	9.35	8.55	7.80	7.51	7.37	7.73	11.65

time(2166) = 45.687500

16.41	12.08	8.46	7.25	6.63	6.14	5.73	5.42	5.28	9.38
17.06	10.46	5.83	5.60	4.89	4.72	4.60	4.59	4.79	9.42
17.24	10.90	5.88	5.60	4.89	4.70	4.57	4.39	4.58	9.47
17.46	11.16	5.98	5.63	4.93	4.73	4.60	4.44	4.63	10.70

**Run9: 4.0 mgd for 13 hours; flood then ebb with diffuser:**

---

time(2160) = 45.625000 (3:00PM)

6.02	11.57	15.14	12.12	9.19	6.35	5.24	3.58	2.93	2.77
5.34	9.73	15.13	14.40	9.59	7.90	7.30	4.34	3.91	3.16
2.09	4.45	13.04	11.31	10.18	8.86	8.75	8.63	7.84	4.10
0.39	0.33	2.81	10.98	6.67	8.39	9.16	9.08	7.92	5.76

time(2161) = 45.635420

11.60	13.70	11.11	7.23	5.40	4.18	2.19	1.67	1.58	2.00
11.78	15.23	11.51	8.49	5.84	4.59	2.25	1.85	2.13	3.26
6.38	10.78	11.65	8.96	6.40	5.23	4.61	3.81	5.94	4.03
5.83	1.62	7.80	9.38	6.85	5.67	4.93	4.64	8.35	6.70

time(2208) = 46.125000

3.17	6.31	10.59	12.31	12.05	11.76	11.35	10.75	10.28	9.25
3.45	5.94	10.56	13.75	12.27	12.97	12.72	11.70	11.59	9.70
3.46	5.65	10.01	10.00	11.78	10.65	12.08	14.80	13.16	12.31
3.45	3.86	4.27	9.72	6.99	7.99	12.05	14.42	12.63	10.24

## Run10: 6.7 mgd for 13 hours; flood then ebb with diffuser:

---

time(2158) = 45.604170  
0.02 0.16 1.36 5.39 9.81 12.90 14.22 13.86 12.32 9.16  
0.00 0.07 0.59 2.18 5.57 9.36 13.50 14.86 13.26 8.84  
0.00 0.02 0.17 0.80 1.89 4.49 9.55 12.38 13.94 8.94  
0.00 0.00 0.09 0.67 1.13 2.09 2.75 3.15 4.67 8.74

time(2159) = 45.614580  
0.84 3.91 13.54 22.96 22.48 19.90 14.35 11.40 9.01 6.62  
0.44 2.69 13.43 23.21 22.19 20.67 19.33 15.18 11.15 6.85  
0.09 0.51 2.13 6.24 14.98 16.90 16.95 13.08 15.83 9.97  
0.01 0.05 0.74 4.97 3.27 2.84 4.85 11.43 8.15 9.51

time(2160) = 45.625000 (3:00PM)  
10.19 19.51 25.45 20.31 15.38 10.64 8.78 6.01 4.91 4.63  
9.03 16.42 25.43 24.09 16.04 13.19 12.18 7.25 6.52 5.29  
3.53 7.47 21.90 18.97 17.03 14.74 14.56 14.31 12.94 6.82  
0.66 0.55 4.71 18.42 11.16 14.02 15.24 15.06 12.98 9.48

time(2161) = 45.635420  
19.51 23.03 18.61 12.09 9.03 6.99 3.67 2.79 2.63 3.33  
19.79 25.57 19.27 14.20 9.77 7.67 3.76 3.09 3.54 5.41  
10.76 18.07 19.49 14.98 10.68 8.73 7.68 6.36 9.80 6.68  
9.81 2.72 13.06 15.68 11.43 9.45 8.22 7.76 13.76 11.03

time(2162) = 45.645830 (3:30PM)  
17.64 17.13 10.91 6.90 5.52 3.67 2.38 1.74 1.84 3.62  
17.46 18.23 11.07 7.22 6.44 5.12 3.53 1.88 2.55 5.46  
17.07 19.38 11.30 7.42 6.82 5.45 4.66 4.41 6.75 7.26  
16.54 7.00 11.73 7.59 7.09 5.72 5.18 4.62 9.49 11.70

time(2163) = 45.656250  
15.96 10.70 6.30 4.80 3.75 2.63 1.96 1.54 1.55 4.80  
16.03 12.09 6.28 4.88 3.88 3.48 2.83 2.11 2.12 4.93  
16.12 13.76 6.32 4.97 4.00 3.68 3.08 2.34 2.14 6.29  
16.20 10.91 6.46 5.06 4.11 3.82 3.26 2.46 5.62 10.28

time(2206) = 46.104170  
3.05 3.10 3.42 4.72 6.29 7.94 9.49 10.53 11.17 10.62  
3.75 4.41 3.67 4.74 5.82 7.72 10.21 12.76 14.02 10.34  
4.78 6.63 4.41 4.93 6.46 7.92 11.09 14.29 17.43 13.30  
6.48 6.47 5.35 4.89 6.14 6.77 6.84 7.38 8.77 10.90

time(2207) = 46.114580  
3.27 4.09 7.16 12.51 15.40 17.13 16.62 16.16 15.61 13.58  
3.73 4.93 7.11 12.53 15.49 17.32 19.88 21.54 19.89 12.80  
5.12 6.10 5.57 8.46 13.31 16.56 19.84 20.25 20.03 16.00  
6.53 6.34 5.70 7.57 7.44 7.23 7.92 11.35 13.44 15.54

time(2208) = 46.125000  
5.30 10.56 17.73 20.62 20.20 19.70 19.02 18.01 17.22 15.50  
5.77 9.93 17.68 23.00 20.58 21.72 21.30 19.59 19.41 16.26  
5.80 9.45 16.69 16.73 19.77 17.84 20.23 24.81 22.09 20.67  
5.78 6.44 7.17 16.27 11.74 13.41 20.20 24.17 21.21 17.14

time(2209) = 46.135420  
11.59 17.85 20.01 20.24 19.98 19.11 17.58 17.26 17.17 17.23  
13.93 19.13 18.92 20.81 20.26 20.12 17.59 18.08 18.71 20.03  
9.14 16.43 18.63 21.03 20.53 21.33 21.74 20.98 22.80 21.54  
8.93 6.87 18.05 21.04 20.72 22.01 22.09 21.56 21.27 19.33

time(2210) = 46.145830  
14.46 18.59 19.51 17.90 17.58 17.12 16.88 16.64 16.70 18.41  
14.06 19.65 19.65 17.88 17.80 17.45 17.05 16.67 17.08 20.86  
13.44 19.49 19.80 17.95 17.96 18.40 17.09 17.29 18.13 21.49  
12.84 12.55 20.04 18.06 18.06 18.61 17.16 17.38 18.30 20.49

time(2211) = 46.156250  
16.15 17.52 17.11 16.70 16.63 16.35 16.24 16.09 16.12 19.50  
15.84 18.33 17.11 16.70 16.67 16.40 16.49 16.47 16.35 19.78  
15.57 18.63 17.15 16.70 16.71 16.44 16.56 16.56 16.85 19.85  
15.33 15.52 17.25 16.71 16.76 16.47 16.62 16.65 16.88 19.90

time(2212) = 46.166670  
16.83 16.21 15.67 15.34 14.98 14.63 14.40 14.43 14.84 18.46  
16.88 16.74 15.69 15.41 15.06 14.91 14.82 14.88 15.34 19.10  
16.92 16.86 15.74 15.48 15.14 15.01 14.88 14.94 15.44 19.07  
16.95 16.31 15.83 15.55 15.22 15.09 14.96 15.02 15.52 19.15

time(2213) = 46.177080  
14.87 12.82 11.61 10.61 9.99 9.76 9.77 10.05 10.94 16.50  
15.24 14.17 11.72 10.74 9.71 8.94 8.72 9.30 10.41 17.24  
15.58 14.49 11.88 10.90 9.80 9.00 8.60 8.63 9.91 18.45  
15.86 14.86 12.11 11.09 9.96 9.14 8.70 8.77 10.00 18.58

time(2214) = 46.187500  
9.80 7.80 6.26 5.88 5.76 5.71 5.76 5.95 6.63 14.02  
11.33 7.94 5.18 4.59 3.87 3.97 4.36 4.73 5.73 14.05  
11.92 8.16 5.25 4.58 3.77 3.38 3.00 3.30 4.38 16.66  
12.31 8.42 5.39 4.64 3.77 3.34 3.04 3.36 4.38 16.65

**Run13: 6.7 mgd for 13 hours; flood then ebb with extended diffuser:**

---

time(2160) = 45.625000 (3:00PM)  
5.72 11.11 14.93 12.56 9.95 6.94 5.92 4.09 3.39 3.40  
5.05 9.31 14.91 14.98 10.50 9.35 8.57 5.22 5.24 3.97  
2.01 4.37 12.85 11.63 11.08 10.02 10.23 10.88 10.85 5.34  
0.38 0.33 2.84 11.25 7.17 9.49 10.82 11.27 10.67 7.91

time(2161) = 45.635420  
12.15 14.45 11.47 7.37 5.62 4.46 2.29 1.87 1.92 2.53  
12.18 15.75 11.92 8.86 6.20 5.00 2.40 2.17 2.84 4.31  
6.56 11.19 12.08 9.40 6.92 5.81 5.46 4.74 8.03 5.40  
5.97 1.66 8.11 9.88 7.49 6.37 5.87 5.82 10.94 8.99

time(2206) = 46.104170  
2.20 2.25 2.51 3.55 4.77 5.99 7.11 7.79 8.20 7.74  
2.75 3.41 2.74 3.63 4.53 5.98 7.77 9.67 10.57 7.66  
3.62 5.36 3.45 3.88 5.17 6.42 8.96 11.58 14.19 11.06  
5.07 5.31 4.35 3.85 4.93 5.56 5.77 6.20 7.57 9.19

time(2207) = 46.114580  
2.37 3.10 5.73 10.15 12.48 13.75 13.20 12.71 12.19 10.43  
2.74 3.76 5.68 10.16 12.48 13.91 15.96 17.37 16.03 9.80  
3.98 4.86 4.43 6.82 10.80 13.51 16.18 16.72 16.84 13.64  
5.10 5.26 4.59 6.10 6.11 6.09 6.66 9.29 11.39 13.22

time(2208) = 46.125000  
4.39 9.40 15.81 18.24 17.65 17.08 16.29 15.18 14.33 12.48  
4.82 8.68 15.77 20.33 17.84 18.83 18.21 16.48 16.05 13.32  
4.52 8.01 14.80 14.33 17.20 15.34 17.45 21.23 18.87 17.64  
4.49 5.22 5.93 13.91 10.00 11.60 17.39 20.49 17.94 14.79

time(2209) = 46.135420  
11.28 17.56 19.32 19.13 18.72 17.67 15.82 15.21 14.86 14.45  
13.13 18.60 18.13 19.52 18.98 18.81 15.89 16.03 16.49 16.94  
8.31 15.78 17.70 19.47 19.23 20.03 20.22 19.47 20.12 18.55  
8.05 5.57 16.87 19.38 19.36 20.72 20.55 19.69 18.30 16.71

time(2210) = 46.145830  
15.36 19.10 19.39 17.20 16.59 15.79 15.34 14.80 14.68 15.75  
14.92 20.45 19.57 17.27 17.02 16.40 15.84 14.86 15.28 18.21  
13.80 20.04 19.74 17.39 17.25 17.66 15.96 16.23 16.94 19.05  
12.96 11.77 20.01 17.53 17.41 17.94 16.09 16.40 17.08 18.02

time(2211) = 46.156250  
17.01 17.54 16.24 15.79 15.58 15.19 14.98 14.65 14.49 17.10  
16.67 18.67 16.22 15.79 15.61 15.24 15.31 15.21 15.11 17.69  
16.35 19.13 16.26 15.80 15.65 15.27 15.39 15.30 15.87 18.20  
16.08 15.63 16.37 15.80 15.71 15.30 15.44 15.40 15.96 18.21

time(2212) = 46.166670  
17.45 16.10 14.99 14.68 14.30 13.95 13.64 13.52 13.71 16.42  
17.51 16.52 14.98 14.74 14.36 14.20 14.08 14.12 14.37 17.39  
17.56 16.63 15.02 14.80 14.42 14.29 14.15 14.19 14.50 17.62  
17.60 16.09 15.09 14.85 14.48 14.36 14.22 14.27 14.57 17.68

```

time(2213) =      46.177080
15.56 13.10 11.50 10.44  9.75  9.44  9.34  9.50 10.18 14.83
15.92 14.16 11.58 10.53  9.44  8.61  8.31  8.73  9.75 15.72
16.26 14.45 11.72 10.68  9.53  8.68  8.24  8.27  9.48 17.01
16.55 14.81 11.93 10.86  9.69  8.82  8.35  8.40  9.56 17.12

```

```

time(2214) =      46.187500
10.63  8.28  6.37  5.88  5.68  5.57  5.54  5.63  6.17 12.70
12.00  8.08  5.10  4.44  3.71  3.77  4.09  4.41  5.28 12.73
12.53  8.28  5.16  4.43  3.61  3.22  2.85  3.11  4.08 15.29
12.90  8.53  5.29  4.49  3.62  3.18  2.89  3.16  4.08 15.28

```

**Tabular output from all time-steps at which an exceedance occurred on the *Hammersley Inlet* sanitary line:**

**Run4: 6.7 mgd for 13 hours; ebb then flood with diffuser:**

---

```

time(2131) =      45.322920
 4.44  6.45 10.50 14.38
 4.42  6.11 10.52 14.46
 4.41  5.91 10.40 14.46
 4.39  5.77 10.28 14.46

```

```

time(2132) =      45.333330
 4.54  6.16 10.48 14.42
 4.56  5.96 10.52 14.46
 4.58  5.80 10.42 14.46
 4.59  5.68 10.30 14.46

```

```

time(2133) =      45.343750
 4.05  5.56  9.99 14.19
 4.09  5.35  9.96 14.46
 4.12  5.17  9.81 14.46
 4.15  5.04  9.65 14.46

```

```

time(2134) =      45.354170
 3.30  5.83 10.87 14.44
 3.33  5.61 10.88 14.46
 3.35  5.42 10.71 14.46
 3.38  5.28 10.51 14.46

```

```

time(2135) =      45.364580
 2.97  5.65 10.92 14.50
 2.96  5.49 10.92 14.50
 2.96  5.33 10.76 14.50
 2.98  5.20 10.57 14.50

```

```
time(2136) =      45.375000
  2.79  5.68  10.90  14.50
  2.78  5.50  10.90  14.50
  2.78  5.32  10.74  14.50
  2.79  5.18  10.54  14.50
```

```
time(2137) =      45.385420
  2.73  6.02  11.21  14.50
  2.69  5.86  11.22  14.50
  2.67  5.69  11.08  14.50
  2.67  5.55  10.90  14.50
```

```
time(2138) =      45.395830
  2.80  6.20  11.16  14.50
  2.76  6.07  11.18  14.50
  2.73  5.91  11.06  14.50
  2.71  5.78  10.91  14.50
```

```
time(2139) =      45.406250
  2.95  6.31  11.17  14.50
  2.91  6.19  11.21  14.50
  2.87  6.04  11.09  14.50
  2.85  5.92  10.95  14.50
```

```
time(2140) =      45.416670
  3.09  6.50  11.15  14.46
  3.08  6.39  11.18  14.50
  3.05  6.24  11.08  14.50
  3.03  6.12  10.96  14.50
```

```
time(2141) =      45.427080
  3.16  6.79  11.39  14.39
  3.14  6.69  11.42  14.50
  3.12  6.55  11.32  14.50
  3.09  6.43  11.20  14.50
```

```
time(2142) =      45.437500
  3.25  7.24  11.71  14.39
  3.23  7.13  11.75  14.50
  3.20  6.98  11.64  14.50
  3.17  6.85  11.52  14.50
```

```
time(2143) =      45.447920
  3.45  7.90  12.31  14.43
  3.42  7.79  12.35  14.50
  3.39  7.64  12.25  14.50
  3.35  7.51  12.13  14.50
```

```
time(2144) =      45.458330
  3.73  8.49  12.62  14.45
  3.70  8.38  12.65  14.50
  3.66  8.23  12.55  14.50
  3.61  8.10  12.44  14.50
```

```
time(2145) =      45.468750
  4.08  9.23  13.07  14.34
  4.04  9.12  13.09  14.50
  3.98  8.97  12.97  14.50
  3.92  8.83  12.85  14.50
```

```
time(2146) =      45.479170
  4.50  9.63  13.16  14.09
  4.42  9.53  13.17  14.49
  4.33  9.38  13.07  14.50
  4.26  9.26  12.98  14.50
```

```
time(2147) =      45.489580
  4.93 10.02  13.16  13.70
  4.82  9.87  13.14  14.29
  4.71  9.69  13.01  14.33
  4.62  9.54  12.89  14.39
```

```
time(2149) =      45.510420
  5.74 11.28  14.10  13.10
  5.59 11.16  14.09  13.11
  5.44 10.98  13.96  13.22
  5.31 10.85  13.84  13.30
```

```
time(2150) =      45.520830
  6.05 11.68  14.59  13.46
  5.92 11.52  14.54  13.17
  5.75 11.31  14.35  13.14
  5.61 11.14  14.18  13.14
```

```
time(2151) =      45.531250
  6.47 12.23  15.22  13.86
  6.34 12.11  15.21  13.38
  6.16 11.90  14.99  13.15
  6.00 11.72  14.79  13.15
```

```
time(2152) =      45.541670
  6.94 12.31  15.55  14.39
  6.81 12.22  15.55  13.79
  6.60 12.02  15.26  13.15
  6.39 11.86  15.02  13.15
```

```
time(2153) =      45.552080
  7.74  12.45  15.72  14.91
  7.54  12.29  15.74  14.91
  7.09  11.93  15.21  13.41
  6.57  11.70  14.75  13.27
```

```
time(2154) =      45.562500
  9.71  13.17  15.78  15.51
  9.27  12.93  15.59  15.54
  8.48  11.95  14.38  15.06
  6.51  11.21  13.80  14.07
```

```
time(2155) =      45.572920
 11.61  13.83  14.63  14.91
 10.57  12.82  13.61  14.79
  7.88  10.73  12.46  14.90
  6.89   9.57  12.73  14.94
```

```
time(2156) =      45.583330
 10.96  13.18  13.32  14.27
  8.51  12.27  12.27  14.24
  8.43  10.91  11.40  14.19
  8.38  10.90  11.49  14.15
```

### Run10: 6.7 mgd for 13 hours; flood then ebb with diffuser:

---

```
time(2180) =      45.833330
  4.94   9.36  14.25  14.31
  4.87   9.25  14.19  13.68
  4.83   9.09  13.96  13.46
  4.80   8.92  13.65  12.81
```

```
time(2181) =      45.843750
  6.71  10.31  15.65  19.15
  6.70  10.02  15.70  19.31
  6.68   9.81  15.58  19.32
  6.66   9.65  15.44  19.32
```

```
time(2182) =      45.854170
  7.49  10.13  15.44  19.30
  7.50   9.84  15.47  19.32
  7.50   9.62  15.33  19.32
  7.50   9.47  15.16  19.32
```

```
time(2183) =      45.864580
  7.22   9.57  15.03  19.48
  7.24   9.29  14.99  19.74
  7.26   9.06  14.79  19.75
  7.28   8.90  14.58  19.74
```

```
time(2184) =      45.875000
  6.61 10.09 16.31 20.33
  6.61  9.78 16.32 20.39
  6.63  9.51 16.09 20.39
  6.66  9.30 15.81 20.39
```

```
time(2185) =      45.885420
  6.39  9.99 16.39 20.48
  6.36  9.78 16.38 20.51
  6.34  9.56 16.17 20.51
  6.34  9.39 15.94 20.51
```

```
time(2186) =      45.895830
  6.37 10.12 16.63 20.75
  6.33  9.89 16.62 20.90
  6.32  9.66 16.41 20.90
  6.32  9.47 16.16 20.90
```

```
time(2187) =      45.906250
  6.36 10.56 17.32 20.98
  6.32 10.35 17.34 20.99
  6.29 10.11 17.15 20.99
  6.28  9.92 16.92 20.99
```

```
time(2188) =      45.916670
  6.43 11.06 17.64 21.08
  6.37 10.87 17.64 21.22
  6.33 10.64 17.46 21.22
  6.31 10.45 17.25 21.22
```

```
time(2189) =      45.927080
  6.49 11.38 18.09 21.35
  6.42 11.22 18.13 21.37
  6.37 11.01 17.97 21.37
  6.35 10.83 17.76 21.37
```

```
time(2190) =      45.937500
  6.58 11.80 18.44 21.37
  6.50 11.63 18.49 21.37
  6.44 11.41 18.33 21.37
  6.42 11.23 18.14 21.37
```

```
time(2191) =      45.947920
  6.71 12.32 18.73 21.52
  6.63 12.16 18.75 21.68
  6.57 11.96 18.59 21.68
  6.53 11.77 18.40 21.68
```

```
time(2192) =      45.958330
  6.87  13.02  19.45  21.79
  6.81  12.87  19.50  21.81
  6.74  12.67  19.35  21.81
  6.70  12.48  19.16  21.81
```

```
time(2193) =      45.968750
  7.14  13.83  20.06  21.81
  7.10  13.69  20.12  21.81
  7.02  13.48  19.98  21.81
  6.96  13.29  19.80  21.81
```

```
time(2194) =      45.979170
  7.54  14.62  20.42  21.81
  7.51  14.49  20.47  21.81
  7.45  14.30  20.33  21.81
  7.37  14.11  20.17  21.81
```

```
time(2195) =      45.989580
  7.99  15.53  20.81  21.79
  7.94  15.40  20.84  21.81
  7.87  15.20  20.70  21.81
  7.79  15.01  20.54  21.81
```

```
time(2196) =      46.000000
  8.53  16.31  21.02  21.46
  8.46  16.21  21.05  21.77
  8.37  16.02  20.92  21.79
  8.28  15.85  20.78  21.81
```

```
time(2197) =      46.010420
  9.13  16.74  20.89  20.64
  9.03  16.62  20.90  21.17
  8.91  16.44  20.77  21.20
  8.79  16.28  20.64  21.66
```

```
time(2198) =      46.020830
  9.76  17.16  20.85  20.00
  9.60  17.01  20.83  20.11
  9.44  16.80  20.68  20.13
  9.29  16.64  20.53  21.08
```

```
time(2199) =      46.031250
 10.44  17.79  21.25  19.79
 10.20  17.55  21.22  19.72
  9.96  17.28  20.99  19.72
  9.76  17.08  20.77  20.66
```

time(2200) = 46.041670  
11.35 18.52 21.67 19.91  
10.93 18.20 21.68 19.72  
10.47 17.86 21.38 19.72  
10.17 17.63 21.07 20.35

time(2201) = 46.052080  
12.95 19.23 21.95 20.43  
12.34 18.80 21.98 19.86  
11.19 18.21 21.47 19.77  
10.07 17.90 20.92 19.97

time(2202) = 46.062500  
16.10 20.12 22.07 21.41  
15.10 19.53 21.97 20.91  
12.89 18.19 20.57 20.89  
9.34 17.17 19.90 20.62

time(2203) = 46.072920  
18.40 20.58 20.57 21.04  
17.50 19.63 19.30 21.09  
14.02 16.36 18.04 21.15  
10.09 14.19 18.23 21.21

time(2204) = 46.083330  
16.48 19.42 19.36 20.32  
13.15 19.24 18.52 20.29  
12.99 16.45 17.84 20.24  
12.90 16.37 17.93 20.20

time(2205) = 46.093750  
15.59 18.17 19.15 19.43  
15.44 18.17 19.07 19.32  
15.33 15.11 16.11 19.10  
15.20 14.74 15.66 18.93

time(2206) = 46.104170  
16.02 16.87 18.11 19.34  
16.02 15.71 17.63 19.30  
15.98 15.18 16.41 19.20  
15.92 14.92 16.16 19.11

time(2207) = 46.114580  
15.09 15.40 16.90 18.80  
15.09 15.36 16.99 18.82  
15.09 15.37 17.06 18.81  
15.10 15.41 17.14 18.80

```
time(2208) =      46.125000
14.53 13.09 13.23 17.22
14.54 13.13 13.35 17.23
14.52 13.18 13.49 17.20
14.50 13.25 13.68 17.16
```