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WA-30-1010

STATE OF WASHINGTON

DEPARTMENT OF ECOLOGY

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To:

Alan Newman, Central Regional Office

From:

Joseph Joy, Water Quality Investigations Section

Subject:

Little Klickitat River Receiving Water Survey in the

Vicinity of Goldendale STP

INTRODUCTION

As you will recall, you and I performed a receiving water survey on the Little Klickitat River in the vicinity of Goldendale on October 27 and 28, 1981. The purpose of the study was to document the impact of the Goldendale STP effluent on water quality. It is hoped that these data may also be useful to you in managing the effluent from Goldendale's new lagoon and land application system due to be functional in the spring of 1983. Finally, these data may provide a baseline for evaluation of the new system especially if a receiving water survey is done in the future.

SITE DESCRIPTION

The Little Klickitat River flows in south central Washington from the Simcoe Mountains and Horse Heaven Hills, through the Klickitat Valley to the Klickitat River (Figure 1). The entire drainage is 280 mi² and includes range, agricultural, and forest lands. The river has been designated as Class A, and is used primarily for irrigation, stock watering, and aquatic life habitat.

Goldendale STP lies at river mile (R.M.) 14.9 of the Little Klickitat River and serves a population of approximately 3,500 people. It is a small, trickling filter system built in the early 1940s with some improvements made in the early 1960s (WDOE file 14-30-01). The plant has experienced hydraulic overloading and periods of inadequate treatment in the past. Also, winter flooding has exposed and broken a portion of the effluent pipe so that effluent flows in at least 100 feet of open channel before mixing with the river at approximately R.M. 14.6.

The new sewage lagoon and land irrigation system under construction is expected to be functional in the spring of 1983 (Newman, 1982). The

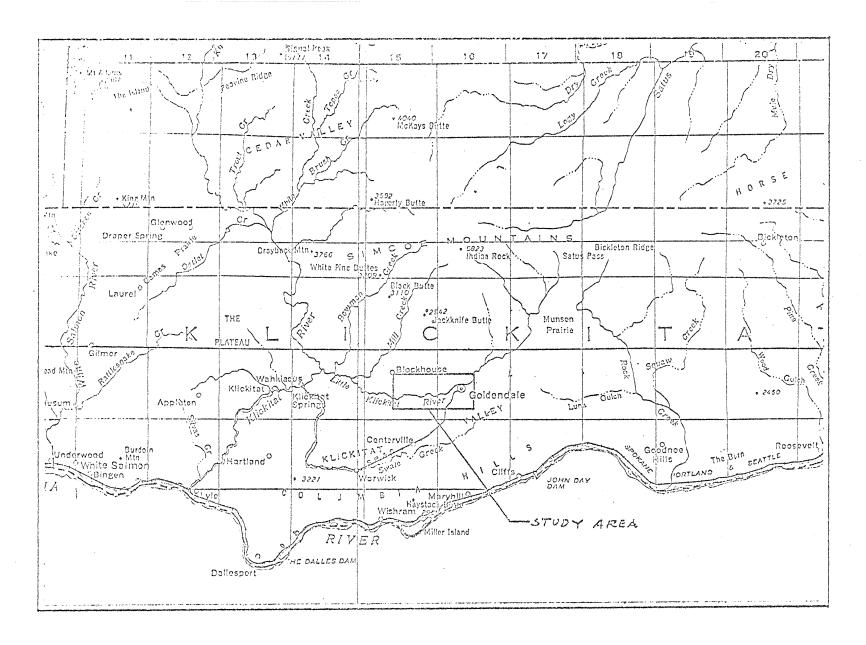


Figure 1. Location of the Little Klickitat River drainage in southcentral Washington, and the receiving water study area near Goldendale STP of October 27-28, 1981.

system will allow domestic sewage to be held in lagoons and applied to surrounding fields throughout the growing season. Depending upon permit requirements, effluent will be discharged into the Little Klickitat River at R.M. 14.1 (Figure 2) only when a river-to-effluent dilution ratio of 20:1 or greater is present, or it will be eliminated only during low-flow months of the year; e.g., July-October (Newman, 1981).

The general work area for this receiving water study included 5.8 miles of river from the Pipeline Road Bridge at R.M. 16.3, to the Esteb Road Bridge at R.M. 10.5 (Figure 2). Within this area were the current STP discharge at R.M. 14.6, the proposed discharge at R.M. 14.1, the confluence of Bloodgood Creek with the Little Klickitat at R.M. 14.9, and the area where channelization work was underway by the City of Goldendale at approximately R.M. 15.7.

METHODS

Water quality grab samples were taken on two separate occasions at seven sites within the study area (Figure 2). Samples were taken at two sites above the STP outfall (Stations 1 and 2); three sites below the outfall (Stations 3 through 5), as well as from Bloodgood Creek (Station 1A) and the open effluent channel (Station EFF.). In addition, 24-hour composite samples were taken at Stations 2 and 3 with Manning $^{\rm R}$ field compositors set to collect 250 mLs at 30-minute intervals. Composite samplers were also set at the STP headworks and at the chlorine contact chamber for collection of influent and effluent samples.

Field analyses for the following water quality parameters were performed at each site: temperature, by mercury thermometer; dissolved oxygen, by Winkler method; total residual chlorine, by DPD ferrous titrametric method; and pH and specific conductivity, by field meters. Other water quality samples were collected, packed on ice, and transported to arrive at the WDOE Environmental laboratories within 24 hours. The following analyses were performed on those samples:

Total Solids Specific Conductivity Total Non-volatile Solids Biochemical Oxygen Demand Total Suspended Solids Chemical Oxygen Demand Total Non-volatile Suspended Solids Nitrate-N Chloride Nitrite-N Total Hardness Ammonia-N Fecal Coliform Orthophosphate-P Turbidity Na+, Mg++, and Ca++ Total Phosphorus-P

All laboratory analyses were performed according to standard procedures (USEPA, 1979; AWWA, 1981). Un-ionized ammonia concentration and sodium absorption ratio (SAR) were calculated using laboratory and field results and applying the appropriate formula (Thurston, Russo, and Emerson, 1979; Hem, 1970). Influent and effluent 24-hour composite samples were split with the Goldendale STP lab so that ${\rm BOD}_5$ and TSS results could be compared.

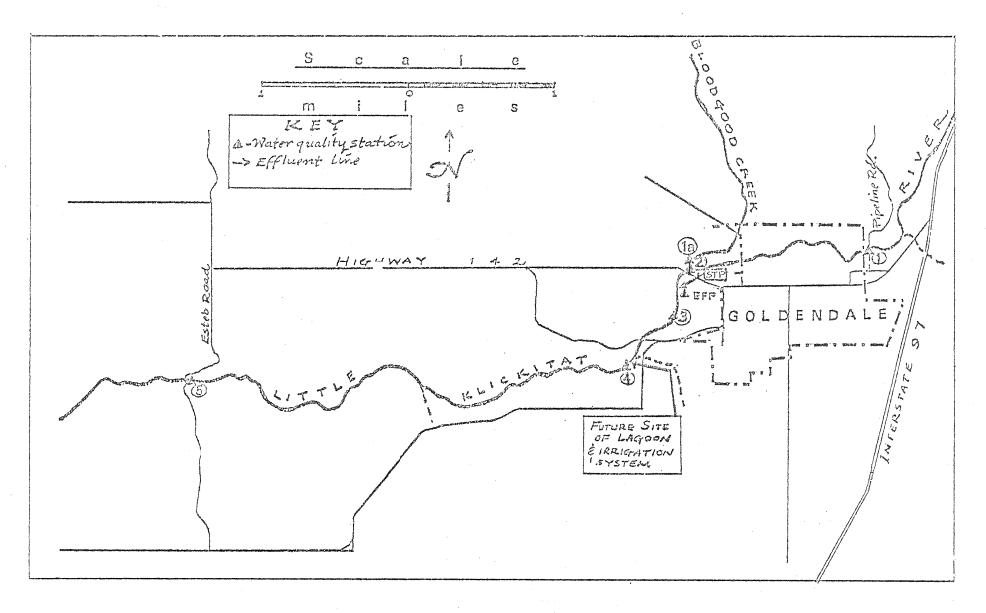


Figure 2. Little Klickitat River receiving water study area showing the location of water quality monitoring stations, and the current and future Goldendale sewage treatment facilities.

Discharge measurements were taken at Stations $1,R^2$, 4, 5, Bloodgood Creek, and the effluent ditch. A Marsh-McBernie magnetic flow meter, staff rod, and measuring tape were used to conduct stream velocity and cross-sectional measurements. The STP discharge was measured at the chlorine contact chamber using a Manning Dipper flow meter over a 24-hour period.

A 42- to 45-hour in-stream fish bioassay was performed at Stations 1, 2, 3, and 4. Fingerling rainbow trout (Salmo gairdneri) (5 months old) were obtained from Bob Neal of the Washington State Department of Game hatchery on Spring Creek. Nine or ten fish were placed in a fiberglass container (18" diameter by 24" length) with fine mesh netting at each end. Two of these containers (19 - 20 fish total) were placed at each of the four stations mentioned above. Containers were placed in areas of similar water velocity and depth, and oriented for maximum circulation through the cylinder. Fish were observed for signs of stress (e.g., erratic swimming, gaping mouths, etc.) or mortality after the 42-to 45-hour period and enumerated.

RESULTS AND DISCUSSION

Field and laboratory analytical results of receiving water quality parameters sought during the study are presented in Table 1. Included are calculated results for un-ionized ammonia (un-ion. NH_3-N) and SAR. Also, influent and effluent sample analytical results from both the WDOE and Goldendale STP labs are presented in Table 2. Mean station values for selected parameters are presented in Table 3.

Influent and Effluent

Analytical results performed by the WDOE and Goldendale STP laboratories on split influent and effluent 24-hour composite samples are presented in Table 2. Results of various grab samples are also included, as are results from the open effluent channel at the confluence with the river.

Results from both laboratories show good agreement on the effluent sample -- poor agreement on the influent sample. The Goldendale STP BOD results are more realistic based on the following equation:

3500 people x 0.2 lbs BOD/person/day = 700 lb. BOD/day

The low BOD and TSS values in the WDOE portion of the split sample may have resulted from improper mixing during the sample splitting.

Based on Goldendale and WDOE effluent sample results, the plant was meeting 85 percent removal of BOD and TSS as required. The pH and effluent volume were also within permit requirements. Coliform levels were within permit requirements, although TRC concentrations were excessive.

Table 1. Chemical analyses results on samples taken October 27 and 28, 1981, from the Little K'ickitat River near Go'dendale. All values mg/L unless otherwise stated.

Station Number	Location	River Mile	Date	Discharge (cfs)	Time of Travel (days)	Temperature (°C)	рн (s.u.)	Dissolved Oxygen	p.0. % Sat. 1/	800 ₅	000	N03-N	W2-14	NH3-N	Un-ionized NH3-N	0-P04-P	Total PhosP	Turbidity (NTU)	Total Solids	Total Non-volatile Solids	Total Suspended Solids	Total Non-volatile Suspended Solids	Chloride	Na	Mg	Ça	SAR- <u>2</u> /	Total Hardness	Fecal Coliform col/100 mL	Residual Chlorine	Specific Conductivity (umhos/cm)
1	Pipeline Road Bridge	16.3	10/27 10/28	4.2		11.0 9.6	8.3 8.1	11.2	102 99	ı.i		<0.01 <0.01	<0.01 <0.01		<0.001 <0.001	0.01	0.03 0.04	1	88 87	78 63	2 11	2	1.5					58 67	9* 24		124 (130) 123 (120)
1A	Bloodgood Creek	14.9	10/27 10/28	5 *		11.8 9.8		10.8	95			0.03	<0.01 0.01	0.05 0.03	<0.001	0.04 0.07	0.04	4	120	120	5	3	2.3 3.1					54			(100) 100 (100)
2	Highway 142 Bridge	14.8	10/27 10/28	7.3 10.2		11.2 9.8	8.0 8.0	10.5 10.8	96 95	5	8	0.02	<0.01 <0.01	<0.01 0.03	<0.001 <0.001	0.03 0.05	0.04	6 7	92 90	90 85	4 4	2 4	1.5	4.7	5.0	9.0	0.312		160 180		· 103 (110) 107 (120)
UC	Upstream Compositor	14.8	.10/27-28		, 4		7.9			4	12	0.01	<0.01	0.04	<0.001	0.12	0.09	74	190	140	53	31	2.3					54			110 (110)
STP	Goldendale Effluent		10/27-28	0.69 0.70 0.68		15.1	7.6	7.2	71	14	100	1.4	0.05	12 .	0.130	4.7	6.9	24	170	150	18	2	23					54	1-9*	2.0-1.5	339 (350)
EFF	Effluent Ditch	14.6	10/27 10/28		0	13.8 12.5	7.2 7.2	2.4 2.9	23 27	9	49	0.48 0.49	0.17 0.19	10 8.4	0.040 0.03	4.0 3.6	4.1 3.3 ⁺	16 8	210 100	160 66	5 5	3 2	23 22		*			63 79	44 84	0.10	357 (350) 325 (355)
3	Mik Area ~ \$00' Downstream of Outfall	14.5	10/27 10/28	9.0* 10.9*	,02	11.3 9.7	7.6 7.7	8.2 8.6	84 76	<4	24	0,15 0.13	0.02	0.76 0.68	0.006 0.006	0.42 0.35	0.42 _†	8	100 130	70 120	3 2	- 1	3.8	6.3	5.0	10	0.406	67	280 140	0.10 0.15	124 (134) 127 (140)
DC	Dovnstream Compositor	14.5	10/27-28				7.8	••		6	15	0.12	0.02	0.87	0.010	0.40	0.25	57	120	120	30	. 2 8	3.1					54			129 (140)
4	Future Outfall Site	14.1	10/27 10/28	9.0 10.9*	,06	10.8 9.8		9.4 9.1	85 80	4	12	0.24 0.25	0.02 0.02	0.44 0.53	0.003 0.004	0.34 0.36	0.34 0.35	. 9 8	100 120	96 88	10 10	8 6	3.8 3.8					58 54	100 86	0.10 0.10	122 (130) 131 (120)
5	Esteb Road Bridge	10.5	10/27 10/28	11.6	,52	11.1 9.5	9.0 8.0	12.4	113 98	7 4	2‡ 12	0.44 0.43	0.02 0.03	0.09	0.015 0.002	0.28 0.30	0.28 0.27	10 10	120 220	100 120	30 6	23 5	3.8 3.1	6.6	10	19	0.305	50 54	27 54	<0.10 <0.10	124 (130) 123 (115)

^{* =} Estimate
+ = Interference in sample; value may be inaccurate
() = Field measurement

// Percent D.O. saturation adjusted for elevation of 1,500 feet
// SAR: sodium absorption ratio

Table 2. Results of analyses on 24-hr. influent and effluent composite samples by both WDOE and Goldendale STP laboratories. Also, results of grab samples and effluent channel samples. All values mg/L unless otherwise noted.

			Chlorina			Monthly	F.C.C.3	01 7
Parameter	Influent/24-hr. WD0E	Composite Goldendale	Effluent/24-hr NDOE	. Composite Goldendale	Effluent Grab	Permit Limit	Effluent 10/27	10/28
Flow (cfs)			0.66	0.69	0.75	1.5		
BOD5 (1bs/day)	86 324	193 728	14 53	13 49		30 375		9
TSS (1bs/day)	53 200	132 498	18 58	20 75		30 375	5	5
Dissolved Oxygen		6.2, 6.0**	k		7.2, 7.8			
pH (S.U.)	7.3		7.6				7.2	7.2
Sp. Cond. (umhos/cm)	325		339		350		325	357
COD	240		100					49
Turbidity (NTU)	65		24				8	16
NH ₃ -N	13		12				8.4	10
NO ₂ -N	0.13		0.05				0.19	0.17
NO ₃ -N	0.13		1.4				0.49	0.48
Ortho-PO ₄ -P	3.6		4.7				3.6	4.0
Total PhosP	4.7		6.9				3.3	4.1
Total Solids	230		170				100	210
T. Non-vol. Solids	160		150				66	160
T. Non-vol. Susp. Sol.	21		2				3	3
Temperature (°C)		16.6**			15.1		12.5	13.8
T. Hardness as CaCO ₃	42		54				79	63
C1	15		23				22	23
F. Coli. (org/100 mL)					1*, 6*	200	84	44
T. Resid. Chlorine				1.5, 1.5	2.03, 1.5			

^{*}Estimated value based on non-ideal plate count.

**Field grab sample.

Table 3. Mean values from samples taken on the Little Klickitat River, October 27 and 28, 1981. All values are in mg/L unless otherwise noted. Mean value ± standard deviation (number of observations).

Station Number	Location	River Mile	Temperature (°C)	(s.U.)	Dissolved Oxygen % Saturation	NO ₃ -N	NO ₂ -N	NH ₃ -N
1	Pipeline Road	16.3	10.3 ± 0.7(3)	8.2 ± 0.1(2)	98.3 ± 2.7(3)	<0.01(2)	<0.01(2)	<0.02 ± 0.01(2)
1A	Bloodgood Creek	14.9	10.8 ± 1.4(2)	7.9(1)	94.9(1)	0.03(2)	0.01(2)	0.04 ± 0.01(2)
2*	Highway 142 Bridge	14.8	$10.4 \pm 0.7(3)$	7.97 ± 0.06(3)	94.2 ± 1.5(3)	0.02 ± 0.01(3)	<0.01(3)	<0.03 ± 0.02(3)
EFF	Effluent Channel	14.6	13.2 ± 0.9(2)	7.2(2)	25.1 ± 2.9(2)	0.48 ± 0.01(2)	0.18 ± 0.01(2)	9.2 ± 1.1(2)
3*	Below Outfall	14.5	10.2 ± 1.0(3)	7.7 ± 0.1(3)	78.7 ± 4.3(3)	0.13 ± 0.02(3)	0.02(3)	0.8 ± 0.1(3)
4	Future Outfall	14.1	10.1 ± 0.6(3)	7.65 ± 0.07(2)	81.5 ± 2.6(3)	0.24 ± 0.01(2)	0.02(2)	0.48 ± 0.06(2)
5	Esteb Road Bridge	10.5	10.3 ± 1.1(2)	8.5 ± 0.7(2)	104.9 ± 10.2(2)	0.46 ± 0.03(2)	0.02 ± 0.01(2)	0.09(2)

Station Number	Un-ionized NH ₃ -N	0-P0 ₄ -P	Total PhosP	Chlorides	Sp. Cond. (umhos/cm)	Total Solids	Total Susp. Solids	Discharge ^{1/} (cfs)
1	<0.001(2)	0.02 ± 0.01(2)	0.04 ± 0.01(2)	1.9 ± 0.6(2)	125 ± 7(2)	88 ± 1(2)	6 ± 6(2)	4.2(1)
1A	<0.001(1)	0.06 ± 0.02(2)	0.06 ± 0.02(2)	2.7 ± 0.6(2)	100(2)	120(1)	5(1)	≈5(1)
2*	<0.001(3)	0.07 ± 0.05(3)	0.06 ± 0.03(3)+	1.9 ± 0.6(2)	107 ± 4(3)	120 ± 60(3)	20 ± 30(3)	8.8 ± 2.1(2)
EFF	0.04 0.01(2)	3.8 ± 0.3(2)	3.7 ± 0.6(2)+	22 ±1(2)	352 ± 4(2)	160 ±80(2)	5(2)	0.69 ± 0.02(3)
3*	0.007 ± 0.002(3)	0.39 ± 0.04(3)	0.33 ± 0.09(3)+	3.4 ± 0.5(2)	138 ± 3(3)	120 ± 20(3)	12 ±16(3)	10.0 ±1.3
4	0.004 ± 0.001(2)	0.55 ± 0.01(2)	0.34 ± 0.01(2)+	3.8(2)	130 ±10(3)	110 ± 10(2)	10(2)	10.0 ±1.3(1)
5	0.008 ± 0.009(2)	0.29 ± 0.01(2)	0.28 ± 0.01(2)+	3.4 ± 0.5(2)	122 ±11(2)	170 ± 70(2)	18 ± 17(2)	10.6 ± 1.4(1)

^{*}Includes results from compositor samples **Field measurement +Results with interference problems included

 $[\]frac{1}{\text{Most}}$ discharge values estimated for two days from one measurement at each station

Some nitrification was occurring in the plant during the time of the survey. The large nitrate concentration in the effluent is indicative of this. The condition may be only intermittent, and is not harmful to treatment processes. In fact, the conversion of ammonia to nitrate in the plant helps reduce the oxygen demand in the receiving water.

Further nitrification of the effluent, along with other biochemical processes, was evident from effluent ditch (EFF) data. The dissolved oxygen concentrations were sharply reduced. BOD, NH3-N, and TSS were also at lower concentrations, indicative of an oxygen-consuming process. The ditch also contained a verdant growth of grasses and attached algae. Effluent nutrients, especially nitrate and phosphorus, probably contributed to this growth.

The low coliform count and the high total residual chlorine (TRC) concentration in the effluent at the chlorine contact chamber were also altered in the ditch. The regrowth of fecal coliform populations is enhanced by temperatures above 13°C, excessive nutrients, and a BOD above 30 mg/L (Geldreich, 1978). All above conditions but BOD were true at the ditch station (EFF). The TRC concentration, which is a measure of free and combined available forms of chlorine for disinfection, was reduced. This is not uncommon since chlorine is highly volatile and also easily binds and reacts with organic debris.

Dissolved Oxygen

Dissolved oxygen (D.O.) concentrations in the river were within the Class A criterion of 8 mg/L. Cold water temperatures, good channel configuration, and swift water velocities were among the factors reducing the impact of the low-D.O. effluent. Conversion of the D.O. concentrations to percent saturation levels better illustrates the impact of the effluent on the river (Figure 3). Water above the STP at nearly 100 percent D.O. saturation is reduced to 80 percent saturation after addition of the effluent, but is again 100 percent saturated four miles downstream of the outfall. Mean D.O. saturation levels (Table 3) were significantly different at Stations 3 and 4 compared to those at Stations 1, 2, and 5, using a Newman-Keuls test and ANOVA at α = 0.05 (Zar, 1979). There was no significant difference between saturation levels at stations within the two groups above.

The D.O. depletion will be further discussed in later sections.

Nutrients

The heavy contribution of nutrients from the STP to the river is quite evident from the data collected (Figure 4). Both phorphorus and total

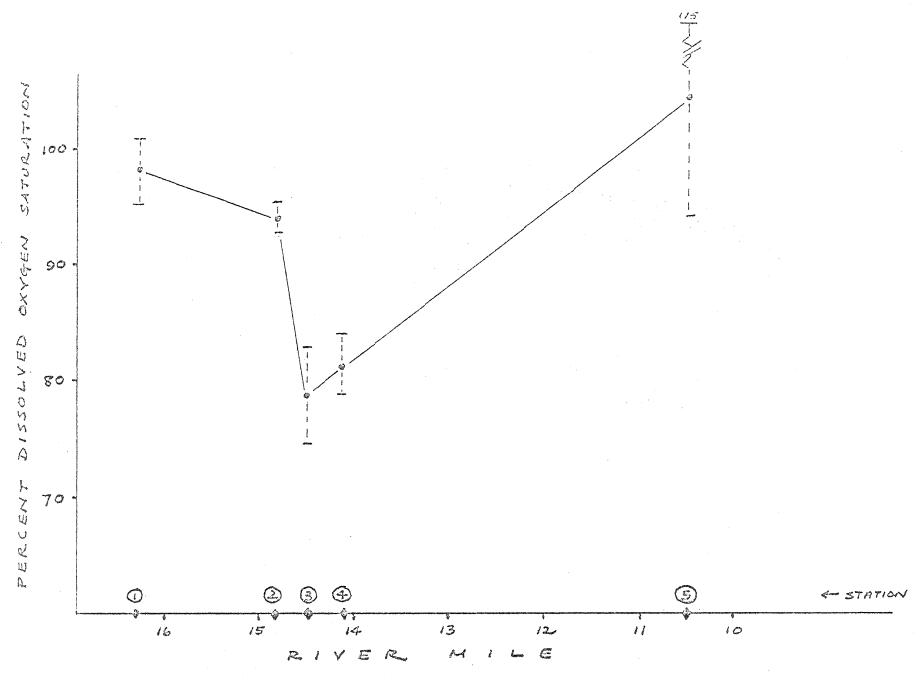


Figure 3. Dissolved oxygen saturation levels (adjusted for 1,500' altitude) at water quality stations above and below the Goldendale STP (R.M. 14.6), Little Klickitat River, October 27-28, 1981.

Mean and standard deviation are shown.

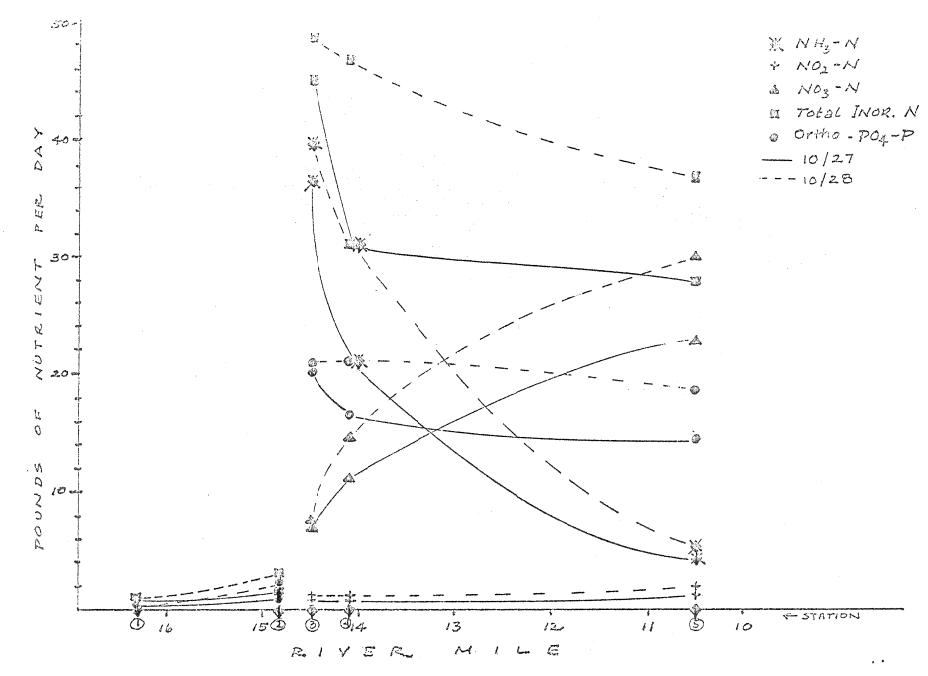


Figure 4. Nutrient loads at water quality stations above and below the Goldendale STP (R.M. 14.6), Little Klickitat River, October 27-28, 1981.

nitrogen were elevated an order of magnitude by the addition of the STP effluent. These elevated concentrations were only slightly diminished by R.M. 10.5.

Field observations revealed a heavy growth of organisms attached to the channel substrate. Although this community was not tested for the presence of nitrifying bacteria, their presence was quite apparent from the rapid nitrogen species conversions between Stations 3 and 5 (Figure 4). The aerobic conditions, slightly alkaline pH, and stable rock habitat are ideal conditions for a nitrifying community. Reduced competition for nutrients from macrophytes and algal communities during this season may have made the nitrifying community more productive than at other times of the year. The effects of this nitrification in the form of nitrogen oxygen demand (NOD) will be further discussed later.

Some photosynthetic activity may have been present. The slight decrease in orthophosphorus and total inorganic nitrogen concentrations between Stations 3 and 5, coupled with the D.O. supersaturation at Station 5, are indicative of this.

Un-ionized ammonia concentrations downstream of the STP did not exceed the 0.016 mg/L NH3-N criterion for protection of aquatic life (USEPA, 1976). One sample taken at Station 5 approached the criterion with a concentration of 0.015 mg/L un-ionized ammonia. At the observed pH of 8.7 to 9.0 and with ammonia concentrations similar to those detected at Station 5, the criterion could be violated with a rise in temperature of less than 5 degrees over temperatures recorded during this survey. The criterion has a safety factor of ten built into it; most toxic effects to fish occur at concentrations around 0.16 mg/L un-ionized NH3-N (USEPA, 1976). Fish bioassays were performed to test for such acute toxicity. No fish died (see Fish Bioassay section).

BOD, NOD, and Oxygen Depletion

As described previously, the river experienced some oxygen depletion for at least 0.5 mile downstream of the outfall location. At approximately 4 miles downstream, D.O. concentrations had recovered. The phenomena primarily responsible for this were biochemical oxygen demand (BOD) and nitrogenous oxygen demand (NOD).

A simple water quality computer model can be used to illustrate the oxygen depletion process, and simulate the reaction of river water quality when variables are manipulated. The model incorporates oxygen demand from three primary sources: carbonaceous oxygen demand (CBOD), nitrogeneous oxygen demand (NOD) both from the effluent, and sediment oxygen demand (SOD) from previously discharged and settled effluent solids.

The basic equation used in the model is:

$$\begin{aligned} & \text{Dt} &= \text{Do } \exp(-k_2 t) \, + \frac{\text{Lok}_1}{k_2 - k_1} \left[\exp(-k_1 t) \, - \exp(-k_2 t) \right] \, + \, \frac{\text{Nok}_n}{k_2 - k_n} \\ & \left[\exp(-k_n t) - \exp(-k_2 t) \right] \, + \, \frac{\text{Sb}}{k_2} \left[1 - \exp(-k_2 t) \right] \end{aligned}$$

where Dt = oxygen deficit at time (t)

 $k_1 = CBOD rate$

Lo = ultimate, initial BOD concentration at t = 0

k2 = reaeration rate
t = time in days

Do = initial oxygen deficit

 $k_n = NOD rate$

Sb = benthic oxygen demand

As mentioned previously, the system downstream of the outfall was favorable for a nitrifying benthic community. Based on data in Table 1, the rate of nitrate + nitrite production between downstream stations was 1.5 to 9.1/day (log e). Ammonia reduction rates were from 3.7 to 9.4/day (log e). Where NH3 rates exceeded NO2 + NO3 rates, the difference may indicate NH3 taken as a nutrient source for autotrophs (Hammer and McKichan, 1981), or precipitating out on clay or detrital particles (Manahan, 1975). The nitrate + nitrite production rates were taken as NOD rates.

Other factors in the model were calculated or hypothesized from field data or literature. No long-term BOD analysis was performed so that BOD rates and the ultimate CBOD concentrations were estimated (Zison, et αl ., 1977). Channel depths and widths and water velocities were measured and used to determine reaeration rate and time of travel. Benthal sediment depth was estimated from field observations.

The model calculates D.O. depletion on a reach-by-reach basis. For simplicity, the river was separated into four reaches below the outfall based primarily on hydrologic characteristics present during the survey: R.M. 14.6 - 14.3; 14.3 - 14.2; 14.2 - 12.0; and 12.0 - 10.5. The boundary between the third and fourth reach is estimated, although its actual placement is important to D.O. recovery rates. This will be seen in the simulations below. A more thorough investigation could have been conducted to account for each specific area of photosynthetic activity and in-stream reaeration, had there been more time. The completed program listing for the model can be found in Appendix I.

The model output based on mean field results from this survey are presented in Figure 5 and Appendix II. As can be seen, the model simulation compares fairly well to actual field data. Two points where the

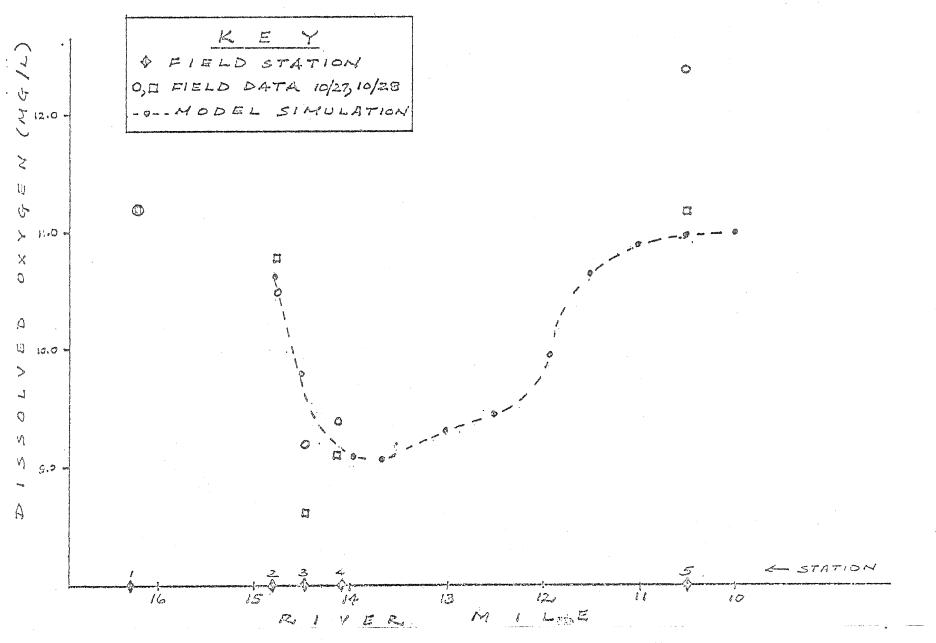


Figure 5. Computer-generated model simulation of dissolved oxygen concentrations in the Little Klickitat River below the Goldendale STP (R.M. 14.6) under conditions found on October 27-28, 1981. Field D.O. concentrations taken during that period are also shown. In-stream discharge:effluent yolume = 14:7.

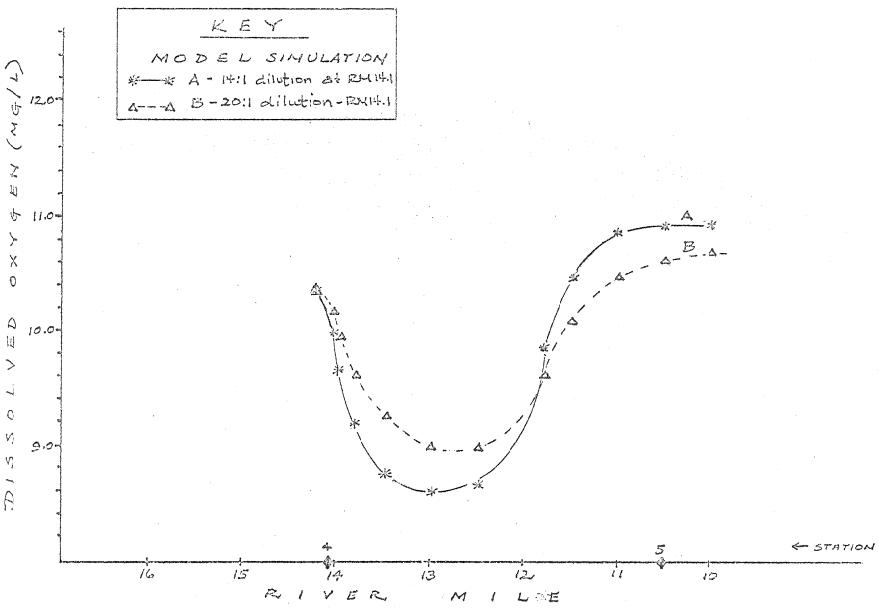
simulation doesn't fit are Stations 3 (R.M. 14.5) and 6 (R.M. 10.5). Station 3 D.O. levels may be lower than those of the model possibly because of an immediate oxygen demand from sediment, benthic organism respiration, or some other oxygen-demanding reaction. Station 6 D.O. levels probably are influenced by either a higher reaeration rate or photosynthetic oxygen production as was hypothesized earlier.

Simulations were also run with effluent entering the river at the site of the proposed outfall at R.M. 14.1 (Figure 6). The model was run including both effluent BOD and NH3 concentrations and a discharge volume equal to those found during this survey. Two river discharge volumes were simulated with river-to-effluent dilution ratios of 20:1 and 14:1. The former is the proposed permit minimum dilution ratio; the latter approximates conditions found during this survey. The latter simulation was run to check on the effects of outfall placement alone. Printouts of all simulations are found in Appendix II.

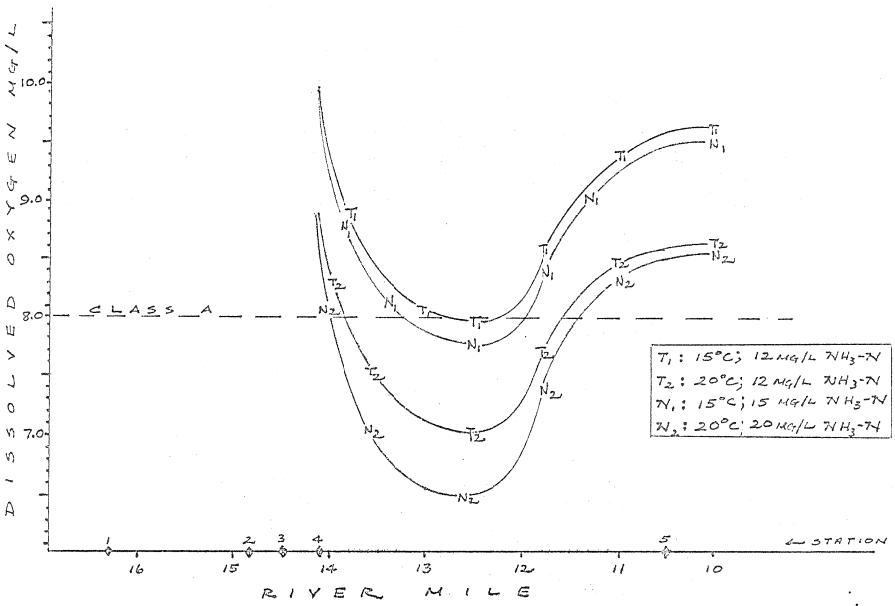
These two simulations show that perhaps a 1.0 to 1.5 mg/L D.O. depletion may occur below the new outfall with dilution ratios of 14:1 and 20:1, and effluent strengths similar to those found during the receiving water survey. The 14:1 simulation shows a similar amount of D.O. depletion as was found in the previous simulation in Figure 5. Figure 6 shows that the shallower water and higher velocity below R.M. 12.0 will play a major role in overcoming effluent oxygen demand in the deeper, slower water from R.M. 14.1 to 12.0. Also, this slower water (less than 0.6 ft/sec) will let sludge accumulate until perhaps a 25:1 dilution ratio. The sludge will exert an oxygen demand until velocities are high chough to achieve scour during winter freshets. Slower or deeper water also has lower reaeration rates. The impact of deeper water during periods of greater discharge can be seen in Figure 6, where the return to normal D.O. levels below R.M. 12.0 is slower at a 20:1 dilution ratio than at 14:1. Under these effluent conditions, a D.O. sag below 8.0 mg/L (Class A criterion) is not expected at a 20:1 dilution ratio unless benthal growth respiration and an immediate oxygen demand greatly influence the area below the new outfall.

Since the quality of effluent from the new lagoon system is unknown and since the impact of minimum 20:1 dilution ratio on water quality under possible adverse in-stream temperatures is unknown, simulations were run with modified effluent ammonia concentrations and in-stream temperatures (Figure 7).

The simulations suggest that the Class A D.O. criterion may be violated downstream of the new outfall during a 20:1 dilution ratio under some of these "worst-case" conditions. In-stream temperatures greater than 15° C with ammonia concentrations similar to those found during the receiving water survey could cause such a deficit (Curve T1, T2 - Figure 7). Higher ammonia concentrations would tend to drive oxygen sag deeper and affect a longer stretch of river (Curve N1, N2 - Figure 7). These



Simulation of dissolved oxygen concentrations in the Little Klickitat River below the proposed Goldendale treatment system outfall with effluent and in-stream temperatures, NH3 and BOD5 concentrations, and effluent volumes similar to October 27-28, 1981.



simulations assume the presence of nitrifying bacteria and the presence of some sludge -- conditions which may not be present under a minimum 20:1 dilution ratio or seasonal removal permit stipulation. They also do not take into account photosynthetic oxygen production or algal respiration which would further drive D.O. concentrations in erratic patterns.

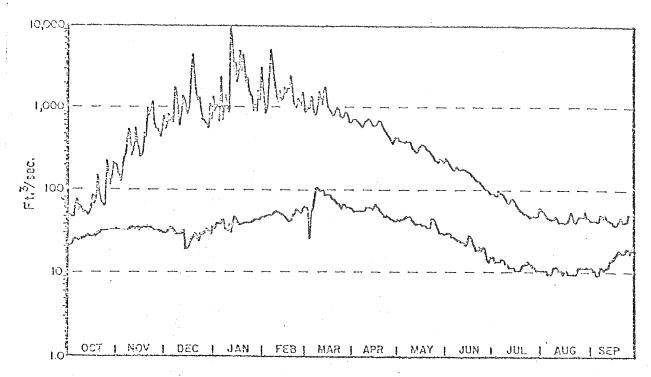
Discharge

The new treatment system for Goldendale will only have seasonal discharge into the Little Klickitat River. The permit will be written either to eliminate the effluent in certain months (e.g., July-October), or to eliminate the effluent during periods of a 20:1 or less dilution ratio (Newman, 1982). Effluent will be stored in the treatment lagoons when permit requirements for season or dilution are not met. Treated effluent will be seasonally applied to fields as irrigation water. A review of historical discharge data suggests that, of the two proposed options, the 20:1 dilution requirement may be more restrictive.

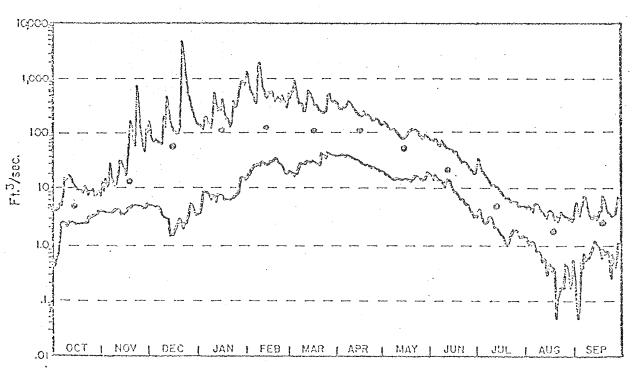
Historical discharge data are available from two stations on the Little Klickitat operated by the U.S. Geological Survey (USGS). One station was located at R.M. 18.0 near Goldendale, Gage #14-1120. The other was located at R.M. 0.3 near Wahkiacus, Gage #14-1125. Maximum and minimum daily flows for both stations are presented in Figure 8, from a previous water resource analysis (Brown, 1979). Mean monthly flows were also calculated from USGS records and are included for the station near Goldendale in Figure 8.

As can be seen in this survey, the contribution of Bloodgood Creck is very important to the discharge volumes above the outfall (Table 1). Although the discharge from Bloodgood Creek has only been measured once previously (7.2 cfs in February, 1911), some flow characteristics may be gathered from the gaging record of other nearby spring-fed creeks. The spring-fed creeks tend to have a nearly constant minimum flow. The combined influence of these can be seen in the flatness of the record of daily minimum flows at the Wahkiakus station on the Little Klickitat (Figure 8). Since the discharge of 4.2 cfs recorded at Station 1 during this survey was near the mean monthly value of 4.8 cfs for October, the 5 - 6 cfs flow from Bloodgood Creek may be conservatively taken as a near minimum flow. This may be presumed in light of the discharge records from Spring Creek where flows have never dropped below 11 cfs and are about 15 cfs year-round (USGS, 1958-1970; Brown, 1979).

Even with the additional water from Bloodgood Creek of 5 cfs, a total of 10 cfs from the Little Klickitat above the confluence of Bloodgood sometimes is not available as early as June or as late as January (Figure 8). In addition, the 183-day, 2-year calculated low flow for the gaging station near Goldendale is 7.35 cfs or 18:1 at current effluent volumes (Figure 9). It is apparent that, at existing flows, the new



Maximum-minimum discharge, 1950-1975, Little Klickitat River near Wahkaicus, Washington.



Maximum-minimum discharge and mean monthly discharge, 1958-1970, Eittle Klickitat River near Goldendale, Washington.

Figure \leq From Brown, 1979.

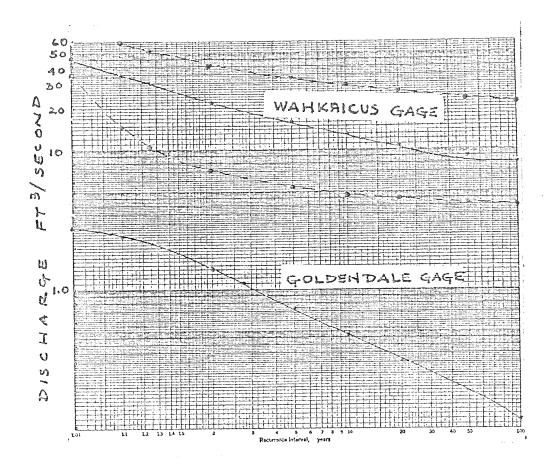


Figure 4. Low-flow recurrence intervals at two USGS gaging stations along the Little Klickitat River. Data for 183 day (- - -), and 7 day (—), from Brown, 1979.

Goldendale treatment system will not be allowed to discharge to the Little Klickitat for extended periods if the permit restricts discharge at dilution ratios of 20:1 or less. Substantial storage capacity and treatment capabilities would be necessary.

Solids

Stream channelization work was being performed by the City of Goldendale between R.M. 15.3 and 16.3 on the 26th and 27th of October. Heavy silt loading at Station 2 was observed during this time. High non-volatile solids and turbidity in both 24-hour composite samples reflect the influence of this silt loading (Table 1). Some settling occurred in the slow water under the highway bridge (R.M. 14.8), and in the marshy area above the STP outfall.

The Class A criterion for turbidity of 5 NTU greater than background was clearly violated as indicated by the composite samples and possibly even grab samples collected at Stations 4 and 5. Background would be about 3 NTU, calculated from Station 1 and 1A values.

The disruption in solids data from the channelization work makes evaluation of the effluent solids data impossible.

Chlorine, Coliform, and SAR

As discussed previously, TRC and coliform levels were altered between chlorine contact chamber and effluent ditch stations. Results of instream coliform and TRC are also presented in Table 1.

Chlorine concentrations continued to be found at detectable levels (greater than 0.1 mg/L) 0.5 mile downstream of the outfall. Concentrations were greater than 0.003 to 0.005 mg/L TRC level recommended by many researchers (DeGraeve, $et\ al.$, 1979).

Continuous exposure of trout (Salvelinus fontinalis, S. namaycush, Salmo gairdneri, and Salmo clarkii) to TRC concentrations of 0.06 to 0.20 mg/L at temperatures of 10 to 14°C have been found to be lethal (Seagert and Bogardus, 1979). The authors noted that it is important to know whether the Cl2 is in the form of monochloramine and dichloramine when determining toxicity. In ammonia-laden water with an alkaline pH, less toxic monochloramine will predominate (Thomas, Bartos, and Brooks, 1979). The non-lethal TRC concentrations for rainbow trout (S. gairdneri) under such conditions are as high as 0.7 to 0.94 mg/L.

Other chemicals such as chlorophenols, chloroform, and dichloromethane can be formed with high chlorine concentrations in wastewater (Jolley, $et\ al.$, 1979). The effects of these chemicals on the aquatic environment are only now becoming known.

The rejuvenation of the coliform population discussed earlier continued to Station 4 and had declined by Station 5. The mean coliform level at Station 4 did not exceed the Class A criterion.

Station 2, above the outfall and below the stream channelization work, had mean coliform levels greater than the 100 organisms/100 mL specified in the Class A criterion. The source of these elevated levels could be Bloodgood Creek or coliform-laden sediments from the channelization work. It is uncertain if these concentrations of coliforms were carried to the outfall area where regrowth was in progress. Coliform often settle with suspended solids and the slow-velocity water in the marshy area above the outfall may have had that effect.

The sodium absorption ratio (SAR) measurements were made to evaluate the salinity hazard of the water for irrigation use. All values were Class Cl and Sl, salinity and alkali hazards, respectively. These classes both indicate low hazards (Hem, 1970).

Fish Bioassay

No fish died or were observed to be stressed after the 42- to 45-hour in-stream bioassay in spite of the existence of potentially toxic concentrations of TRC, un-ionized NH3, and total solids. If the time of exposure were lengthened to 96 hours or more, or if detailed gill tissue analyses had been performed, perhaps some differences would have been seen. For example, six weeks' exposure of fingerling chinook salmon, Oncorhynchus tshawytscha, to un-ionized ammonia concentrations as low as 0.002~mg/L has been reported to have caused gill hyperplasia (Willingham, et al., 1979). Values detected in samples from Stations 3, 4, and 5 exceeded this concentration.

CONCLUSIONS

Data from the receiving water survey conducted on the Little Klickitat River indicate that the current Goldendale STP effluent has a potentially serious local impact on water quality. However, natural assimilation processes in the river minimized these impacts under the conditions found during the survey. Impacts such as oxygen depletion, un-ionized ammonia and chlorine toxicity, and nutrient enrichment may affect a large downstream area during summer low-flow conditions.

Current effluent loads were not found to be highly toxic (<42 hrs.) to fingerling rainbow trout. Further testing would be necessary to evaluate if there were effluent elements exhibiting acute toxicity within the standard 96-hour duration, or chronic toxicity to aquatic organisms.

The model simulation of Little Klickitat D.O. concentrations below the proposed outfall reveals that low-velocity water at a 20:1 river-to-effluent dilution ratio will result in a D.O. sag of 1.5 mg/L if NH3-N and BOD concentrations are similar to what is currently discharged. The river is an ideal environment for autotrophic and nitrifying benthal communities and their existence greatly influences local D.O. concentrations. Ammonia concentrations in effluent should be minimized to reduce the NOD loading. Temperatures greater than 15°C may result in Class A D.O. criterion violations. Evaluated effluent ammonia concentrations will exacerbate the situation.

Low-flow records indicate that if a 20:1 permitted dilution ratio is necessary for effluent discharging to the river, there will usually be no discharging in the months of August through October. In certain years, this may extend from June through January.

Stream channelization work conducted during the study increased turbidity and possibly coliform bacteria to levels greater than the Class A criterion. Evaluation of effluent solids on river water quality was not possible because of this situation. D.O. and nutrient concentrations may have also been affected by the work being done.

JJ:cp

Attachments

cc: Bob Neal, WDG Goldendale Hatchery Files

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APPENDIX I

LITTLE KLICKITAT/GOLDENDALE D.O. MODEL PROGRAM

```
OO10 DEFFN'14"SELECTLISTOOS(80)";HEX(OD)
OORO DEFEN:15"SELECTLISTR15(182)";HEX(OD)
OOBO DEFEN'BO "Gs="; HEX(22); "KLICK"; HEX(223A); "GCRATCH F Gs"; HEX(OD)
0040 DEFFN'31 "GAVE DC F$(Q4)Q$" "HEX(OD)
OOSO REM %
THE RESIDENCE OF THE PROPERTY 
0050 REM THIS MODELS THE DO CONCENTRATION IN THE LITTLE KLICKITAT AT GOLDENDALE
0070 REM PROGRAMMER JOE JOY
0080 REM JULY, 1982
0030 REM SOURCES; HAMMER AND MACKICHAN, 1980 AND YAKE, 1981; SINGLETON, 1981
0100 P1=760
Olio REM X
0120 INPUT "TOTAL NUMBER OF REACHES TO MODEL", A
A DT 1=OA 907 OE10
OISO INPUT "ARE THERE POINT SOURCES IN THIS REACH (1=Y, 2=N)", Y1
0160 IF Y1=2 THEN 230
0170 INPUT "UP FLOW, PT. SOURCE FLOW (CFS)",F1,F2
0180 INPUT "TEMP UP, PT, SOURCE TEMP", T1, T2
0190 INPUT "D.O. UP, PT. GOURCE D.O.", D1, D2
0200 INPUT "NHE-N UP, PT. SOURCE NHE-N", N1, N2
OBIO INPUT "IS BOD FIVE DAY (1) OR LLTIMATE (2)", C
0220 INPUT "ENTER BOD UP, PT. SOURCE BOD", C1, C2
       : GOTO 270
OBBO INPUT "ENTER TOP OF REACH FLOW(CFS), D.O., TEMP., & NHB", F.CO,TO,NO
0240 NO=NO#4, 33
DESO INPUT "IS BOD FIVE DAY (1) OR LLTIMATE (2)",C
OBSO INPUT "ENTER BOD", CI
ORTO INPUT "DO YOU WISH TO CALCULATE SED. GXY.DEMAND.1=Y.Z=N",Y
0280 IF Y=2 THEN BOO
0290 INPUT "DEPTH OF SEDIMENT?" G
OBGO INPUT "DOES THE BOD RATE NEED TEMP. ADJUSTMENT (Y=1,N=2)", YE
0310 INPUT "ENTER BOD & NOD RATES", KI, K3
OBGO INPUT "HAVE YOU A REAERATION RATE (Y=1,N=2)",YB
0330 IF Y3=2 THEN 345
0340 INPUT "ENTER REAERATION RATE @20 DEG. C", K2
OB45 INPUT "HAVE YOU A NOW-NOD AMMONIA DEPLETION RATE(Y=1,N=2)",Y5
0346 IF Y5=2 THEN 350
0347 INPUT "ENTER NHS DEPLETION RATE", RS
0350 INPUT "DO YOU HAVE GINGLE VALUES (1), OR PARTIAL VALLES (2) FOR DEPTH, WIDTH, AND VELOCITY", E
0360 IF E=1 THEN 400
```

```
0350 INPLT "DO YOU HAVE SINGLE VALUES (3), OR PARTIAL WALLES (2) FOR DEPTH, WIDTH, AND VELOCITY", E
MAKE THEN 400
0370 INPUT "EMTER FARTIAL MEAN DEFTHS AND WIDTHS", Z1, Z2, X2, X3
0380 INPUT "ENTER PARTIAL DISCHARGES" F3. F4
OBOO INPUT "ENTER PARTIAL VELOCITIES", V1, V2
0400 IF E=2 THEM 430
0410 INPUT "ENTER MEAN DEPTH AND WIDTH", Z5, X5
0420 INPUT "ENTER VELOCITY", V5
0430 INPUT "ENTER RIVER MILE AT TOP OF REACH", 19
0440 INPUT "ENTER CALCULATION INTERVAL (MILES)", 15
0450 INPUT "ENTER LENGTH OF REACH IN MILES", 16
0460 IF AO>1 THEN 470
      " PRINT HEX (OC)
0470 PRINT HEX (CAOA)
      · 原田四 发
是你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们不要你,我们也不要你,我们
0480 GOSUB 610
0490 05=8
      ; IF D5/2>5 THEN D6=D5/2
 0500 G05U8 910
OSIO FOR TS =0 TO IG STEP IS
 0520 GOSUB 1280
 OSBO IF X=1 THEM 550
0540 PRINT "RIVER MILE", "DAYS", "BOD", "NOO", "D.O.", "DEFICIT"
 (1555) X=1
 0560 PRINT ROUND(R,3),ROUND(T,3),ROUND(L9,2),ROUND(N9,2),ROUND(D,2),ROUND(D9,2)
 0570 NEXT TS
 OCTO PRINT
        (S, EE, AVEN) CHALOR: "HEHM" THIRD :
 0580 X, F, F1, F2, T1, T2, D), D2, N1, N2, C, C1, C2, N0, G, K2, Z1, Z2, Z5, X2, X3, X5, F3, F4, V1, V2, R2=0
 0590 NEXT AO
 0600 END
 OCIO REM X
               RESIDENT RESIDENCE STRUCK STRU
 0620 IF Y1=2 THEN 570
 0630 R1=F1/F2
 0640 CO=(F1*D1*F2*D2)/(F1*F2)
 0650 TO=(F1*T1+F2*T2)/(F1+F2)
  0660 MO=((Fi*Mi*F2*M2)/(Fi*F2))*4.33
  0670 IF Y=2 THEN 720
  0680 IF E=1 THEN 710
  0690 23=((21+22)/2)*,3048
  0700 SP={(TOM, 15)+("B*G))/ZB
         # 25 203 of cycle of control of
```

```
(4656) [Yika(新生物]](47) #72/数(2) [新生物](47)
  CONTRACTOR (CERTARD -FEDERAL) / (FifeE) ) *4, TE
  0670 IF Y=2 THEN 720
  0680 IF E=1 THEN 710
 QGGO ZB=(|Z1+Z2)/2>*,3048
 0700 SR=((TO+,15)+(,3%G))/ZR
           : 60TO 720
0710 SB=((TO*,15)+(,B*G))/(Z5*,B048)
OZZO REM PERPERSE D.O. Z SAT PREPERSE
0730 P={Pi-4,B7922#EXP(,06378#T0))/{760-4,87922#EXP(,06379#T0))
0740 DS=(14.5214-.4026*T0+6.8516E-03*T072*E.2619E-04*T073-2.4998E-05*T0748.5254E-07*T075-1.0513E-08*T0
0750 IF Y2=2 THEN 770
0760 M1=K1*1,047*(T0-20)
0770 IF YB=1 THEN 810
0780 IF E=2 THEN 800
0790 K2=(21,6%V5*,67/25*1,85)
            # GOTO 810
OBOO BEH((FB/(FB/FF4))*(E1,6*V1*,67/Z1*1,85))*((Fa/(Fa/(FB*F4))*(E1,6*VE*,67/ZE*1,85))
0810 R2=R2*1.022*(T0-20)
OSZO IF C=2 THEN 950
0830 B1=C1/(1-EXP(-54K1))
       : IF Y1=2 THEN 880
0840 BB=08/(1-6%P(-5%K1))
0950 GOTO 870
0860 B1=01
           : IF Y1=2 THEN 880
            -: B2=02
0870 L0=(F1481+F2482)/(F1+F2)
            -: GOTO 890
0880 L0=B1
0990 D0=D5-00
0900 RETURN
4910 BEM X
                                           to the later than the state of 
OBBO IF AONI THEN 950
OBEO PRINT HEX (OE); TAB(4); "LITTLE KLICKITAT D.O. MODEL"
 0940 PRINT HEX(OE); TAB(R); "INSTREAM CLASS A, D.D. CRITERION & MG/L"
 0950 PRINT
           : IF Y1=2 THEN 1130
OBSO FRINT "NAMES AND ASSESSMENT AND THE TRANSPORT OF A SECTION OF A S
 0970 IF E=1THEN 1030
0980 PRINT "LESTREAM FLOW, PT. SOUNCE FLOW (CFS)" F1; F2, "PARTIAL AND THE . .
                                                                                                                                                                                             "#TI#TE, "PARTIAL DISCHE
OSSO PRINT "TERP UP, PT, SOURCE TEMP
```

```
OSAO FRINT FERKOE); TAROS; "INSTREAM CLASS A. D. D. CRITERION & MRZL"
DOBO PRINT
 : IF Y1=2 THEN 1130
·OOCO PRINT "当在前来的最终并不是要的的对象的是是是对象的。 INPUT ECHO 经数据设计的的对象的数据表现的的对象的对象的数据
0970 IF E=1THEN 1020
OSSO PRINT "LPSTREAM FLOW,PT.SOLRCE FLOW (CFS)";F1;F2,"PARTIAL DEPTHS AND WIDTHS";71;72;72;73
0980 PRINT "TEMP LP, PT. SOURCE TEMP "; T1; T2, "PARTIAL DISCHARGES"; F3; F4
                                            ";D1;De,"PARTIAL VELOCITIES ";V1;VE
1000 PRINT "D.O. LP.PT.SOURCE D.O.
1010 GOTG 1050
loro print "Upstream Flow.ft.scurce Flow (CFS)"(F1(F2," DEFTH AND WIDTH - "(Z5(X5
1030 PRINT "TEMP UP, PT. SOURCE TEMP
                                          ";T1;T2
                                          ";D1;D2,"VELOCITY";V5
1040 PRINT "D.O. LF,PT.SOLRCE D.O.
1050 PRINT "NH3-N UP, PT, SOORCE NH3-N
                                          1060 IF C=2 THEN 1090
1070 PRINT "FIVE DAY BOD UP, PT.SCURCE BOD
                                            "#C1#C2
1080 GSTS 1110
1030 PRINT "ULTIMATE BOD UP, PT.SOURCE UBCO
                                          " " C1 " C2
1100 81=01
  : B2=C2
   : GOTO 1110
1120 PRINT HEX(OAOA)
   : GOTO 1140
1130 IF E=2 THEN 1135
   : PRINT "DOWNSTREAM FLOW (CFB) " "F" "DEPTH & WIDTH"; Z5; X5; "VELOCITY"; V5
   : GOTO 1150
1135 PRINT "PARTIAL DEPTHS & WIDTHS"; Z1; Z2; X2; X2; X3; "PARTIAL DISCHARGES & VELOCITIES"; F3; F4; V1; V2
   : 6070 1160
1140 PRINT "DOWNSTREAM FLOW (CFS)
                                  " : F14F2
                                 ";ROLNO(R1,2)
1150 PRINT "DILLTION BATIO
                                 ":RCLND(LO,2)
1160 PRINT "MIXED ULT, BOD (MG/L)
                                " # ROLNO (NO # Z)
1170 PRINT "MIXED ULT, NOD (MG/L)
                                  "; MOLND(TO, 2)
1180 PRINT "MIXED TEMPERATURE (C)
                                  "#ROUND(CO, B)
1130 PRINT "MIXED D.O. (MG/L)
1200 PRINT "D.O. 100% SAT =
                                  ";ROUND(D5,2)
                                  1210 PRINT "K1=
                                  ";ROUND(KE,2)
1220 PRINT "K2=
                                  " "ROUND(KB, 2)
1230 PRINT "K3=
1240 PRINT "BEDIMENT DEPTH IN INCHES"; $
   : PRINT "NON-NOO AMMONIA DEPLETION RATE" ; R2
1250 PRINT
  " PRINT "
                                   27 # 1 1 (S
               R E A C H
1260 PRINT HEX(OAOA)
1270 RETURN
```

```
; PRINT "
                                                                  FREACH "#AO
 1260 PRINT HEX (OAOA)
 1270 RETURN
1.220 自EM 3
 THE THE TAXABLE PARTY OF THE PA
 1290 R=19-T5
 1300 IF E=1 THEW 1920
 1310 T=(({TS#5/380*X2*Z1)/{FB}+(T5*5/280*X3*Z2)/{F4})//(FB+F4))/86400
         # GOTO 1350
1320 IF Y1=2 THEN 1340
1930 T=((T5*5280*X5*Z5)/(F1*F2))/85400
         # GOTO 1350
1340 T= ((T5*5280*X5*Z5)/F)/86400
1350 IF T5=0 THEN T=,00001
1340 DB=((K1aF0)\(K5-R1))a(EXb(-K1a1)-EXb(-R5a1))+((K3aM0)\(K5-R3))a(EXb(-R3a1)-Exb(-R5a1))+(D0aEMb(-R2
1380 L9=LOMEXP(-K1#T)
        - # - M9=NO#EXP(-K3#T)
1385 IF YS=1 THEN NO=NO*((EXP(-KB*T)+EXP(-RB*T))-1)
 1390 D=D5-D9
 1400 RETURN
   0230 - 0160
   0270 - 0220
   0880 - 0880
    OEEO - OEEO
    0350 - 0346
    0400 - 0360
    0430 - 0400
    0470 - 0460
    0560 - 0530
    0630 - 0490
    0670 - 0520
    0710 - 0590
    0720 - 0570 0700
    0770 - 0750
    0200 - 0720
    0810 - 0770 0790
    0860 - 0880
    0220 - 0250
    0880 - 0880 0860
    0890 - 0870
    0910 - 0500
    0950 - 0920
    1020 - 0970
```

```
0970 - 0500
OSSO
     - 0920
1020 - 0970
0:01 - 0:00
1090 - 1060
1110 - 1080 1100
1130 - 0950
1185 - 1180
1140 - 1180
1160 - 1130 1135
1280 - 0520
1320
     - 1300
1340
     - 1920
(NEEL L
      - 1910 1990
      - 0120 0130
\Delta y_{ij}^{(1)}
      - 0130 0460 0590 0920 1250
      - 0830 0860 0870 0880 1100
81
88
      - 0840 0880 0870 1100
      - 0210 0250 0530 0820 1060
(C)
      - 0230 0640 0830 1130
C1
      - 0220 0250 0530 0330 0860 1070 1090 1100
C2
      - 0220 0580 0840 0860 1070 1090 1100
      - 0560 1390
\Box
\{[[\lambda]]\}
      - 0890 1370
D3
      - 0190 0580 0540 1000 1040
\sum_{i=1}^{n}
      - 0190 0580 0640 1000 1040
()E
      - 0490 0740 0890 1200 1390
06
      - 0490
DB
      - 0560 1370 1390
{-::
      - 0350 0360 0400 0680 0780 0970 1130 1300
      - 0230 0580 1130 1340
F- 1
      - 0170 0580 0680 0640 0650 0660 0870 0980 1080 1140 1330
}==;=?
      - 0170 0580 0630 0640 0650 0660 0870 0980 1020 1140 1330
      - 0380 0880 0800 0990 1135 1310
1-4
      - 0380 0580 0800 0990 1135 1310
G
      - 0290 0580 0700 0710 1240
15
      - 0440 0510
145
      - 0450 0510
19
      - 0430 1230
      - 0310 0760 0830 0840 1810 1870 1880
161
142
      - 0340 0580 0790 0800 0810 1820 1970
      - 0310 1230 1370 1380 1395
КЭ
      - 0870 0880 1160 1370 1380
LO
1.49
       - 0560 1380
      - 0230 0240 0580 0660 1170 1370 1380 1385
140
1911
      - 0200 0580 0560 1050
1.174
       - 0200 0580 0660 1050
143
      - 0569 0575 1380 1385
      - 0730 0740
p
13.1
       - 0100 0730
```

```
345
      - 0450 OS10
370
      - 0480 1890
1:1
      - 0310 0760 0830 0840 1810 1370 1390
163
      - 0340 0580 0790 0800 0810 1820 1370
      - 0310 1230 1370 1380 1385
\mathbb{H}\mathbb{B}
      - 0870 0880 1160 1370 1380
1,1)
1.3
      - 0560 1380
      - 0230 0240 0580 0660 1170 1370 1380 1385
$5E)
      - 0200 0580 0660 1050
141
1,50
      - 0200 0580 0560 1050
143
      - 0560 0575 1980 1385
      - 0730 0740
1...
      OE70 0010 -
F-1
      - 0560 1290
17
F1.
      - 0630 1150
82
      - 0347 0580 1240 1385
532
      - 0700 0710 1370
      - OSGO 1310 1330 1340 1350 1370 1380 1385
7
      - 0230 0650 0700 0710 0730 0740 0760 0310 1180
T()
T3
      - 0190 0580 0650 0990 1030
T
      - 0190 0580 0650 0990 1030
      - 0510 0570 1290 1310 1330 1340 1350
T5
      - 0390 0580 0800 1000 1135
VI
VE
      - 0390 0580 0800 1000 1135
      - 0420 0790 1040 1130
VS
Y,
      - 0530 0550 0580
7, 53
      - 0370 0580 0980 1135 1310
      - 0370 0580 0980 1135 1310
\mathbb{E}_X
       - 0410 0580 1020 1130 1330 1340
7, 87
       - 0270 0280 0670
      -- 0150 0160 0620 0830 0860 0950 1320
Y 3
Y.23
      - 0300 0750
      - 0380 0330 0770
YΒ
\forall \Xi
      - 0345 0346 1385
Z 3
      -- 0370 0580 0690 0800 0980 1135 1310
\mathbb{Z}\mathbb{H}
      - 0370 0580 0690 0600 0880 1135 1310
7.3
      - 0590 0700
75
       - 0410 0580 0710 0730 1020 1130 1330 1340
```

APPENDIX II

LITTLE KLICKITAT/GOLDENDALE STP SIMULATIONS

LITTLE KLICKITAT D.O. MODEL INSTREAM CLASS A, D.O. CRITERION 8 MG/L

LIPSTREAM FLOW	, PLANT FLOW (**************************************	ытртн i.i 15	
TEMP UP,PLANT D.O. UP,PLANT NH3-N UP, PLAN EIUF DAY ROD N	TEMP D.C N-EHN TM DD TMAJH JAN	10.4 13.2 10.6 2.6 .02 12 5 20 *********	VELOCITY .6	3.	
DOWNSTREAM FLOOR DILUTION RATION MIXED ULT. NO MIXED TEMPERA MIXED D.G. (M. D.G. 100% SAT K1= K2= K3= SEDIMENT DEPT NON-NOD AMMON	Q 1 D (MG/L) 7 D (MG/L) 3 TURE (C) 1 G/L) 10 = 11	1			
· · · · · · · · · · · · · · · · · · ·	E A C H	3	The second secon		n in 26 julijan (j. 1944). Nasilian (j. 1945). Nasilian (j. 1945). Nasilian (j. 1946).
RIVER MILE 14.7 14.6 14.5 14.4 14.3	DAYS 0 .01 .02 .03 .04	BDD 7.4 7.38 7.35 7.33 7.3	NOD 3.72 3.5 3.29 3.09 2.91	0.0. 10.04 9.32 9.64 9.49 9.37	DEFICIT 1.14 1.35 1.53 1.69 1.81
NH3= .67	, and the same that the same that the same that the same that the	na ann ann ann ann ann ann ann ann ann		· .	
	ID (MG/L) 3 ID (MG/L) 2 ITURE (C) 1 IG/L) 9 IE 11 IE 13 IE 3	DEPTH & WIDTH 1. 7.3 1.9 10.2 37 1.28 34 62 .1	SS 20 VELOCIT	/ . 32	
R""	E A C H	2		***************************************	
RIVER MILE 14.3 14.2	DAYS 0 .019	800 7.3 7.25	NOD 2.9 2.49	p.0. 9.37 8.92	DEFICIT 1.91 2.36
NH3= .57	and their thing down their field had been print that the time the	the time can the time that had been the same time that the man time to the same time to	man time time time time time time time time	1864 mag vin 1967 A 44 F447 3 46 F438 3 546	

```
жалапапананананананананан INPUT ECHO анананананананананананананана
UPSTREAM FLOW, PLANT FLOW (CFS) 10 .4
                                              DEPTH AND WIDTH
                              10.1 10.1
TEMP LP, PLANT TEMP
D.O. LP, PLANT D.O.
                                              VELOCITY .43
                               8.9 8.9
                               .57 .02
7.25 6
NH3-N UP, FLANT NH3-N
ULTIMATE BOD LP, PLANT BOD 7.25 6
DOWNSTREAM FLOW (CFS)
                         10.4
DILUTION RATIO
                         25
                         7.2
MIXED ULT. BOD (MG/L)
MIXED ULT. MOD (MG/L)
                         2.33
MIXED TEMPERATURE (C)
                         10.1
MIXED D.G. (MG/L)
                        3.9
D.O. 100% SAT =
                        11.31
K1=
                        . 34
K2=
                        7.06
КЭ=
                        1.5
SEDIMENT DEPTH IN INCHES O
NON-NOD AMMONIA DEPLETION RATE 2.2
         REACH
               DAYS
                               800
                                               NOO
                                                              n.a.
                                                                              DEFICIT
RIVER MILE
                                                                             2.41
                                                               3.9
                                               2.38
 14.2
                0
                                7.2
 14.1
                .014
                                                2.25
                                                               8,95
                                                                               2.35
                                7,17
                                                                               2.3
                                                               9
 14
                .028
                                7.13
                                                2.14
                                               2.02
                                                               9,05
                                                                              2,26
 13.9
                .042
                                7.1
                .056
                                7.07
                                                1.91
                                                               9.09
                                                                               2.21
 13.8
                                                                              2.17
                                                               9.13
                .071
                                7.03
                                                1.8
 13.7
                                                               9.17....
                                                                               2.14
                ~035
                                                1,69
 13.5
                                7
                                                               9.2
                .099
                                6.96
                                                1.59
                                                                               2.1
 17.5
                                                                               2.07
                                                               9.23
 13.4
                .113
                                6.93
                                                1.48
                                                                              2.04
                                6.9
                                                1.39
                                                               9.26
 13.3
                .127
                                                                               2.01
                                                               9.29
                .141
                                6.86
                                                1.29
 13.2
                .155
                                                1 . 0
                                                               9.32
                                                                              1.99
 13.1
                                6.83
                                                                             1.95
                                                               9.34
 13...
                .169
                                6.8
                                              1.1
                                                                              1.94
                                                               9.37
                .183
                                                1,02
 12.9
                                6.77
                .197
                                                .93
                                                               9.39
                                                                               1.92
                                6.73
 8.51
                                                                             1.89
                                                . 85
                                                               9,41
 12.7
                .212
                                6.7
                                                               9,43
                                                                               1.37
                                6,67
                                                .76
 12.6
                 .226
                .24
                                                . 68
                                                                              1.86
 12.5
                                6.64
                                                               3.45
                                                                              1 , 24 --
                .254
                                               .61
                                                               9.47 ...
 12.4
                                6,61
                                                                               1.82
                                                .53
                                                               9.48
 12.3
                . 268
                                6,57
                                                .46
                                6.54
                                                               9.5
                                                                               1.8
 12.2
                 .282
                 .295
 12.1
                                6.51
                                                . 39
                                                               9.52
                                                                               1.79
                                                                9.57
                                                                               1.77
 12
                 . 31
                                6.48
                                                - 72
70. =EHM
每本部外的各种的的特殊的特殊的基础的基础的特殊的 INPUT ECHO 的复数拉特斯斯特斯特特特拉特斯特斯特特特特
UPSTREAM FLOW, PLANT FLOW (CFS) 10.4 .2 DEPTH AND WIDTH - .6 28
                             10.1 10.1
TEMP UP, PLANT TEMP
D.O. UP, PLANT D.O.
                                               EB. YTIDOLIBY
                               9.5 9.5
```

.07 .02 6.5 6

NH3-N UP, PLANT NH3-N ULTIMATE BOD UP, PLANT BOD

DOWNSTREAM FLOW (CFS)	10.6	
DILUTION RATIO	52	
MIXED ULT. BOD (MG/L)	6.49	
MIXED ULT. NOD (MG/L)	.3	
MIXED TEMPERATURE (C)	10.1	
MIXED D.O. (MG/L)	9.5	
D.O. 100% SAT =	11.31	
1(1=	. 34	
K5=	33.88	
K3=	1.5	
SEDIMENT DEPTH IN INCHES	0	
NON-NOD AMMONIA DEPLETIO	V RATE	O

REACH 4

RIVER MILE	DAYS	800	NOD	0.0.	DEFICIT
12	O	6,49	<i>"</i> 3	9.5	1.8
11.9	"O1	6.47	.29	9, 91	1.39
11.8	.019	6.45	.29	10,21	1.09
11.7	.023	6.43	.29	10.43	. 88
11.6	.039	6.41	.28	10.59	.72
11.5	.048	6.38	.28	10.7	
11.4	.058	6.36	.27	10.79	.52
11.3	. OE8	6.34	.27	10.85	, 46
11.2	.077	6.32	.27	10.89	.41
11.1	.037	6.3	. ac	10.93	. 38
11	. 0 9 7	6.28	.26	10.95	. 35
10.9	.107	6.26	.25	10.97	. 34
10.8	.116	6.24	.25	10.98	. 32
10.7	.126	6.22	.25	10.99	.32
10.6	.136	6.2	24	11	
10.5	.145	6.18	.24	11	.3
10.4	.155	6.16	24	11.01	. 3
10.3	.165	6.14	.23	11.01	.3
10.2	. 174	6.13	.23	11.01	.29
10.1	.184	6.1	.23	11.01	.29
10	.194	e* 08	.22	11.01	

NH3= .05

HARRAGE REPRESENTATION OF THE PROPERTY OF THE

DOWNSTREAM FLOW (CFS) 14.7 DILLITION RATIO \odot 6,99 MIXED ULT, BOD (MG/L) 3,5 MIXED ULT, MOD (MG/L) MIXED TEMPERATURE (C) 10,4 MIXED D.O. (MG/L) 10,44 11.23 D.C. 100% SAT = . B4 K1= 6.87 K**≘**= KB= SEDIMENT DEPTH IN INCHES 6 NON-NOD AMMONIA DEPLETION RATE .5

RIVER MILE	DAYS	EGGO	NEO	$\bigcap_{n}\bigcap_{n}$	DEFICAT
14, 3	$\langle \rangle$	6,99	2.6	10.44	. 7/9
1 44		6.,94	2.17	9.95	1,28
13.8	,Q48	6.88	1.8	9,6	1.63
19.5	071	6.83	1.49	9.34	1,89
13.4	.095	6,77	1.21	9.37	E4. O45
13.2	,119	6.71	, S	9.05	27 . 37
13	,143	6.65	.78	8.99	2.74
12,8	, 164	6.61	"B	8.95	2.E7
12.6	,19	G.SS	n dis City	8.95	2,28
1,2 ,4	"214	6.5	. 32	8.96	2.27
13.2	.238	6,45	n X	8.98	
13	. DCC	8,4	n 🐧	9.02	e. e1

TEMP LP, PT. SOURCE D.O. 8.8 8.8 VELOCITY .63

NH3-N UP, PT. SOURCE NH3-N .03 .03

ULTIMATE BOD LP, PT. SOURCE UBDO 6.6 5.8

DOWNSTREAM FLOW (CFS) 10,6 DILLTION RATIO 377 MIXED ULT. BOD (MG/L) e, se MIXED ULT, MOD (MG/L) . 13 MIXED TEMPERATURE (C) 10,4 MIXED D.O. (MG/L) 8.8 0.0. 100% SAT = 11.23 }{ <u>1</u> ::: , 34 16 77 12 33,09 **₩** 1,5 SEDIMENT DEPTH IN INCHES O NON-NOD AMMONIA DEPLETION RATE O

REACH Z

RIVER MILE	DAYS	800	NOD	$O_{\pi}O_{\pi}$	DEFICIT
13	\Diamond	6.58	# 1 B	8.8	2,43
11.8	.019	Eva Evaly	- 1 B	9.78	1,45
11.5	"OBO	6.5	" 3 Z	10.3	,93
11.44	"OS8	6.45	3. 22	10.57	.es
11.3	"O77	6.41	# 1 EF	30.78	.51
1.1	, DAT	6. B7	n 1 1	S.O.t	, 4 B
10,8	.115	6. BB	n 3 1	10.84	. EE .
10,5	. 1 345	er ere	, 3 ±	30.86	, B7
1(),4	. 155	6.25	. 3.	10.87	, 34G
10,2	, 174	6.21	n 3.	10.88	.35
3.40	. 194	6,17	n 3.	30.88	##E .

SC, =EHM

THE DESCRIPTION OF THE PERSON OF THE CHEST O

RESIDENCE SCIENCE PART DIO DE PREVIOUS EN 1985 AL

DOWNSTREAM FLOW (CFS) 10.4 DILUTION PATID 13,5% MIXED ULT. BOD (MC/L) 7,35 MIXED ULT. HOD (MG/L) 3,52 MIXED TEMPERATURE (C) 10.4 MIXED D.O. (MG/L) 10,37 D.O. 100% SAT = 11,33 H(1 = " B4 H2= 7,11 月(日= ...7 SEDIMENT DEPTH IN INCHES 6 NON-NOD AMMONIA DEPLETION RATE .5

RIVER MILC	DAYS	E(110)	NOD	0,0,	DEFICIT
14, 41	$\langle \rangle$	7.35	3.62	10.37	. 86
) 4)	EEC.	7.28	2.92	964	1,58
13,8		7,21	2. B4	9.15	2.08
13,45		7.34	1,55	8.84	2.39
13.4	.113	708	1.44	8.69	2,58
19.3	. 141	7.01	1, 1	9.55	2,67
1.3	, 160	6.94	. 83	8.53	2.7
12.8	.197	6.28	.57	9.55	8.68
13,5	n 52 52 55	e. Bi	" BG	8.59	2,64
13.4	" EFF.44	€., 74	,, 18	8.66	2.57
12.2		6,468	JOB	8,73	2.5
12	" II.1	6.68	-,11	8.81	2.42

15.7 DOWNSTREAM FLOW (CFS) 14.7 DILLTION BATIO MIXED ULT. BOD (MG/L) 8,36 MIXED ULT. NOD (MG/L) , 13 10.39 MIXED TEMPERATURE (C) MIXED D.O. (MG/L) 11,23 D.O. 100% SAT = , 34 K 1 == 19,84 }(@= 1,5 KB⊯ SEDIMENT DEPTH IN INCHES O NON-NOD AMMONIA DEPLETION RATE O

REACH

RIVER MILE	DAYS	BOD	NOD	D <u>.</u> O .	DEFICIT
13	٥	6. B5	" 1 B	9	2.23
11.8	.019	6.32	. 1 B	9. Hi	1.71
11.6	.037	6,28	, 12	9.88	1,35
11,4	. DEG	(5 ., 274)	" 1 "	10.13	1 1
11.3	.07 <u>5</u>	6.2	.12	10 . B	. 9B
	Ē¢Ģ.	6,16	, 11	10.492	81
10.8	.112	6,12	. 3. 3	10.51	"73
10,6	.131	6,09	. 11	10.56	"67
3 () ,, 4)	. 145	6,05	, 1	10.6	"63
10.2	,168	6.01	,, 1	10.63	# 12N
3 (C)	, 157	5,97	, 1	10.65	, SB
9.50	,206	5.93	" <u>1</u>	10.66	., 57

MHE: "OE

LTTTLE KLKSKETIMT D.C. WODEL KNSKTREMST DLASSE M. D.C. WRKKTERKSKW SKIMKL

DOWNSTREAM FLOW (CFS) 14,7 DILUTION RATIO 20 MIXED ULT, GOD (MG/L) 8,36 MIXED ULT. MOD (MG/L) 4, , 335, MIXED TEMPERATURE (C) 30 MIXED D.O. (MG/L) 8,98 0.0. 100% SAT = 9.18 K2= 8,47 }<∃:= SECIMENT DEPTH IN INCHES 6 NON-NOD AMMONIA DEPLETION RATE .5

RIVER MILE	DAYS	BOO	NOO	D.O.	DEFICIT
1 4 m E	\Diamond	8. P6	4,25	8.92	, <u></u>
14	, J.	e. ee	I. HI	8.08	1 1
13,8	048	8.27	E . 194	7.48	1 7
13,5	073	8.29	2.43	7.05	2.11
13,4	. :073·5	8.18	1.99	6.79	2.B
13.3	.119	Et ., 3 4	1.6	6,62	2,55
1	.143	8., O3	1.27	EE.B	22 / 54
1.3.8	" 1 (SAS	S. 05	.99	6.,5	2.68
1 E . E	. 19	8	, " JA	6.5	B.68
13,4	A15.	7.95		6.53	2,64
الناء بر الناء لا	, 338	7.98	# BE	6.58	2.59
13	 365	7,87	. 16	6.64	2.53

中央政策的政策的基础的企业实际的基础的基础的基础。(CEC) TUPAN 中国政策的基础的基础的基础的基础的基础的 UPSTREAM FLOW, PT. SOURCE FLOW (CFS) 14.7 1 OE 8, HIGHW GNA HIGHD TEMP UP, PT. SOURCE TEMP 20 20 D.G. LP, PT. SOURCE D.G. 6.6 3 VELOCITY ,65 NHB-N UP, PT. SOCRCE NHB-N ...04 ...03 ULTIMATE BOD UP, FT.SOURCE UBOD 7.9 5.8

15.7 DOWNSTREAM FLOW (CFS) DILLSTION RATIO 14,7 7,77 MIXED ULT, BOD (MG/L) MIXED ULT, MOD (MG/L) . 17 MIXED TEMPERATURE (C) 20 MIXED D.O. (MG/L) 6, 69 0.0. 100% SAT = 9,18 **K**3= "BB }(??= 24,46 K E == SEDIMENT DEPTH IN INCHES O

NON-NOD AMMONIA DEPLETION RATE O

R E A C H

RIVER MILE	DAYS	2(20)	NCO	D , () ,	DEFICIT
133	\bigcirc	7.77	. 17	6, 69	2,49
11.8	.019	773	.15	7.38	3 , (2)
11.5	,OB7	7.7	# 1 B	7.81	1 , 305
11,4	, 1755	7.67	* 3.527	8.09	1.08
11.5	.075	T. S.B	" <u>1</u>	8.27	,91
1.1	O93	7.6	,09	8. B8	,79
8.01	- 1 1 E	7.57	. 08	8.45	, 7 EZ
10.6	. 1.B1	7.54	" O7	8.51	# EN7
113.4	,149	7.5	. OS	8,54	" (54)
10.2	. 1.68	7.47	"OS	8.54	, (D)
10	187	7.44	" {"\\ <u>\</u> "\\	8.57	4 JES

ic, =EHM

INSTRUME OF PASS 14. D.C. CRITERION S WELL.

DOWNSTREAM FLEW (CFS) 14,7 DILLUTION MATIO 20 MIXED ULT. BOD (MG/L) 8,36 MIXED ULT, NOD (MG/L) 3,22 MIXED TEMPERATURE (C) MIXED D.O. (MG/L) 3,93 D.O. 100% SAT = 9,18 K1= . 23 }(Z)= 8,47 **}**(E)== ...y SEDIMENT DEPTH IN INCHES 6 NON-NOD AMMONIA DEPLETION RATE .5

RIVER MILE	DAYS	E400	MEED	o.c.	DEFICIT
14.3	0	8.54	3,33	8.92	, 25
1 2 4	" 1 E 1 7 5	e. be	2.69	E., 22	
B.E1	048	8.27	2.23	7,,72	1,45
13.6	.: O71	e.ze	1 . 34	7.56	1,81
13.4	ZEO.	8.18	i, E	7.18	2.05
13,3	.119	8.14	1., 21	6.96	2.21
13	.14B	8.09	# 94 5	6 37	a, 3
12.8	. 166	8.05	.75	6.BB	2.35
184.48	.19	8	, 56	6.88	2,36
12.4	"214	7.95	. 99	6.83	27, 34
12.E	EEE,	7.93	" 23 Th	6.85	3. 33
1.2	* E(F)	7.57	, 13	$\mathfrak{S}_n \mathfrak{D}$	2.28

UPSTREAM FLOW, PT. SOURCE FLOW (CFS) 14.7 1 DEPTH AND WIDTH .8 30 TEMP UP, PT. SOURCE TEMP 20 20 0.0. UP, PT. SOURCE D.O. 6.9 8 VELOCITY .65 NH3-N UP, PT. SOURCE NH3-N .03 .03 ULTIMATE BOD UP, PT. SOURCE LBOO 7.9 5.8

DEWNGTREAM FLOW (CFS) 15.7 DILUTION RATIO 14.7 MIXED ULT, BOD (MG/L) 7.77 MIXED ULT. MOD (MG/L) .13 MIXED TEMPERATURE (C) \cong MIXED D.O. (MG/L) 6.97 D.O. 100% SAT = 9,18 }{ } := # Z 3 **₩**2= 24,46 }(=(== 1 , 5 SEDIMENT DEPTH IN INCHES O NON-NOD AMMONIA DEPLETION RATE ()

REACH 2

RIVER MILE	DAYS	BCO	NOD	D ., O.,	DEFICIT
1 23	\Diamond	7.73	E L .	6.97	2.2
11,8	019	7.73	.1B	7, 57	1,61
11.45		7,,7	" 1 E	7.95	1.23
3.3 " 4		7.67	, 1 H	8, 19	. 99
11,2		7.63	" 3 2	8.34	. 84
1.1		7 " ⊖	. 3 3	8,48	74
10.8	, 112	7.57	, 1 1	田。田	. 5B
10.5	, 1 E1	7.54	a 3 3	9.59	" #54
10 , 4	.149	7.5	и .i.	8 . S43	" (5.12)
10.2	E23 f	7.47	σ 1	8.58	# \$100 h
10	. 187	7,44	n .1	9.59	59

50, =EHM

LIVE KERCETAT BLO. WORDEL Krantelan class a. D.C. Creterion species

TEMP LP, FT. SOURCE D.O. 10.1 7.2 VELOCITY .51
NH3-N UP, FT. SOURCE BOD 5 20
STANSSTRAND UP, FT. SOURCE BOD 5 20

DOWNSTREAM FLOW (CFS) 14.7 DILUTION MATIO 22() MIXED ULT. BOD (MG/L) 5,51 MIXED ULT. NOD (MG/L) 3,22 MIXED TEMPERATURE (C) 1 15 MIXED D.O. (MG/L) 9,98 D.G. 100% SAT = 10,15 K 1 == " 14 **天**選= 7.59 **K∃**::: SEDIMENT DEPTH IN INCHES 6 NON-NOO AMMONIA DEPLETION RATE "S

RIVER MILE	DAYS	800	NED	D.O.	DEFICIT
14.3	$\langle \rangle$	6,61	8.28	9,,98	.19
1 14	$_{H}$ $\left\langle \left\langle \cdot\right\rangle \right\rangle \left\langle \left\langle \cdot\right\rangle \right\rangle \left\langle \left\langle \cdot\right\rangle \right\rangle \left\langle \cdot\right\rangle \left\langle $	6.55	2.69	9. Z5	. 89
13,8	.:048	6.48	2,23	8.75	3 4
13.5	,071	6.48	1.84	8.B8	3 . 77
13,4	.095	6. B5	3 # 🖽	8.12	2.03
13.E	,119	6.3	3 - 573	7.95	<u>~</u>
1 B	.143	€., 2 4	, 96	7.84	2.31
12.8	. 188	6.18	, 3 5	7,78	2.37
122.45	, 19	6.12	. E6	7,75	2.4
12.4	" <u>214</u>	6.07	"B9	7.75	2.39
18.8	,235	6.01		7,, 78	F. IT
1.3	. Per	5.95	. 12	7.83	

DOWNSTREAM FLOW (CFS) 3 15 DILLUTION RATIO 1 4 MIXED ULT. BOD (MG/L) 5,99 MIXED ULT, MOD (MG/L) . 13 MIXED TEMPERATURE (C) 15 MIXED D.G. (MG/L) 7.8 D_*O_* 100% SAT = 10.15 **K** 1 :== ,, st. K2= 21,93 KE= SEDIMENT DEPTH IN INCHES O NON-NOD AMMONIA DEPLETION RATE O

R E A C H Z

RIVER MILE 12 11.8 11.6	DAYS 0 .02 .039	800 5,99 5,94 5,89	NOD "13	D.G. 7.8 8.41	0EFICIT 2.35 1.74
11.4	. 620 . 870 .	5, 89 5, 8	, 3 , 00 , 00	8.8 9.06 9.23	1.35 1.09 .92
11 10.2 10.5	.098 .117 .187	5.75 5.71 5.67	.07 .06 .05	9.35 9.42 9.47	. 8 . 73 . 78
10.4 10.2 10.2	.156 .176 .196	S. 62 S. 58 S. 54	204 204 203	9.5 9.53 9.54	,68 ,65 ,62 ,61

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DOWNSTREAM FLOW (CFS) 14,7 DILUTION MATIO 20 MIXED ULT, BOD (MG/L) 6.99 MIXED ULT. MOD (MG/L) 3,22 MIXED TEMPERATURE (C) 10,4 MIXED D.O. (MG/L) 10,44 D.O. 100% SAT = 11,23 K 1 ## . 34 天津中 6,87 ₩3::: SEDIMENT DEPTH IN INCHES 6 NON-NOD AMMONIA DEPLETION RATE "5

RIVER MILE	DAYS	800	NOD	0.0.	DEFICIT
14.3	0	6.99	3,22	3.40 , 444	, T'9
3.4	" 1 5 5 5 4 4 5 5 5 5 6 5 6 5 6 5 6 5 6 5 6	6,94	2.69	9.87	135
13.8	. 948	6.88	7.29	9,49	1.,78
13.6	* 17.13 F	6. BE	3.,84	9,15	₩. (\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
13,4	, O95	6,77	1 5	8.95	2,27
13.2	.119	6.71	1,21	8,83	∃ , 4,
13	.143	6.68	.96	8.76	₽.47
13.3	.166	6,61	" 7 S	8.73	e. S
18.5	.19	6.55	. BG	B.73	2.5
13.4	. 2114	6.5	" B9	8.75	2.48
13,3	. 238	€., 45	* 53 E	8.79	2,44
13	. ECE	6.4	.12	8.89	E, 39

```
UPSTREAM FLOW, PT. SOURCE FLOW (CFS) 14.7 1 DEPTH AND WIDTH .8 30 TEMP LP, PT. SOURCE TEMP 10.4 10.2 D.O. UP, PT. SOURCE D.O. 8.8 VELOCITY .65 NH3-N UP, PT. SOURCE NH3-N .03 .03 ULTIMATE BOD LP, PT. SOURCE LBOD 6.4 5.8 SERVER RESERVED AND WIDTH .8 30 TEMP LP, PT. SOURCE LBOD 6.4 5.8 SERVER RESERVED AND LBOOK LB
```

DOWNSTREAM FLOW (CFS) 15,7 DILLTION MATIO 14.7 MIXED ULT. BOD (MG/L) 6.36 MIXED ULT. MOD (MG/L) , 13 MIXED TEMPERATURE (C) 10,39 MIXEO D.O. (MGZE) 8.8 D.G. 100% SAT = 11,23 K1= " B4 天罡== 19,84 1.5 SEDIMENT DEPTH IN INCHES O

NON-NOD AMMONIA DEPLETION RATE O

R E A C H 2

RIVER MILE	DAYS	E(00)	NCO	n.a.	DEFICIT
1,23	\circ	€,, ⊒€,	.13	2.8	2,43
11.8	"O19	6. Ba	#13	9.38	1,85
11,5	#037	6.28	.12	9,79	1 , 44
1 1 2 2		€., 24}	, 1 2	10.07	1,17
11.2	"O7S	6. E	. 122	10.26	,97
1.1	"NGB	6.15	. 11	10.39	, E4
10.8	" 1 1 2	6.12	. 1. 1	30,48	, 7 S
10.6	. 131	6.09	. 11	10.55	. GB
10.4	149	6.05	. 1	10.59	" 15.4
10,2	.168	601	» 3	10.62	.61
10	.187	5.97	o 3	10.65	, 59

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· 动物等特别或者有效的动物的特别的特别特别的特别的 INPUT ECHO 网络特别特别的特别特别特别特别特别特别特别特别的特

UPSTREAM FLOW, FT. SOURCE FLOW (OFS) 14 .7 DEFTH AND WIDTH 1.3 22

TEMP UP, PT, SOURCE TEMP D.O. LP.FT. SOUNCE D.O.

20 20

9.01 7.2 VELOCITY .51

NHS-N UP, PT.SOORCE NHS-N .03 12

FIVE DAY BOD UP, PT. SQUACE BOD 5 20

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DOWNSTREAM FLOW (CFS)	14.7
DILLTION MATIC	20
MIXED ULT, BOD (MG/L)	8.36
MIXED LET. MOD (MG/L)	2.5
MIXED TEMPERATURE (C)	20
MIXED D.G. (MG/L)	8.98
D.O. 100% SAT =	9.18
₩1 =	"BB
}{? <u>?</u> =	0.47
} € 3=	7
SEDIMENT DEPTH IN INCHES	6
NON-NOD AMMENIA DEPLETIO	N RATE .5

RIVER MILE	DAYS	BCD	NEO	0,0,	DEFICIT
14,2	\Diamond	8 . ISS	æ. G	8.92	20 M
1.4	,, / <u>[]</u> }; <u>[]</u> / <u>4</u> }	8.59	2.17	8.31	. 87
13.8	#O48	8,27	1.8	7.85	1,32
13.6	"O71	0.23	1,49	7. 54	1.4
13,4	.095	8.18	1,21	7. 362	1,55
13.2	.119	8.14	98	7.17	<u>;;,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,</u>
13	.143	8.09	" 7 (3)	7.,08	iF . 1
12.8	.166	8.05	" ©	7.03	2,15
12.6	.19	8	4 24 KZ	7.03	2,17
4 52 2	" <u>114</u>	7.95	" 32	7. O1	2.17
A i B i B	BEE.	7.98	<i>"</i> a	7 " O.E	Z.15
12	, E6E	7,87	" \$.	7.05	2,13

UPSTREAM FLOW, FT. SOURCE FLOW (CFS) 14.7 1 DEPTH AND WIDTH .8 30 TEMP LP, FT. SOURCE TEMP .20 20 D.O. UP, FT. SOURCE D.O. 7.05 9 VELOCITY .65 NH3-N UP, FT. SOURCE NH3-N .03 .05 ULTIMATE BOD UP, FT. SOURCE LBOD 7.87 5.8

DEMNSTREAM FLOW (CFS) 15.7 14.7 DILLITION RATIO 7 , 7 L; MIXED BLT. BOD (MG/L) MIXED ULT. MOD (MG/L) Ei. MIXED TEMPERATURE (C) \odot MIXED D.O. (MG/L) 7.17 D.G. 100% SAT = 9,18 H(1 == 24.45 }{??= 1.5 GEOIMENT DEPTH IN INCHES O NON-NOD AMMONIA DEPLETION RATE O

REACH 2

RIVER MILE	DAYS	800	NOD	D.O.	DEFICIT
1.2	$\langle \rangle$	7,74	"33	7,17	27
11.8	019	77	" 3 E	7.7	1.48
11.6	"OBG	7.67	» 3. Œ	E0.48	1,15
11,4		7.64	n 3 27	13 m 13 m	, 94
11.22	" O75	761	.12	8.87	E
1.1	093	7.57	× 3. 3.	8.46	7 E
10.8	.11E	7.54	. 1.1	2.51	, 67
10.6	,, 131	7.51	. 3. 1	8.54	.63
10,4	, 149	7.,48	n 3.	8.57	. 45.1
10,42	.168	7,44	. 3	8.58	» 6
30	. 3 557	7.41	. 1	8.59	. 59

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DOWNSTREAM FLOW (CFS) 14,7 DILLUTION RATIO 20 MIXED ULT, BOD (MG/L) 6,62 MIXED ULT, MOD (MG/L) 3,5 MIXED TEMPERATURE (C) 3 55 MIXED D.G. (MG/L) 9,95 D.O. 100% SAT = 10.15 }{ <u>1</u> == , il. }(@= 7,59 }(3) SEDIMENT DEPTH IN INCHES 6 NON-NOD AMMONIA DEPLETION RATE .5

RIVER MILE	DAYS	800	NOD	D O	DEFICIT
14.8	\bigcirc	e. ee	₽"€	9.96	,19
1.4	" (C) (E) 44	€ EVS	2.17	9.85	" 🖾
13.8	"O48	e.s	1.8	8.9	1.25
13.8	.071	Es., 4445	1.49	8.54	1,59
13.4	095	6.38	1.21	8.89	1,82
13,3	.119	e. be	.98	8,16	1,99
13	.143	6.25	.78	8.05	2,09
12.8	. 165	6.2	\$	7.99	2,14
12 , 6	1/3	6.14		7.98	2,19
133.4	.21A	6.08	u 1957	7.95	SI . II
157 , 57	#BB	6.03	# XII	7.98	2,19
12	. 26E	5.97	" 1.	7,98	2.17

UPGIREAM FLOW, PT. SOURCE FLOW (CFS) 14.7 1 DEPTH AND WIDTH .8 30 TEMP LF, FT. SOURCE TEMP 15 15 15 VELOCITY .65 NH3-N UP, PT. SOURCE NH3-N .03 .03 .03 ULTIMATE 80D LF, PT. SOURCE LBOD 6 5.8

DOWNSTREAM FLOW (CFS) 15.7
DILUTION RATIO 14.7
MIXED ULT. BOD (MG/L) 5.99
MIXED ULT. NOD (MG/L) .13
MIXED TEMPERATURE (C) 15
MIXED D.O. (MG/L) 8.13
D.O. 100% SAT = 10.15
K1= .4
K2= 21.93
K3= 1.5
SEDIMENT DEPTH IN INCHES 0

NON-NOD AMMONIA DEPLETION RATE O

REACH 2

RIVER MILE	DAYS	BUD	NCID	$\square_{\pi} \square_{\pi}$	DEFICIT
13	Ø	5.93	.13	8.13	2.02
11.8	.O19	594	, 3 B	8.61	1 154
11.5	.OEG	5.9	. 1 E	8.93	1,22
1 1 1 1		B., 888	. 3 ZE	9,15	1
3.1.2	"O75	5.81	# 1 E	929	. 86
3. 3.	"O93	5.77	# 3 A	9.38	77
10.8	112	5,73	, 11	9,45	
10.5	,1B1	5.68	. 3.1	9,49	66
10.4	. 149	E. 64	" J.	9,53	. 63
10.8	. 168	5.45	., 3	9.54	n 45 3
10	. 1 B7	E E.E.	" 1.	9,55	n \$2.5

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