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Earth and Environmental Technologies

Lake Sawyer Hydrogeologic Study Black Diamond, Washington

Prepared for Washington State Department of Ecology

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CONTENTS

	Page
EXECUTIVE SUMMARY	1
INTRODUCTION	4
Study Location	5
Scope of Work	5
Report Structure	7
DATA COLLECTION METHODS	8
Septic Leachate Survey	8
Monitoring Well and Wellpoint Installation	9
Aquifer Testing	10
Water Level Monitoring	11
Groundwater Sampling	11
RESULTS	15
Septic Leachate Survey	16
Monitoring Well and Wellpoint Installation	16
Aquifer Testing	17
Water Level Monitoring	17
Groundwater Sampling	18
HYDROGEOLOGIC CONDITIONS	20
Introduction	20
Subsurface Stratigraphy	20
Two Aquifer System - Qvr and Qc	23
Conceptual Model - Groundwater/Lake Interactions	25
GROUNDWATER INFLOW/OUTFLOW ESTIMATES	29

	Page
SEPTIC SYSTEM PERFORMANCE EVALUATION	34
Septic System Site Selection Process	34
Septic System Monitoring Sites: Physical Conditions	35
Septic System Performance Characteristics	36
Septic System Loadings	43
REFERENCES	45

TABLES

1	Lake Sawyer Groundwater and Surface Water Elevation Data
2	Aquifer Testing Results
3	Groundwater Field Measurements: August 1989
4	Groundwater Field Measurements: December 1989
5	Groundwater Field Measurements: March 1990
6	Groundwater Field Measurements: May 1990
7	Groundwater Sampling Results: August 1989
8	Groundwater Sampling Results: December 1989
9	Groundwater Sampling Results: March 1990
10	Groundwater Sampling Results: May 1990
11	Average Groundwater Inflow and Outflow Estimates
12	Summary of Lake Sawyer Septic System
13	Septic System Loading Uncertainties

FIGURES

1	Vicinity	Map
---	----------	-----

- 2 Well Location Map
- 3 Hydrographs for Monitoring Wells
- 4 Hydrographs for Wellpoints #1 through #5
- 5 Hydrographs for Wellpoints #6 through #12
- 6 Hydrographs for Wellpoints #13 through #15

Page

FIGURES (Continued)

7	Hydrographs for Selected Domestic Wells
	Wells Completed in Qvr
8	Hydrographs for Selected Domestic Wells
	Wells Completed in Qc
9	Groundwater Elevation Map - September 1989
10	Groundwater Elevation Map - January 1990
11	Groundwater Elevation Map - May 1990
12	Conceptual Surface Geology and Well Inventory Map
13	Generalized Geologic Cross Section A-A'
14	Generalized Geologic Cross Section B-B'
15	Generalized Geologic Cross Section C-C'
16	Generalized Geologic Cross Section D-D'
17	Generalized Geologic Cross Section E-E'
18	Lake Sawyer Water Budget
19	Chloride Frequency Distribution
	Lake Inflow - Vashon Outwash Areas
20	Chloride Frequency Distribution
	Lake Outflow - Vashon Outwash Areas
21	Total Phosphorus Versus Chloride
	Black Diamond/Lake Sawyer Wastewater
22	Total Nitrogen Versus Chloride
	Black Diamond/Lake Sawyer Wastewater
23	Total Phosphorus Frequency Distribution
	Lake Inflow - Vashon Outwash Areas
24	Total Nitrogen Frequency Distribution
	Lake Inflow - Vashon Outwash Areas
25	Phosphorus Removal Frequency
	All Wastewater Detection Samples
26	Nitrogen Removal Frequency
	All Wastewater Detection Samples

		Page
	NDIX A ITORING WELL AND WELLPOINT INSTALLATION	A-1
		A 1
	oring Well Drilling and Installation Methods	A-1 A-2
	escriptions	A-2 A-2
	oring Well Installation	A-3
Monitoring Well Development Wellpoint Installation Water Level Measurements		A-3
		A-4
FIGUI	RES	
A- 1	Key to Exploration Logs	
	arough Boring Log and Construction Data for	
A-5	Monitoring Wells MW-1 through MW-4	
APPEI	NDIX B	
GROU	UNDWATER SAMPLING	B-1
APPEI	NDIX C	
AQUIFER TESTING		C-1
In Situ Hydraulic Conductivity Testing		C-1
Pumpi	ing Tests in Selected Domestic Wells	C-2
FIGUI	RES	
C- 1	MW-1 Falling Head Test Data	
~ •	MW-1 Rising Head Test Data	
C-2	MW-1 Recovery Test Data	
C-3	MW-2 Falling Head Test Data	
	MW-2 Rising Head Test Data	

		Page
FIGU	RES (Continued)	
C-4	MW-2 Recovery Test Data	
C-5	MW-3 Falling Head Test Data MW-3 Rising Head Test Data	
C-6	MW-3 Recovery Test Data	
C-7	MW-4 Falling Head Test Data	
O 1	MW-4 Rising Head Test Data	
C-8 MW-4 Recovery Test Data		
•	Recovery Test Data from Domestic Well No. 4	
	Recovery Test Data from Domestic Well No. 8	
APPE	NDIX D	
	LOG INVENTORY	D-1
	WMP DATABASE	
APPE	NDIX E	
	SAWYER SEPTIC LEACHATE SURVEY	E-1
	ANCO ENGINEERS, INC.	
APPE I	NDIX F	
	TORING NETWORK ELEVATION SURVEY	F-1

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LAKE SAWYER HYDROGEOLOGIC STUDY BLACK DIAMOND, WASHINGTON

EXECUTIVE SUMMARY

Lake Sawyer is a natural lake, of approximately 300 acres, located in southwest King County, 1.5 miles northwest of Black Diamond, Washington. This Lake Sawyer Hydrogeologic Study is one component of an overall lake Diagnostic Study being conducted by the Washington State Department of Ecology (Ecology). This study, and the companion Waste Load Allocation Evaluation completed last year, were prompted by recent nutrient loading increases to the lake attributed to the Black Diamond wastewater treatment system.

The objective of this hydrogeologic study was to provide data to supplement the water and nutrient budgets developed in the overall Diagnostic Study. This was accomplished by estimating groundwater inflow/outflow and nutrient loadings, incorporating an assessment of the nutrient inputs from nearshore septic systems.

Geologic conditions around Lake Sawyer were assessed by reviewing existing reports, studying available geologic logs of regional water wells, and logging four borings in which monitoring wells were installed. Based on an initial review of this information, a monitoring network was developed to provide groundwater flow and quality information. The network included the 4 monitoring wells, 15 wellpoints, 10 domestic wells, and five staff gages. The wellpoints were installed along the shore, between Lake Sawyer and residential septic systems, in order to assess leachate flow to the lake. Domestic wells were selected according to their location and availability. From August 1989 to May 1990, monthly water level measurements were obtained from the network and approximately quarterly water quality samples were collected from the monitoring wells and wellpoints. In addition, to assess the permeability of aquifer materials and potential flow volumes, aquifer testing was performed in the monitoring wells and some of the domestic wells.

The available geologic data indicate a very complex stratigraphic and hydrogeologic system around Lake Sawyer. It appears that the lake sits in a trough of low permeable till material, between bedrock to the east

and surficial till outcrops to the north and west. Higher permeable outwash deposits overlie the till to the east of the lake, but are largely absent to the west. Partly because of this condition, groundwater flow enters Lake Sawyer predominantly along its eastern shore, via the outwash deposits. The estimated groundwater inflows averaged approximately 0.1 cubic feet per second (cfs), with the bulk of the inflow occurring during the winter months.

Permeable recessional meltwater channels of sand and gravel are located in the northeast and southwest corners of the lake, and complicate hydrogeologic conditions within the basin. The meltwater channels appear to act as outflow conduits from Lake Sawyer, with such outflow likely providing recharge to a deeper aquifer system. The estimated groundwater outflows, through the lake bottom and via the meltwater channels, average approximately 3 cfs, with the highest calculated outflows occurring during the summer/fall months. The estimated monthly groundwater outflows are consistent with unexplained lake outflows calculated in Ecology's preliminary surface water budget.

The inputs of total nitrogen (TN) and total phosphorus (TP) to Lake Sawyer via the regional groundwater system were assessed by multiplying the average estimated groundwater inflow rate (0.1 cfs) by measured background concentrations of these nutrients within the basin. Because of the presence of cultural (i.e., septic tank) influences at the monitoring well locations, background water quality was assumed to be represented by water quality monitoring data within Ravensdale Creek, which drains a relatively pristine watershed in the eastern areas of the basin. Ravensdale Creek also appears to be a predominant recharge source for groundwater inputs to Lake Sawyer. Based on these data and assumptions, the estimated nutrient inputs to the lake from the regional groundwater system average approximately 50 kgTN/year and 0.8 kgTP/year. The uncertainty in these estimates is large, owing primarily to variabilities in measured and assumed flow parameters. Loading rates of these parameters may range from 5 times higher to 15 times lower than the estimated average values.

Septic system releases of nitrogen and phosphorus to Lake Sawyer were assessed by monitoring groundwater quality within approximately 100 feet of a variety of nearshore septic systems within the basin.

Detectable quantities of effluent, as determined using chloride tracer

data, were observed in groundwater samples collected at ten of these locations. Based on a mass balance relationship, removals of TP and TN occurring during transport from the septic tank to the groundwater monitoring locations averaged 94 ± 2 percent and 79 ± 11 percent, respectively. These measured removals are consistent with values reported from other similar studies conducted in the region. Applying these observed removals to the total number of dwellings within the "effective" Lake Sawyer catchment area results in an estimated existing (1989-1990) wastewater loading of approximately 190 ± 90 kgTN/year and 9 ± 4 kgTP/year.

Based on the available data, it is likely that the bulk of groundwater inputs of nitrogen and phosphorus to Lake Sawyer are represented by wastewater contributions. These estimated groundwater loading values, however, do not appear to be major components of the overall nutrient budget for Lake Sawyer.

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INTRODUCTION

This report presents the results of a hydrogeologic study of Lake Sawyer, a natural lake (approximately 300 acres in size) located near Black Diamond, Washington. This investigation is a component of Ecology's overall Diagnostic Study, which assesses existing water quality characteristics of Lake Sawyer. Principal objectives of these efforts are 1) to evaluate the influence of the Black Diamond Wastewater Treatment Facility on the present and potential eutrophication of Lake Sawyer, recommending the discharge option which maintains acceptable Lake Sawyer water quality; and 2) to perform a comprehensive baseline water quality assessment, which includes an evaluation of nutrient inputs from a variety of sources, and their relative contribution to the trophic condition of Lake Sawyer.

Recent increases of nutrient loadings to Lake Sawyer have been reported (Pelletier, 1989). Much of the apparent increase of the most critical nutrient -- phosphorus -- has been attributed to the Black Diamond wastewater treatment system. The system presently consists of a two-cell aerated lagoon discharging into a natural wetland where nutrient removal was designed to take place utilizing natural processes. The wetland, via Rock Creek, eventually drains to Lake Sawyer.

The U.S. Environmental Protection Agency (EPA) recommended that a study be performed to assess the amount of phosphorus that needs to be removed by the Black Diamond wastewater treatment system in order to protect the water quality of Lake Sawyer. In response, Ecology developed a two-phased approach to the study. The initial phase, the Waste Load Allocation Evaluation, used primarily existing data to evaluate the sources and rates of nutrient inputs to Lake Sawyer, and considered the lake's trophic response to current and future loadings. This phase of the study was completed in the summer of 1989, and resulted in an Ecology staff recommendation to discontinue discharge of Black Diamond wastewater to the Lake Sawyer basin.

Field work for the Diagnostic Study, the second phase, began in the spring of 1989 and was completed in the spring of 1990, incorporating a year of data collection. The study consisted of monitoring limnological parameters within the lake itself, assessing the surrounding watershed, and developing lake nutrient/water budgets. This hydrogeologic study of

Lake Sawyer is one portion of the overall Diagnostic Study, supplementing the development of the nutrient and water budgets.

As stated above, one of the principal objectives of the Diagnostic Study is to provide an adequate database to evaluate nutrient inputs and trophic conditions. The assessment of residential wastewater (i.e., septic system) inputs of nutrients to the lake is presented in this report. A more general evaluation of groundwater inflow and outflow characteristics is also presented, primarily to support the limnological evaluation reported elsewhere.

Study Location

Lake Sawyer is located in southeastern King County and lies within the Big Soos Creek sub-basin of the Green River drainage near the city of Black Diamond (Figure 1). Approximately 83 percent of its 36,000-foot shoreline is developed, primarily with residential dwellings (Figure 2).

Lake Sawyer's 13-square mile watershed consists largely of forested areas or unproductive land. Two inflow streams, Rock and Ravensdale Creeks, discharge into the southeastern end of Lake Sawyer. An extensive wetland is also located in this area. Covington Creek exits the lake from the central-western shoreline. A concrete dam installed across Covington Creek controls the lake level.

Scope of Work

The data collection scope of work for the hydrogeologic portion of the Diagnostic Study was divided into the following tasks:

Task 1 - Literature Review and Well Inventory

The purpose of this task was to develop a basic understanding of the hydrogeologic system(s) around Lake Sawyer in order to orient the data collection efforts (e.g., placement of monitoring wells and wellpoints). This effort involved a review of relevant information in the South King County Groundwater Management Program database, a homeowner survey to locate regional water wells and obtain volunteers to participate in the study, and a septic leachate survey along select shoreline areas of Lake Sawyer.

Task 2 - Design and Install Monitoring Network

Based on the findings of Task 1, a monitoring network was designed which involved the installation and/or monitoring of 4 monitoring wells, 15 wellpoints, 10 domestic wells, and 4 staff gages (installed by Ecology). The purpose of the 4 monitoring wells was threefold: 1) to aid in the assessment of regional stratigraphy; 2) to provide background water quality data; and 3) to serve as water level monitoring points for the assessment of regional groundwater flow. The purpose of the wellpoints was to provide nearshore groundwater quality data to assess septic/drainfield system inputs to Lake Sawyer and to assess shallow groundwater flow near the lake. The domestic wells served as water level monitoring points and, in some instances, as water quality sampling locations. The staff gages provided lake and creek level information.

Task 3 - Aquifer Testing

To aid in the groundwater inflow/outflow assessment of Lake Sawyer, short-term pumping tests and *in situ* hydraulic conductivity tests (slug tests) were conducted to assess aquifer permeabilities.

Task 4 - Elevation Survey

The monitoring wells, wellpoints, and 4 staff gages were surveyed for elevation by a licensed surveyor. In addition, Hart Crowser conducted an elevation survey of the remaining domestic wells used in the monitoring well network.

Task 5 - Groundwater Sampling

To assess nutrient inputs to Lake Sawyer via groundwater, water samples were obtained from the monitoring wells and wellpoints on a quarterly basis. During two of the sampling rounds, samples were also obtained from two septic tanks.

Task 6 - Water Level Monitoring

To assess groundwater flow near the lake, water level measurements were obtained on a monthly basis from the monitoring network and the staff gages.

Task 7 - Public Participation

This task involved preparing a publicity article for the local newspaper (Voice of the Valley), and meeting with the public to provide information on the study and to encourage public participation.

Task 8 - Reporting

This task involved preparing monthly progress reports and developing a draft, final draft, and final report for the study. This document presents the final report. The report includes: 1) the various data collected during the study; 2) an interpretation of the hydrogeologic system in the area; 3) an assessment of septic system performance; and 4) an assessment of water and nutrient inputs to and from Lake Sawyer from groundwater and septic system sources.

Report Structure

The previously presented **EXECUTIVE SUMMARY** highlights the methodology and findings of this report - presented below in their order of appearance:

- DATA COLLECTION METHODS;
- ► RESULTS:
- HYDROGEOLOGIC SYSTEM;
- ► GROUNDWATER INFLOW/OUTFLOW ESTIMATES; and
- **▶ SEPTIC SYSTEM PERFORMANCE EVALUATION.**

Appendix A presents the monitoring well/wellpoint installation methods along with the boring logs and as-built diagrams for the monitoring wells. Appendix B presents the groundwater sampling methods and the laboratory raw data results. Appendix C presents the aquifer testing methods and results. Appendix D contains the relevant well information/geologic logs from the South King County Groundwater Management Program database. Appendices E and F contain Entranco Engineers septic leachate survey report and Meridith Incorporated elevation survey report, respectively.

In general, tables and figures for this document are located after the text of the report and prior to the appendices.

DATA COLLECTION METHODS

To support the hydrogeologic study of Lake Sawyer, a variety of data were collected to assess groundwater quality and flow into the lake. The field activities included a septic leachate survey, the installation of monitoring wells and wellpoints, aquifer testing, groundwater level monitoring, and groundwater quality sampling. General descriptions of these activities are discussed in the following sections. Supporting information on the data collection methods can be found in Appendices A through F.

Septic Leachate Survey

A septic leachate survey was conducted by Entranco Engineers, Inc. along 1.1 miles of Lake Sawyer's eastern shoreline to assess the approximate location of leachate plume inputs to the lake. The survey was conducted using a K-V Associates Model 15 septic leachate detector in the shallow littoral zone of the shoreline. The detector is a portable fluorometer which operates at a fixed wavelength responsive to the fluorescence (specifically amino acids) in human urine and laundry whiteners (EPA, 1980). The unit was generally calibrated hourly with a one percent urine solution. A field measurement comparable to the response from the solution is generally used as the threshold value strongly indicative of an inadequately treated wastewater plume. The operator of the unit conducted the survey by sweeping the intake wand of the instrument along a 2 meter swath while wading the shallow littoral zone of Lake Sawyer.

The purpose of the survey was twofold; first, to aid in locating shoreline wellpoints within leachate plumes, and second, to provide a preliminary evaluation of leachate inputs to Lake Sawyer. The survey was originally scheduled for the entire eastern shoreline of Lake Sawyer where groundwater inflow was considered likely (confirmed with subsequent hydrogeologic data). However, instrument failure precluded surveying a portion of the southeastern shoreline. The complete report of the survey, including a more detailed account of the methods, is provided in Appendix E.

Monitoring Well and Wellpoint Installation

Monitoring Well Installation

Prior to locating the four monitoring wells installed during this study, available water well information in the South King County Groundwater Management Program database was reviewed. The database includes geologic logs, well construction information, water level, owner, and pumping test data. The water well inventory listings from the database, for wells within approximately 2 miles of Lake Sawyer, are presented in Appendix D.

Information from this database indicated that regional groundwater flow is generally to the west-northwest. It was also noted that there was considerably more stratigraphic information for the west and northwest sides of the lake as opposed to the northeast, east, and south sides. Primarily because of these geologic data gaps, and because the eastern portion of the study area was likely to contribute the majority of groundwater inflow to Lake Sawyer, monitoring wells MW-1, MW-3, and MW-4 were installed along the southeast, east, and northeast sides of the lake, respectively. MW-2 was installed along the southwest side of the lake. These locations are indicated on Figure 2.

Originally, attempts were made to drill all four wells using a hollow-stem auger rig. However, due to the cobbly subsurface materials, refusal was reached within seven feet of ground surface at all four locations. Consequently, a cable tool drilling was eventually used to drill MW-1. The remaining three monitoring wells were drilled with an air rotary drill rig. Geologic logs and detailed installation methods for these monitoring wells are presented in Appendix A.

Wellpoint Installations

The wellpoints were generally located along selected shorefront lots in suspected discharge zones to Lake Sawyer, although several wellpoints were distributed around the lake to define flow zones. Homeowners were initially identified via a house-to-house canvass and news release describing the project and the need for assistance from local homeowners. At this time, domestic wells for the water level monitoring network were also identified. The location of the wellpoints and domestic wells are shown on Figure 2.

Again, given the regional west-northwest groundwater flow direction (i.e., indicating lake outflow along the western shore), the majority of the wellpoints were placed along the eastern shore of Lake Sawyer. Their primary purpose was to assess nutrient inflow to the lake. The individual wellpoints were installed between the lake and the homeowner's septic system drainfield, within the suspected leachate plume. The general location of the drainfield was determined from the owner. Then, in six locations conducive to this approach, water samples within 0 to 15 feet of the lake were obtained from test probes. Chloride determinations were performed in the field on these samples using an Orion No. 9617 BN ion probe. Elevated levels of chloride potentially indicated the location of the drainfield leachate plume. Wellpoint placement was influenced by the ion probe readings in four of the six locations tested. General installation methods are presented in Appendix A.

Elevation Survey

The top-of-casing (TOC) elevations of the monitoring wells and wellpoints, and the staff gages were surveyed by Meridith Incorporated - a licensed surveyor in the State of Washington. They used a Wild NA-2 automatic level. They followed three wire leveling practices done to second order Class 2 accuracy as defined by NOAA (1979). Their report is presented in Appendix F. The TOC elevations for the domestic wells were surveyed by Hart Crowser using a Lietz level. The wellpoints were used as benchmarks. The accuracy of this survey, based on instrument precision, is approximately ± 0.01 feet.

Aquifer Testing

Aquifer testing was conducted in some of the monitoring and domestic wells within the monitoring network to assess the permeability of the various aquifer materials deposited around the lake. *In situ* hydraulic conductivity (slug) tests, pumping tests, and bail tests were performed.

Slug tests were conducted in MW-1 through MW-4. Both falling and rising head slug tests were performed. For the falling head slug test a rod was quickly lowered into the well causing an instantaneous rise in the water level. A Terra Systems automatic data acquisition system was used to measure the water level as it returned to the static level.

For the rising head slug test, the rod was removed causing an instantaneous drop in the water level. Again the water level was then measured with the Terra unit as it rose to the original static level. The collected data was analyzed using the Bouwer and Rice (1976) method.

Pumping tests were conducted in domestic wells No. 4 and No. 8. In each of the tests, the pump was turned on and allowed to run for 15 to 20 minutes during which drawdown in the pumping well was measured. Discharge from a garden hose was monitored by recording the time it took to fill a 4-gallon bucket. After turning off the pump, the water level recovery data was also monitored. The pumping and recovery test data were analyzed by the Cooper-Jacob (1946) method.

Bail (recovery) tests were also conducted in MW-1 through MW-4. In this instance, the water was removed from the monitoring well as rapidly as possible with a bailer, recording the volume removed and the bailing time. Then, the Terra unit was used to monitor the water level recovery to the static level. The data was then analyzed similar to pump test recovery data using the Cooper-Jacob method.

Appendix C presents a detailed description of the methods used to conduct these tests and analyze the results.

Water Level Monitoring

Water level measurements for the monitoring network were obtained on a monthly basis. Hart Crowser conducted the measurements from August through December of 1989, and in March and May of 1990. The Washington State Department of Ecology (Ecology) collected the water level data during the three intervening months.

Hart Crowser measured water levels to an accuracy of approximately 0.05 foot with an Olympic Model 150 Electric Well Probe and a decimally graduated tape measure. Ecology measured water levels to the same accuracy with a Slope Indicator well probe and tape measure.

Groundwater Sampling

Groundwater samples were collected from the 15 wellpoints and the 4 monitoring wells in accordance with the "Groundwater Sampling Plan" presented in Appendix B. Additionally, groundwater samples were

collected from a community well during three sampling rounds and a domestic well during the first sampling round. A quarterly sampling schedule, beginning in August 1989, was originally projected. However, the November 1989 sampling was postponed until December 1989 due to low lake and groundwater levels, and the associated lack of water in a number of wellpoints. The December sampling was conducted after lake levels had begun to recover, and immediately following a major rain storm. Subsequent sampling events took place in March and May of 1990.

The groundwater samples were filtered in the field and submitted to the Turnbull Laboratory of Ecological Studies in Cheney, Washington for analysis of total soluble phosphorus (TSP), soluble reactive phosphorus (SRP), total soluble nitrogen (TSN), nitrate-nitrite as nitrogen (NO₂+NO₃-N), ammonia as nitrogen (NH₃-N), and chloride. Samples from the May 1990 event were analyzed for TSP, TSN, and chloride only. Temperature, pH, conductivity, and dissolved oxygen (DO) measurements were performed in the field.

The following section presents a review of the laboratory data quality.

Data Quality

Data quality control measures included field and laboratory duplicates, spike recoveries, laboratory blanks, internal QA recoveries, and field blanks and filter blanks. Generally, one to two field duplicates were submitted to the laboratory during each sampling round, in addition to one field blank and two filter blanks. Spike recoveries involved adding a known concentration of an analyte to one of the groundwater samples in the laboratory, and then performing the analysis. Given the concentration of the analyte in the spike and of the groundwater sample, the percent of recovery can then be calculated. Internal QA recoveries involve the determination of a known concentration of analyte.

Overall, the data were deemed acceptable with one qualification; several of the TSP results were flagged as estimated values due to phosphorus input from field filtering. This, and the specific quality control results for each analyte, are discussed in further detail in the remainder of this section.

Total Soluble Phosphorus. With some exceptions, quality control associated with the TSP data was generally good. Spike recoveries averaged 94 percent, ranging from 82 to 99 percent. Internal QA recoveries (omitting 5 ug/L analyses due to accuracy difficulties with relatively low concentrations) averaged 100 percent with a range from 93 to 113 percent. Lab blanks ranged from 1.1 to 3.1 ug/L, with an average of 1.9 ug/L. No sample corrections were deemed necessary based on the lab blank data.

Sample duplicate analyses indicated a high precision with an average standard deviation between lab duplicates of 0.7 ug/L. The field duplicates were somewhat more variable, with an average standard deviation between duplicates of 2.5 ug/L or a coefficient of variation of less than 25 percent for higher level samples.

Two types of 0.45 micron in-line filters were used in the field for groundwater sampling. The majority of the samples were collected using a Gelman Acro 50 A filter (termed New Filter in the raw data base). However, a larger QED Environmental Systems, Inc., Model FF-8200 was used for some of the more turbid samples because the smaller Gelman filters clogged too readily. During each sampling round both Gelman and QED filter blanks were analyzed along with a deionized water blank.

TSP increases in the QED filter blanks ranged from 2 to 12 ug/L. The average increase was statistically significant at the 90 percent confidence level (P<0.1; t-test, n=4). Consequently, TSP data for the samples filtered with the QED filters were adjusted based on filter blanks for the particular sampling round. The adjusted samples with less than a 25 percent blank correction were flagged with a "B", to denote that blank contamination had been found and a correction made. Samples for which the blank correction exceeded 25 percent of the raw data value were flagged with a "J", denoting that the associated numerical value is an estimated quantity. Overall, approximately 30 percent of the database was "B" flagged and 17 percent was "J" flagged.

Conversely, TSP increases associated with the Gelman filter were not significant (P>0.50, t-test, n=4). Samples filtered in this manner thus did not require blank corrections.

Soluble Reactive Phosphorus. Quality control associated with SRP data was generally good. With one exception, spike recoveries averaged 98 percent and ranged from 94 to 100 percent. The exception was a 148 percent recovery for the December sampling round. Potentially, calculation errors may account for the elevated recovery percentage, since 100 percent recovery was obtained for the other spike in that data set. Internal QA recoveries (again omitting 5 ug/L analyses) averaged 99 percent and ranged between 99 and 100 percent. Lab blanks averaged 2.5 ug/L, and ranged from 1.9 to 3.5 ug/L. No correction was deemed necessary based on the lab blank data.

The laboratory duplicates indicated a high degree of precision, with an average standard deviation between lab duplicates of 0.6 ug/L. Like the TSP data, the field duplicates were somewhat more variable, with an average standard deviation between duplicates of 3.5 ug/L. Differences between the deionized water field blank and both the QED and Gelman filter blanks were not significant.

TSP vs SRP. In a number of incidences throughout the data sets the concentration of SRP for a particular sample was greater than the concentration of TSP. This issue was discussed with the Turnbull Laboratory Manager. He indicated this problem is potentially attributable to differences in the calibration curves during sample runs and/or pH differences in sample aliquots (due to sample preservation) which influence instrument readings. For the purposes of this report, both TSP and SRP data were considered equally valid based on QA/QC considerations.

Total Soluble Nitrogen. Quality control associated with TSN was acceptable. Spike recoveries averaged 100 percent and ranged between 94 and 108 percent. Internal QA recoveries were somewhat more variable, averaging 107 percent and ranging between 92 and 137 percent. Lab blanks ranged from 0 to 10 ug/L, and were not statistically different from analytical "zero". No correction was deemed necessary based on the lab blank data.

The precision of the laboratory duplicates was good with an average standard deviation between duplicates of 7 ug/L. Again, the field duplicates were somewhat more variable with an average standard deviation of 46 ug/L. Differences between the deionized water field blank and both the QED and Gelman filter blanks were not significant.

Nitrate-Nitrite as Nitrogen. Quality control associated with NO₂+NO₃-N was good. Spike recoveries ranged between 99 and 108 percent and averaged 104 percent. Internal QA recoveries averaged 104 percent and ranged between 101 and 105 percent. No correction was deemed necessary due to lab blanks, which ranged from 0 to 5 ug/L.

Again, the precision of the laboratory duplicates was good. The average standard deviation between laboratory duplicates was 3 ug/L. For field duplicates, the average standard deviation between duplicates was 24 ug/L. Differences between the deionized water field blank and both the QED and Gelman filter blanks were not significant.

Ammonia as Nitrogen. Quality control associated with NH₃-N was generally good. Spike recoveries averaged 103 percent and ranged from 93 to 114 percent. Internal QA recoveries averaged 91 percent and ranged from 72 to 105 percent. No correction was deemed necessary based on lab blank data.

The precision of the NH₃-N laboratory duplicates was very good with an average standard deviation between duplicates of 1 ug/L. Again, field duplicates were somewhat more variable with an average standard deviation between duplicates of 8 ug/L. Differences between the deionized water field blank and both the QED and Gelman filter blanks were not significant.

Chloride. The quality control associated with chloride was very good. Internal QA recoveries averaged 101 percent and ranged from 96 to 103 percent. The precision for both laboratory and field duplicates was excellent. The average standard deviation was 0.08 mg/L between laboratory duplicates and 0.18 mg/L between field duplicates.

RESULTS

The results of the data collection activities are presented in this section, according to each specific collection activity performed. Then, in the final three sections of this report, these results are synthesized together to develop a model of the hydrogeologic system, estimates of groundwater inflow and outflow, and to evaluate lakeside septic system performance.

Septic Leachate Survey

In general, the survey did not detect septic leachate plume activity along the portion of shoreline monitored. And, although equipment failure caused a premature termination, more than half of the survey was conducted along shoreline areas later determined to be part of a groundwater inflow zone. The lack of detections indicates that it is likely that septic system wastewater is either being adequately treated or is undergoing sufficient vertical and/or lateral dispersion from the drainfield to: 1) reduce plume concentrations below detection levels; or 2) discharge into the lake at deeper littoral locations.

Based on the results of the septic leachate survey, locating specific points of septic system wastewater effluent discharge into Lake Sawyer appeared unlikely. The assessment of wastewater inputs from more diffusive sources was therefore initiated, based on the groundwater monitoring network program and, specifically, the wellpoints.

Because groundwater flow directions were consistent throughout the year, and since perched groundwater conditions were not indicated on the western shore, the septic leachate survey was not repeated during the winter months. The seasonal consistency of groundwater flow patterns is discussed further under the HYDROGEOLOGIC CONDITIONS; Conceptual Model section of this report.

Monitoring Well and Wellpoint Installation

Generally, the only soil materials encountered in MW-2, MW-3, and MW-4, which were drilled to depths of 72, 48, and 59 feet, respectively, were recessional outwash sands and gravels (Qvr). In MW-1, till (Qvt) was encountered below approximately 15 feet of sand and gravel. Bedrock (Tbr), with traces of coal, was encountered at approximately 38 feet below ground surface at this location. The location and relative extent of these geologic units will be discussed further in the **HYDROGEOLOGIC CONDITIONS** section.

The TOC elevations are indicated in Table 1, along with well/wellpoint depths and monthly water level elevation measurements. The elevation survey indicated that the Lake Sawyer surface elevation generally fluctuates between 515 and 520 feet Mean Sea Level (MSL). This approximate lake level was corroborated by information obtained from

Dave Garland of Ecology, who collected survey data on the lake and groundwater near Lake Sawyer in the early 1980s (Dave Garland, 1990). However, the USGS Quad map for the area (Black Diamond Quadrangle; 7.5 minute series; photorevised 1973) previously reported the lake surface elevation at approximately 495 feet MSL. The discrepancy between these values appears to represent an error associated with the USGS map.

Aquifer Testing

Table 2 presents the aquifer testing results. The hydraulic conductivity testing estimates ranged from $5x10^{-5}$ cm/sec to $>10^{-3}$ cm/sec. The varying permeabilities suggests that there are a variety of different aquifer materials located near Lake Sawyer. This issue will be discussed further in the HYDROGEOLOGIC CONDITIONS section. The hydraulic conductivity testing results are used in conjunction with Darcy's Law to estimate inflow to and outflow from Lake Sawyer, as described in the GROUNDWATER INFLOW/OUTFLOW ESTIMATES section.

Water Level Monitoring

Monthly groundwater elevation data for the various monitoring locations are presented in Table 1. Based on these data, the hydrographs on Figures 3 through 8 were developed. They present the monthly groundwater elevations, relative to the lake, for the various measurement points within the monitoring network. The newly installed monitoring wells are grouped together on one hydrograph. Generally, the wellpoint hydrographs have been grouped together according to areas of the lake which they represent (i.e., northeast, east-south, and west-northwest). Domestic wells are grouped according to the geologic unit in which they are screened.

As indicated, the water level in wellpoints no. 1 through no. 5 and in MW-4, on the northeast side of the lake, are below lake level indicating outflow to the groundwater. On the other hand, the water level in wellpoints no. 6 through no. 12, MW-1, MW-3, and domestic well numbers 1, 2, 6, and 7, located along the east and south sides of the lake, are generally near or above lake level, indicating groundwater inflow. On the west and northwest sides of the lake the water levels in wellpoints no. 13 through no. 15, MW-2, and domestic well numbers 3,

4, and 8 are also generally below lake level, indicating outflow. Domestic well no. 5 appears to be an anomaly. The lower water elevation indicates that it is either screened in deeper pre-Vashon glacial deposits (Qc), or that it is pumped regularly to serve several homes and is slow to recover due to being at least partially screened in till or lower permeable materials.

Figures 9 through 11 provide water level elevations for the monitoring network during the months of September 1989, and January and May 1990. Of note is the lake versus MW-4 groundwater elevation difference which exists during September 1989, which is different from that observed during January 1990. A strong groundwater gradient away from the northeast part of the lake appears to exist during the drier portion of the year. Conversely, in the wetter portions of the year groundwater levels northeast of the lake are only slightly lower than lake level. A similar situation exists between the lake and MW-2. This, and the overall hydrogeologic flow system around Lake Sawyer will be further discussed in the HYDROGEOLOGIC CONDITIONS section.

Groundwater Sampling

Field Sampling Results

Field Measurements. The field measurement readings are presented in Tables 3 through 6. Groundwater temperatures measured in the four monitoring wells ranged from 9 to 15°C over the sampling year, with only slight seasonal variations. Conversely, wellpoint groundwater temperatures ranged from 7 to 22°C for the year, showing a larger seasonal influence. This is as expected, due to the wellpoints close proximity to Lake Sawyer and the much larger seasonal temperature variations observed in surface water as compared with groundwater.

Monitoring well and wellpoint pH measurements ranged from 5.6 to 7.7 and from 5.9 to 7.8, respectively. Electrical conductivity readings in the monitoring wells generally ranged from 90 to 170 μ mhos, although the readings in MW-1 ranged up to 360 μ mhos. Differences in conductivity may be attributable to different geologic units screened by the monitoring well (MW-1 is partially screened in Qvt and partially in Qvr, roughly 20-feet above bedrock; the other wells are screened in Qvr). Wellpoint electrical conductivity readings ranged from 80 to 280 μ mhos.

These pH and conductivity measurements were generally within the expected range for groundwater.

Dissolved oxygen measurements varied widely. Overall, DO ranged from 0.4 to 12.3 mg/L. Some of the lowest values were detected within septic leachate plume areas.

Laboratory Results

The laboratory results are presented by sampling round in Tables 7 through 10. TSP and SRP data remained relatively stable for individual sampling locations among sampling rounds. Phosphorus concentrations in the Community Well and Wellpoint No. 10 were generally near or greater than 100 ug/L for all of the sampling rounds. Phosphorus concentrations in MW-2 generally averaged around 50 ug/L, and the concentrations for the remaining sampling sites were generally below 30 ug/L for the sampling rounds. MW-1 is an anomaly to this scenario, again potentially attributable to its being partially screened in Qvt, and also to varying fine sediment concentrations in the sample, requiring additional filtering in the laboratory prior to analysis. Wellpoint no. 14 also indicates some fluctuation in phosphorus concentrations between sampling events, which may be attributed to changing precipitation and/or lake level conditions.

Nitrogen data were somewhat more variable for individual locations among sampling rounds. However, generally only the concentrations of TSN and NO₂+NO₃-N in MW-1, MW-2, MW-4, and wellpoints no. 9, no. 10, and no. 11 were near or greater than 500 ug/L for the sampling rounds. NH₃-N concentrations varied among sampling rounds for specific locations but, with one exception, were generally below 100 ug/L. NH₃-N concentrations in wellpoint no. 14 exceeded 200 ug/L during the two sampling rounds in which samples were obtained from this location.

Generally, the chloride concentrations at each of the various sampling locations were relatively stable among sampling rounds. Notable exceptions were concentrations in wellpoints no. 4 and no. 7, where concentrations varied among and between each sampling round. Variations in chloride concentration in these wellpoints likely represent changes in the volumetric fraction of wastewater present in the

groundwater sample (see SEPTIC SYSTEM PERFORMANCE EVALUATION)..

HYDROGEOLOGIC CONDITIONS

Introduction

The purpose of this section is to develop a conceptual model of the hydrogeologic system functioning around Lake Sawyer. The conceptual model is then used to estimate groundwater inflow to and outflow from the lake. The model is based on the previously mentioned data collected during this study and other information obtained from the literature. As such, the regional subsurface stratigraphy is considered first, followed by a discussion of the local aquifers which interact with Lake Sawyer. Finally, the conceptual hydrogeologic model for the system is presented and the interactions between the groundwater and the lake are discussed.

Subsurface Stratigraphy

The subsurface stratigraphy in the Lake Sawyer vicinity consists of the following sequence:

- Lake Sediments (Qs);
- Peat (Qp);
 Vashon recessional outwash deposits (Qvr);
- Vashon till (Qvt);
- Pre-Vashon glacial deposits (Qc);
- ▶ Undifferentiated till and interglacial deposits (Qi); and
- Tertiary bedrock (Tbr).

Not all units are present in all areas around the lake. For instance, as shown on the surficial geology map (Figure 12) the peat, Vashon recessional outwash, and the Vashon till all crop out at the surface at various locations around Lake Sawyer. Figures 13 through 16 are generalized subsurface cross sections that represent our interpretation of the subsurface stratigraphy in the project area. This interpretation of the subsurface stratigraphy is based on interpretation of geologic logs from Hart Crowser's four monitoring wells, information obtained from available drillers' logs for the area, published and unpublished reports on the geology and hydrology of the area, and from personal communication with individuals familiar with the local geology. It is

important to note that these are interpretations. The subsurface distribution of geologic units around the lake appears to be very complex and consequently cannot be fully delineated from available data.

Each of the stratigraphic units is described briefly below.

Lake Sediments (Os)

Sediments occur along the bottom of Lake Sawyer due to deposition of organic and inorganic matter. The relative depth and extent of these sediments is not known. It is presumed that the lake sediments have a relatively low permeability.

Peat (Op)

Post-glacial peat deposits occur at the surface along the southeastern corner of the lake, where Rock Creek flows into Lake Sawyer. The localized peat deposits are typically saturated, but are generally very thin and have low permeability. Therefore, the peat probably has minimal influence on shallow groundwater flow into or out of Lake Sawyer.

Vashon Recessional Outwash (Ovr)

The recessional outwash consists predominantly of stratified sand and gravel with abundant cobbles and small boulders. The outwash was deposited by high energy meltwater during the retreat of the Vashon glaciation. The recessional outwash occurs along most of the east and south sides of the lake, and in the northwest corner of the lake.

Luzier (1969) maps a number of relatively large recessional meltwater channels which trend northeast-southwest across much of south King County. One of these may occur in the northeast and southwest corners of Lake Sawyer. The data indicate that monitoring well MW-4, with a depth of 59 feet, is completed within a thick sand and gravel deposit. MW-2 and MW-3 were also drilled into sand and gravel presumed to be the recessional outwash to depths of 48 and 72 feet, respectively. Approximately 14 feet of the outwash was encountered overlying till at MW-1.

Vashon Till (Ovt)

Vashon till generally occurs beneath the recessional outwash along much of the east, north, and south sides of Lake Sawyer. Where the outwash is absent, the till crops out at the surface, especially west and north of the lake (Figure 12). The till, consisting of low permeability, unsorted, silty, gravelly sands and silts, is often referred to as hardpan in the drillers' logs. Information from available drillers' logs indicate that the till is generally 40 to 60 feet thick in the Lake Sawyer vicinity.

Not enough geologic data are available to discern if the till underlies the entire lake or only portions of the lake. Our cross sections indicate that the till is probably at least present beneath the shallow parts of the lake, but whether it underlies the deeper portions is unknown (Figures 13 through 16). There also is some question as to whether the Vashon recessional meltwater channel scoured away the existing till in the northeast, leaving a direct connection between the Qvr and the underlying Qc unit.

Pre-Vashon Glacial Deposits (Qc)

Data from drillers' logs and information from the South King County Groundwater Management Program indicate that these deposits, defined as Pre-Vashon in the South King County Groundwater Management Program, underlie Lake Sawyer and the surrounding area, but are absent in the rest of South King County. The Qc unit consists of clean sand and gravel with lesser quantities of slightly silty to silty sand. The South King County Groundwater Management Program refers to this unit as Qc(2) to differentiate it from similar but older coarse-grained units (Qc(3), etc.). However, because these other older units are not distinguished for this study, the coarse-grained deposits below the till are referred to simply as Qc.

As indicated earlier, the Qc unit may intersect the deeper portions of the lake. Additionally, in the northeast corner (and possibly in the southwest) meltwater channels may have scoured away the till unit leaving recessional deposits in direct contact with the Qc unit.

Older, Undifferentiated Till/Interglacial Deposits (Oi)

Several City of Kent wells located northeast of the lake have been advanced into non-water-bearing materials beneath the Pre-Vashon glacial deposits (Qc). The Kent Springs deep test well was drilled to a depth of 695 feet and encountered 550 feet of predominantly clay and silt below the Qc deposits. The origin of the thick clay is uncertain, but may have been deposited in glacial, interglacial, and/or possibly marine environments. Since the unit does not appear to interact directly with Lake Sawyer nor is it known to produce significant groundwater in the area, all deposits below the Qc are undifferentiated and grouped as Qi for this study.

<u>Tertiary Bedrock (Tbr)</u>

Tertiary bedrock is encountered in wells east and southeast of Lake Sawyer, including MW-1. Because the bedrock is not encountered in wells elsewhere in the vicinity of Lake Sawyer, the full extent of the bedrock is uncertain. The bedrock consists of light gray, fine-grained sandstone. In MW-1, light gray clay containing traces of coal or lignite was encountered immediately overlying the bedrock. (A number of underground coal mines exist in the region, including one located immediately southeast of Lake Sawyer.) The clay and sandstone are found together commonly in the area, and are part of the Hammer Bluff Formation (Luzier, 1969). The bedrock is a secondary source of groundwater in some locations, but is not considered a major water-bearing unit in the project area.

Two Aquifer System - Qvr and Qc

The recessional outwash (Qvr) and Pre-Vashon deposits (Qc) are the two primary water-bearing units which potentially interact with Lake Sawyer. Available hydrogeologic data indicate that the two systems are hydraulically distinct in most areas around the lake, separated by the Vashon till (Qvt). Groundwater in the Qvr flows both into and out of Lake Sawyer. Direct interaction between the Qc and Lake Sawyer remains questionable, due to lack of information concerning the extent of till beneath the lake and lake sediments on the lake bottom. Each of these aquifers is discussed below.

<u>Ovr</u>

The recessional outwash is an unconfined (water table) aquifer. Data from our monitoring network indicate that the water table elevation fluctuates seasonally by up to 12 feet. Depths to water range typically from less than 5 feet in wellpoints near the lake to greater than 40 or 50 feet in monitoring wells on the east (MW-3 and MW-4) and south (MW-2) sides of the lake where the topography rises.

<u>Oc</u>

The Pre-Vashon glacial deposits form a confined (artesian) aquifer in the project area. The overlying Vashon till forms a low permeability confining unit above the Qc Aquifer. On the basis of available drillers' logs and our cross sections, it appears that domestic wells numbers 3, 4, and 8 are completed in the Qc. Data are not available to ascertain whether domestic well number 5 is completed in the Qvr, Qc, or possibly in a more permeable zone within the Qvt.

Available drillers' logs for City of Kent's Kent Springs wells and KCWD 105 wells (located immediately northeast of the lake) suggest that both wellfields are tapping the Qc Aquifer, as indicated in Cross Section B-B' (Figure 14). Due to water rights issues, increased pumping rates, and lower groundwater levels, several studies have investigated the aquifer system(s) in the area immediately northeast of Lake Sawyer (Robinson and Noble, 1979 and 1982; Anderson & Kelly, 1981 and 1982; James M. Montgomery, 1987). These data indicate there is considerable debate on which aquifer the wellfields tap (Qvr or Qc or other?), and whether there is hydraulic connection between the water-bearing zones tapped by the City of Kent and KCWD 105.

Pumping tests conducted in both the Kent Springs wells and the Covington Water District wells indicate that the aquifer is highly transmissive, with transmissivities of greater than 1,000,000 gpd/ft estimated. Some tests indicate the system is influenced by a significant recharge boundary - possibly Lake Sawyer. Ecology, in their ongoing review of water rights for this area, has considered a possible direct hydraulic connection between the lake and the production wells (Garland, 1990). To date, however, the information is inconclusive.

Water level elevation data with survey control are scarce for wells completed in the Qc Aquifer. Water level data collected for this study from domestic wells in the Qc Aquifer indicate a general flow direction toward the northwest. These data are supported by water level data available through the South King County Groundwater Management Program database, which indicate regional groundwater flow toward the west-northwest. Luzier (1969) also maps regional groundwater flow to the west across this area. Potentially, the combined pumping from both the City of Kent and KCWD 105 wellfields may be drawing the hydraulic gradient in the Qc toward the northwest.

Conceptual Model - Groundwater/Lake Interactions

The purpose of the conceptual model of the hydrogeologic system is to assess groundwater inflows to and outflows from Lake Sawyer. As discussed previously, the hydrogeologic system around the lake is very complex due to the irregular distribution and thickness of Qvr and Qvt. To simplify the following discussion, the lake has been divided into the following five subsections:

- ► East and South Sides;
- Northeast corner:
- ► Northwest and West Sides;
- Southwest corner; and
- Bottom.

As will become apparent, these subsections were developed based on inflow and outflow characteristics, and the particular unit (Qvr or Qvt) in contact with the lake. Figures 9, 10, and 11 indicate the approximate location of the four surficial subsections.

East and South Sides

Groundwater in the Qvr flows into Lake Sawyer along its east and south sides. This is indicated by the water level measurements from MW-1 and MW-3, from wellpoints no. 6 through no. 11, and from domestic well numbers 1, 2, 6, and 7. The relation between the lake elevation and the groundwater elevations in these wells are shown on the hydrographs on Figures 3, 5, and 7.

The groundwater elevation in MW-1, screened predominantly in Qvt and partially in Qvr, and in MW-3, screened entirely in Qvr, are

consistently 5 to 15 feet above lake level. Water levels in the wellpoints, screened in Qvr, are approximately at or slightly above lake level throughout the year. The same is generally true for the domestic wells.

Wellpoint no. 12 is located on the southwest side of the lake. Its water level elevations also fluctuate with the lake, potentially indicating that it is situated in Qvr and that there is groundwater inflow. However, the actual wellpoint was placed within a few feet of the lake in what appears to be a fill area due to residential landscaping. Consequently, the looser fill material may provide a direct contact with Lake Sawyer. Thus the water levels may only reflect fluctuating lake levels and not actual groundwater flow conditions.

Northeast Corner

In the northeast corner of Lake Sawyer, water appears to flow out of the lake through the Qvr unit. This is demonstrated by the consistently lower groundwater elevations, relative to lake level, in MW-4 and in wellpoints no. 1 through no. 5. As indicated by the hydrographs on Figure 3, the groundwater elevation in MW-4 ranges from roughly 1 to 7 feet below the lake elevation, depending on the season. Similarly, the water levels in the five wellpoints (Figure 4) fluctuate seasonally with the lake, but remain consistently below lake level.

There appears to be a relatively sharp distinction between the east-south inflow area and the northeast outflow area. The boundary between the inflow/outflow zones lies between MW-3/wellpoint no. 6 on the northern edge of the inflow section, and wellpoint no. 5 situated along the southern edge of the outflow section.

It is speculated that MW-4 is located in a thicker portion (i.e., closer to the center) of the outwash channel mapped by Luzier (1969) than MW-3. This is shown schematically in Cross Section D-D' on Figure 16. Although the two monitoring wells are only 1,400 feet apart and are both screened in Qvr, the groundwater elevation at MW-3 ranges from 12 to 18 feet higher than at MW-4.

The groundwater elevation at MW-4 shows an interesting seasonal fluctuation. During the drier months of the year the groundwater elevation is as much as 7 feet below lake level. Consequently, a strong

hydraulic gradient for outflow from the lake exists. Conversely, during the wetter winter months the groundwater elevation at MW-4 is similar to the lake surface elevation, and thus outflow potential is reduced due to the shallower hydraulic gradient.

Potentially, several factors may influence this seasonal phenomenon. In the summer evapotranspiration increases while recharge to the Qvr unit from precipitation decreases. Additionally, increased draining to the deeper Qc unit may result due to seasonally lower water levels in the Qc and increased regional pumping of groundwater during the summer months. During the wet season the opposite occurs. There are less drains on the unit due to decreases in evapotranspiration and discharges to the Qc. Additionally, precipitation recharge to the Qvr increases.

Northwest and West Sides

Along the northwest and west sides of Lake Sawyer little or no water flows out of the lake through Qvt because it has a very low permeability. This is indicated by the hydrographs for wellpoints no. 13 through no. 15 on Figure 6. As shown, some of these wellpoints were dry for much of the year despite the fact that they were located near the lake and screened at depths well below the lake surface level.

Substantial interflow (in the topsoil along the top of the Qvt) to Lake Sawyer is not believed to occur based on the data collected in the study. Two wellpoints (WP-13 and WP-14) on the lake's west side are completed within the surficial till. In addition, wellpoint WP-15 in the northwest corner of the lake also appears to be completed in the Qvt, based on the soil characteristics observed during wellpoint installation.

Three geologic logs are available for water wells located within the area of surficial Qvt along the west side of the lake - 21N/06E-04E01, 21N/06E-4K01, and 21N/06E-4Q02 (see Appendix D). The well locations are shown on Figure 12. The three geologic logs indicate topsoil, or weathered till ranging from 2 to 5 feet thick, overlying the Qvt. The average thickness from the three values is 4 feet. Based on field observations and available geologic logs, the surficial soils may be more permeable than the underlying unweathered till (Qvt). Therefore, the question exists as to whether this thin topsoil layer may be saturated, either seasonally or during short term storm events, and whether there is groundwater flow from this unit into the lake.

The three Qvt wellpoints (WP-13, WP-14, and WP-15) were dry during many of the monthly water level measurements. WP-13 and WP-15 were both dry on January 8, 1990, which was the second day of a major storm event. Both wellpoints contained measurable water three days later (January 11, 1990), with nearly continuous rain in the interim. Water was measured in WP-14 during most of the year. On January 11, 1990, WP-14, which is located approximately 5 feet from the lake edge, was submerged by the lake with 0.16 feet of water above the wellpoint cap. This measurement is a measurement of lake level, and is not a groundwater level.

Groundwater levels in wellpoints 13 through 15 were below lake level during all dates of measurement (Figure 6), indicating flow out from the lake at all times. However, the question remains whether the wellpoints are screened to intercept flow within the topsoil, if it does occur. The top of the wellpoint screens are as follows:

- ➤ 2.1 feet below ground in WP-13;
- ▶ 4.1 feet below ground in WP-14; and
- ➤ 2.8 feet below ground in WP-15.

The top of the screened intervals in all three wellpoints are within the topsoil depth range of 2 to 5 feet, and all are at or above the average thickness of 4 feet, suggesting that the three wellpoints are partially screened within the topsoil. Because the water levels in the wellpoints are always below lake level, the available data indicate that flow is not occurring from the topsoil to the lake.

Southwest Corner

Outflow is believed to occur from the southwest corner of Lake Sawyer in a seasonal manner similar to that of the Northeast corner - although on a less dramatic scale. The Qvr in this area may also have been deposited within a channel, as shown schematically in Cross Section B-B'. As indicated by the hydrograph for MW-2 shown on Figure 3, the groundwater levels in this well closely mimic the fluctuating lake level. Again, during the drier summer months the groundwater level in MW-2 is generally several feet below lake level. Then, during the wetter months the groundwater elevation approaches, but stays slightly below the lake surface elevation. Consequently, in the summer months the increased hydraulic gradient between Lake Sawyer and the Qvr results in an increased rate of recharge to the Qvr unit from the lake.

Conversely, in the wetter season, the Qvr receives additional recharge from precipitation causing the water table to rise. This causes a reduced potential head difference between the lake and the Qvr, and results in a reduced rate of recharge from Lake Sawyer.

Bottom

As discussed earlier, there are insufficient data to completely assess the extent of the Qvt underlying Lake Sawyer.

However, based on surrounding stratigraphic information and the lake bathymetry, it is speculated that much of the lake rests on the till. The potential exceptions to this would be the two deeper zones in the northeast and central portions of the lake and/or any discontinuous areas of Qvt. In this case the lake would be in direct contact with the Qc.

Given the differences between the lake surface elevation and the water level in the domestic wells screened in the Qc, and the lake bathymetry, flow from the bottom of the lake to the Qc would be primarily downward. However, despite this downward hydraulic gradient, the relatively low hydraulic conductivity of the Qvt restricts flow. Even in areas where the lake is in direct contact with the more transmissive Qc, accumulated bottom sediments would also restrict flow.

In summary, Lake Sawyer is located between bedrock to the east and till outcrops to the north and west. It appears to be situated in a till trough with major recessional meltwater channels extending out from its northeast and southwest corners. Groundwater inflow to the lake occurs along its east and south sides. Outflows to the groundwater occur predominantly through the recessional deposits in the northeast and southwest, and also through sediment and till deposits lining the lake's bottom.

GROUNDWATER INFLOW/OUTFLOW ESTIMATES

Flows to and from Lake Sawyer were estimated using Darcy's Law (Freeze and Cherry, 1979):

Q=KIA,

where;

Q = discharge;

K = hydraulic conductivity;

I = hydraulic gradient; and

A = area of flow.

Due to the lack of specific data in certain areas and for certain parameters, a number of assumptions were made in order to estimate groundwater inflow and outflow from Lake Sawyer. For instance, hydraulic conductivities (K) were only measurable in several discrete locations around the lake. Consequently, the K values used in the estimates are either based on the aquifer testing results, where appropriate, or, where there is no data (e.g., the lake bottom), assumed based on expected K values for similar media in the region.

By necessity, hydraulic gradients are often based on only the difference between the Lake Sawyer surface elevation and the water level from one well. The diverse flow system into and out of the lake generally resulted in a condition where only one well with adequate geologic data and survey control, was available to monitor a particular flow path.

One of the most difficult parameters to estimate was the cross sectional area of flow for a particular pathway to or from the lake. For instance, there is insufficient stratigraphic data to pinpoint the width and depth of the recessional meltwater channels in the northeast and southwest corners of the lake. Similarly, along the east side of the lake the depth to Qvt influences the inflow estimates through the Qvr. However, Qvt was only encountered in our explorations in MW-1, and is only poorly described in available well logs for this area. Consequently, assumptions, based on trends in the stratigraphy, needed to be made concerning the depth to Qvt and thus the cross sectional area of flow within the Ovr.

Given the assumptions inherent in the calculations, the following inflow estimates should be viewed as rough approximations, and more as indicators of relative seasonal flow patterns and discharges, rather than specific values for inflow/outflow. In each case, the assumptions used for the flow estimate are discussed.

<u>Inflows</u>

As indicated previously, inflows to Lake Sawyer occur in the Qvr along the east and south sides of the lake. The components of Darcy's equation were determined in the following manner. Hydraulic conductivity was based on the average test values presented in Table 2 for MW-3. The hydraulic gradient was estimated using the monthly groundwater elevations in MW-1 and MW-3 as compared to the monthly lake level elevations. Several assumptions were made in determining the area of flow. First, it was assumed that all of the groundwater flow around the south-southeast portion of the lake entered via Rock Creek, and thus was monitored by Ecology's stream gaging station. As such the width of the inflow zone was assumed to be 4,500 feet, or roughly from Ravensdale Creek to wellpoint no. 6. Second, for the east side of the lake it was assumed that the depth to till angled linearly down from where it was encountered in MW-1 northward to approximately 10 feet beneath the bottom of MW-4. Consequently the thickness of Qvr in the inflow zone varied roughly from 3 to 10 feet (depending on seasonal water levels) at MW-1, to 25 to 40 feet at MW-3.

Table 11 presents the calculated average inflows to Lake Sawyer. Maximum and minimum inflow values were also considered. The maximum inflow was estimated using the maximum K value obtained during the aquifer tests. Additionally, it was assumed that the till near MW-1 dropped off sharply to the north so that the thickness of the inflow zone varied seasonally from 25 to 40 feet along the entire eastern shoreline. The maximum inflow values were approximately five times greater than the average estimates.

The minimum inflow was estimated using the lowest K value obtained during aquifer tests. The thickness of the inflow zone was estimated by assuming that Qvt existed immediately below the bottom of MW-3 and angled linearly up to where it was encountered of MW-1. The minimum monthly inflow estimates were approximately 15 times less than the average monthly inflow values.

Outflows

Outflows from Lake Sawyer to the groundwater system occurred from the lake bottom (incorporating the northwest and west zones) and from the recessional meltwater channels located in the northeast and southwest. Again, in each case due to the lack of specific data various assumptions had to be made in order to calculate outflows.

To estimate outflows through the bottom, the lake was divided into north and south quarters, and a central section which contained half the lake area. Flow from the lake to the Qc Aquifer was assumed to go through relatively low "permeability" till and bottom sediment with a K value of approximately 10⁻⁵ cm/sec - a typical hydraulic conductivity value for till and silt in the area. As shown in Table 2, aquifer testing in MW-1, which is largely screened in the Qvt, indicated K values in the 10⁻⁵ range. The hydraulic gradient was estimated using the difference between the lake surface elevation and the water elevation for domestic wells screened in the Qc (e.g., domestic well no. 4 for the north section, domestic well no. 3 and no. 8 for the central section, and domestic well no. 5 for the south section). It was assumed that all the wells were screened near the top of the Qc unit, estimated at an average elevation of 475 feet. Maximum and minimum flows were assessed using K values of 10⁻⁴ and 10⁻⁶ cm/sec, respectively. This changed flow values, relative to the average, by a factor of ten

Outflow via the northeast meltwater channel was estimated by assuming the channel was approximately 1,650 feet wide - roughly the distance between wellpoint no. 1 and just south of wellpoint no. 5. The thickness of the Qvr channel, and thus the interface of the channel with the lake, was estimated to extend approximately 10 feet below MW-4 to an elevation of 480 feet. The hydraulic gradient was calculated using the difference between the lake surface level and the groundwater level in MW-4. Since hydraulic conductivity testing was ineffective in this area due to high permeability of the material, an average K value of 10⁻¹ was assumed. Again, maximum and minimum flows were assessed by changing the K values one order of magnitude in each direction from the average - which produced an order of magnitude change in the calculated values.

Outflow via the southwest meltwater channel was estimated by assuming a channel width of 1,200 feet, which is somewhat less than the distance between wellpoints no. 11 and no. 12. The depth of the channel was assumed to be roughly 5 feet beneath the bottom of MW-2 at elevation 495 feet. The hydraulic gradient was calculated using the difference between the lake surface level and the groundwater level in MW-2.

Again the average of the aquifer tests were used for the K value. The maximum and minimum hydraulic conductivity test values were used to calculate the maximum and minimum outflow values from this area.

The total estimated outflow from these three areas of the lake are presented in Table 11. The estimate average outflows through the bottom of Lake Sawyer remained steady throughout the seasons, ranging between 1 to 2 cfs. Conversely, the average outflow via the northeast channel showed considerable seasonal fluctuations, varying from an average of 2 cfs in August to 0.2 cfs in February. As discussed previously, a reduced hydraulic gradient between the lake and the Qvr unit in the northeast during the wet season results in reduced outflow from Lake Sawyer. Finally, based on this analysis it appears as if there is minimal outflow via the southwest channel. This appears largely due to a relatively smaller hydraulic gradient from the lake to the groundwater and a lower hydraulic conductivity.

Nutrient Loading Estimates. The inputs of total nitrogen (TN) and total phosphorus (TP) to Lake Sawyer via the regional groundwater system was assessed by multiplying the average estimated groundwater inflow rate (0.1 cfs) by measured background concentrations of soluble (and thus potentially transportable) forms of these nutrients within the basin. Because of the presence of cultural (i.e., septic tank) influences at the monitoring well locations (see Section VI), background water quality was assumed to be represented by water quality monitoring data within Ravensdale Creek, which drains a relatively pristine watershed in the eastern areas of the basin. Ravensdale Creek also appears to be a predominant recharge source for groundwater inputs to Lake Sawyer. The average TSN and TSP concentrations in Ravensdale Creek observed over the study period were 450 ± 30 ug/L and 7 ± 1 ug/L, respectively.

Based on these data and assumptions, the estimated nutrient inputs to the lake from the regional groundwater system average approximately 50 kgTN/year and 0.8 kgTP/year. The uncertainty in these estimates is large, owing primarily to variabilities in measured and assumed flow parameters discussed above. The true regional groundwater loading rates of these parameters may range from 5 times higher to 15 times lower than the estimated average values.

Summary

As indicated in Table 11, there is a net monthly groundwater discharge from the lake. This result corresponds favorably with the surface water budget developed by Ecology. Using measurements for all inflow and outflow components except groundwater, Ecology calculated a significant unexplained output for June through December, and an insignificant residual value for the January through May period. The surface water budget thus indicated that there were a number of months with unexplained outputs from the lake. Based on this hydrogeologic study of Lake Sawyer, the estimated groundwater outflows can account for at least a portion of, if not all, the unexplained output.

Figure 18 graphically displays the monthly groundwater outflows and the monthly surface water unexplained output (i.e., residuals) for Lake Sawyer. Of note, the large unexplained output in August through December are reflected in the higher groundwater outflow values. Correspondingly, as the lake residuals shrink during the wetter winter months, the estimated groundwater outflows also are considerably reduced.

SEPTIC SYSTEM PERFORMANCE EVALUATION

One of the principal objectives of this study was to evaluate performance characteristics of existing and possible future on-site wastewater disposal systems within the Lake Sawyer basin. As discussed above, fifteen (15) wellpoints and four (4) monitoring wells were installed to accomplish this objective.

Septic System Site Selection Process

The septic systems selected for evaluation were determined based on telephone surveys and interviews with homeowners. Following the homeowner interviews, septic systems were selected for monitoring if they met the following criteria:

► The system had been in operation two years or more. (New systems tend to have limited impact on groundwater because of limited use and slow travel time to downgradient monitoring points).

- ▶ The residence was occupied on a year-round basis.
- ▶ The systems were located on soil types characteristic of the basin.

Based on the above, 15 sites were identified for investigation, and covered a range of hydrogeologic settings.

At each site, a wellpoint was installed within approximately 30 m (100 ft) of the drainfield location in a suspected downgradient direction. The wells were generally completed within the first groundwater zone encountered below the depth of the drainfield (during late fall installations).

As discussed previously in the DATA COLLECTION METHODS section of this report, the wellpoints were located in the field in areas most likely to receive wastewater discharges. These field determinations were based on a consideration of: 1) location of the drainfield; 2) regional groundwater flow directions; and 3) field determinations of chloride concentrations (a wastewater tracer parameter) both in the nearshore lake zone and in several wellpoint test holes driven at each location

For comparative purposes, the four monitoring wells, selected water supply wells, Ravensdale Creek, and Lake Sawyer were utilized as sampling points to assess local upgradient conditions. The specific upgradient locations used in the septic system performance analysis are described below.

Septic System Monitoring Sites: Physical Conditions

Based on available hydrogeologic and engineering data for each of the septic system monitoring sites, the generalized drainfield and groundwater flow system below each site can be described. The flow systems encountered can generally be described as a relatively simple unsaturated vertical discharge into an underlying saturated zone. In the case of drainfields located in the permeable Qvr, effluent flow to the saturated zone is relatively rapid, with minimal diffusion. The opposite is true for the Qvt, due to the considerably lower permeability in this material.

Septic System Performance Characteristics

The approach used in this investigation to assess septic system performance and potential constituent transport rates to Lake Sawyer was based on the determination of local subsurface attenuation parameters (Kerfoot and Skinner, 1981; Gilliom and Patmont, 1982; Patmont et al., 1989). Briefly, the methodology provides estimates of pollutant removals within the soil environment by comparing the relative transport of a conservative septic effluent tracer parameter (i.e., chloride) with the constituents of interest. Such an assessment of attenuation properties relies solely on water quality determinations, and not on estimated (and generally highly uncertain) flow rate estimates. This attenuation-based methodology also incorporates the effects of a variety of pertinent physical processes such as dilution and diffusion.

Estimates of subsurface attenuation evaluation were also based on the assumption that surface transport of wastewater in the vicinity (i.e., within 30 m; 100 ft) of the drainfield is negligible. The assumed absence of surficial wastewater "failures" is indicated by the results of the wastewater leachate detection survey, limited inspections conducted by the Seattle-King County Health Department (Larry Kirschener, King County, personal communication, 1990), and also by additional field reconnaissance activities performed during this study. Although septic system wastewaters ultimately discharge into basin streams and/or into Lake Sawyer, local scale transport (i.e., within 30 m of the drainfield) appears to be generally limited to subsurface groundwater components.

Previous studies of septic system performance conducted in a variety of environmental settings have demonstrated that subsurface pollutant removal is often restricted to the local, and sometimes immediate drainfield vicinity (Dillon and Kirchener, 1975; Jones and Lee, 1977; EPA, 1980a; Gilliom and Patmont, 1982; Johnson and Atwater, 1985; Patmont et al., 1989). Local scale removals of wastewater constituents have generally been attributed to attenuation processes within the unsaturated soil horizon. More limited removals of nitrogen and phosphorus would be expected in the saturated groundwater zone. Based on these data, most of the removal (i.e., attenuation) of wastewater constituents within the Lake Sawyer basin likely occurs within the local unsaturated zone. Attenuation data obtained from the wellpoint and monitoring well network installed within the saturated

zone, therefore, are probably representative of removals occurring after more extended transport through the groundwater.

Wastewater Detection Based on Chloride Tracer Data

As stated above, concentrations of a relatively conservative septic effluent tracer parameter -- chloride -- were used to assess the presence and volumetric fraction of wastewater present within the groundwater samples. The utility of chloride as a tracer parameter for this application has been well established (Kerfoot and Skinner, 1981; Gilliom and Patmont, 1982; Patmont et al., 1989). Within the Lake Sawyer basin, background concentrations of chloride typically ranged from approximately 1 to 3 mg/L, based on data collected from Ravensdale Creek and from monitoring wells and water supply wells located in generally undeveloped areas. In contrast, wastewater effluent within the basin (sampled at the Black Diamond Wastewater Treatment Plant and in basin septic tanks) averaged approximately 20 mg/L.

The accuracy of the chloride tracer approach to detect the presence of wastewater effluent in a given groundwater sample is partially dependent on the determination of local upgradient chloride concentrations not affected by the septic system discharge. The rather complex hydrogeologic conditions within the Lake Sawyer basin somewhat complicate this determination. Specifically, consideration must be given to the following factors: 1) groundwater flow direction within the saturated zone (i.e., to or from Lake Sawyer); 2) possible dispersion of wastewater flows during vertical transport through the unsaturated zone; and 3) different geologic units within the basin, which may exhibit differences in background chloride concentrations. Table 12 presents a summary of the geologic units, flow directions, and measured chloride concentrations at the various monitoring points used in this study.

Recessional Outwash Inflow Zone

The first flow system considered in this analysis is discharge to Lake Sawyer through the Qvr. Most of the monitoring points within the basin are located within this zone along the eastern shore of Lake Sawyer. Hydrogeologic analyses of the basin suggest that much of the discharge within this unit originates in (or is otherwise represented by) flows from Ravensdale Creek, since leakage from the creek contributes to recharge

of the Qvr. Little development occurs within the Ravensdale Creek watershed. Accordingly, upgradient chloride concentrations measured in Ravensdale Creek were initially assumed to be representative of upgradient (pre-septic system release) conditions within this zone.

The cumulative frequency distributions of chloride concentrations measured within Ravensdale Creek and within wells of the Qvr zone are depicted on Figure 19. The data reveal that many of the chloride concentrations measured in wellpoints (and monitoring wells) completed within this zone significantly (P < 0.01; t-test) exceeded creek concentrations. The presence of elevated chloride concentrations in these wells is indicative of the presence of wastewater leachate.

Elevated chloride concentrations were also consistently observed in MW-3. Although this well was initially intended to serve as an upgradient reference well, access restrictions necessitated the placement of MW-3 within 30 m (100 ft) of an existing septic system drainfield. Because chloride concentrations in MW-3 were significantly (P < 0.01; t-test) elevated above creek concentrations, and also above many other Qvr wells, the presence of wastewater leachate in this well is indicated. Accordingly, MW-3 was designated as a downgradient septic system monitoring well for the purposes of this study.

For the purposes of this analysis, therefore, upgradient groundwater quality in the Qvr Inflow zone to Lake Sawyer was assumed to be represented by Ravensdale Creek. The upper 99th percentile chloride concentration in the creek was approximately 2.9 mg/L (based on t-test; depicted on Figure 19). Groundwater samples collected from this zone which exhibited chloride concentrations in excess of this value were interpreted to contain a statistically significant (P < 0.01) volumetric fraction of wastewater.

Recessional Outwash Outflow Zone

The second flow system considered in this analysis was discharge from Lake Sawyer via the Qvr zone. Although chloride concentrations in most of the Qvr outflow zones were equivalent to concentrations measured in Lake Sawyer, several samples nevertheless exhibited significantly (P < 0.01; t-test) elevated chloride concentrations relative both to Lake Sawyer and Ravensdale Creek (Figure 20). For the purposes of this evaluation, groundwater samples collected from this

zone which exhibited chloride concentrations in excess of 2.9 mg/L were interpreted to contain a statistically significant (P < 0.01) volumetric fraction of wastewater.

It should be noted that the concentrations of chloride, TSP, and TSN in the majority of wellpoints located in the lake **outflow** zone were nearly identical to those measured in Lake Sawyer. The correspondence of water quality conditions between these sampling locations supports the use of lake water quality data to define upgradient conditions within this flow pathway.

The primary outflow well which exhibited elevated chloride concentrations was MW-4, located downgradient of a developed residential area northeast of Lake Sawyer. One sample collected from WP-4 exceeded the chloride threshold concentration. Accordingly, this groundwater sample was interpreted as a downgradient observation for the purposes of the septic system performance evaluation.

Other Discharge Zones

Although other groundwater discharge zones such as the Vashon till unit exist in the Lake Sawyer area, these areas were not targeted for evaluation in this study. Based on the regional hydrogeologic evaluation, wastewater migration is most likely to occur only via the Qvr discussed above. Effluent migration via discharge through other units is not likely to be a predominant transport pathway to Lake Sawyer.

Based on the available hydrogeologic data, significant septic system discharges through or above the Qvt on the western and northern shores of Lake Sawyer is considered very unlikely. This conclusion is based on the groundwater flow direction consistently directed away from Lake Sawyer, and the lack of an identified perched groundwater zone, as discussed in the HYDROGEOLOGIC CONDITIONS section of this report.

Constituent Attenuation Methodology

Based on the chloride tracer data, the equivalent volumetric fraction of undiluted wastewater present at a given groundwater sampling site can be estimated as follows:

Volumetric Fraction =
$$\frac{Cl_d - Cl_u}{Cl_z - Cl_u}$$

where Cl denotes chloride concentration in mg/L, and subscripts u, d, and e denote upgradient, downgradient, and effluent values, respectively. Using this formulation, the volumetric percentage of effluent at all sites which exhibited a significant local increase in chloride levels was calculated.

The wastewater volume calculations are summarized in Table 12 and reveal that some of the groundwater samples were composed of substantial quantities of septic system effluent (up to approximately 30 percent by volume). Considering the observed variability in the background chloride data, effluent levels of approximately one (1) percent by volume in the groundwater samples were generally required to reliably determine wastewater contributions. Lower volumetric fractions of effluent were not distinguishable from apparently random variations in background chloride levels. Most of the potentially downgradient monitoring locations, however, contained more than this minimum detectable effluent fraction.

For the downgradient groundwater samples which contained a significant wastewater component, fractional constituent (C) removals within the soil system were calculated as follows:

C Removal = 1 -
$$[C]_d$$
 - $[C]_u$
{ $[C]:Cl}*{Cl_d - Cl_u}$

where [C] denotes the measured constituent (e.g., TSP) concentration and [C]:Cl denotes the constituent to chloride ratio present in wastewater as it is discharged through the septic tank/drainfield system.

The [C]:Cl ratios were similar among a variety of septic tanks and wastewater collection systems sampled in the Lake Sawyer area, even though the concentrations of individual constituents varied by more than an order of magnitude. (Samples collected from ST-1 were excluded from this evaluation owing to sampling uncertainties — e.g., chloride concentrations, in one instance, not significantly elevated above background; it is likely that the location receives only grey water (e.g., laundry) discharges). The relationships between TP and TN levels with chloride concentrations in the wastewater samples are depicted on

Figures 21 and 22, respectively (data are presented in Appendix B). The mean and standard error of the observed constituent ratios for these key parameters (based on regression analyses which correct for background values -- see Gilliom and Patmont, 1982) are summarized below:

TP:Cl (wt:wt) =
$$0.282 \pm 0.027$$

TN:Cl (wt:wt) =
$$1.73 \pm 0.39$$

The mass balance formulation presented above examines differences between upgradient and downgradient mobile constituent (e.g., TSP and TSN) concentrations, and compares the observed differences with expected (unattenuated) values based on chloride tracer data. Comparisons of downgradient ([C]_d) and upgradient ([C]_u; Ravensdale Creek; Appendix B) TSP concentrations in the Qvr inflow zone are depicted on Figure 23. The elevated TSP concentration in many of the downgradient samples, presumably due to wastewater contributions, is evident in this comparison.

Upgradient and downgradient TSN concentrations in the Qvr inflow zone are depicted on Figure 24. Although many of the downgradient groundwater samples exhibited TSN concentrations above those measured in Ravensdale Creek, several of the samples nevertheless contained TSN concentrations significantly (P < 0.01; t-test) below the assumed upgradient values. (A similar condition was not observed with the TSP data).

The presence of lower TSN concentrations in downgradient groundwater samples may be indicative of nitrogen removals which occurred during groundwater transport between the assumed point of recharge (Ravensdale Creek) and the various downgradient wellpoints and monitoring wells. Nitrogen uptake by plants has been reported in a variety of shallow groundwater systems in the Northwest, owing to the general deficiency of this plant nutrient in regional soil systems (Gessel et al., 1969; Harper-Owes, 1985). Based on these data, it is likely that the Ravensdale Creek TSN concentrations overestimate local upgradient TSN concentrations in the immediate vicinity of the septic systems evaluated in this study. The true upgradient TSN concentration in this case may range from a lower bound of zero to an upper bound

defined by Ravensdale Creek. This "background" uncertainty will be assessed further in the performance calculations presented below.

Constituent Attenuation Results

Based on the available data and the mass balance removal model described above, constituent removals associated with each groundwater sample which significantly (P < 0.01; t-test) exceeded background chloride concentrations were calculated. The results of the calculations are presented on Figures 25 and 26, respectively.

In order to address uncertainties in the background (upgradient) concentrations of these constituents, two sets of calculations were performed. The first assumed a local upgradient concentration of zero as a lower bound. The second (upper-bound) calculation was based on the measured Ravensdale Creek or Lake Sawyer concentrations, depending upon which flow system was represented (see above discussion). Both sets of calculations are depicted on Figures 25 and 26.

The TP removal estimates were relatively insensitive to the assumed upgradient concentration (Figure 25). Individual sample TP removal estimates ranged from a low of approximately 50 percent to a high of 100 percent. Lower TP removals tended to occur in areas with a relatively shallow depth to water below the drainfield trench, although a full statistically-based analysis of this condition could not be performed with the available data. Overall, more than 60 percent of those samples with a significant (P < 0.01) volumetric fraction of effluent were associated with an apparent TP attenuation statistically equivalent (P > 0.10) to 100 percent removal. Average P removal over the study period, based on all of the attenuation calculations, was 93.8 ± 2.5 percent. Average P removals measured during this study are similar to values reported in the Pine Lake and Lake Chelan basins (Gilliom and Patmont, 1982; Patmont et al., 1989).

In contrast to the TP removal results, TN attenuation estimates varied depending upon the assumed background concentration (Figure 26). Average N removal over the study period, based on all of the attenuation calculations, was 71.8 ± 4.8 percent assuming a zero background concentration, and 85.4 ± 4.7 percent assuming the creek/lake values represent local upgradient conditions. Individual sample TN removal estimates ranged from a low of approximately 20

percent to a high of 100 percent. The overall average TN attenuation, assuming equal weight is given to both background concentration assumptions, is approximately 78.6 ± 10.7 percent. Again, these measured TN removals are similar to those observed in the Pine Lake and Lake Chelan basins (Gilliom and Patmont, 1982; Patmont et al., 1989; R. Gilliom, USGS, unpublished data).

Septic System Loadings

The wastewater characterization and attenuation data discussed above formed the basis for an assessment of constituent loadings to Lake Sawyer from on-site septic systems. In the areas of the watershed where groundwater flow is directed toward the lake (defined generally as the drainage area extending from wellpoint no. 6 clockwise to wellpoint no. 11; see Figure 27), approximately 80 ± 4 homes currently (1989-1990) utilize septic systems (based on Hart Crowser field observations). Most of these dwellings appeared to be full-time residences. The septic systems monitored were assumed to be representative of these basin dwellings.

Total loadings of TP and TN were calculated based on the following:

Load = HOME x DWEL x
$$Cl_L x [C]:Cl x (1 - C_R)$$

where HOME denotes the total number of basin homes within the groundwater catchment zone (80 \pm 4); DWEL denotes the average people per household in the area (3.0 \pm 0.2 based on Puget Sound Council of Governments unpublished data, 1989; Census tracts #316 and #320.01); Cl_L denotes the characteristic per capita chloride excretion rate (2.2 \pm 0.5 kg Cl/person-year; Metcalf and Eddy, 1972; Brandes, 1978), [C]:Cl denotes the constituent to chloride ratios discussed previously, and C_R denotes the overall average fractional constituent removal (93.8 \pm 2.5 percent for TP; 78.6 \pm 10.7 percent for TN). The results of these current (1989-1990) septic system loading calculations are summarized below:

TP Loading = $9 \pm 4 \text{ kgP/year}$ TN Loading = $190 \pm 90 \text{ kgN/year}$

Uncertainty Analysis

Overall, the total coefficients of variation (standard error divided by the mean) associated with the average annual wastewater TP and TN loading calculations were similar at approximately ± 50 percent. The various sources of uncertainty, expressed as a percentage of the total variance contributing to the loading calculations, are summarized in Table 13.

In both cases (i.e., TP and TN), the major source of uncertainty contributing to the total variance in the loading calculations was groundwater sampling variability, particularly site-to-site variations in parameter attenuation (Table 13). This variability is represented by those ten (10) septic system monitoring sites sampled during the study which contained detectable quantities of wastewater (Table 12). Variations in the per capita chloride excretion load (Cl_L) also contributed significantly to the overall loading uncertainty, representing approximately 22 to 24 percent of the total variance.

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Table 1 - Groundwater and Surface Water Elevation Data (page 1 of 2)

Station	Well Depth	TOC Elevation	8/7-8/89	9/6/89	10/3/89	11/2/89	12/5-6/89	1/8/90 (Storm Event)	1/11/90
Deep Monitoring Wells:									
MW-1	23.0	535.64	524.81	524.19	523.33	522.83	530.22	NM	529.88
MW-2	70.0	572.46	516.08	515.33	514.31	513.71	517.16	NM	519.36
MW-3	48.0	560.84	523.74	523.42	522.78	522.35	527.11	528.74	535.86
MW-4	48.0	549.29	509.32	511.06	509.84	508.90	512.78	515.54	518.29
Wellpoints:							•		
WP-1	6.1	521.44	<515.3	<515.3	<515.3	<515.3	517.39	517.32	518.31
WP-2	6.5	521.74	517.77	517.15	516.39	<515.2	518.64	518.55	519.40
WP-3	9.4	522.69	513.60	<513.3	<513.3	<513.3	<513.3	<513.3	518.31
WP-4	6.0	520.56	515.12	514.53	514.46	<514.6	515.49	NM	518.40
WP-5	6.7	520.64	<513.9	<513.9	<513.9	<513.9	515.53	NM	519.46
WP-6	6.5	521.13	517. 9 7	517.37	516.56	516.10	518.83	NM	519.43
WP-7	5.2	520.68	518.28	517.38	516.54	516.08	518.96	NM	519.59
WP-8	3.9	520.90	518.01	517.34	<517.0	<517.0	518.99	NM	519.76
WP-9	5.4	521.82	518.03	517.32	516.58	<516.4	519.13	NM	519.86
WP-10	6.4	521.75	518.02	517.56	516.55	516.08	519.08	NM	519.82
WP-11	4.9	520.91	518.22	517.36	516.55	516.09	519.33	NM	519.61
WP-12	3.2	519.55	517.94	517.42	516.27	<516.4	519.05	518.97	NM
WP-13	5.1	521.93	<516.8	<516.8	<516.8	<516.8	<516.8	<516.8	515.58
WP-14	7.1	519.38	513.29	<512.3	<512.3	<512.3	516.40	516.35	NM
WP-15	4.8	520.27	<515.5	<515.5	<515.5	<515.5	518.09	<515.5	518.14
Domestic Wells:									
DOM-1	24	540.10	518.10	516.9 9	<516	<516	518.87	NM	519.73
DOM-2	50	526.40	517.88	517.44	516.42	516.02	518.95	NM	516.51
DOM-3	?	541.27	502.99	504.23	503.36	502.75	507.12	NM	512.76
DOM-4 (4B07)	80	530.36	NM	480.19	479.40	480.37	483.70	NM	490.65
DOM-5 (3P02)	?	540.96	513.51	512.90	512.07	511.59	514.79	NM	515.54
DOM-6	12	526.68	518.35	517.24	516.66	516.15	519.16	NM	518.74
DOM-7	?	553.23	518.14	517.54	516.66	516.22	519.46	NM	520.12
DOM-8	80	524.19	505.64	505.61	504.81	504.23	508.00	NM	511.13
Staff Gages:		levation of 0.00 M							
1a) Rock Creek-Old	NA	517.57	NM	519.29	518.85	519.54	>520.5	521.17	NM
1b) Rock Creek-New	NA	519.28	NM	NM	NM	NM	NM	NM	NM
2) Eaton's Dock	NA	511.37	NM	517.35	516.64	516.18	519.06	519.00	519.55
3) Covington Creek-Post*	NA	517.68	NM	515.43	515.41	515.32	517.18	516.85	>517.7
4) Covington Creek-Dam	NA	516.53	NM	517.37	516.63	516.27	519.02	518.98	519.41

Notes:

- 1) All elevations in feet (MSL). Well depths are relative to ground surface.
- 2) Elevations denoted with "<" indicate a dry wellpoint. Elevation listed is the elevation of the wellpoint bottom.
- 3) Elevations denoted with ">" indicate a submerged staff gage. Elevation listed is the elevation of the top of the staff gage.
- 4) TOC: Top of casing; NA: Not applicable; NM: No measurement taken.
- 5) *: Top of metal fence post is "0.00 Mark".

Table 5.1

Station	2/2/90	3/13-14/90	4/3/90	5/1/90
Deep Monitoring Wells:				
MW-1	528.97	528.96	528.33	528.97
MW-2	518.16	518.10	517.86	517.56
MW-3	533.31	531.56	530.25	527.14
MW-4	518.40	517.52	516.88	515.71
Wellpoints:				
WP-1	517.32	516.22	515.63	<515.3
WP-2	518.91	518.45	518.71	518.21
WP-3	518.34	517.45	517.03	515.67
WP-4	518.38	517.71	517.26	516.28
WP-5	518.83	518.09	517.95	516.62
WP-6	518.97	518.79	518.63	518.54
WP-7	519.10	518.90	518.93	518.69
WP-8	519.11	518.92	518.73	518.67
WP-9	519.62	518.92	518.88	518.35
WP-10	519,03	518.93	518.90	518.70
WP-11	519.55	518.99	518.78	518.83
WP-12	518.95	518.88	518.66	518.72
WP-13	<516.8	<516.8	515.20	<516.8
WP-14	516.77	515.64	515.48	515.63
WP-15	<515.5	<515.5	<515.5	<515.5
Domestic Wells:		•		
DOM-1	518.97	518.80	518.60	<516
DOM-2	518.94	518.82	518.72	518.56
DOM-3	510.13	511.06	508.27	507.26
DOM-4 (4B07)	490.61	489,69	488.96	488.26
DOM-5 (3P02)	514.79	514.57	514.39	514.28
DOM-6	519.06	518.95	518.81	518.71
DOM-7	519.29	519.23	519.01	518.96
DOM-8	510.35	510.27	509.32	509.03
Staff Gages:				
1a) Rock Creek-Old	520.71	520.67	519.97	520.29
1b) Rock Creek-New	520.72	520.68	519.91	520.29
2) Eaton's Dock	519.01	518.95	518.77	518.68
Covington Creek-Post*	515.23	517.5	NM	516.9
4) Covington Creek-Dam	>519.5	518.94	518.78	518.76

Notes:

- 1) All elevations in feet (MSL).
- 2) Elevations denoted with "<" indicate a dry wellpoint. Elevation listed is the elevation of the wellpoint bottom.
- 3) Elevations denoted with ">" indicate a submerged staff gage. Elevation listed is the elevation of the top of the staff gage.
- 4) TOC: Top of casing; NA: Not applicable; NM: No measurement taken.

70618502

Table 2 - Aquifer Testing Results

		Hydraulic Conductivity in cm/sec				
SITE	Screened Interval	Pumping Test	Bail Test	Slug Test		
Domestic Well No. 4	Qc	10 ⁻³ *				
Domestic Well No. 8	Qc	5x10 ⁻⁵ *				
MW-1	Qvr/Qvt		3x10 ⁻⁵ *	5x10 ⁻⁵ ***		
MW-2	Qvr		4x10 ⁻⁵ *	9x10 ⁻⁴		
MW-3	Qvr		4x10 ⁻⁴ *	7x10 ⁻³		
MW-4	Qvr		** (>10 ⁻³)	** (>10 ⁻³)		

- * indicates depth of aquifer was assumed in order to convert transmissivity data to hydraulic conductivity values.
- ** indicates water table recovered too rapidly to monitor, thus hydraulic conductivities are probably greater than 10⁻³ cm/sec.
- *** MW-1 is completed partially in both Qvr and Qvt. Early time slug test data indicate a higher K value of approximately 10⁻³, and may indicate the contribution of Qvr. The complete data set indicates K = 5x10⁻⁵ cm/sec, indicative of Qvt.

Table 3 - Groundwater Field Measurements: August 1989

	Tomporatura	Electrical	,	Dissolved
1 1	Temperature	Conductivity		Oxygen
Location	in oC	in umhos	рН	in mg/L
Monitoring Wells				
MW-1	13	120	6.3	3.9
MW-2	11	90	7.0	2.0
MW-3	11	90	6.3	7.8
MW-4	15	110 -	6.2	3.6
Wellpoint #				
1	Dry			
2	22	130	5.9	2.6
3	Dry			
4	Dry			
5	Dry			
6	18	140	6.4	2.8
7	19	110	5.4	1.5
8	20	80	5.6	16
9	16	120	5.9	2.7
10	17	110	5.9	3.0
11	15	100	70	2.8
12	20	130	5.9	2.0
13	Dry			
14	Dry			
15	Dry			

Table 4 - Groundwater Field Measurements: December 1989

		Electrical		Dissolved
	Temperature	Conductivity		Oxygen
Location	in oC	in umhos	pН	in mg/L
Monitoring Wells				
MW-1	11	250	7.5	8.8
MW-2	10	150	7.7	6.8
MW-3	10	130	7.5	7.3
MW-4	10	160	7.5	8.4
Wellpoint #				
1	10	210	75	10.9
2	12	190	7.5	4.9
3	Dry			
4	10	210	7.8	8.8
5	11	170	7.8	12.3
6	12	120	7.2	4.7
7	11	120	6.9	2.4
8	11	90	7.1	10.4
9	11	280	75	3.0
10	11		7.7	4.0
11	11	150	7.0	8.3
12	9	150	7.6	6.2
13	Dry			
14	10	270	7.4	2.0
15	9	160	6.8	10 6
Others				
Community Well	10	180	8.5	2.4

Table 5 - Groundwater Field Measurements: March 1990

		Electrical		Dissolved
	Temperature	Conductivity		Oxygen
Location	in oC	in umhos	pН	in mg/L
Monitoring Wells				
MW-1	7	360	7.2	
MW-2	. 8	170	7.2	
MW-3	11	90	7.2	
MW-4	10	150	7.1	
Wellpoint #				
1	7	170	6.1	2.9
2	7	150	6.1	1.2
3	8	120	5.9	6.4
4	8	140	6.1	7.2
5	7	120	6.4	4.0
6	9	160	6.0	5.2
7	10	120	6.2	1.2
8	9	100	6.0	3.7
9	8	120	6.1	3.2
10	9	110	6.1	5.6
11	8	140	7.0	
12	7	130	7.4	
13	Dry			
14	8	140	73	
15	Dry		•	
Septic Systems				
ST-1				
ST-2	9	190	5.8	4.1
Others				
Community Well	10	170	6.3	5.8

Table 6 - Groundwater Field Measurements: May 1990

		Electrical		Dissolved
	Temperature	Conductivity		Oxygen
Location	in oC	in umhos	рН	in mg/L
A F 15 11 3 A F - 13 -				
Monitoring Wells		040		5,6
MW-1	9	310	6.3	
MW2	9	130	5.6	2.3
MW-3	11	100	6.4	9.5
MW-4	13	120	6.4	10.3
Wellpoint #				
1	Dry			
2	11	190	6.4	2.7
3	12	260	6.7	4.2
4	12	190	7 2	4.8
5	14	160	7.5	4.2
6	11	140	7.2	0.7
7	13	110	68	04
8	12	100	72	09
9	12	160	70	0.5
10	13	130	70	0.5
11	11	80	6.1	5.9
12	13	130	6.4	2.1
13	Dry			
14	16	130	6 4	5.3
15	Dry			
Septic Systems				
ST-1	14	1080	7.8	0.8
ST-2	18	440	72	3.6
Others				
Community Well	10	150	8.3	6.0

Table 7 - Groundwater Sampling Results: August 1989

	Total Soluble	Soluble Reactive	Nitrate-nitrite	Ammonia	Total Soluble	
	Phosphorus	Phosphorus	as N	as N	Nitrogen as N	Chloride
Location	in ug/L	in ug/L	in ug/L	in ug/L	in ug/L	in mg/L
Monitoring Wells		45.0	000	10	640	2.80
MW-1 MW-2	4.4 J	15.3 56.1	628 10	19 3	642 50	185
MW-3	39.0 B 9.5 J	219	3,071	8	3,225	590
MW-4	9.5 J 8.2 J	21.0	222	11	3,225	2.45
Wellpoint #						
1						
2	27.4	25.6	6	295	295	2.45
- 3						
4						
5						
6	20.0	21.5	5	93	145	2.90
7	12.1	11.7	6	9	59	643
8	12.3	10.9	12	(2)	58	2.80
9	(6.0) J	10.2	223	(0)	516	5.45
10	230.4	226.6	1,529	25	1,510	3.90
11	15.7	18.7	267	(0)	378	3.13
12	14 8	125	6	33	118	2.53
13						
14						
15						
Others						
Domestic Well	38.5 B	12.7	422	25	574	3.30
"J" - indicates that the associated numerical value is an estimated quantity						

Table 8 - Groundwater Sampling Results: December 1989

	I							
	Total Soluble	Soluble Reactive		Ammonia	Total Soluble			
	Phosphorus	Phosphorus	as N	as N	Nitrogen as N	Chloride		
Location	in ug/L	in ug/L	in ug/L	in ug/L	in ug/L	in mg/L		
Monitoring Wells								
MW-1	5.2 J	77.6	2,050	5	2,666	325		
MW-2	50.6 B	605	99	16	544	2.30		
MW-3	12.0 J	18.4	442	(3)	2,165			
MW-4	175 J	19.2	553	3	575	2.85		
Wellpoint #								
1	21.8	18.7	(5)	(6)	123	2.80		
2	12.5	9.7	258	21	427	2.00		
3								
4	25 1 J	14.5	272	86		7.00		
5	39	8.8	25	(3)	166	2.65		
6	(6.1) J	8.9	66	(1)	246	4.08		
7	5.2	4.4	20	4	94	3.40		
8	(1.9) J	8.4	351	1	341	4.20		
9	4.4	8.4	1,722	47	1,507	585		
10	160.3 B	165.3	2,019	92	1,924	5.28		
11	6.2	8.4	1,132	1	690	175		
12	9.7 J	97	51	(2)	305	305		
13								
14	18.8	23.5	35	371	505	425		
15	13.5	8.1	24	(1)	175	2.60		
Others								
Domestic Well								
Community Well	112.4	116.2	39	13	38	1.35		
Community 11611	1 1 <u>2</u> - 7	110.2	•	.5	30			
	"J" - indicates that the associated numerical value is an estimated quantity							

Table 9 - Groundwater Sampling Results: March 1990

	Total Soluble	Soluble Reactive	Nitrate-nitrite	Ammonia	Total Soluble	
	Phosphorus	Phosphorus	as N	as N	Nitrogen as N	Chloride
Location	in ug/L	in ug/L	in ug/L	in ug/L	in ug/L	in mg/L
				· · · · · · · · · · · · · · · · · · ·		
Monitoring Wells						
MW-1	318.6	3 72.0	590	21	1,280	5 30
MW-2	48.9 E	3 48.2	87	50	233	1 85
MW-3	12.8 E	3 160	733	3	3,249	5.25
MW-4	28.8	3 19.6	488	21	821	2.35
Wellpoint #						
1	71	6.4	28	6	116	1.85
2	24.5	10.3	107	33	186	2.10
3	5.1	5.4	459	2	505	2.15
4	3.0	3.9	385	7	443	2.20 *
5	42	4.6	286	(1.3)	349	1 83
6	3.3 .	4.6	163	3	198	3 25
7	9.3	10.0	816	20	782	7.75
8	7.6 E	7.4	15	23	33	2.75
9	16	8.4	747	30	701	4.25
10	91.5 E	81,2	782	4	709	3.60
11	4.2	9.2	1,040	38	1,106	2 85
12	47	10.5	267	2	378	1.95
13						
14	89.8 E	16.5	73	239	464	2.70
15						
Septic Systems						
ST-1	22,892 B	21,900	(3)	57,348	63,980	12.35 J
ST-2	378 B	348	1,530	9,817	14,105	5.40
Others						
Domestic Well						
Community Well	105	119	43	13	29	1.00
	"J" - indicates that the associated numerical value is an estimated quantity					

Table 10 - Groundwater Sampling Results: May 1990

	Total Soluble		Total Soluble	
	Phosphorus		Nitrogen as N	Chloride
Location	in ug/L		in ug/L	in mg/L
Monitoring Wells				
MW-1	197.7	В	750	1.79
MW-2	47.3	В	1,250	2.40
MW-3	18.3	В	3,652	5.45
MW-4	242	В	1,463	3.40
Wellpoint #				
1				
2	12.7		120	2.15
3	100		445	2.70
4	9.3		374	2.15
5	26.9		284	2.15
6	12.4		105	2.50
7	212		201	9.25
8	10.5		56	2.65
. 9	5.6		236	2.88
10	133.4	В	743	408
11	71		1,139	3,30
12	6.3		135	2.30
13				
14	31.5	В	461	2.70
15				
Septic Systems				
ST-1	10,812	В	65,382	0.85
ST-2	4,180		30,556	15.65
Others				
Domestic Well				
Community Well	108.3		71	1.90

Table 11 - Average Groundwater Inflow and Outflow Estimates

Month	Average Inflow in cfs	Average Outflow in cfs					
August	0.05 (0.01 - 0.3)	4 (0.5 - 40)					
September	0.05 (0.01 - 0.3)	3 (0.5 - 35)					
October	0.05 (0.01 - 0.3)	4 (0.5 - 35)					
November	0.05 (0.01 - 0.3)	4 (0.5 - 35)					
December 1989	0.1 (0.01 - 0.5)	3 (0.5 - 35)					
January 1990	0.2 (0.01 - 1)	2 (0.2 - 15)					
February	0.2 (0.01 - 1)	1 (0.1 - 15)					
March	0.2 (0.01 - 1)	2 (0.2 - 15)					
April	0.1 (0.01 - 0.5)	2 (0.2 - 20)					
May	0.1 (0 01 - 0.5)	2 (0.2 - 20)					

^{() -} indicates potential range of flows

Table 5.4

Table 12 - Summary of Lake Sawyer Septic System Phosphorus and Nitrogen Removal Calculations

							Average						1	····					
ì	Geologic	Flow to	Chloride					TSP in u		TSP Removal				TSN in	ug/L		TSN Removal		
Location	Unit	Lk. Sawyer	Mean	+/-	S.E.		Fraction(a)	Mean	+/-	S.E.	Mean	+/-	S.E.	Mean	+/-	S.E.	Mean	+/-	S.E.
RAV-CK	Qvr	·In	2.16	+/-	0.08			7.0	+/-	0.8				453	+/-	31			
LK SAWYER		_	2.24	+/-	0.05			13.0	+/-	1.1				295	+/-	79			
MW-1	Qvi	In	3.29	+/-	0.74			131.5	+/-	77.2				1.335	+/-	465			ļ
MW-2	Qvr	Out	2.10	+/-	0.15			46.5	+/-	2.6				519	+/-	264			;
MW-3	Qvr	In	5.53	+/-	0.19	•	21%	13.2	+/-	1.9	99%	+/-	0%	3,073	+/-	318	46%	+/-	9%
MW-4	Qvr	Out	2.76	+/-	0.24	•	3%	19.7	+/-	4.5	94%	+/-	0%	811	+/-	235	47%	+/-	5%
 WP-1	Qvr	Out	2.33	+/-	0.47			14.5	+/-	7.3				120	+/	4			
WP-2	Qvr	Out	2.18	+/	0.10			19.3	+/-	3.9				257	+/-	67			
WP-3	Qvr	Out	2.43	+/	0.27	:		7.6	+/-	2.4				475	+/-	30			
WP-4	Qvr	Out	3.78	+/-	1.61	. •	10%	12.5	+/-	6.6	99%	+/-	n=1	392	+/-	26	97%	+/-	n=1
WP-5	Qvr	Out	2.21	+/-	0.24			11.7	+/-	7.6		-		266	+/-	54	•	•••	
WP-6	Qvr	In	3.18	+/-	0.34	*	6%	7.4	+/-	5.6	100%	+/-	196	174	+/-	31	100%	+/-	6%
WP-7	Qvr	In	6.71	+/-	1.24	•	28%	12.0	+/-	3.4	99%	+/	0%	284	+/-	169	100%	+/-	7%
WP-8	Qvr	In	3.10	+/-	0.37	•	6%	7.1	+/-	3.2	100%	+/-	n=1	122	+/-	73	97%	+/-	n=1
WP-9	Qvr	In	4.61	+/-	0.67	•	15%	1.4	+/-	2.6	100%	+/-	096	740	+/-	273	87%	+/-	9%
WP-10	Qvr	In	4.22	+/-	0.37	•	13%	153.9	+/-	29.2	73%	+/-	6%		+/-	298	73%	+/-	1196
WP-11	Qvr	In	2.76	+/-	0.35	*	4%	8.3	+/-	2.5	97%	+/-	2%	828	+/-	182	72%	+/-	24%
WP-12	Qvr	Out	2.46	+/-	0.23		į	8.9	+/-	2.2				234	+/-	64			
WP-13	Qvt	Out		Dry			·		Dry						Dry				
WP-14	Qvt	Out	3.22	+/-	0.52	(b)		46.7	+/	21.9				477	+/-	14	ļ		
WP-15	Qvt	Out	2.60		n=1			13.5		n=1				175		n=1			
DW-6	Qvr	In	3.30		n=1		7%	38.5		n=1	89%		n=1	574		n=1	82%		n=1
cw .	?	l n	1.42	+/-	0.26			108.6	+/-	2.1]		•	46	+/	13]		••

NOTES:

a) Average effluent fraction calculated based on an average effluent chloride concentration of approximately 18 mg/L (see Figures 21 and 22).

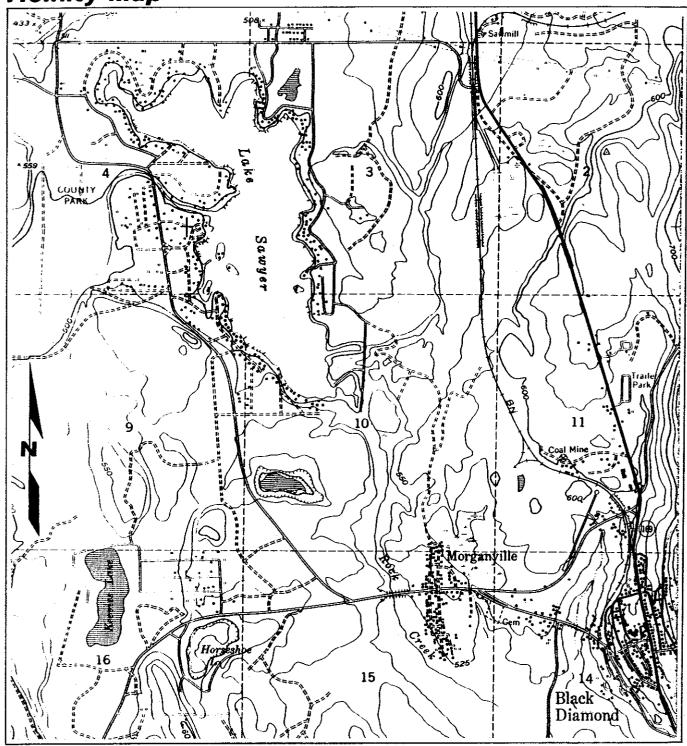
b) Relative to the Qvt background well (MW-1), chloride concentration in these wells are not significantly elevated.

[&]quot;"" denotes that the measured chloride concentration was significantly (P<.05; t-test) greater than the corresponding reference value, indicating a detection of wastewater at that location.

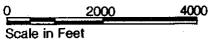
Table 13 - Septic System Loading Uncertainties

Parameter	Percent of TSP Loading Uncertainty	Percent of TSN Loading Uncertainty			
Removal: Background Estimate	3	17			
Sampling Variability	67	31			
[C]:Cl Ratio	4	23			
Chloride Load (Cl _L)	22	24			
People/Dwelling (DWEL)	1	1			
Total Dwellings (HOME)	4	4			

Vicinity Map

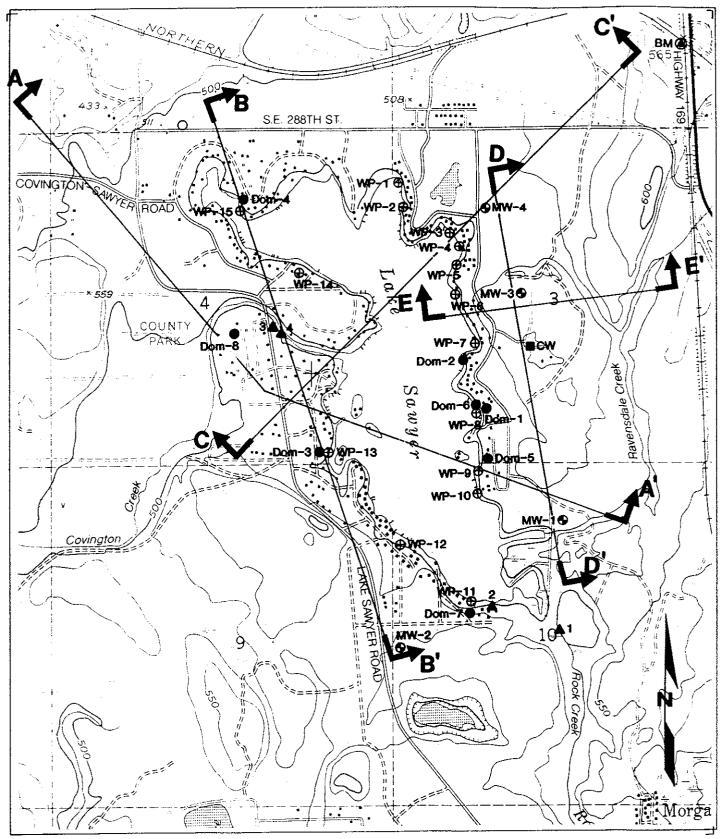


Note: Base map prepared from USGS 7.5-minute quadrangle of Black Diamond, Washington.





Well Location Map



♠ MW-1 Monitoring Well Location and Number
 ♠ WP-1 Wellpoint Location and Number
 ♠ Staff Gage Location and Number
 ♠ Dom-1 Domestic Well Location and Number
 ♠ CW Community Well Location
 ♠ BM Benchmark
 ♠ CM Springs Well No. 1

Cross Section Location and Designation

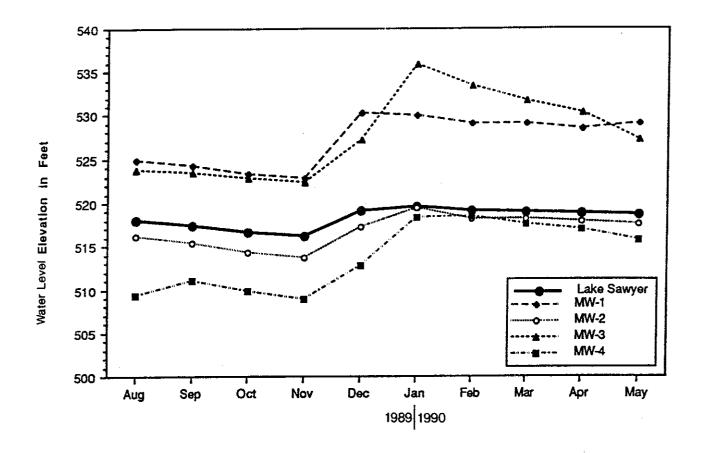
Note: Base map prepared from U.S.G.S. 7.5 minute quadrangle of Black Diamond, Washington:

> 9 1500 3000 Scale in Feet

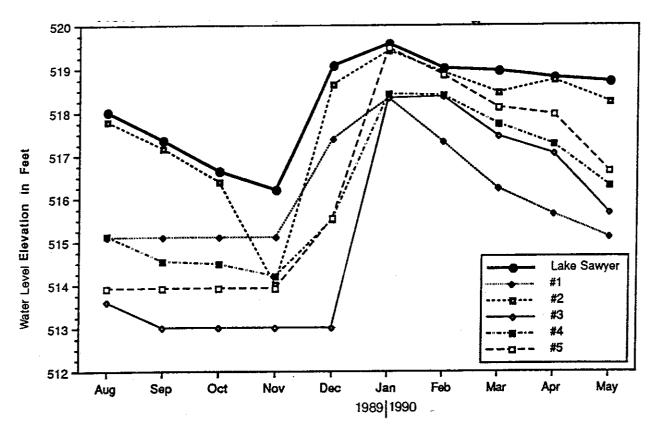
HARTCROWSER
J-2484 4/90
Figure 2

Figure 3.1

Hydrographs for Monitoring Wells

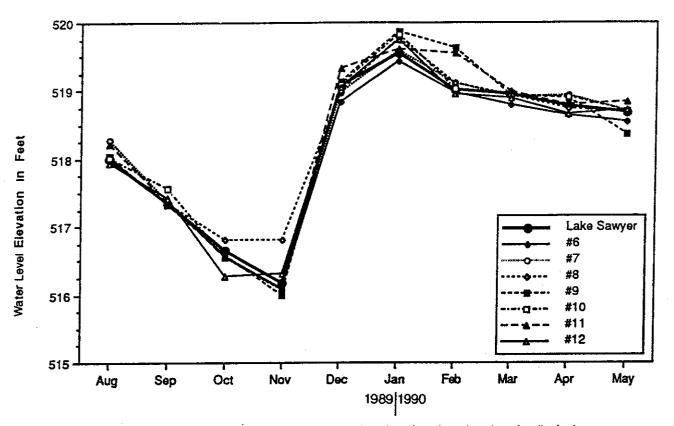


Hydrographs for Wellpoints #1 through #5 Northeast Side of Lake



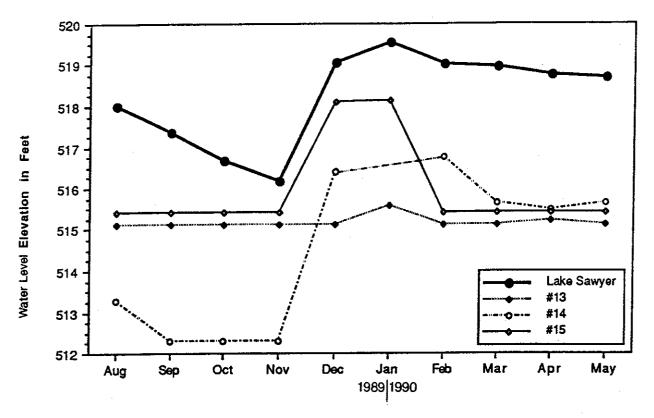
Note: For months when wellpoint was dry, groundwater elevation plotted as elevation of wellpoint bottom. Actual groundwater elevation will be lower than that plotted.

Hydrographs for Wellpoints #6 through #12 East and South Sides of Lake



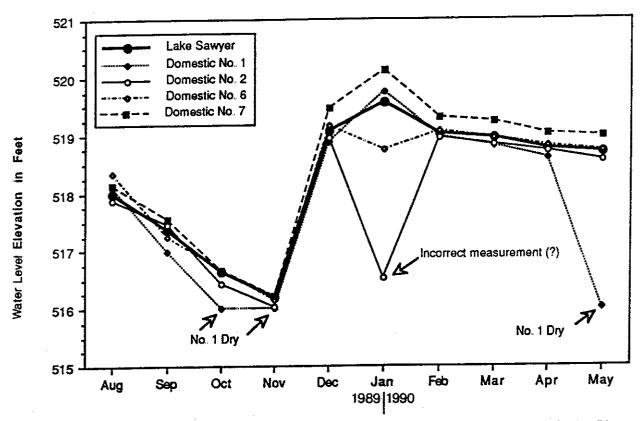
Note: For months when wellpoint was dry, groundwater elevation plotted as elevation of wellpoint bottom. Actual groundwater elevation will be lower than that plotted.

Hydrographs for Wellpoints #13 through #15 West Side of Lake



Note: For months when wellpoint was dry, groundwater elevation plotted as elevation of wellpoint bottom. Actual groundwater elevation will be lower than that plotted.

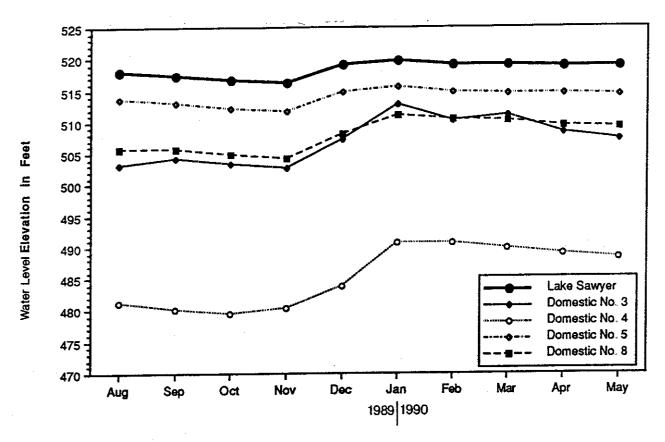
Hydrographs for Selected Domestic Wells Wells Completed in Qvr



Note: For months when Domestic Well No. 1 was dry, groundwater elevation plotted as elevation of well bottom.

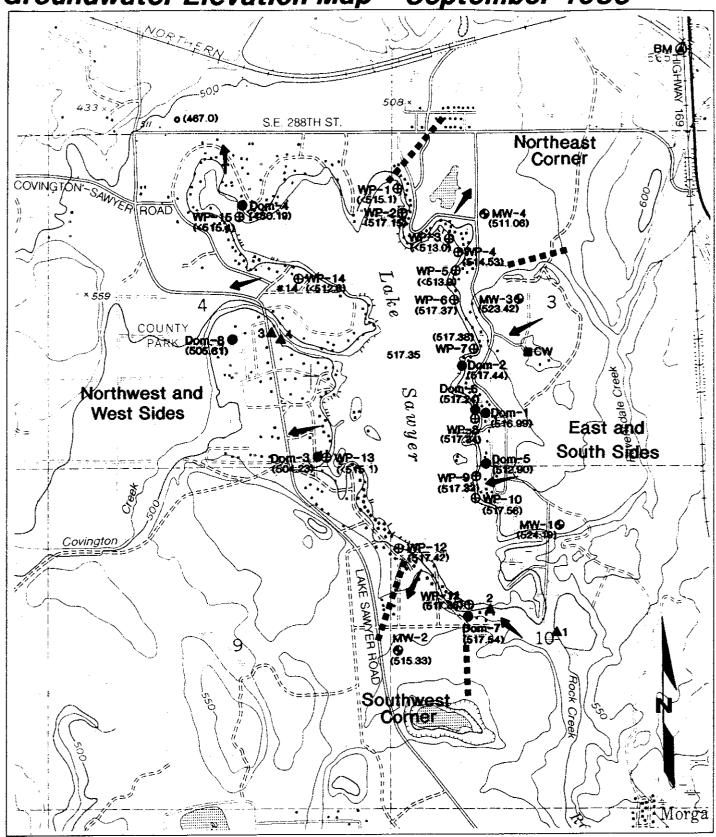
Actual groundwater elevation will be lower than that plotted.

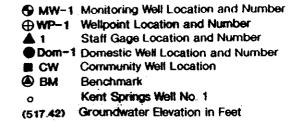
Hydrographs for Selected Domestic Wells Wells Completed in Qc

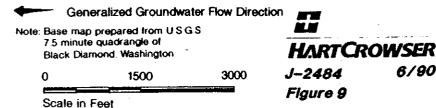


Note: It is not known which unit Domestic Well No., 5 is completed in.

Groundwater Elevation Map - September 1989

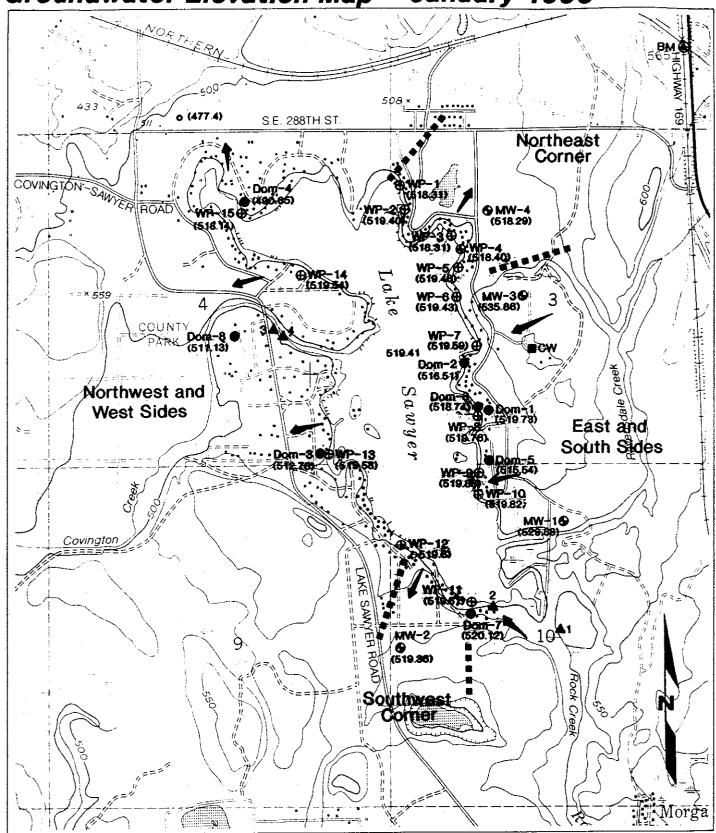


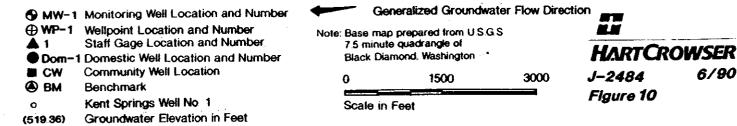




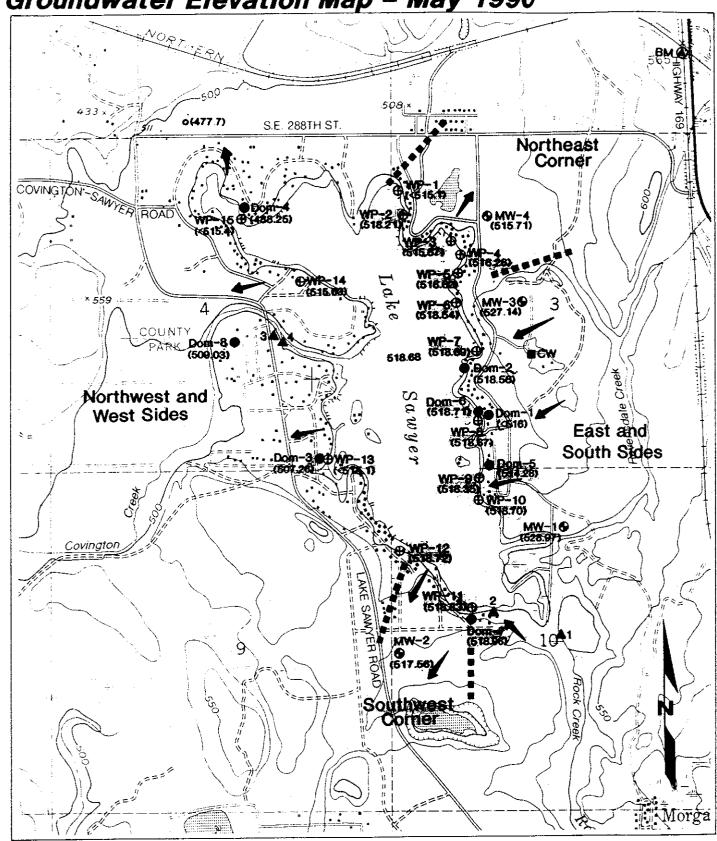
6/90

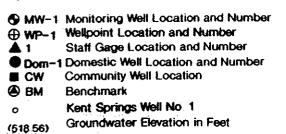
Groundwater Elevation Map - January 1990

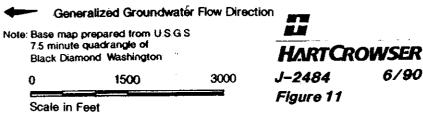




Groundwater Elevation Map - May 1990

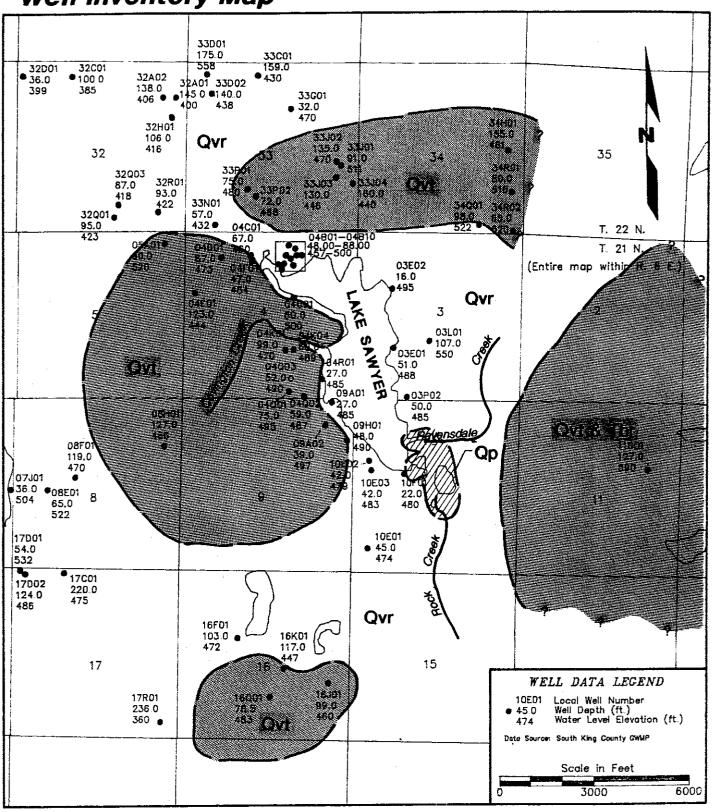






6/90

Conceptual Surface Geology and Well Inventory Map





Peat

Qvr

Vashon Recessional Outwash

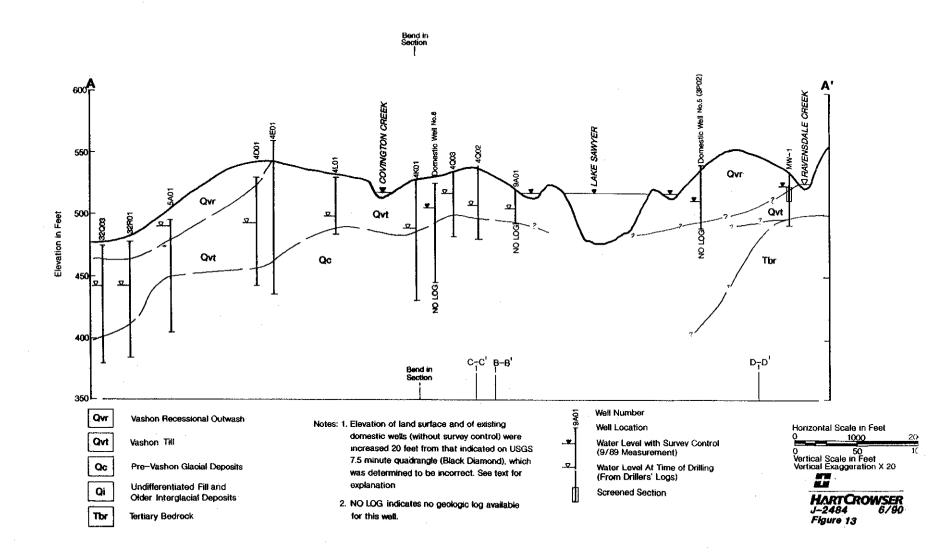


Vashon Till

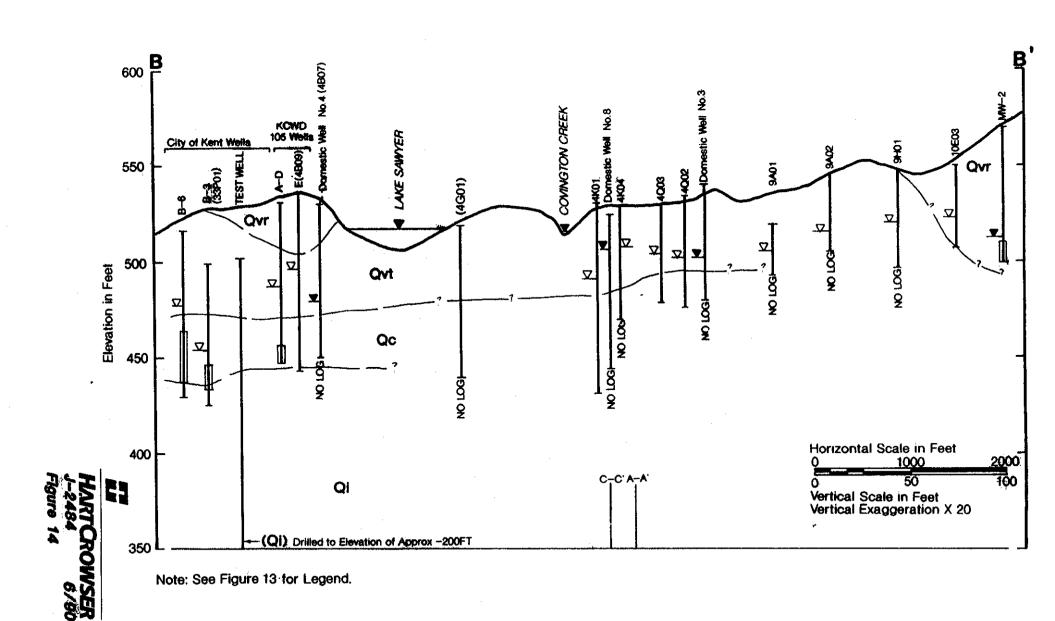


Tertiary Bedrock

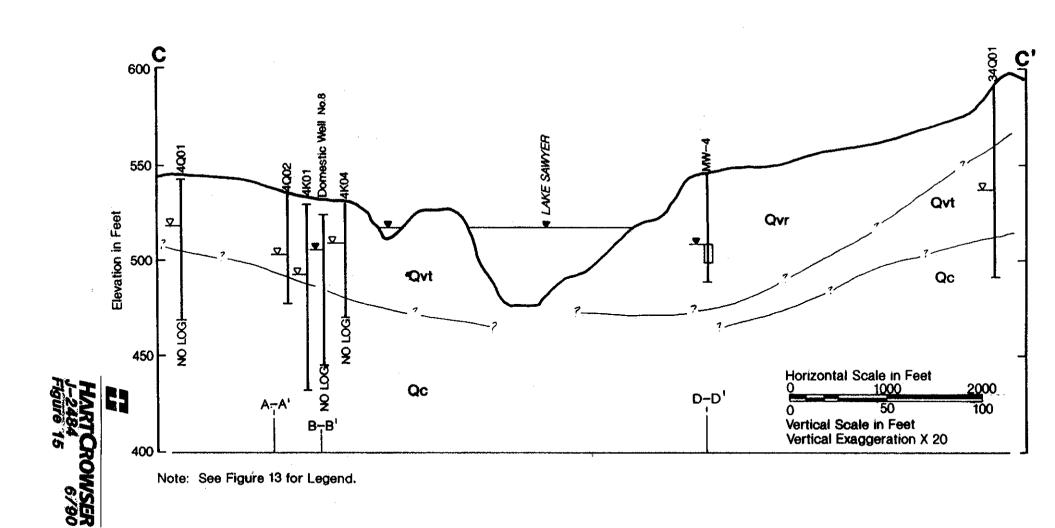




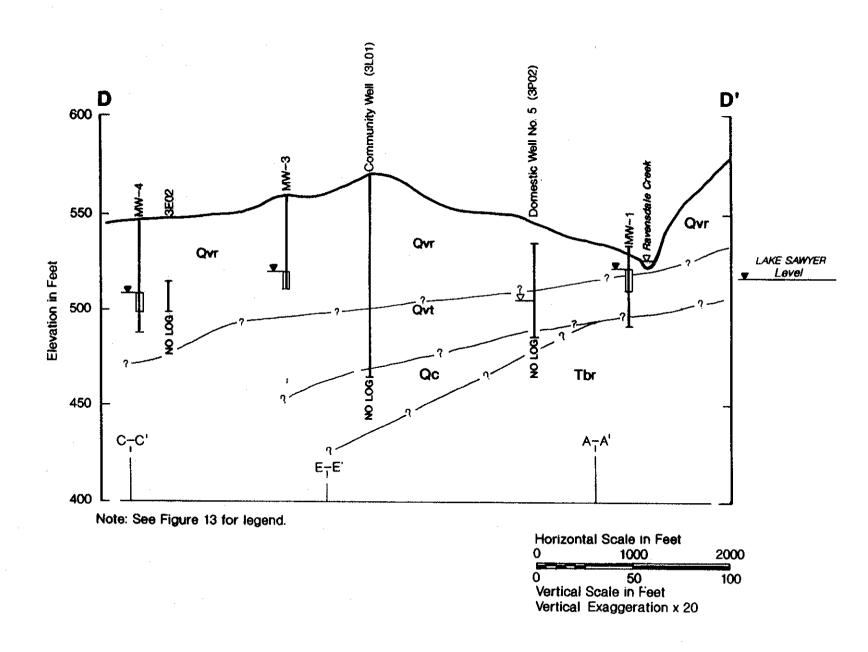
Generalized Geologic Cross Section B-B'



Generalized Geologic Cross Section C-C'

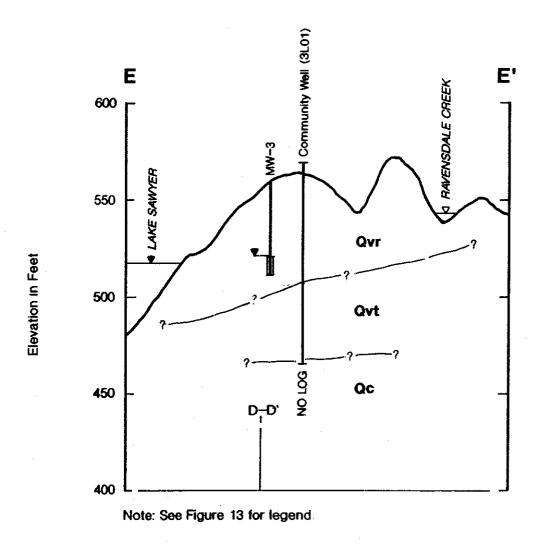


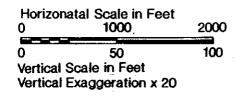
Generalized Geologic Cross Section D-D'



HARTCROWSE J-2484 6/1 Figure 16

Generalized Geologic Cross Section E-E'

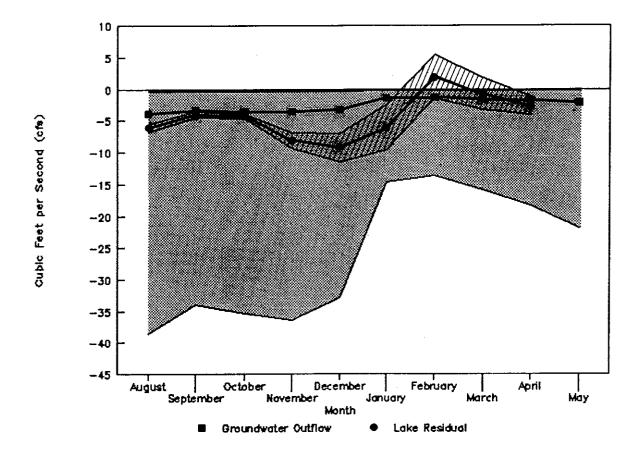






Lake Sawyer Water Budget

Groundwater Outflow/Lake Residual

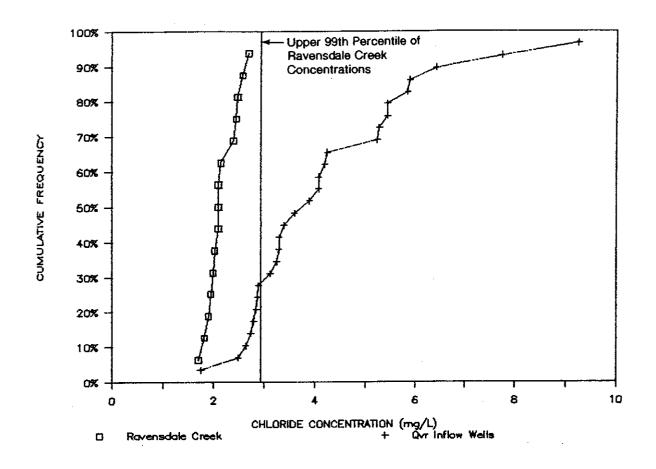




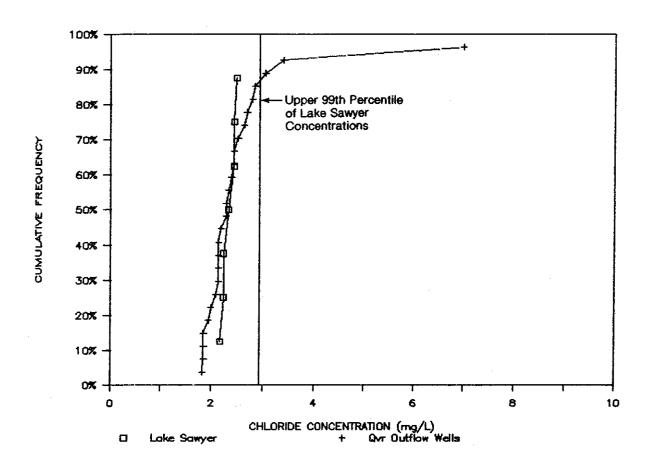
Groundwater Outflow Confidence Interval

Lake Residual Confidence Interval

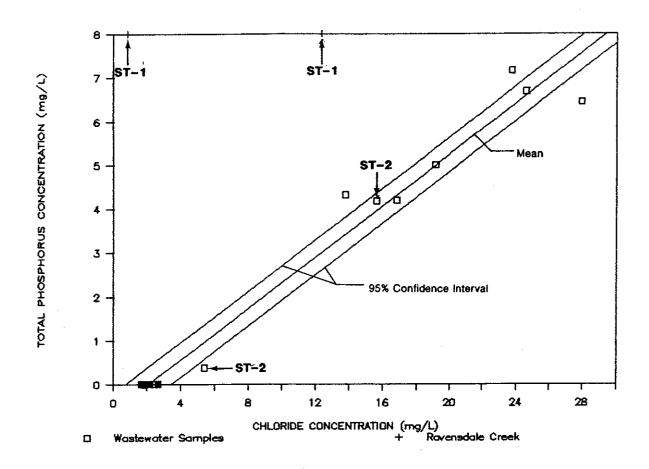
Chloride Frequency Distribution Lake Inflow - Vashon Outwash Areas



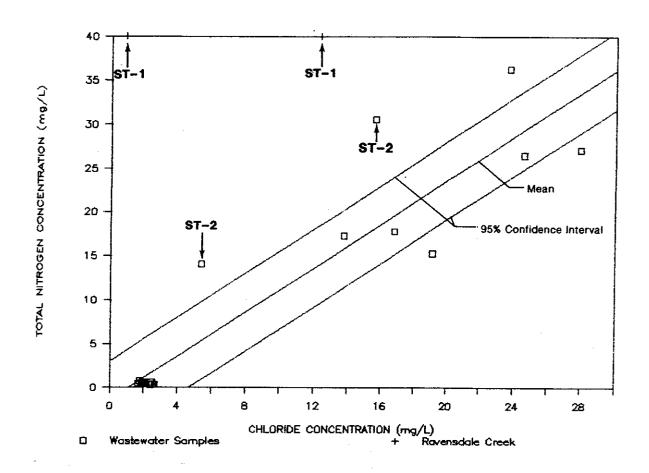
Chloride Frequency Distribution Lake Outflow - Vashon Outwash Areas



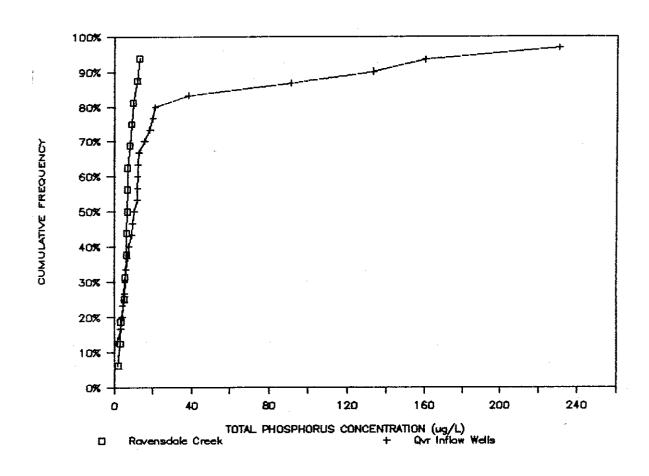
Total Phosphorus Versus Chloride Black Diamond/Lake Sawyer Wastewater



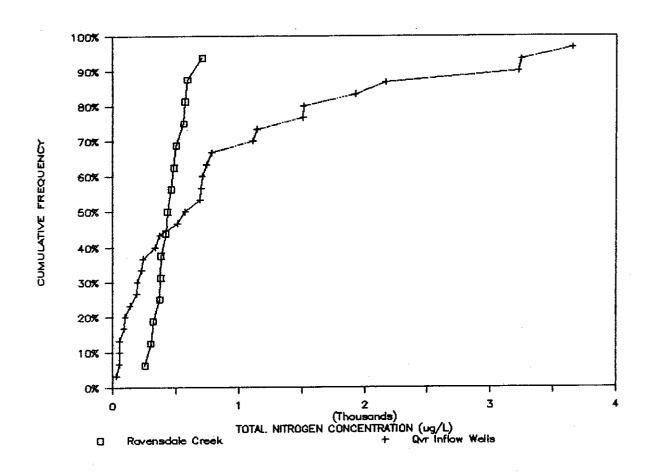
Total Nitrogen Versus Chloride Black Diamond/Lake Sawyer Wastewater



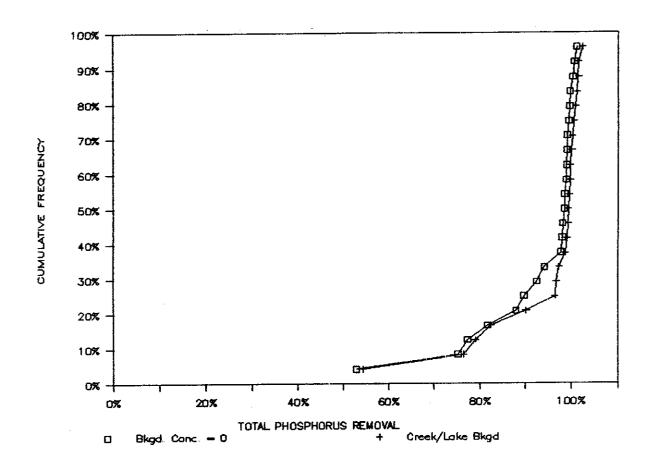
Total Phosphorus Frequency Distribution Lake Inflow - Vashon Outwash Areas



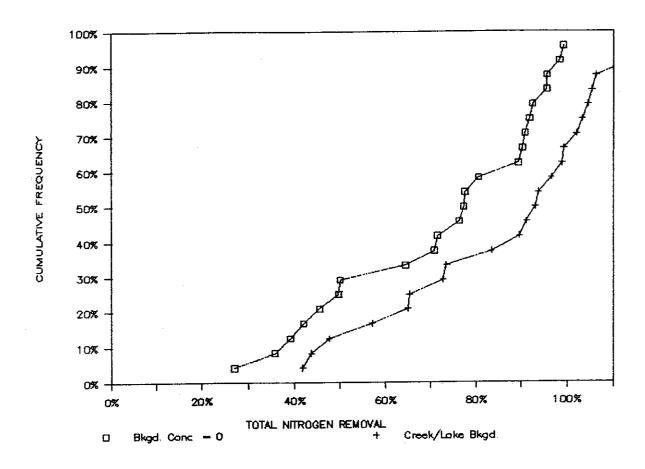
Total Nitrogen Frequency Distribution Lake Inflow - Vashon Outwash Areas



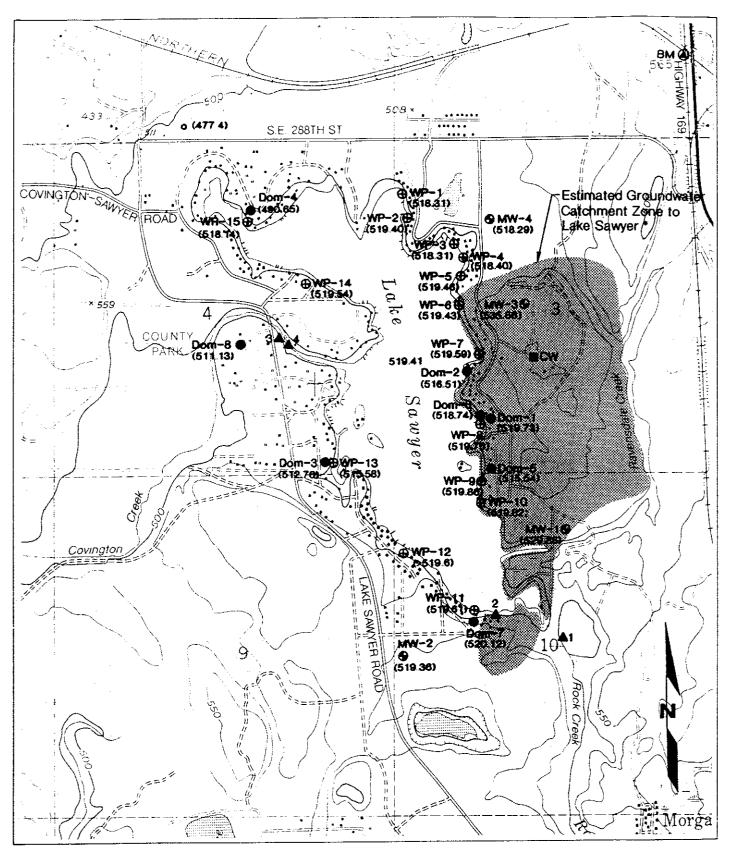
Phosphorus Removal Frequency All Wastewater Detection Samples

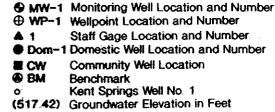


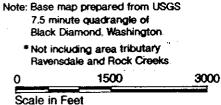
Nitrogen Removal Frequency All Wastewater Detection Samples



Groundwater Catchment Zone







HARTCROWSER
J-2484 8/90
Figure 27

APPENDIX A MONITORING WELL AND WELLPOINT INSTALLATION

APPENDIX A MONITORING WELL AND WELLPOINT INSTALLATION

The field exploration program included the completion of four soil borings/monitoring well installations (MW-1 through MW-4) and fifteen wellpoint installations (no. 1 through no. 15). The drilling of the four monitoring wells was completed between June 21, 1989, and July 7, 1989. The locations of the monitoring wells and wellpoints, as well as domestic wells and staff gages used in the monitoring network, are shown on Figure 2.

Drilling was performed under continuous supervision of a Hart Crowser hydrogeologist, who prepared an exploration log for each boring (Figures A-2 through A-5). The exploration logs represent our interpretation of subsurface stratigraphy compiled from drilling and soil sampling.

Monitoring Well Drilling and Installation Methods

Initially, Hokkaido Drilling and Development, of Puyallup, Washington, mobilized a Mobile B-61 drill rig with 4-inch inside diameter hollow-stem auger for drilling and installing the monitoring wells. However, because of the cobbly subsurface materials around the lake, the auger was unable to be advanced to the desired depths. The hollow-stem auger rig was moved to all four of the proposed monitoring well locations. At each location, refusal was reached within seven feet of ground surface because of the exceptionally dense, cobbly soils.

Following the lack of success with the hollow-stem auger drill rig, Hokkaido Drilling and Development mobilized a cable tool drilling rig to the location of MW-1. The boring at MW-1 was advanced by sequentially drilling the cobbly soil, driving a 6-inch-diameter steel casing into the advancing borehole, and then removing the cuttings with a stop valve bailer. Water was added to the borehole to allow more efficient removal of the soil cuttings. Split-spoon samples could not be collected because the soils were too gravelly to allow sample recovery. Therefore, samples of the soil cuttings were collected at five-foot-depth intervals from the bailer.

Because of the time and expense involved with cable tool drilling, the remaining monitoring wells were installed with an air rotary drill rig. Borings (MW-2 through MW-4) were advanced with a truck-mounted drilling rig using a top drive percussion bit inside of a 6-inch inside diameter threaded steel casing. Following a one- to three-foot advance of the drilled borehole, the steel casing was driven approximately the same distance by means of the drill rig-mounted air hammer. Soil cuttings were removed from the casing by compressed air. The air, forced down the drill pipe, escapes through small ports in the drill bit and lifts the cuttings to the surface, where they are directed through a discharge hose. Because the subsurface soils were generally too gravelly for split-spoon samples to be collected, soil samples were collected from the air-lifted drill cuttings at approximately 5-foot-depth intervals.

Soil Descriptions

Soil samples were visually classified in the field in general accordance with the system presented on Figure A-1. The geologic logs are presented on Figures A-2 through A-5. The soil descriptions include the following properties: relative density of sands and gravels/consistency of silts and clays, moisture, color, minor constituents, and major constituents. For both the cable tool and air rotary soil borings, relative density was determined principally from drill action and the relative ease with which the steel casing could be driven.

Monitoring Well Installation

We completed each of the four soil borings as 2-inch-diameter monitoring wells. The monitoring well construction methods were in accordance with WAC 173-160 and consistent in all four monitoring wells, regardless of the method of drilling. The wells are constructed of Schedule 40 PVC with 10-foot sections of 0.02-inch slotted screen. The monitoring wells were installed by lowering the bottom of the PVC casing to the selected depth within the cased hole. Silica sand (Colorado 10-20) was used to backfill the annulus around the screen to a level two to four feet above the top of the screen. The purpose of the sand pack is to prevent fine-grained materials from reaching and clogging the well screen. An annular seal of either bentonite chips or bentonite slurry was placed above the sand pack to a level two to three feet below ground. All wells have a concrete surface seal and are

protected by a locking steel monument. The well construction details are also presented on Figures A-2 through A-5.

Monitoring Well Development

We developed the four deep monitoring wells on July 25, 1989. The wells were developed to remove fine-grained material from the well bottom and clear material from the well screen, thus improving the well's hydraulic connection with the formation. During development, we attempted to purge approximately 15 to 20 casing volumes from the well using a stainless steel bailer. MW-1 was bailed dry after approximately 16 casing volumes and subsequently recovered slowly.

Wellpoint Installation

Fifteen wellpoints, designated no. 1 through no. 15, were installed in the presumed downgradient location of the septic drainfield on each property. Once the property owner's permission was obtained for installation, the specific location of the wellpoint on the property was determined primarily by the location of the septic system drainfield. At four locations the results of chloride measurements taken in the groundwater at various locations along the property shoreline were also used to help place the wellpoints. These measurements were obtained in an attempt to delineate a chloride plume from the septic system. At these locations, a 1/2-inch-diameter, perforated hollow steel pipe was driven into the ground until groundwater was hit. Measurements were taken at each of these temporary locations by lowering a chloride probe into the perforated hollow pipe.

The wellpoints consist of a 3-foot section of 1-1/8-inch-diameter stainless steel 60 mesh screen, or 20-slot PVC screen within a steel point, attached to a 3-foot length(s) of 1-1/8-inch-diameter blank steel pipe with a threaded steel couple. To start the wellpoint installation, a post hole digger was used to dig a hole approximately two feet deep. The wellpoint was then driven with a drive hammer to a depth at which water rose to approximately 2 to 3 feet within the wellpoint, or until refusal. A 1- to 2-foot seal of bentonite chips, overlain by cement was then placed into the hole around the wellpoint. The wellpoint is protected by a locking PVC well cap.

Key to Exploration Logs Sample Descriptions

Classification of soils in this report is based on visual field and laboratory observations which include density/consistency, moisture condition, grain size, and plasticity estimates and should not be construed to imply field nor laboratory testing unless presented herein. Visual-manual classification methods of ASTM D 2488 were used as an identification guide.

Soil descriptions consist of the following: Density/consistency, moisture, color, minor constituents, MAJOR CONSTITUENT, additional remarks.

Density/Consistency

Soil density/consistency in borings is related primarily to the Standard Penetration Resistance. Soil density/consistency in test pits is estimated based on visual observation and is presented parenthetically on the test pit logs.

SAND or GRAVEL	Standard Penetration Resistance	SILT or CLAY	Standard Penetration	Approximate Shear
Density	in Blows/Foot	Consistency	Aesistance in Blows/Foot	Strength in TSF
Very loose	0 - 4	Very soft	0 - 5	<0125
Loose	4 - 10	Soft	2 - 4	0 125 - 0 25
Medium dense	10 - 30	Medium stiff	4 - 8	0.25 - 0.5
Dense	30 - 50	Stiff	8 - 15	0.5 - 1.0
Very dense	>50	Very stiff	15 - 30	1.0 - 2.0
		Hard	>30	>20

Moisture

Dry Little perceptible moisture Damp Some perceptible moisture, Moist

probably above optimum

probably below optimum Probably near optimum moisture content Much perceptible moisture.

Minor Constituents Estimated Percentage Not identified in description 0 - 5Slightly (clayey, silty, etc.) 5 - 12Clayey, silty, sandy, gravelly 12 - 30Very (clayey, silty, etc.) 30 - 50

Legends

Tube Pushed, Not Driven

Sampling BORING SAMPLES

Split Spoon Shelby Tube Cuttings No Sample Recovery

Monitoring Well Observations Flush Mounted Monument Concrete Surface Seal 8-inch Ø Borehole Bentonite Grout 2 Ø Riser Pipe Water Level 10/20 Sand Pack 2 Ø 0.020 Slot **PVC Screen** Native Material

Test Symbols

Grain Size Classification

CN Consolidation

TUU Triaxial Unconsolidated Undrained

TCU Triaxial Consolidated Undrained

TCD Triaxial Consolidated Drained

QU Unconfined Compression

DS. Direct Shear

Κ Permeability

PΡ Pocket Penetrometer

Approximate Compressive Strength in TSF T۷

Torvane

Approximate Shear Strength in TSF CBA California Bearing Ratio

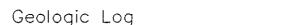
Moisture Density Relationship МΠ

AL Atterberg Limits

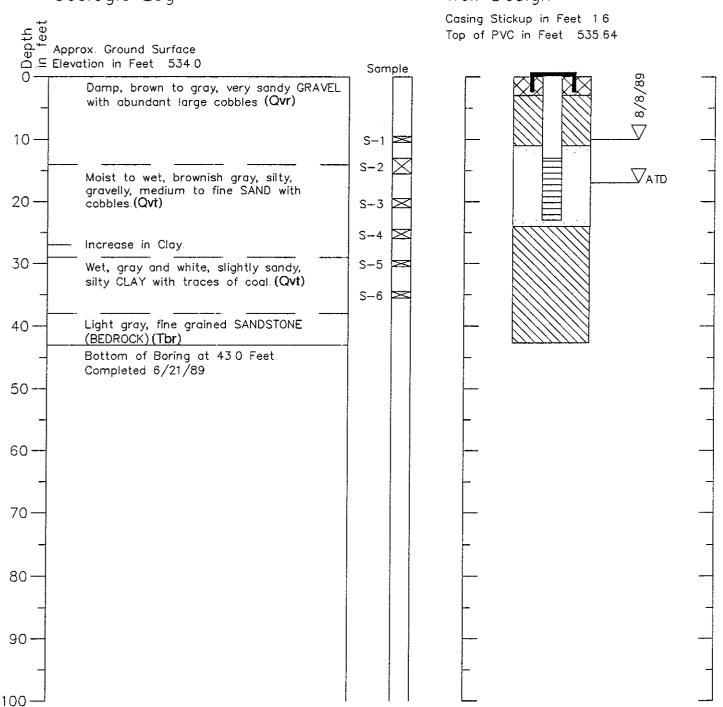
> Water Content in Percent Liquid Limit -Natural Plastic Limit

> > **HARTCROWSER** J-2484 7/90 Figure A-1

Boring Log and Construction Data for Monitoring Well MW-1



Monitoring Well Design Casing Stickup in Feet 16



1 Refer to Figure A-1 for explanation of descriptions and symbols.

2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

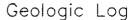
3 Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

4 All elevations based on record elevation of 564 684 feet at USCGS benchmark No. Z253



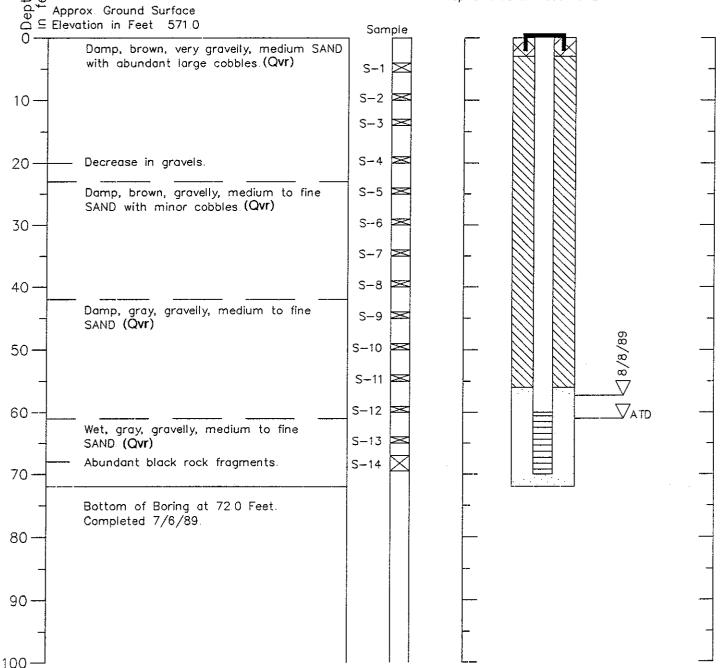


Boring Log and Construction Data for Monitoring Well MW-2



Monitoring Well Design

Casing Stickup in Feet 15 Top of PVC in Feet 572.46



 Refer to Figure A-1 for explanation of descriptions and symbols.

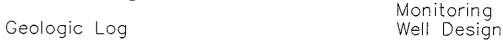
2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

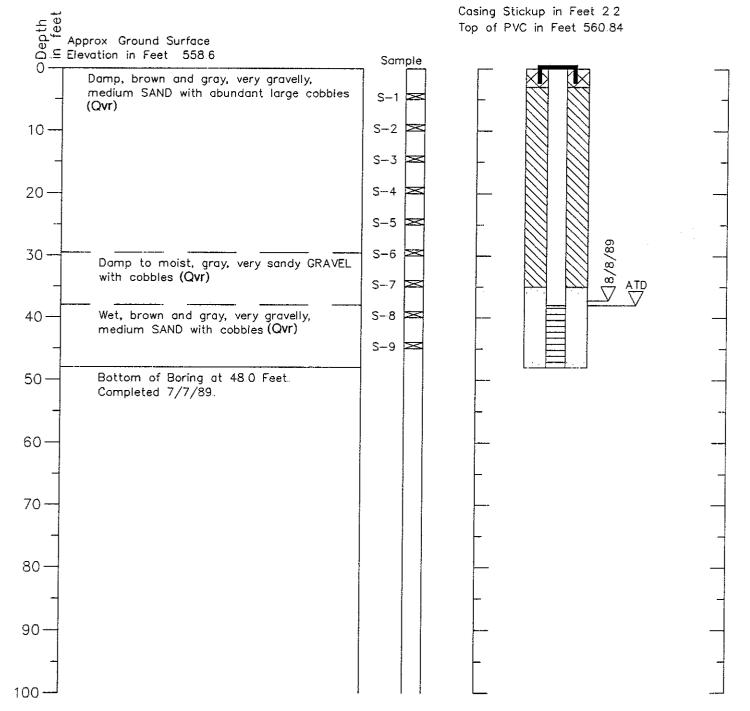
3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.

4. All Elevations based on record elevation of 564 684 feet at USCGS benchmark No. Z253.



Boring Log and Construction Data for Monitoring Well MW-3



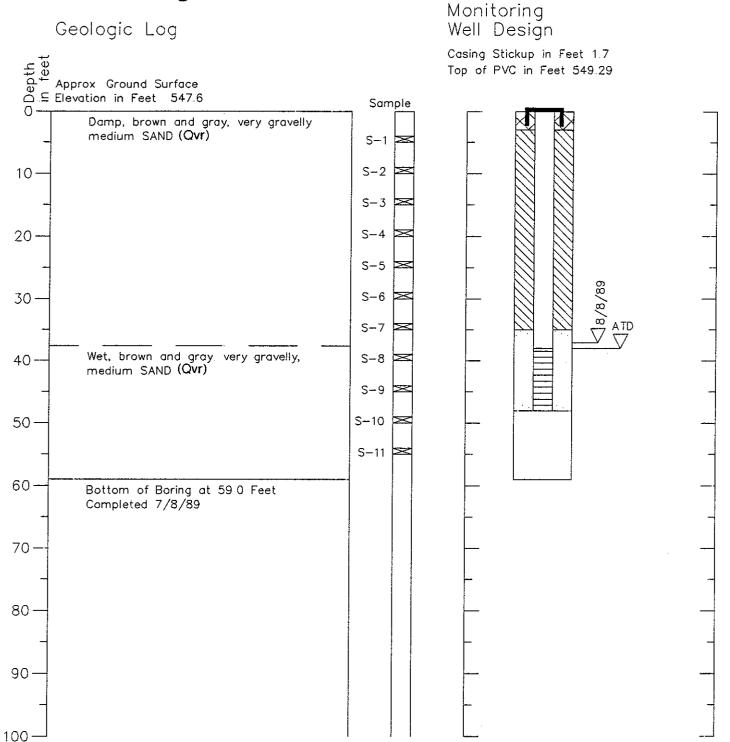


- 1 Refer to Figure A-1 for explanation of descriptions and symbols.
- 2. Soil déscriptions and stratum lines are interpretive and actual changes may be gradual.
- 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time
- 4. All elevations based on record elevation of 564 684 feet at USCGS benchmark No. Z253.





Boring Log and Construction Data for Monitoring Well MW-4



- 1 Refer to Figure A—1 for explanation of descriptions and symbols.
- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
- 3. Ground water level, if indicated, is at time of drilling (ATD) or for date specified. Level may vary with time.
- 4. All elévations based on record elevation of 564.684 feet at USCGS benchmark No. Z253.





APPENDIX B GROUNDWATER SAMPLING

APPENDIX B GROUNDWATER SAMPLING

This appendix presents the "Groundwater Sampling Plan for the Lake Sawyer Hydrogeologic Study" and the raw laboratory analytical data generated during the study.

GROUNDWATER SAMPLING PLAN

FOR

LAKE SAWYER HYDROGEOLOGIC STUDY

1. Introduction

The following plan details the sampling locations, analytical parameters, sampling frequency, and methodology to be used when sampling the monitoring wells and wellpoints for the Lake Sawyer Hydrogeologic Study. For a period of one year, quarterly groundwater samples and water level data will be collected from 4 background monitoring wells and 15 wellpoints located along the Lake Sawyer shoreline. Additionally, water levels will be obtained from approximately 6 to 8 domestic water wells and 4 staff gages situated around the lake.

2. Purpose

The purpose of the sampling program at the Lake Sawyer site is to assess the amount/concentration of leachate from individual septic systems which reaches Lake Sawyer. Additionally, the water level measurements will be used to define groundwater flow characteristics within the area. Samples from the 4 deep monitoring wells will be used for determination of regional background water quality conditions, for comparison with the samples from the wellpoints. The wellpoints were placed in an attempt to intercept leachate plumes emanating from individual septic systems.

The purpose of this sampling plan is to provide procedural guidance for field sampling and data collection. Tentatively, groundwater samples and water level data will be collected in August and November of 1989, and February and May of 1990.

During the November and May sampling rounds an as yet undetermined number of septic systems will also be sampled

3. Data Collection

During each sampling round there will be 3 categories of wells from which data will be collected. Groundwater quality samples and water level data will be obtained from the 4 monitoring wells and the 15 wellpoints. Concurrently with the wellpoints and monitoring wells, the water levels in several domestic wells will be determined.

4. Equipment

Groundwater will be withdrawn from the four monitoring wells with a stainless steel bailer, and braided nylon or polypropylene rope with monofilament "leaders". Groundwater will be removed from the wellpoints with a teflon bailer and/or a peristaltic pump. Additional equipment requirements include an electric well sounder, an engineer's rule, and a graduated plastic bucket for measurement of extracted well water.

Equipment used for the measurement of field parameters will be calibrated at least twice during each sampling round in accordance with the manufacturer's instructions. This equipment includes a pH meter, a specific conductance meter, dissolved oxygen (D.O.) meter, and a thermometer. A cooler packed with "blue ice" will be used to maintain the samples at approximately 4°C prior to delivery to the analytical laboratory.

5. Groundwater Sampling

The following operations will be performed at each of the monitoring wells/wellpoints:

- Label sample containers before sampling, checking that the appropriate bottles were supplied by the laboratory. Note any significant observations for each location in the Field Notes.
- Measure and record (to the nearest 0 01 feet) the static water level in the well with a clean electric tape to the mark on the top of the PVC casing Record level on groundwater sampling data form. This methodology will also be used at the domestic wells monitored for water levels.
- Purge well until at least 3 well volumes of groundwater are removed or the well pumps dry, using either a bailer or peristaltic pump. (During the first (August 1989) sampling round additional volumes of water will be removed from the wellpoints in an attempt to obtain a relatively sediment free sample.) Record purge volumes on groundwater sampling data form. Dispose of purged water away from wellhead.
- Using either a peristaltic pump or bailer, remove water from well cautiously to minimize turbulence and prevent sediment from getting into the sample. Tubing will be dedicated for each wellpoint for use with the peristaltic pump. Four (4) bailers for the monitoring wells will be decontaminated prior to going into the field, and dedicated for the particular monitoring well during that sampling round.
- If the peristaltic pump is used to remove groundwater, a 0.45 micron membrane filter will be placed in-line, and sample containers can be filled directly from the well. If a bailer is used, the peristaltic pump with in-line filter will be used to filter the groundwater samples prior to filling sampling containers.
- At each monitoring well/wellpoint 5 sample containers will be filled with filtered water for analysis of:

Total phosphorus Soluble reactive phosphorus Nitrate-nitrite as N Ammonia as N Total nitrogen as N Chloride

Samples for total phosphorus, soluble reactive phosphorus, and total nitrogen will each be put in separate 125 ml bottles. One 125 ml container will be used for the sample to be analyzed for ammonia and nitrate-nitrite.

One 500 ml container will be filled for the chloride analysis. Sample jars will be rinsed with groundwater prior to filling and capping. After filling the total dissolved phosphorus container, approximately 2 drops of concentrated sulfuric acid will be added to lower the pH to < 2. Sample containers will then be placed in a cooler with "blue ice" for transport.

- Two (2) extra sets of containers will be available for each sampling round in case some of the containers are contaminated. During each sampling round, 2 duplicates and 2 blanks (deionized water passed through the filters) will be collected.
- 7. A clean container will be filled with groundwater for temperature, pH, conductivity, and D.O. measurements. Data will be recorded on the groundwater sampling data form.
- Well sounders and monitoring meters will be decontaminated with a triple rinse in deionized water. Bailers for the monitoring well samples and dedicated tubing for use with the peristaltic pump will be decontaminated with a dilute sulfuric acid solution, followed by a triple rinse of deionized water.
- 9. It is anticipated that the septic tanks will be sampled in a manner similar to the wellpoints. The peristaltic pump with in-line filter will be used to remove water from the tank and fill the containers. Septic tank samples will be analyzed for the same suite of parameters mentioned above.

6. Sample Delivery for Analysis

As discussed above, all samples will be placed in a cooler with "blue ice" to maintain an approximate 4°C and appropriate Chain of Custody forms will be filled out. The coolers will then be delivered (overnight) to the Turnbull Laboratory of Ecological Studies.

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SUMMARY OF BACKGROUND, LAKE SAWYER, AND WASTEWATER WATER QUALITY DATA

Table B-1 - Summary of Background, Lake Sawyer, and Wastewater Water Quality Data

	Sampling			<u> </u>
STATION	Date	Cl (mg/L)	TSP (ug/L)	TSN (ug/L)
Ravensdale Creek:	·			
RAV	Mar-89	2.10	3.3	561 E
RAV	Арг-89	190	18	424
RAV	May-89	200	29	484
RAV	May-89	203	53	437
RAV	Jun-89	2.15	9.8	386
RAV	Jul-89	2.10	8.2	384
RAV	Aug89	2.45	13.2 E	263 E
RAV	Oct-89	2.70	9.0 E	311 E
RAV	Oct89	2.58	7.0 E	329 E
RAV	Nov-89	2.40	5.7 E	378 E
RAV	Dec-89	2.48	6.4 E	591 E
RAV	Jan-90	1.82	11.9 E	708 E
RAV	Feb-90	195	6.8 E	572 E
RAV	Mar-90	210	7.0 E	505 E
RAV	Apr-90	1.70	6.3 E	466 E
Covington Creek/Lake Sawyer:				
cov	Mar-89	250	20.9	840
cov	Apr-89	2.25	20.9	498
cov	May-89	235	16.8	364
cov	May-89	218	188	287
cov	Jun-89	225	192	274
cov	Jul-89	245	17.8	272
COV	Aug-89	245	25.3	284
		Cl (mg/L)	TP (mg/L)	TN (mg/L)
Wastewater Effluent:				
WTPIN-C	Mar-89	13.80	4.31	17.27
WTPIN	Apr-89	1915	500	1530
WTPIN-C	May-89	24.60	6.68	26,49
WTPIN-C	May-89	2795	6.4 4	27.09
WTPIN	Dec-89	1685	4,20	17 79
WTPIN	Apr-90	23.75	7.16	36.22
ST-1	Mar-90	1235	22.89	63. ₉ 8
ST-1	May-90	0.85	10.81	6538
ST-2	Mar-90	5.40	0.38	1411
ST-2	May-90	15.65	4.18	30.56

NOTES:

[&]quot;E" denotes that the total soluble concentration was estimated as the average of soluble reactive and total components.

AUGUST 1989 RAW LABORATORY DATA Sawyer Lake - Chloride (surface and groundwater) Collection date: 31 July & 7,8 Aug 89 Analysis date: 2 & 18 Aug

	mg/l Cl		mg/l
WTP-EFF-COMP	29.90	B1 ank	0.00
RCA-A	1.85	MM-81	2.80
8CA-8	1,90	MM-83	1.85
RCMBA	4.70	MM-83	5.75
RCM8-8	4.65	MM-84	2.45
P5B	-0.25	1.1	3.05
PS-8 (dup)	-0.30	11b	3,20
BDLC-B	i.35	12	2.45
RCLS-8	4,10	12 (dup)	2,60
RAV-B	2.45	ring Citi	2.45
COV-B	2.45	e	2.80
COV-B (dup)	2.48	iã	2.90
15թթտ (ԱՌ)	15.25	4	5.45
5 ppm (QA)	5.05	10	3.90
		7	6.40
		7 (dup)	6.45
		New filter	0.05
		Filter blank	1.65
		5 mg/l (0fl)	5.15
		15 mg/! (8A)	15.29
		8b	3.301
		ми-ва вер	$\{e_n(t)\}_{t=0}^n$

Sawyer take - Total Phosphorus (surface and groundwater) Collection Date: 31 July, 1,7,8 Aug 89 Analysis date: 4,7,10 Aug 89

	μg/1		pg/1
L1-0	13.5	RCA-A (dup)	42.6
Li-3	14.2	PAV-B	12.5
Li-6	40.1	L5-ta	21.9
L2-0	12.3	RCMB-B	272.9
L2-3	14.5	BLK	1.0
L2-6	25.1	BLK + 50 ug/l spike	51.69 (103% rec)
L2-6 (dup)	25.3	OA-7	11.8
L3-0	11.5	250ppb (QA)	253.0
L9-3	11. i	20 ppb (Off)	22.9
L3-6	25.1	• •	
L3-9	16.0	Y = 0.0041(X) + 0.0016	
L3-12	42.3	r = 0.9998	
L3-15	52.6		
L4~0	11.5	ST3-LWR-B	\$15.4
BLK	2.7	ST3-UPR-B	299.5
BLK + 50 ug/l spike	47.99 (96% nec)	HTP-EFF-FI	7223.1
L4-3	12.0	WTP-EFF-COMP	7889. i
L.46	23.0	€:QV~A	23.6
L4-9	19.4	COV-8	26.8
50 ppb (QA)	46.5	60V-8 (dup)	29.8
L4-12	18.4	BDLC-A	30.9
L415	42.6	BDLC-8	42.9
L5-0	11.8	PAY-A	15.5
L5-0 (dup)	12.8	GA-1	17
L5-3	10.0	Ufl-1 + 50 ug/l spoke	49.27 (99% rec)
ST3-LWR-A	534.5	OD 2	J. O
BLK	2.7	[14] - 발	12.5
BLK + 50 ug/1 spike	50.70 (101% rec)	HT-4	12.3
ST3-UPR-A	241.)	MP-E	ţ «cķ
ST4-LWR-8	336.9	Off-6	english of the second
ST4-UPR-A	207.7	QH-8	42.4
ST4-UPR-8	278.3	<u> 195-19</u>	37.6
RCA-A	42.6	6/5 -6	36.9

	µg≠1		ng/1
RCLS-A RCLS-A (dup) RCLS-B RCA-B RCMB-A ST4-LWR-A BLK BLK + 50 ug/l spike BLK 20 ppb (QA) ATP-EFF-B SOVIE PRECIP	253.3 253.3 264.1 44.2 269.6 338.8 2.5 51.27 (103% rec) 1.7 20.3 20.8 7580 23.1	BLK + 50 ug/l spike 5 ppb (QR) BLK BLK + 50 ug/l spike 250 ppb (QR) Filter blk BLK (field) NEW Filter 7 8b y = .004(x) + .006 r = .999	50.25 (96% rec) 5.9 / 2.0 50.25 (96% rec) 247.2 / 15.0 / 4.6 3.1 (12.1 / 48.9 /
Y = .0040(%) + 0.0133 r = .9999			
MW-B1 MW-B2 MW-B3 Rep MW-B4 MW-B4 (dup) 11 11B 12 2 (dup) 8 6 9	14.8 49.4 21.0 18.7 19.0 18.2 15.5 15.8 14.8 26.9 27.9 12.3 20.0 4.4 230.4 2.4		

Sawyer Lake - Soluble Reactive Phosphorus (surface and groundwater) Collection Date: 31 July, 1,7,8 Aug 89 Analysis date: 1,2,18 Aug 89

	yg/l		μg/1		pq/1
£3-0	4.6	HTP-EFF-COHP	6941	HH-82 (dup)	55.3
L3-3	4.1	RCAA	17.4	HH-83	21.5
L3-6	5.9	RCA-A (dup)	21.8	MH-83 Rep	22.2
L3-9	7.7	RCA-B	10.0	HH~B4	21.0
L3-12	20.1	RCNE-A	188.2	11	17.9
L3-12 + 50ug/l spike	49.58 (99% nec/	RCNB-A (dup)	191.8	1.1B	19.4
L3+15	33.8	RCMB-B	182.5	12	12.7
L4~0	5.2	PS-B	24.4	12 CdupO	12.2
L4-0 (dup)	4.6	BDLC-8	21.6	2	25.6
L4-3	4.4	RCLS-B	166.1	В	10.9
L4-6	5.4	RAV- B	14.1	6	21.5
L-1-9	7.5	COV-B	8.2	9	10.2
L4-12	13.7	QA-5	3.6	10	226.6
L4-15	25.6	ùA-5 + 50 ug/l spik⊕	48.08 (96% rec)	BLK	\$.5
BLK	3.3	QA-6	8.2	BLK + 50 ug/l spike	49.80 (100% re
5 ug/1 (QA)	4.4	QA-6 + 50 ug/l spike	51.16 (102% rec)	5 ug/1 (QA)	5.0
ũH− i	3.6	QA-7'	7.4	8LK	2.7
88-S	2.3	มิñ8	14.6	8LK + 50 ug/l spike	49.80 (100% re
0A-2 (dup)	2.3	BLK	2.8	100 ug/l (QA)	99.8
E-RQ	3.3	20 ug/1 (QA)	20.3	Filter blk	9.7
QA-4	3.6	BLK	2.8	BLK (field)	5.3
0A-4 + 50ug/l spike	50.35 (101% rec)	250 ug/1 (0A)	245.3	NEW Filter	·1.5
250 ug/1 (QA)	251.2			7	11.7
		P00. + Cx)S00. = p		8ხ	12.7
$y = .0040x^3 + .004$		r = .999			
;- = .999				$6100.$ \sim Categora \approx μ	
		MH-B1	15.3	r = .9999	
		HH82	56.9		

Sawyer Lake - Total Nitrogen (surface and groundwater) Collection Date: 31 July, 1,7,8 Rug 09 Analysis date: 3,7,9 Rug 09

	Hg/l		mg/l		a g/1
L1-0	0.253	L2-3	0.257	⊭ ≪ST⊴~LHR~A	3.425
.75 mg/1 (QA)	0.735	L5-3	0.266	≠ ≭STH-LHR-B	i.668
L1-3	0.236	L5-3 + .33 mg/l spike	.299 (91% rec)	51'4-UPR-8	1.739
L1-6	0.308	LS-6	0.360	ST4-UPR-8	1.683
L2-0	0.322	BLK	~0.018	RCLS-8	0.563
L2-b	0.346	.75 mg/l (QA)	0.756	RCLS-A (dup)	0.581
L2-0 + .33 mg/l spike	.272 (82% rec)	RCHB-Ā	0.481	RCLS-B	0.520
L3-0	0.246	RCMBB	0.463	BLK	-0.027
1.3-3	0.260	RAV-A	0.379	BLK + 0.33mg/l spike	.320 C972 red
L3-6	0.341	RAV-B	0.389	0.75 mg/l (8A)	u.705
L3-6 (dup)	0.323	C0V-8	0.311	QH-1	0.005
L3-9	0.477	COA-B	0.290	มิศ-⊇	0.001
L3-12	0.487	COV-B (dup)	0.278	QA-3	0.250
L3-15	0.337	FS-H	i.i36	014 - 4	0.245
L4-0	0.245	PS~8	i.085	QA−4 (dup)	0.229
14-0 + .33 mg/l spike	.309 (94% rec)	BLK	-0.008	QA-5	-0.003
L4-3	0.235	BLK + .33 mg/l spike	.317 (962 nec)	QB~F.	0.019
BLK	-0.009	0.30 คg/1 (มีคว	0.317	Q0~ ?	0.135
L4-6	0.379	HTF-EFF-A	13.95	QA~8	0.177
1.4-9	0.497	HTP-EFF-B	16.39		
L4-12	0.537	HTP-EFF-COHP	14.79		
L4-15	0.518	⊁ ×St3 LMR-A	i.938	g = .9826xx + .04	
L4-15 (dup)	0.533	≠×ST3-LHR-B	2.070	ñ ≈ .999	
L5+0	0.264	ST3-UPR-A	1.897		
ж.30 нg/1 (QR)	0.235	ST3-UPR-B	3.730		

* readent proposite office

** RED-robarial delans present all itry this North

Total Nitrogen continued

	mg/l
BOLC-A	0.487
BOLC-A (dup)	0.469
BDLC-B	0.545
RCA-A	0.593
RCA-8	0.563
SOVIE PRECIP	D.259
2	0.295
6	0.145
7	0.059
8	0.058
86	0.574
フ 9	0.516
	.327 (99% rec)
BLK (field)	0.021
10	i.510
11	D.352
116	0.403
12	0.118
BLK	-0.016
.30 mg/l (QA)	0.308
MW-81	0.642
MM-82	0.050
MM-B3	3.225

	act t
MW-83 Rep	3,225
MW-84	0.409
MM-B4 (dup)	0.364
.75 mg/t (OA)	0.710
ÐLK	0.001
BLX + .33 mq/l spike	.350 (106% rec)
Filter blk	U.061
MEW Filter	0.040
Y = 1.03(%) + .032 r = .399	

Sawyer Lake - Nitrate+Nitrite (surface and groundwater) Collection Date: 31 July, 1,7,8 Aug 89 Analysis date: 2,10 Aug 89

	mg/l		mg/l
L.3-0	0.003	RCMD-A	0.036
L9~3	0.002	RCMB-8	0.046
L3-6	0.038	RCA-A	0.024
L3-9	0.2 9 6	RCA-A (dup)	0.024
13-9 (dup)	0.305	RCA-B	0.025
L3-12	0.336	RCLS-B	0.000
L3-15	0.33 8	RCLS-B (dup)	-0.001
L3-15 + .40 mg/l spike	.377 (94% rec)	RAV-B	0.253
L4-0	0.003	СОУ-В	0.005
L4-3	0.000	.75 mg/l (QA)	0.703
L4-6	0.022	Р5-В	-0.001
L4~9	0.263	MTP-EFF-COMP	11.44
L4-9 + .40 mg/l spike	.368 (92% rec)	91k	0.002
L4-12	0.373	BLK + .40 mg/l spike	.383 (96% mec)
L4-15	0.316	0A-5	0.002
GA-1	0.002	QH~ts	0.003
QA-2	0.002	.30 տց/է (ԹՌ)	0.303
QA-3	0.001	0 11 -7	0.005
QH-4	0.001	0A8	0.024
BLK	-0.001		
0.30ppm (QA)	0.291	Y = 1.73(%) + .013	
BDLC-B	0.011	r = .999	

Nitrate Witrite continued

1 ²	0.006
<pre>2 + .33 mg/l spike</pre>	.431 (108% red)
6	0.005
7	0.006
8	0.012
9 5	Ü. 422
9	0.223
10	1.529
BLK (field)	0.007
11	0. 180
116	0,353
12	0.006
BLK	0.004
.30 mg/l (QA)	0.314
Filter blk	0.013
NEW filter	0.005
MH-81	0,629
МИ-81 (dup)	0.626
MH-82	0.010
F WW F WOODS	

mg/1

	mg/l	
MW-B3 MW-B3 Rep BLK BLK + .33 mg/l spike .75 mg/ (QA) MW-B4 MW-B4 (dup)	3.043 3.098 0.005 .418 (105% red) 0.771 0.224 0.220	7
y ≈ 1.55(x) + .002 r ≈ .998		

Sawyer Lake - Ammonia (surface and groundwater) Collection date: 31 July, i,7,8 Hug 89 Analysis date: 2,10 Hug 89

	µg/l		pg/1		pq/1
L3-0	9.64	RCHB-A	44.01	HH-81	19.2
L3-3	6.04	RCHB-B	45.35	HH-82	3.4
L3-6	9.62	PS-B	1523	ни-вэ	7.i
L3-9	10.73	BBLC-B	60.09	HH-B 3 Rep	9.4
1.3-12	27.71	RCLS-B	57.10	BLK	9.3
L3-12 + 100yg/l spike	102.1	RAV-0	11.40	BLK + 100 vg/l spike	111
L3-15	60.76	RAV-B (dup)	12,74	HH-B4	11.1
L4-0	6.71	COV-B	29,49	1.1	-D.i
L4-3	5.Ü4	COV-B (dup)	31.05	11b	-0.6
L4-6	8.05	QA-5	37.08	12	32.8
L4-9	17.66	ÑÐ∽5	48,25	2	294
L4-12	6.28	ÑH-7	23.02	2 (dup)	296
L4-12 (dup)	8.72	0A-8	57.18	8	-2.2
L4-15	60.31	QA-8 (dup)	55.62	25 ug/1 (OA)	18.i 🧷
L4-15 + 100yg/1 spike	102.3	BLK	-i.55	100 ug/1 (QA)	93.4 7
QA-i	35.52	BLK + 100 pg/l spike	99.60	5	93.0 -
QA-2	37.08	25 ug/1 (QA)	20.78	6 + 100pg/l spike	114
QA-3	7.61	BLK + 100yg/l spike	96.92	9	-0. į
QA-4	Ę.04	250 ug/1 (QA)	251.7	10	24.4
BLK	4.40	BLK	-i.55	10 (dup∋	24.6
25 ug/l (QA)	26.37			8b	25.i
BLE	7.38			7	8.5
250 ug/1 (QA)	258.4	g = .0045(x) + .0480		BLK	-5.3
ATP-EFF-COHP	7.16	r = 0.998		Filter blk	12.9
RCA-A	63.44			MER filter	12.9
RCA-B	65.00				
				a = 60437 es + 6736	

y = .0043 cs / 1.0736r = .999

DECEMBER 1989 RAW LABORATORY DATA

```
Ammonia/Nitrogen - Sawyer Lake Ground Water
Collection Date: 5,6 Dec. 89
Analysis Date: 7 Dec 89
                                 \mu q/1
1
                                -5.54
                                21.06
4
                               83.66
5
                               -3.14
4 (dup)
                                89.27
6
                               -0.551
6 (dup)
                               -2.21
7
                                 4.43
blank
                               ~0.181
8
                               0.742
9
                               46.72
10
                               69.25
100
                               115.23
11
                               1.111
12
                               -2.03
14
                               371.36
15
                                -1.1
com-well
                                 13.3
mա~1
                                  4.8
m\omega-1 + 100 \mu g/l (spike)
                              102,12
տա-2
                               18,28
mu-3
                                -2.95
                                2.96
mw-4
mw-2 (field dup)
                                13.65
filter blank (field)
                                -2.03
new filter blank (field)
                               -3.69
                                -2.4
blank (field)
25 µg/1 (QA)
                                23.27
250 pg/1 (QA)
                               241.73
Y = 0.0052 (x) + 0.068
r = 0.998
* Y = 0.0022(x) + 0.032
× r = 0.990
```

.997

TOTAL NITROGEN - Sawyer Lake Ground Water Collection date: 5,6 Dec 89 Analysis date: 19 Dec 89

	mg∕l		mg/1
1	0.123	15 (dup)	0.175
2 5	0.427	COMWELL	0.038
	0.167	MM-1	2.666
5 (dup)	0.165	MM-2	0.666
6	0.246	MM-3	2.165
7	0.094	MW-4	0.575
	ike .309 (94% rec)	BLK	-0.028
BLK	-0.023	.75 mg/l (QA)	0.747
.30 mg/l (QA)	0.307	MW-2 DUP (field)	0.422
8	0.341	Filter Blank (field)	0.032
9	1.507	New Filter Blank (field)	-0.013
10	1.911	Blank (field)	-0.015
109	i.936		
11	0.69	y = 1.015(x) + .039	
12	0.305	ř ≈ .999	
14	0.505		
15	0.174		

CHLORIDE - Sawyer Lake <u>Ground Water</u> Collection Date: 5,6 Dec. 89 Analysis Date: 8 Dec 89

	mg/l
1	2.60
2	2.00
2 4 5	7.00
5	2.65
6	4.00
6 (dup)	4,15
7	9,40
8	4.20
9	5.85
10	5.50
108	5.05
11	1.75
12	9.05
14	4.25
15	2.60
5 mg/1 (QA)	5.15
15 mg/l (QA)	15.20
com-well	1.35
mu1	9.30
mw-1 (dup)	3.2 0
mω−2 '	2.20
mw-3 bottle broken during	
mu-4	2.85
mw-2 (field dup)	2.40
filter blank (field)	0.05
new filter blank (field)	0.05
blank (field)	-0.05

SOLUBLE REACTIVE PHOSPHORUS - Sawyer Lake Ground Water Collection Date: 5,6 Dec. 89 Analysis Date: 7 Dec 89

```
pq/1
                                          18.70
2
                                          9.71
4
                                          14.47
5
5 (dup)
6
                                          9.45
                                          0.13
                                          8.92
7
                                           4.43
blank
                                          2.31
7 + 50 pq/l (spike)
                                         50.21 (100% rec)
                                          8.39
q
                                          8.39
10
                                         161.43
108
                                         169.09
11
                                          8.39
12
                                           9.71
14
                                          23:46
15
                                          8.13
15 (dup)
                                           8.13
com-well
                                         116,29
๓ฌ~1
                                         77.64
mw-1 + 50 \text{ pq/l (spike)}
                                          74.01 (148% rec)
mw-2
                                          62.31
aω~3
                                          18.43
ma-4
                                          19.23
mw-2 (field dup)
                                          50.61
filter blank (field)
                                          3.90
new filter blank (field)
                                          2.05
blank (field)
                                          2,58
5 pg/1 (QA)
                                           2.31
250 pg/1 (0A)
                                         247.86
```

Y = 0.004 (x) + 0.4012 r = 0.999 TOTAL PHOSPHORUS - Sawyer Lake Ground Water Collection Date: 5,6 Dec. 89 Analysis Date: 9 Dec 89

Pq/1 1 21.77 12.2 2 (dub) 12.7 37.38 + 50 µg/l (spike) 41.04 (82% rec) 5 3.89 6 6.16 7 5.15 blank 1.12 5 µg/1 (QA) 3.39 Θ 10.44 9 4.4 10 171.59 108 173.59 11 6.16 12 22.02 14 18.75 15 13.71 15 (dup) 13.21 com-well 112.41 mw−1 17.49 mw-1 + 50 pq/l (spike)49.35 (99% rec) mu−2 61.3 mu-3 24.29 29.83 mw-4 mw-2 (field dup) 64.57 filter blank (field) 16.73 new filter blank (field) 4.65 blank (field) 4.4 blank 1.37 250 pg/1 (QA) 246.61 Y = 0.0040(x) + 0.0035r = 0.999

Nitrate/Nitrite-Nitrogen - Sawyer Lake Ground Water Collection Date: 5,6 Dec. 89 Analysis Date: 8 Dec 89

mq/1

```
-0.005
2
                                   0.255
2 (dup)
                                   0.26
    .40
                                   0.272
  + 50 pq/l (spike)
                                   0.418 (104% rec)
                                   0.025
6
                                  0.066
7
                                   0.02
blank
                                 -0.005
0.30 \text{ pg/l} (QA)
                                   0.315
                                  0.351
\mathbf{q}
                                   1.722
10
                                  2.022
108
                                  2.016
11
                                   1.132
12
                                  0.0514
14
                                  0.035
15
                                  0.024
15 (dup)
com-well
                                  0.039
mu-1
                                   2.041
mw~i (dup)
                                   2.059
mw-2
                                  0.116
៣ម∽3
                                  Ü. 442
mu--4
                                  0.553
mw-2 (field dup)
                                  0.082
filter blank (field)
                                 -0.005
new filter blank (field)
                                 -0.004
blank (field)
                                  0.001
b Larak
                                 -0.006
mw-2 + 40 \text{ mg/I (spike)}
                                  0.416 (104% rec)
0.75 \text{ mg/1 (QB)}
                                  0.774
     1,62
Y = \frac{1}{12} (n) + 0.028
r = 0.999
```

MARCH 1990 RAW LABORATORY DATA

form : Hutthutus - Sawyer Lake Ground Water Collection Date: 13,15 Mar 90 Analysis Date: 19 Mar

	RG/1
81ank	2 3
Filter Blank	4, "
New Filter	4 . 7
250 Pa/l (QA)	245
Com Well	105.
井 1	7 1 -
#2	æ4. um
#3	± 200 m cm − 0
#4.	3.0.
# 5	4 m 😅
非 否	5.7.
并 7	9 .3 -
#8	10.0
Lab Blank	2.1
20 Pq/1 (QA)	18.5
神 学	1 . 6
转主 ()	94,4
#10B	93.4
非 1 1	4 ₁ <u>22</u>
#11 + 50kg/l spike	48.2 (9 6% rec)
#12	4,7.
#14	92.2
Lab Blank	1 - 1
MU-B1	321 -
tim-B5	47.9 •
MM-SB DOF	54.6·
MM-3	14.60
MIN-3 (dup)	15.8
MW-B4	31,2,
S1-1	22894 .
ST-2	380 '
#3 (dup)	4.0
Y = 0.0040(X) + 0.013	
r = 0.999	

CHLOFIDE - Sawyer Lake Ground Water Collection Date: 14 Mar 90 Analysis Date: 16 Mar

	mg/l
Unmarked bottle ** MW-B1 #3 #1 #5 #5 (dup) #2 #7 #8 MW-3 #6 #9 Com Well Filter Blank #10 #11	mg/1 2.20 5.30 2.15 1.85 1.75 1.70 2.75 5.25 4.20 0.40 2.85 -0.10
Blank #14	2.70
** ST-1 ST-2 #108 #10B (dup) MW-2B DUP MW-B2 MW-B2 MW-B4 New Filter #12	12.35 5.4 3.65 3.75 1.9 0 2.35 -0.10 1.95
5mg/l (QA) 15mg/l (QA)	4.80 14.95

** samples very cloudy; titration χ^{0} endpoint difficult to see

```
Ammonia Mitrogen - Saujer Lake Ground Water
Collection Date: 13.14 Mar 90
Analysis Date: 20 Mar
                                  Fq:1
                                   2.6
Blank
                                  \sim 0.6
Filter Blank
                                  1.5
New Filter Blank
                                   5.6
                                   97.0
\#1 + 100 \mu g/l spike
                                  32.4
#2
                                   2.4
排圖
                                   5.6
                                  -0.9/1.4 } 2cm cusatta readings
#4
 #4 (dup)
_Lab Blank
 25Mq/1 (QA)
                                  -1.3
 芒件
                                   2.6
 拼合
                                  19.9
 # 7
                                  23.2
                                   93.0
 #8 + 100 mg/l spike
                                  29.6
                                   4.2
 #10
                                   3.3
 #10B
                                  37.9
 #11
                                   2.1
 #12
                                  0.3/2.2) zem cuvelle readings
 #14
 #14 (dup)
 Lab Blank
                                  21.3
 ** HW-B1
                                  56.8
 MM-B2
                                  43.4
 MM-SB DUP
                                   2.6
 HW-3B
                                  20.6
                                   262 / 250 / Zem reading
 MW-B4
_Com Well
 250/g/l (QA)
                                 57348 } zem cuvette
 ST-1
 ST-2
 5cm' cuvette: Y = 0.0057(X) + 0.0553
           r = 0.999
```

r = 0.999
** sample turbid even after refilteri

 $2cm \ cuvette: Y = 0.0024(X)+0.0187$

** sample turbid even after refiltering using 0.45µm filter; read sample at 750nm to subtract out turbidity

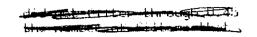
```
--0.00à
Blank
                               --(),QQ&
Filter Blank
                               ~0.005
New Filter Blank
                                0.028
                                0.108
井田
                                 0.105
#2 (dup)
                                 0.459
#3
                                 0.385
#4
                               --0.006
Lab Blank
                                 0.312
_0.30 mg/l (QA)
                                 0.286
井西
                                 0.163
                             0.395 (99% rec)
\# 6 + 0.40 \text{mg/l spike}
                                 0.815
                                 0,015
** #8
                                 0.747
#9
                                 0.810
 #10
                                 0.753
#10日
                                  1.040
#11
                                 0.267
_#12
                                 0.073
 #14
                                -0.005
Lab Blank
                                 0.759
 0.75mg/I (QA)
                                 0.590
 *** MM-B1
                                 0.096
 MM-B5
                                 0.077
 MM-SB DUF
                                 0.733
 MW-38
                                 0.488
 MW-B4
                                 0.043
 Com Well
                             ().416 (104% rec)
 Com Well+0.40mg/l spike
                                -0.0Q3
 ST-1
                                  1.53
 ST-2
 Y = 1.67 (x) + 0.023
 r = 0.999
 ** Sample was not filtered when shipped
 *** Sample very turbid; filtered through GF/C filter;
```

mog l

did not filter through 0.45µm filter because of the number of filters that would ve been required.

32

TOTAL NITHOGEN - Sawyer Lake Ground Waber Collection date: 13,14 Mar 90 Hnalysis date: 28 Mar. 90



mq/l		mg/1
•	MW-3B (1:5)	3.249
Filter Blank -0.018	#1	0.116
New Filter -0.017	#2	0.186
#3 0.5	#5	D.345
#3 (dup) 0.51	#5 (DUP)	0.353
#4 0.443	LAB BLANK	-0.03 Ø
Lab Blank -0.024	0,38 ppm (AA)	0.412
0.397 (QA) 0.397	#7 (1:2)	u.782
#6 0.198	#3	u.033
#6+0.33mg/l spike .315 (95% nec)	#8 + 0.33 mg/l (spike)	0.326 (99% nec)
#9 (1:2) 0.701	#10 (1:2)	0.710
#11 1.106	#10B (1:2)	0.708
MW-81 1.28	#12	0.978
MW-82 0.204	#12 (dup)	u. 383
MA-2 DUP 0.262	#14	0.464
MM-84 0.821	lab blank	-0.026
Lab Blank -0.029	COM WELL	0.029
0.75mq/l (QA) 8.69	COM WELL + 0.33 mg/l (spike)	
#9 (dup) (1:2) 0.701	Blank	-0.008
·	ST-1 (1:150)	63.98(
Y = 1.037(X) + 0.0404	ST-2 (1:60)	14.105
r = 0.999	0.75 ppm (QA)	0.822
18	q = 1.008 (s) + 0.043	
1.3	ř = .999	

22

BOLUBLE REACTIVE PHOSPHORUS - Sawyer Lake Ground Water Collection Date: 12,14 Mar 30 Analysis Date: 15 Max

```
\mu a \pm 1
                                             3.4
5lank
                                             3.1
Filter Blank
                                             2.6
New Filter Blank
                                             6.4
#1
                                            10.0
#2
                                            10.5
#2 (dup)
                                             5,4
#3
                                            1.9
Lab Blank
                                            49.4
50 Ma/1 (QA)
                                             3 9
#4
                                             4.6
#5
                                             4.6
#6
                                            10.0
井フ
                                             7.4
#8
                                        46.8 (94% rec)
\#8 + 50 \text{Mg/l spike}
                                             8.4
#9
                                            77.8
#10
                                            77.0
#10 (dup)
                                            88.7
#10E
                                             2.1
Lab Blank
                                            99.0
100Fg/1 (QA)
                                            72.0
* MM-B1
                                            47.4
MM-2
                                        47.6 (95% rec)
MW-2 + 50kg/l spike
                                            48 9
MM-82
                                            16.0
MW-BB
                                            19.6
MW-B4
                                          217003
ST-1
                                           348 -
ST-2
                                           119-
Com Well
                                             9.2
#11
                                            10.5
#12
                                            15.5
#14
y = 0.0040(x) + 0.0006
r = 0.999
```

* required extensive refiltering with GF/C and $0.45 \mu m$ filters!

MAY 1990 RAW LABORATORY DATA

	pg/1
Field Blank	4.4
Filter Blank	7.1
New Filter Blank	2.4
250 pg/1 (QA)	246.9
Com Well	108.3
#2	12.7
#3	10.0
#4	9.3
#5	26.9
#6	12.4
×#7	21.2
×# ₿	10.5
Lab Blank	3.1
20 وبر 20/1 (QA)	22.5
# 9	5.6
#10	159.8
#10B	112.4
#11	7.1
#6 + 50yg/l spike	46.5 (93% rec)
#12	6.3
#14	34.2
Lab Blank	1.7
MW-1 (filtered)	200.4
MH-2	54.7
MW-2 (field dup)	45.2
MN-3	21.0
MU-4	26.9
5T-1	10,815
#11 (dup)	6.8
MW-2 + 50 µg/l spike	47.4 (95% rec)
Y = 0.0041(x) + 0.0312 r = 0.999	
5 1 -2	4,163
	·
Y = 0.004(x) + 0.005 r = 0.999	
* Samples with yellow/orange	

TOTAL PHOSPHORUS - Sawyer Lake Ground Water ?

Collection Date: 2 May 90

Analysis Date: 7 May 90

Filtered before reading absorbance.

NOTE: New molybdate reagent used for second curve. ABS values lower than for first curve; therefore, only used first curve For calculations.

CHLORIDE - Sawyer Lake Ground ter Collection Date: 2 May 90 Analysis Date: 4 May 90

mq/l

	-
¥¥ MW-1	1.79
#2	2.15
#3	2.70
#4	2.15
#5	2.15
#6	2.50
#7	9.25
#8	2.65
#9	2.90
#9(dup)	2.65
#108	4.15
#10	4.00
#11	3.30
#12	2.30
15 mg/l (QA)	15.10
#14	2.70
COM-WELL	1.90
MH-2	2 .5 5
MW-2 (FIELD DUP)	2.25
MM-3	5.45
MH-4	3.40
FIELD BLANK	0.0 5
FILTER BLANK	ü.20
NEW FILTER	0.00
ST-1	0.85
MH-4 (LAB DUP)	3.25
ST-2	15.65

** Sample very cloudy; titration endpoint difficult to see so the sample was centrifuged and test was run on supernatant.

```
TOTAL NITROGEN - 5 er Lake Ground Water
Collection date: 2 may 90
Analysis date: 7, 9 May 80
```

```
ma/1
#2
                      0.120
#5
                      0.285
#5 (dup)
                      0.283
#6
                      0.105
×#7 ~
                      0.201
*#B --
                      0.056
×#9 ~
                      0.236
filter blank -
                       0.091
New Filter
                       0.011
#3
                       0.445
#3+0.33 mg/l spike < 0.356 (108% rec)
                      0.374
Lab Blank-
                      -0.010
0.30 \text{mg/l} (QA) -
                       0.290
*#10 --
                       0.753
*#8+0.33mg/l spike
                      0.313 (95% med)
×#108 ---
                       0.732
#12 -
                       0.135
*#14 ~
                       U. 461
COM-WELL -
                       0.071
MM-3 (1:5) ~
                       3.652
FIELD BLANK -
                       0.030
Lab blank -
                      -0.005
0.75 \text{mg/l} (QR) > 1
                       0.747
5T-1 (1:150) _
                      65.382
                      30.556
ST-2 (1:60)—
Y = 0.995(X) + 0.019
r = 0.999
#11 (1:2) -
                       1.139
MM-2 (1:2)^{-1}
                       1.307
MW-2 (field dup)~
                       1.193
MW-4 (1:2)
                       1.469
××MW-1 (1:2) ~
                       0.750
```

Y = 0.998(x) + 0.017r = 0.999

*Sample with red-colored debri present after digestion. Filtered before reading absorbance on spectrophotometer.

**5ample centrifuged and test ran on supernatant.

APPENDIX C AQUIFER TESTING

APPENDIX C AQUIFER TESTING

In Situ Hydraulic Conductivity Testing

We conducted in situ hydraulic conductivity tests (slug tests) in the 4 monitoring wells installed for this project. Slug tests are the typical method for estimating hydraulic conductivity (K) of subsurface soils near a well by measuring the rate of water level fall or rise in the well after the static level is suddenly displaced. In a falling head slug test, a 5- or 10-foot-long 1.5-inch-diameter rod is quickly lowered into the well, causing an "instantaneous" rise in the water level. A Terra Systems automatic data acquisition system (using a downhole pressure transducer) measures the falling water level over time as it re-equilibrates toward the static level. In a rising head test, the situation is reversed - the slug rod is withdrawn from the well, causing an "instantaneous" drop in water level, followed by a water level rise to equilibrium.

We analyzed the data from slug tests using the method of Bouwer and Rice (1976), which is applicable for unconfined (water table) conditions. The data plots for the slug tests are presented on figures C-1, C-3, C-5, and C-7. Tabulated data from the tests are presented on tables C-1 through C-9.

In addition to standard slug testing procedures, we also conducted recovery tests on the four monitoring wells. These tests were essentially conducted and analyzed like small scale pumping tests. The well was bailed as quickly as possible using a 5-foot-long bailer. The volume of water removed and the bailing time were recorded, and an equivalent pumping rate (volume/time) was calculated. For the four wells, the rate ranged from 0.6 to 1.2 gallons per minute (gpm). As the last bailer was withdrawn from the well, the automatic data acquisition system was started and the water level recovery to static was recorded.

The recovery test data were then analyzed like pump test recovery data using the Cooper-Jacob (1946) method, which plots residual drawdown versus a dimensionless recovery ratio on a semi-logarithmic plot (Figures C-2, C-4, C-6, and C-8). The tabulated data from these tests are presented in Tables C-10 through C-13. The dimensionless recovery

ratio is equal to the total time since bailing began divided by the time since bailing stopped. Aquifer transmissivity was calculated by this method. If the aquifer thickness is known, hydraulic conductivity can then be calculated from the transmissivity. Since the depth to the bottom of the aquifer was unknown, it was estimated from available information in order to determine hydraulic conductivity - this estimation brings an added uncertainty to the calculated hydraulic conductivity values.

Falling and rising head slug tests and a recovery test were attempted in MW-4. However, no significant water level displacement could be created using either 10-foot-long slug rod, or by bailing with a 5-foot-long bailer. The data plots for all three tests are included (Figures C-4 and C-8). Following the two unsuccessful slug test attempts, we proceeded to take continuous readings at 3 second intervals (using the Terra System) while bailing with the 5-foot-long bailer. As can be seen from the data plot for the MW-4 recovery test, no measurable displacement could be induced by bailing as quickly as possible for 10 minutes, not even when the bailer entered and left the water. At the end of the bailing, the transducer was checked to assure that it was working properly. Although a hydraulic conductivity cannot be calculated from the test data, it is possible to say, qualitatively, that the formation at MW-4 is exceptionally permeable - much more permeable than at the other locations tested.

Pumping Tests in Selected Domestic Wells

We conducted pumping tests in the selected domestic wells which satisfied four criteria: 1) the particular well supplied a single residence and therefore the pump could be turned on and off easily and the discharge measured; 2) the depth to water in the well could be measured; 3) some information as to the well screened depth and geology was available; and 4) the well owners' permission to pump the well was obtained. After evaluating each of the eight domestic wells in the monitoring program, we concluded that only two satisfied all three criteria (No. 4 and No. 8). Pumping tests were conducted in these two wells, both of which we believe are completed in the Qc unit underlying the till.

In each of the tests, the pump was turned on and allowed to run for 15 to 20 minutes during which time drawdown in the pumping well was

measured using an electric well probe and graduated tape measure. Discharge from a garden hose was measured by recording how long it took to fill a 4-gallon bucket. After the pump was turned off, we measured water level recovery in the well until it reached static. Because the data for the tests were collected in the pumping well, it is better to analyze only the recovery data, since these data are much less affected by well efficiency than are the drawdown data collected during pumping.

As with the recovery tests described in the previous section, the recovery data from the two pumping tests were analyzed by the Cooper-Jacob method - plotting residual drawdown versus the dimensionless recovery time ratio on a semi-logarithmic plot. The plots of the recovery data for the two pumping tests are presented on Figure C-9. Tabulated data for the tests are presented in Tables C-14 and C-15.

In addition to the two pumping tests, we attempted to perform a recovery test (using a bailer as described in the section above) in Domestic Well No. 6, which is a shallow 1.5-inch-diameter standpipe completed within the recessional outwash (Qvr). Unfortunately, we could not create any drawdown of the water table in the permeable outwash material using a one-inch bailer.

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
0	16.657		
4 2	16.68		
8	16.68		
11.9	16.68		
15.3	16.68		
20.9	16.68		
31.8	16.703		
33.7	17.072		
34.1	18.225		
35.7	19.471		
36	19.31		
37 7	20.44		
38	20.463	0.000	3.806
39.7	20.371	0.028	3 714
40.1	20.417	0.035	3.760
41.7	19.886	0.062	3.229
42	20,809	0.067	4.152
43.7	20.44	0.095	3.783
44.1	19.817	0.102	3.160
	19.817	0.128	3.160
45.7 46	20.002	0.128	3.345
	19.979	0.162	3 322
47.7	19.609	0.195	2 952
49.7	19.471	0.228	2.814
51.7	19,356	0.262	2.699
53.7 55.7	19,402	0.295	2.745
	19,402	0.328	2.929
57.7 50.7	19.31	0.362	2.653
59.7	19.079	0.395	2.422
61.7		0.428	2.353
63.7	19.01	0.462	2.283
65.7	18.94		2.191
67.7	18.848	0.495	2 122
69 7	18.779	0.528	2.076
71.7	18.733	0.562	
73.7	18.664	0.595	2.007
75.7	18.617	0.628	1,960 1,891
77.7	18.548	0.662	1.776
82.7	18.433	0.745	1.684
87.7	18.341	0.828	1.591
92.7	18.248	0 912	1.522
97.7	18.179	0.995	
102 7	18.11	1078	1.453
107.7	18.064	1.162	1.407
112.7	18.018	1.245	1.361
117.7	17.995	1.328	1.338
122.7	17.948	1.412	1.291
127.7	17.925	1.495	1.268
137 7	17.879	1.662	1 222
147.7	17 833	1.828	1.176
157.7	17.81	1.995	1.153
167.6	17.787	2 160	1.130
177.7	17.787	2.328	1.130
187.7	17.764	2.495	1.107
197.7	17.764	2.662	1107

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
207.7	17.764	2.828	1.107
217.6	17.741	2.993	1084
227.7	17.741	3.162	1.084
257.6	17.718	3.660	1061
2877	17.718	4.162	1.061
317.6	17.695	4.660	1.038
347.6	17.695	5.160	1.038
377.6	17.672	5.660	1.015
4076	17.672	6160	1015
437.6	17672	6.660	1015
467.6	17.672	7.160	1.015
4976	17.649	7.660	0.992
527.6	17.649	8.160	0.992
557.6	17.649	8.660	0.992
5876	17.649	9.160	0.992
6176	17.649	9660	0.992
6477	17.625	10.162	0.968
677.6	17625	10.660	0.968
7076	17.625	11160	0.968
737.7	17.625	11.662	0.968
767 6	17.625	12.160	0.968
7977	17.602	12.662	0.945
8277	17.625	13.162	0.968
887.6	17.602	14.160	0.945
947.6	17579	15160	0.922
10076	17579	16.160	0.922
10676	17579	17.160	0.922
1127.6	17. 57 9	18.160	0.922

2.833

1 338

16,195

203.7

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
213.7	16.241	3.000	1.292
223.7	16.264	3.167	1.269
233.6	16.287	3.332	1.246
243.6	16.334	3,498	1199
253.7	16.357	3667	1176
263.7	16.38	3.833	1.153
273.7	16.403	4.000	1.130
283.6	16.426	4.165	1.107
293.6	16.426	4.332	1.107
303.7	16,449	4.500	1 084
3137	16,449	4.667	1 084
323.7	16.472	4.833	1.061
353.7	16,495	5 333	1.038
3836	16.518	5.832	1.015
4137	16.541	6,333	0.992
443.6	16.541	6.832	0.992
4736	16.564	7.332	0.969
503.6	16.564	7.832	0.969
533.6	16.587	8.332	0.946
563.6	16.587	8.832	0.946
593.6	16.587	9332	0.946
623 6	16.587	9832	0.946
653.6	16.587	10.332	0.946
683.6	16.61	10.832	0.923
713.6	16.61	11.332	0.923
743.6	16.61	11.832	0.923
773.6	16.61	12332	0.923
803.6	1661	12832	0.923
833.6	16.61	13.332	0.923
863.6	1661	13.832	0.923
893.6	16.61	14.332	0.923
923.7	16.61	14.833	0.923
983.6	16.61	15.832	0 923
1043.6	16.633	16.832	0.900
1103 6	16.633	17832	0.900
1163.6	16.633	18.832	0.900
1223.6	16.633	19.832	0.900
1283.6	16,633	20.832	0.900
1343.6	16.633	21.832	0.900
1403.6	16 633	22.832	0.900
1463.6	16.633	23.832	0.900

Table C-3 - MW-2 Falling Head Slug Test Data (page 1 of 2)

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
0	15.688		
3	15 688		
5.9	15.711		
8.8	15.711		
11.6	15.711		
14.2	15.711		
17.3	15.711		
29.5	15.688		
30.9	15.734		
31.4	15.803		
33	16195		
33,3	16.795		
35	17.695		
353	18.248		
37	19.056		
374	18.295		
39	19.932	0.000	4 244
39.3	18.94	0005	3.252
41	19.402	0.033	3.714
41.4	19.748	0.040	4.060
43	18.779	0.067	3091
43.3	18594	0072	2906
45	18.479	0.100	2.791
47	18.456	0.133	2.768
49	18.364	0.167	2.676
51	18.295	0.200	2.607
53	18.248	0 233	2.560
55	18.179	0.267	2.491
57	18.133	0.300	2.445
59	18.087	0.333 0.367	2.399 2.330
61	18.018	0.400	2284
63	17.972 17.925	0.433	2.237
65 67	17.902	0.467	2.214
67 69	17.856	0.500	2.168
71	17.81	0.533	2122
73	17.764	0.567	2.076
75 75	17.695	0.600	2.007
73 77	17.602	0.633	1.914
79	17.51	0,667	1.822
81	17.418	0.700	1.730
83	17.349	0.733	1.661
85	17.256	0.767	1.568
90	17.095	0.850	1.407
95	16.956	0.933	1.268
100	16.841	1.017	1.153
105	16.749	1.100	1061
110	16.657	1.183	0.969
115	16.587	1 267	0.899
120	16.518	1.350	0.830
125	16.472	1.433	0784
130	16 403	1.517	0.715

Time	Height of Water Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
135	16.357	1.600	0.669
140	16.31	1.683	0.622
145	16.287	1.767	0.599
150	16.264	1,850	0.576
155	16.218	1.933	0.530
160	16.172	2.017	0.484
170	16.126	2.183	0.438
180	16.08	2.350	0.392
190	16057	2.517	0.369
199.9	16.011	2 682	0.323
210	15988	2.850	0.300
220	15.964	3.017	0.276
229.9	15.941	3.182	0.253
239.9	15.918	3.348	0.230
250	15.918	3.517	0.230
259.9	15.895	3.682	0.207
270	15.872	3.850	0184
279 9	15.872	4 015	0184
289.9	15.849	4.182	0161
299.9	15.849	4.348	0161
310	15.849	4.517	0.161
339.9	15.803	5.015	0.115
3699	1578	5.515	0092
399.9	15.78	6.015	0.092
429.9	15. <i>7</i> 57	6.515	0.069
459.9	15757	7.015	0069
489.9	15.757	7.515	0069
519.9	15.734	8.015	0046
549.9	15734	8.515	0046
579.9	15.734	9.015	0046
609.9	15734	9.515	0046
639.9	15.734	10.015	0.046
669.9	15.734	10.515	0.046
699.9	15. <i>7</i> 11	11.015	0.023
729.9	15.711	11515	0.023
759.9	15711	12015	0 023
789 9	15.711	12.515	0.023

Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
381.2	15.503	5.567	0.208
411.2	15.526	6.067	0.185
441.2	15 549	6.567	0.162
471.2	15.572	7.067	0.139
501.2	15.595	7.567	0.116
531.2	15.595	8.067	0.116
561.2	15.595	8.567	0.116
591.2	15.618	9067	0 093
621.2	15.618	9.567	0.093
651.2	15.641	10.067	0070
681 2	15.641	10.567	0.070
711.2	15.641	11.067	0.070
741.2	15.641	11.567	0.070
771.2	15.641	12.067	0.070
801.2	15.641	12.567	0.070
831.3	15.665	13.068	0046
891.2	15.665	14.067	0.046
951.2	15 665	15.067	0.046
1011.2	15.665	16.067	0.046

Table C-5 - MW-3 Falling Head Slug Test Data (page 1 of 2)

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
0	15.111		
3.6	15.134		
6.5	15.134		
9.3	15.111		
18.1	15 .111		
20	15.411		
20.4	16.749		
219	19656		
22.3	21132	0.000	6.021
23.9	18.917	0.027	3.806
24.3	17.81	0033	2.699
26	17.995	0062	2.884
26.4	18.202	0.068	3091
28	18.087	0.095	2976
28.3	17.625	0.100	2.514
30	16.657	0128	1546
30 3	16.795	0133	1.684
32	16.564	0.162	1.453
32.4	16.61	0.168	1.499
34	16.357	0.195	1246
34 3	16.426	0200	1.315
36	16.172	0228	1 061
36.4	16.241	0.235	1.130
37.9	16.034	0.260	0.923
38.3	16.103	0,267	0992
40	15.895	0.295	0784
40.3	15.988	0.300	0.877
42	1578	0.328	0.669
42.4	15.849	0.335	0.738
43.9	15.688	0.360	0.577
44.3	15.78	0.367	0.669
46	15.595	0.395	0.484
46.3	15.688	0.400	0.577
48	15.526	0.428	0 415
48.4	15.618	0.435	0.507
51	15.434	0.478	0323
53	15.388	0.512	0.277
55	15.342	0.545	0.231
57	15.318	0.578	0.207
59	15 295	0.612	0.184
61	15.272	0.645	0.161
63	15.249	0.678	0.138
65	15.226	0.712	0.115
67	15.226	0.745	0.115
69	15.203	0.778	0092
71	15.203	0.812	0092
73	15.18	0.845	0069
75 75	15.18	0.878	0.069
73 77	15.18	0.912	0.069
79	15.18	0.912	0.069
81	15.18	0.978	0.069
83	15.157	1.012	0.046
85		1.045	0.046
	15.157 15.157	1.045 1.078	0.046
87	15.157		
89	15157	1.112	0.046

Table C-5 - MW-3 Falling Head Slug Test Data (page 2 of 2)

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
94	15.157	1.195	0.046
99	15.157	1.278	0.046
104	15.157	1.362	0.046
109	15.157	1.445	0.046
114	15.157	1.528	0.046
119	15157	1.612	0.046
123.9	15.157	1.693	0.046
129	15.157	1 778	0.046
134	15.157	1862	0.046
139	15.134	1.945	0023
149	15.134	2.112	0.023
1589	15.134	2277	0.023
168 9	15134	2.443	0.023

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
0	15.134		
2.8	15157		
75	15.134		
12.4	15.134		
39.3	14788		
40.8	12758		
412	12.366		
42.8	10.243		
43.1	10.197	0.000	4.937
44.8	11.004	0028	4.130
45.1	11.258	0.033	3.876
46.8	11766	0.062	3.368
47.2	12.043	0068	3 091
48.7	12.366	0.093	2.768
49.1	12.55	0.100	2.584
50.7	12.85	0.127	2 284
51.1	13 011	0133	2.123
52.8	13.288	0.162	1.846
53.2	13.45	0.168	1.684
54.7	13.588	0.193	1.546
55.1	13.75	0.200	1.384
56.8	13.911	0.228	1.223
57 2	14.05	0.235	1.084
58.8	14.119	0.262	1.015
59.1	14.257	0.267	0.877
60.7	14.303	0.293	0.831
61.1	14.442	0.300	0.692
62.8	14.442	0.328	0.692
63.2	14.58	0.335	0.554
64.7	14.557	0.360	0.577
65.1	14.696	0.367	0.438
66. <i>7</i>	14.649	0.393	0.485
67.1	14.765	0.400	0.369
68.8	14.719	0.428	0.415
69.3 71.8	14.834	0.437	0.300
71.8 73.8	14.811	0.478	0.323
	14.857 14.903	0.512	0.277
75.8 77.0		0.545 0.579	0 231
77.8	14.926	0.578	0.208
79.8	14.949	0.612	0.185
81.8	14.949	0.645	0.185
83.8	14.972	0.678	0.162
85.8	14.972	0.712	0.162
878	14.995	0.745	0.139
89.8	14.995	0.778	0.139
91.8	14.995	0.812	0.139
93.8	15.019	0.845	0.115
95.8	15.019	0878	0.115
97.8	15.019	0.912	0.115
99.8	15.019	0.945	0115
101.8	15.019	0978	0115
103.8	15.019	1012	0.115
105.8	15019	1.045	0 115
107.8	15019	1.078	0.115
109.8	15.019	1 112	0.115

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
114.8	15.019	1.195	0.115
119.8	15.019	1 278	0.115
124.8	15.019	1.362	0.115
129.8	15.019	1.445	0.115
134.8	15.019	1.528	0.115
139.8	15.019	1.612	0115
144.8	15.019	1.695	0.115
149.8	15.019	1.778	0.115
154.8	15.019	1.862	0.115
159.8	15.019	1.945	0.115
169.8	15.019	2.112	0.115
179.8	15.019	2.278	0.115
2098	15.091	2.778	0.043
239.8	15.091	3.278	0.043
269.8	15.091	3.778	0.043

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
0	16.403		
31	16.403		
6 2	16.403		
9.1	16.426		
13	16.403		
16.7	16.403		
26.7	16.403	0000	0000
28.8	16.403	0.035	0000
29.1	16.518	0.040	0.115
30 8	16.426	0.068	0.023
31.2	16.518	0.075	0.115
32.8	16,449	0.102	0.046
33.1	16.541	0.107	0.138
34.7	16 449	0.133	0.046
35.1	16.541	0.140	0.138
36.8	16.449	0.168	0046
37.2	16.541	0.175	0.138
38.8	16.426 16.541	0.202	0.023
39.1	16.426	0.207	0.138
40.8 41.1	16.541	0.235	0023 0138
42.8	16.426	0.240 0.268	0.023
43.2	16.518	0.275	0.115
44.8	16.426	0.302	0.023
45.1	16.541	0.307	0.138
46.8	16.426	0.335	0.023
47.2	16.541	0.342	0.138
48.8	16.403	0 368	0.000
49.1	16.518	0.373	0.115
50 7	16.403	0.400	0.000
51.1	16.518	0.407	0.115
52.8	16.403	0.435	0.000
53.2	16 495	0.442	0.092
54.7	16.403	0.467	0.000
55.1	16.518	0.473	0.115
567	16.403	0.500	0.000
57.1	16.518	0507	0.115
59.8	16.403	0.552	0000
61.8	16.403	0.585	0.000
63.8	16.403	0.618	0.000
65 8	16 403	0.652	0.000
67.8	16.403	0.685	0000
69.8	16.403	0.718	0000
718	16.403	0.752	0000
73.7	16.403	0.783	0.000
75.8	16.403	0.818	0.000
778	16.403	0.852	0000
79.8	16.403	0.885	0000
81 8	16.403	0.918	0.000
83.7	16.403	0.950	0 000
85.8	16.403	0.985	0.000
87 8	16.403	1.018	0000
89.8	16.403	1.052	0.000
91.8	16.403	1.085	0.000
93.8	16.403	1.118	0.000
95.8	16 403	1 152	0.000
97.8	16.403	1.185	0.000
102.7	16.403	1,267	0.000
107.8	16.403	1.352	0.000
112.8	16.403	1435	0000

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
0	16.403		
2.3	16.426		
56	16.403		
8.9	16.426		
12.3	16,403		
16.7	16.403		
20.1	16.403		
37	16 38	0000	0.023
39	16.357	0.033	0.046
39.4	16.426	0.040	-0.023
40.9	16.357	0.065	0.046
41.3	16.449	0.072	-0.046
42.9	16.357	0.098	0.046
433	16 449	0.105	-0.046
44.9	16.334	0132	0 069
45.4	16.426	0.140	-0.023
46.9	16.31	0.165	0.093
47.3	16.403	0172	0 000
48.9	16.264	0.198	0.139
49.3	16.38	0.205	0.023
51	16.241	0.233	0.162
514	16.334	0.240	0.069
52.9	16.403	0.265	0.000
53 3	16.495	0.272	-0.092
55	16403	0.300	0.000
55.4	16.495	0.307	-0.092
57	16 403	0.333	0000
57.3	16.495	0.338	-0.092
58.9	16.403	0.365	0.000
59.3	16.518	0.372	-0.115
60.9	16,403	0.398	0.000
61.4	16.518	0407	-0.115
62.9	16.403	0.432	0.000
63.3	16.518	0.438	-0.115
65	16.403	0.467	0.000
65.3	16.518	0.472	-0.115
67	16 403	0.500	0.000
67.5	16.518	0.508	-0.115
70	16.403	0.550	0.000
72	16.403	0.583	0.000
74	16.403	0.617	0.000
76 70	16.403	0.650	0.000
78	16.403	0.683	0.000
79.9	16.403	0.715	0.000
82	16.403	0.750	0.000
84	16,403	0.783	0.000
86	16.403	0.817	0.000
88	16.403	0.850	0.000
90	16.403	0.883	0000
92	16.403	0 917	0.000
94	16.403	0.950	0 000
96	16.403	0.983	0.000
97.9	16.403	1.015	0.000
100	16.403	1.050	0.000
102	16.403	1.083	0.000

Height of Water

Above Transducer	Elapsed Time	Head Change
in Feet	in Minutes	in Feet
16.403	1 117	0000
16.403	1.150	0.000
16.403	1.183	0.000
16.403	1.267	0000
16.403	1.350	0.000
16.403	1.433	0.000
16.403	1.517	0000
16.403	1.600	0000
16.403	1.683	0.000
16.403	1.767	0.000
16.403	1.848	0.000
16.403	1.933	0000
16.403	2.017	0.000
16.403	2.182	0.000
	in Feet 16.403 16.403 16.403 16.403 16.403 16.403 16.403 16.403 16.403 16.403 16.403 16.403 16.403	in Feet in Minutes 16.403 1.117 16.403 1.150 16.403 1.183 16.403 1.267 16.403 1.350 16.403 1.433 16.403 1.517 16.403 1.600 16.403 1.683 16.403 1.767 16.403 1.848 16.403 1.933 16.403 2.017

	Height of Water		
Time	Above Transducer	Elapsed Time	Head Change
in Seconds	in Feet	in Minutes	in Feet
0	16 449		
4 8	16 449		
11.2	16.449		
16.6	16.449		
21.9	16.449		
37.2	16.449		
394	16.38	0.000	0.023
39.8	16,472	0.007	-0.069
41.4	16.38	0.033	0.023
41.7	16.495	0.038	-0092
43.4	16.38	0.067	0.023
43.7	16.495	0.072	-0.092
45.4	16.38	0.100	0.023
458	16.472	0.107	-0.069
47.4	16.38	0.133	0.023
47.7	16.495	0.138	-0.092
49.3	16.38	0.165	0.023
49.7	16,495	0.172	-0.092
51.4	16.38	0.200	0 023
51.8	16.472	0 207	-0.069
53.3 53.7	16.38	0.232	0.023
53.7 55.3	16.495	0.238	-0 092
55.8	16.403	0.265	0.000
57.3	16.495 16.403	0.273	-0.092 0.000
57.3 57.7		0.298	0.000
59.4	16.518 16.403	0.305 0.333	-0.115
594 597	16.518	0.338	0.000 -0.115
61 4	16.403	0.367	0.000
61.8	16.495	0.373	-0.092
63.4	16.403	0.400	0.000
63.7	16.518	0.405	-0.115
65.3	16.403	0.432	0.000
65.7	16.518	0.438	-0.115
673	16.403	0.465	0.000
67.8	16.495	0.473	-0.092
70 4	16.403	0.517	0.000
72.4	16.403	0.550	0.000
74.4	16.403	0.583	0.000
76.4	16.403	0.617	0.000
78.4	16.403	0.650	0.000
80.4	16.403	0.683	0.000
82.4	16.403	0.717	0.000
84.4	16.403	0.750	0000
86.4	16.403	0.783	0.000
88 4	16.403	0.817	0.000
90.4	16403	0.850	0.000
92.4	16.403	0.883	0.000
94.4	16403	0.917	0.000
96.4	16403	0.950	0000
98.4	16.403	0.983	0000
100.4	16.403	1.017	0000
102 4	16.403	1.050	0000
104.3	16403	1.082	0.000
106.4	16.403	1.117	0.000

133.1

16.403

Time in Seconds	Height of Water Above Transducer in Feet	Elapsed Time in Minutes	Head Change in Feet
108.4	16.403	1.150	0.000
113.4	16.403	1.233	0 000
118.4	16.403	1.317	0000
123.3	16.403	1.398	0.000
128.4	16.403	1.483	0.000

1.562

0.000

	Height of Water				
Time	Above Transducer	t	ť'		Residual Drawdown
in Seconds	In Feet	in Minutes	in Minutes	t/t'	in Feet
0	16.633				
2.4	16 68				
7.6	16.657				
11.5	16.657				
14.8	16.657				
215	16.657				
25.9	16.657				
414.2	4.337	3.250	0000	#DIV/0!	12.320
416.2	4 429	3.283	0.033	98.500	12.228
416.5	4.545	3 288	0.038	85.783	12.112
418.2	4.545	3 317	0067	49.750	12.112
418.6	4,637	3.323	0073	45.318	12 020
420.2	4.637	3.350	0.100	33.500	12.020
4205	4.729	3.355	0.105	31.952	11.928
422.2	4.729	3,383	0133	25.375	11.928
422.5	4.822	3.388	0.138	24.494	11.835
424.2	4.822	3.417	0.167	20.500	11 835
4246	4.914	3.423	0.173	19.750	11.743
426.2	4.914	3.450	0200	17.250	11743
426.5	5.006	3.455	0.205	16.854	11.651
428.2	5.006	3.483	0.233	14.929	11,651
428.6	5098	3.490	0240	14 542	11.559
430 2	5,098	3.517	0.267	13.188	11.559
432.2	5.168	3.550	0 300	11.833	11489
434.2	5.26	3 583	0.333	10.750	11.397
436.2	5.352	3.617	0.367	9 864	11.305
438.2	5.421	3.650	0.400	9.125	11.236
440.2	5.514	3.683	0.433	8.500	11.143
442.2	5.583	3.717	0.467	7 964	11 074
444.2	5 675	3.750	0.500	7.500	10.982
446.2	5.744	3.783	0.533	7.094	10.913
448.2	5.814	3.817	0 567	6.735	10.843
450.2	5.883	3.850	0600	6.417	10.774
452.2	5.929	3.883	0633	6.132	10.728
454.2	5.998	3.917	0.667	5875	10.659
456.2	6.044	3 950	0 700	5 643	10.613
458.2	6.09	3.983	0733	5 432	10.567
460.2	6.114	4.017	0767	5.239	10.543
4652	6.183	4100	0.850	4.824	10.474
470.2	6.275	4.183	0.933	4 482	10 382
475.2	6.367	4.267	1.017	4.197	10.290
480.2	6.437	4.350	1100	3.955	10.220
485.2	6.529	4.433	1.183	3.746	10.128
490 2	6.621	4.517	1.267	3.566	10.036
495.2	6.713	4.600	1.350	3.407	9.944
500.2	6.806	4.683	1.433	3.267	9.851
505.2	6.921	4.767	1.517	3.143	9.736
510.2	7 036	4 850	1.600	3.031	9.621
520.2	7.267	5.017	1 7 67	2.840	9.390
530.1	7.544	5.182	1.932	2.682	9.113
540.2	7.844	5 350	2.100	2.548	8813

Notes: 1) Well bailed at 1.2 gpm for 3.25 minutes.

²⁾ t = Elapsed total time since bailing began; t' = Elapsed time since bailing stopped (recovery began)

Height of Water	Hei	iah	t of	Wat	er
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	neight of water	_			
Time	Above Transducer	t	ť'		Residual Drawdown
in Seconds	in Feet	in Minutes	in Minutes	t/t'	in Feet
550.2	8.144	5.517	2.267	2.434	8.513
560 2	8.444	5683	2.433	2.336	8 213
570.1	872	5848	2598	2.251	7.937
580.2	8.974	6.017	2.767	2.175	7683
5901	9.182	6.182	2932	2.109	7.475
600.2	9.389	6.350	3.100	2.048	7.268
610.1	9.574	6.515	3.265	1.995	7.083
620.1	9736	6,682	3 432	1.947	6 9 21
630.2	9.92	6.850	3600	1.903	6.737
640 2	10.082	7.017	3767	1.863	6.575
650.2	10.243	7.183	3.933	1.826	6.414
660.1	10.405	7.348	4.098	1.793	6252
6702	10.566	7.517	4.267	1.762	6.091
680.2	10.728	7.683	4.433	1733	5,929
690.2	10.866	7.850	4.600	1.707	5791
700.2	11.027	8.017	4767	1 682	5630
710.2	11.166	8.183	4933	1.659	5.491
740.2	11.558	8.683	5.433	1.598	5.099
770.1	11.927	9182	5.932	1.548	4730
800.1	12.273	9.682	6 432	1.505	4.384
830.1	12.573	10.182	6.932	1.469	4.084
8601	12.873	10.682	7432	1.437	3.784
8901	13.127	11.182	7.932	1.410	3.530
920.1	13.358	11.682	8.432	1385	3.299
950.1	13.588	12.182	8.932	1.364	3.069
9801	13.773	12.682	9.432	1.345	2 884
1010.1	13.957	13.182	9932	1.327	2.700
1040.1	14.096	13.682	10.432	1.312	2.561
1070 1	14.257	14182	10.932	1297	2.400
1100.1	14.373	14.682	11.432	1 284	2 284
1130.1	14.488	15.182	11932	1.272	2.169
1160.1	14.603	15.682	12.432	1.261	2.054
1190.1	14.696	16.182	12.932	1.251	1.961
1220.1	14.765	16.682	13.432	1.242	1892
1250.1	14.834	17.182	13.932	1.233	1.823
1280 1	14.903	17.682	14.432	1.225	1.754
1310.2	14.949	18.183	14.933	1.218	1.708
1370.1	15.019	19.182	15.932	1 204	1.638
1430.1	15.065	20.182	16.932	1.192	1592
1490.1	15.088	21.182	17.932	1.181	1569
1550.1	15.111	22.182	18.932	1.172	1.546
1610.1	15.111	23.182	19.932	1.163	1546
1670.1	15.157	24.182	20.932	1155	1.500
1730.1	15.18	25.182	21932	1.148	1,477
1790.1	15.18	26.182	22.932	1.142	1.477
1850 1	15.203	27.182	23.932	1.136	1.454
1910.1	15.226	28.182	24.932 24.932	1.130	1.431
1970.1	15.249	29.182	25.932 25.932	1125	1.408
2030 1	15.249	30.182	26.932 26.932	1.121	1.408
2090.1	15.249		20.932 27.932	1.121	1.385
		31.182			1.385
2150.1	15.272 15.205	32.182	28.932	1.112	
2210.1	15.295	33.182	29.932	1.109	1.362

Notes: 1) Well bailed at 1.2 gpm for 3.25 minutes.

²⁾ t = Elapsed total time since bailing began; t' = Elapsed time since bailing stopped (recovery began).

Time	Height of Water Above Transducer	t	t'		Residual Drawdown
in Seconds	in Feet	in Minutes	in Minutes	t/t'	in Feet
2270.1	15.318	34.182	30.932	1.105	1.339
2330.1	15.318	35.182	31.932	1.102	1.339
2390.1	15.318	36.182	32,932	1.099	1.339
2450.1	15.342	37182	33.932	1096	1.315
2510.1	15.342	38.182	34,932	1.093	1.315
2570.1	15.342	39.182	35,932	1090	1,315
2630.1	15.342	40.182	36.932	1.088	1.315
2690.1	15.342	41.182	37.932	1.086	1.315
2750.1	15.342	42.182	38.932	1083	1.315

Notes: 1) Well bailed at 1.2 gpm for 3.25 minutes.

²⁾ t = Elapsed total time since bailing began; t' = Elapsed time since bailing stopped (recovery began).

	Height of Water				
Time	Above Transducer	t	ť'		Residual Drawdown
in Seconds	in Feet	in Minutes	in Minutes	t/t'	in Feet
0	15.9852				
3	16.1				
5.9	161				
9.2	16.1				
446.4	10.981	6.000	0.000	#DIV/0!	5.119
447.8	11.051	6.023	0.023	258.143	5.049
448.2	11189	6.030	0.030	201.000	4.911
449.8	11.143	6.057	0.057	106.882	4957
450.3	11.258	6.065	0 065	93.308	4.842
451.8	11.235	6.090	0090	67.667	4.865
452.2	11.35	6.097	0.097	63.069	4.750
453.8	11.327	6123	0.123	49.649	4773
454.2	11.42	6130	0.130	47154	4.680
455.8	11.397	6,157	0 157	39 298	4.703
456.3	11.512	6.165	0.165	37.364	4.588
457.8	11.489	6.190	0.190	32.579	4.611
458.2	11604	6197	0.197	31.508	4.496
459.8	11.558	6.223	0.223	27.866	4.542
460.3	11.673	6.232	0 232	26.899	4.427
461.9	11.65	6.258	0 258	24.226	4.450
463.8	11.72	6.290	0.290	21,690	4.380
465.9 467.8	11.789 11.858	6.325 6.357	0.325 0.357	19.462 17.822	4.311 4.242
467.8 469.9	11.927	6.392	0.392	16.319	4173
471.8	11.996	6.423	0.423	15.173	4.104
473.9	12.066	6.458	0.458	14.091	4.034
475.8	12.112	6.490	0.490	13.245	3.988
477.9	12.181	6.525	0.525	12.429	3.919
479 9	12.25	6.558	0.558	11.746	3.850
481.9	12296	6.592	0.592	11.141	3.804
486.9	12435	6.675	0.675	9.889	3.665
491.8	12.573	6.757	0.757	8.930	3.527
496.9	12.712	6.842	0.842	8.129	3.388
501.8	12.827	6.923	0.923	7.498	3.273
506.9	12.942	7.008	1008	6.950	3.158
511.9	13.058	7.092	1 092	6.496	3.042
516.9	13.173	7 175	1.175	6106	2.927
521.8	13.265	7.257	1 257	5.775	2.835
526.8	13.358	7.340	1.340	5.478	2.742
531 9	13.45	7.425	1425	5.211	2,650
541.8	13.611	7.590	1590	4.774	2.489
551.8	13773	7 757	1.757	4.416	2327
561.8	13.934	7 923	1 923	4.120	2166
571.8	1405	8.090	2.090	3.871	2.050
581.8	14.188	8.257	2.257	3.659	1.912
5918	1428	8.423	2.423	3.476	1.820
601.8	14.396	8,590	2.590	3.317	1.704
611.8	14.488	8.757	2.757	3.177	1.612
621.8	14.58	8.923	2.923	3.052	1.520
631 9	14.673	9.092	3 092	2.941	1.427
661.8	14.88	9.590	3.590	2.671	1.220
691.8	15.042	10.090	4.090	2.467	1.058
721.8 751.9	15.18 15.205	10.590	4.590 5.000	2.307	0.920 0.805
751.8 791.9	15.295 15.411	11.090 11.590	5.090 5.590	2.179	0.689
781.8 811.8	15.411 15.48	11.590 12.090	5.590 6.090	2.073 1.985	0.620
811.8 841.8	15.549	12.590	6.590	1.985	0.551
U4 1 .0	10	16.430	0.030	1	0.551

Notes: 1) Well bailed at 0 67 gpm for 6.0 minutes.

²⁾ t = Elapsed total time since bailing began; t' = Elapsed time since bailing stopped (recovery began)

	Height of Water				
Time	Above Transducer	t	t'		Residual Drawdown
in Seconds	in Feet	in Minutes	in Minutes	t/t'	in Feet
871.8	15595	13.090	7.090	1.846	0.505
901.8	15665	13.590	7.590	1 791	0.435
931.8	15711	14.090	8.090	1.742	0.389
961.8	15.734	14 590	8.590	1.698	0366
991.8	15.78	15.090	9.090	1.660	0.320
1021.8	15,803	15.590	9.590	1.626	0.297
1051.8	15.826	16090	10 090	1.595	0.274
10818	15.849	16.590	10.590	1.567	0.251
1111.8	15.872	17.090	11.090	1.541	0.228
1141.8	15.872	17.590	11590	1.518	0.228
1171.8	15895	18.090	12.090	1 496	0.205
12018	15.918	18.590	12.590	1.477	0.182
1231.9	15.918	19.092	13.092	1.458	0.182
1291.8	15.941	20.090	14090	1.426	0.159
1351.8	15.964	21.090	15.090	1.398	0136
1411.8	15.988	22.090	16 090	1.373	0112
1471.8	15.988	23.090	17.090	1.351	0.112
1531.8	16.011	24.090	18.090	1.332	0.089
1591.8	16.011	25.090	19.090	1.314	0089
1651.8	16034	26.090	20.090	1.299	0.066
1711.8	16.034	27.090	21.090	1 284	0.066
17718	16.034	28.090	22.090	1.272	0.066
18318	16.057	29.090	23.090	1.260	0 043
1891 8	16.057	30.090	24.090	1.249	0.043
1951.8	16.057	31.090	25.090	1.239	0.043

Notes: 1) Well bailed at 0.67 gpm for 6.0 minutes.

²⁾ t = Elapsed total time since bailing began; t' = Elapsed time since bailing stopped (recovery began).

	Height of Water				
Time	Above Transducer	t	ť'		Residual Drawdown
in Seconds	in Feet	in Minutes	in Minutes	t/t'	in Feet
0	16 0/1				
0 3.2	16.841 16.98				
7.9	16.98				
13.4	16.98				
517 1	16.956				
518.7	16011	7.300	0000	#DIV/0!	0.969
519.2	16.149	7.308	0.008	877.000	0.831
520.7	16.264	7 333	0.033	220.000	0.716
521.1	16.403	7.340	0.040	183.500	0.577
522.7	16.472	7.367	0.067	110.500	0.508
523.1	16.61	7.373	0.073	100.545	0.370
524.7	16.587	7.400	0.100	74.000	0.393
525.2	16.726	7.408	0.108	68.385	0.254
526.7	16.703	7.433	0133	55.750	0.277
527.1	16.818	7.440	0140	53.143	0.162
528.7	16.772	7 467	0.167	44.800	0.208
529.1	16.887	7.473	0.173	43.115	0093
530.8	16.818	7.502	0.202	37.198	0.162
531.2	16.933	7.508	0.208	36 040	0.047
532.7	16.864	7 533	0.233	32 286	0.116
533.1	16.98	7.540	0.240	31.417	0000
5347	16.887	7.567	0.267	28.375	0.093
535.2	17.003	7575	0.275	27.545	-0.023
536.8	16.91	7.602	0.302	25 199	0.070
537.1	17.026	7.607	0.307	24 804	-0.046
538.7	16.933	7.633	0.333	22.900	0.047
539.1	17.026	7.640	0.340	22.471	-0,046
540.8	16.933	7.668	0.368	20.819	0 047
541.2	17.049	7.675	0.375	20 467	-0.069
542.7	16 933	7.700	0.400	19 250	0.047
543 1	17.049	7.707	0.407	18.951	-0.069
544.7	16.956	7.733	0.433	17.846	0.024
545.1	17.049	7.740	0.440	17 591	-0.069
546.8	16.956	7.768	0.468	16.587	0.024
547.2	17.049	7775	0.475	16.368	-0069
549.8	16.956	7.818	0.518	15.084	0.024
5518	16.956	7.852	0.552	14 233	0.024
553.8	16.956	7.885	0585	13.479	0.024
555.7	16.956	7.917	0.617	12.838	0.024
557 8	16.956	7.952	0.652	12.202	0.024
559.7	16.956	7.983	0 683	11.683	0.024
561.8	16.956	8.018	0.718	11.162	0.024
563.7	16.956	8.050	0.750	10.733	0.024
565 7	16.956	8.083	0.783	10.319	0.024
567.7	16.956	8.117	0.817	9.939	0.024
569.7	16.98	8.150	0.850	9.588	0.000
571.7	16.98	8.183	0.883	9.264	0000
573.7	16.956	8.217	0.917	8.964	0.024
575.7	16.956	8.250	0.950	8684	0.024

Notes: 1) Well bailed at 1.1 gpm for 7.3 minutes.

²⁾ t = Elapsed total time since bailing began; t' = Elapsed time since bailing stopped (recovery began).

-	Height of Water				Backland Burnelows
Time	Above Transducer	t	t'		Residual Drawdown
in Seconds	in Feet	in Minutes	in Minutes	t/t'	in Feet
577.7	16.98	8.283	0983	8.424	0000
579.7	16.98	8.317	1.017	8.180	0.000
581.7	16.98	8.350	1.050	7.952	0.000
583.8	16.98	8.385	1085	7.728	0000
585.7	16.98	8.417	1.117	7.537	0.000
587.8	16.98	8.452	1.152	7.339	0000
592.7	16.956	8.533	1.233	6.919	0.024
597.8	16.956	8.618	1.318	6.537	0.024
602.7	16.98	8.,700	1400	6 214	0.000
6077	16.956	8783	1483	5.921	0.024
612.8	16.98	8.868	1568	5.655	0000
617.7	16.956	8.950	1.650	5424	0.024
6227	16.956	9.033	1.733	5.212	0.024
6277	16.98	9.117	1.817	5 018	0.000
632.7	16.956	9.200	1900	4.842	0.024
637.8	16.98	9 285	1.985	4.678	0.000
647.7	16.956	9.450	2.150	4.395	0.024
6577	16.956	9.617	2.317	4.151	0.024
667.7	1698	9.783	2.483	3.940	0.000
6777	16.956	9.950	2.650	3.755	0.024
687.7	16.98	10.117	2.817	3.592	0000
6977	16.956	10.283	2.983	3.447	0.024
7077	16.98	10.450	3.150	3.317	0.000
717.7	16.98	10.617	3.317	3.201	0.000
727.7	1698	10.783	3.483	3.096	0.000
737.8	16.98	10.952	3.652	2.999	0.000

Notes: 1) Well bailed at 1.1 gpm for 7.3 minutes.

²⁾ t = Elapsed total time since bailing began; t' = Elapsed time since bailing stopped (recovery began)

	Height of Water	
Time	Above Transducer	
in Seconds	in Feet	
0 3.7		
7.5		
11 1	16.403	
14.7		
55.9		BEGIN BAILING AT 0.9 GPM
59.6 62.6		(See text for discussion)
65 6		
68 6		
71.6	16 403	
74 5	16.403	
77.6 80.6	16.403 16.426	
83.6	16 403	
86 5	16.403	
89 6	16.403	
92 6	16 403	
95.5	16 403	
98.6 101.6	16.403 16.403	
104.6	16.403	
107.6	16 403	
110.6	16.403	
1135	16.403	
116.6 119.6	16.403 16.403	
122.6	16 403	
125 6	16.426	
128 5	16.38	
131.6	16.403	
134.5 137.6	16.403	
140 6	16.403 16.403	
143.6	16.403	
146.6	16.403	
149 6	16.403	
152 6	16.403	
155.5 158.6	16.403 16.403	
161 6	16 403	
164 6	16 403	
167.6	16.426	
170.5	16.403	
173.5 176.6	16 403 16 403	
1796	16.403	
182.5	16.403	
185.6	16.403	
188.6	16.426	
191 6 194 5	16 403 16 403	
197.6	16.403	
200.6	16.403	
203.5	16 403	
206 5	16.403	
209 6	16.426	
212.6 215.6	16.403 16.403	
218.6	16.403	
221 6	16 403	

Time in Seconds	Height of Water Above Transdu in Feet	
224 5		16 403 CONTINUE BAILING AT 0.9 GPM
227 6		16.403
230.6		16.426
233.6		16.403
236.6		16 403
239 5		16.403
242 6		16.403
245.6		16.403
248.5		16.426
251.6		16 403
254 6		16.403
257 6		16.403
260.6		16.403
263.5		16 403
266 6		16 403
269 6		16.426
272.6 275.5		16.403 16.403
273.3 278.5		16.403
281 6		16.403
284.6		16.403
287.6		16 426
290 6		16.403
293 6		16.403
296 6		16.403
299.6		16.403
302.6		16 403
305.5		16.403
308 6		16.426
311.6		16.403
314.5		16,403
317.5		16 403
320 6		16.403
323 6		16.403
326.6		16.403
329.5		16.426
332.6		16.426
335 6		16.403
338.5		16.403
341.6		16 403
344 6		16.403
347 6		16.403
350 6		16.426
353.5		16 426
356 5		16.403
359 6		16.403
362.6		16.403
365.6		16.403
368 5		16.403
371 6		16.426
374.6		16.426
377.6		16.403
380.6		16 403 16 403
383 5		16.403
386.6		16.403 16.403
389.5 303.5		16.403 16.403
392.5		16 403 16 426
395.6 398.6		16.426 16.426
401 6		16 426 16.403
404.5		16.403 16.403
G. POP. 3	i	I O. TOO

Time in Seconds	Height of Water Above Transducer In Feet	
407 6	16 403	CONTINUE BAILING AT 0.9 GPM
410.6	16.403	
413.6	16.403	
416 5	16.426	
419 5	16.426	
422.6	16.426	
425.6	16.403 16.403	
428.5 431.6	16 403	
434.6	16 403	
437.5	16.426	
440 6	16.403	
443.5	16 426	
446.6	16 426	
449.6	16.403	
452.5	16.403	
455 5 458 6	16.403 16.403	
461 6	16.403	
464.5	16.403	
467.6	16.426	
470 6	16 426	
473 6	16.403	
476.5	16 403	
479.5	16.403	
482.6	16 403	
485.6 488.5	16.426 16.426	
491 6	16.426	
494 5	16 426	
497.5	16.426	
500.6	16.426	
503.6	16.403	
506 5	16.403	
509.5	16 403	
512.6 515.5	16.403 16.403	
518.5	16 426	
521.5	16.426	
524.6	16.426	
527.6	16.403	
530 5	16.403	
533.5	16 403	
536.6	16.403	
539.6	16.403	
542.6	16.403	
545 6 548 5	16 426 16 426	
551.6	16.403	
554 5	16.403	
557.6	16 403	
560.6	16 403	
563.6	16,426	
566 6	16.426	
569 5	16 426	
572.5 575.5	16.403	
575.5 578 6	16.426 16.403	
581 6	16.403	
584 5	16 403	
587.5	16 426	

	Height of Water	
Time	Above Transducer	
in Seconds	in Feet	CONTRUCT DAILING AT A CORE
590.5 593.6		CONTINUE BAILING AT 0.9 GPM
596.6		
599.5		
602 5		
605 5		
608.5 611.6		
614.5		
617.5	16 426	
620.5	16.403	l .
623.5	16.426	
626.5	16.403	
629 5 632.5	16 403 16 403	
635.6	16.403	
638.5	16.403	
641 5	16.403	
644.6	16 426	
647.6	16.426	
650.5 653.6	16.403 16.426	
656.6		BAILING STOPPED AT 10 MINUTES
657 1		"RECOVERY"
658.5	16.426	
658.9	16.518	
660.5	16.426	
660.9 662.6	16 518 16 426	
663	16.495	
664.5	16.426	
665	16 518	
666.5	16.426	
666 9	16.518	
668.5 668.9	16.403 16.518	
670.5	16 426	
671	16.518	
672.5	16.426	
672.9	16.518	
674 5	16.403	
674.9 676.5	16 518 16 426	
677	16.518	
678.5	16.426	
679	16 518	
680.5	16 426	
680.9	16.518	
682.5 682.9	16.426 16.518	
684.5	16.426	
685	16 518	
686.6	16.426	
688.5	16.403	
690.6	16.403	
692.5	16 403	
694.5 696.6	16 403 16 426	
698.5	16.403	
700.5	16.426	
702.6	16 403	
704 5	16 426	

Time	Height of Water Above Transducer	
in Seconds	in Feet	
706	6	16 426
711	5	16.403
716	5	16.426
721	.6	16 403
726	.6	16.403
731	5	16.403
736	.5	16 403
741	.5	16.403
746	.6	16.403
751	.6	16.403

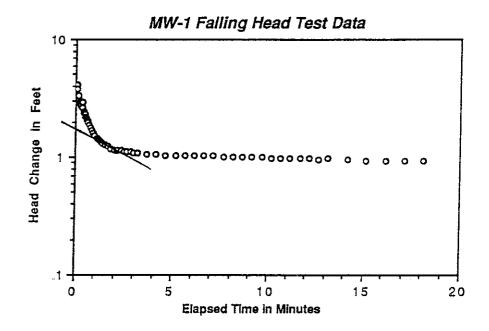
Elapsed Time (t)	Depth to Water	Drawdown		
in Minutes	in Feet	in Feet	<u>t</u>	<u>t/ť</u>
1.00	46.57	4.47	_	
2.00	46.37	4.27		
300	46.56	4.46		
400	4640	4.30		
5.00	46.61	4.51		
550	46.64	4 54		
700	46.45	4.35		
8.00	46.39	4.29		
9.00	46.40	4.30		
10.50	46.38	4 28		
12.00	46.43	4.33		
14.00	46.43	4.33		
15.05	46.37	4.27	0.05	301.00
15.33	46.10	4.00	0.33	4645
15.67	44.45	2.35	0.67	23.39
16.00	44.06	196	100	1600
16.33	43.74	1.64	1.33	12.28
16.67	43.57	1.47	1.67	9.98
17.00	43.43	1.33	2.00	8.50
17.33	43.24	114	2.33	7.44
17.67	43.05	0.95	2.67	6.62
18.00	42.96	0.86	3.00	6.00
1833	42.87	0.77	3,33	5.50
18.67	42.74	0.64	3.67	5.09
19.00	42.67	0.57	400	4.75
19.50	42.63	0.53	4.50	4.33
20.00	42.44	0.34	5.00	400
21.00	42.30	0.20	6.00	3.50
2217	42.22	0.12	717	3.09
23.00	42.16	0.06	8.00	2.88
24.00	42.13	0.03	9.00	2.67
25.50	42 14	0.04	10.50	2.43

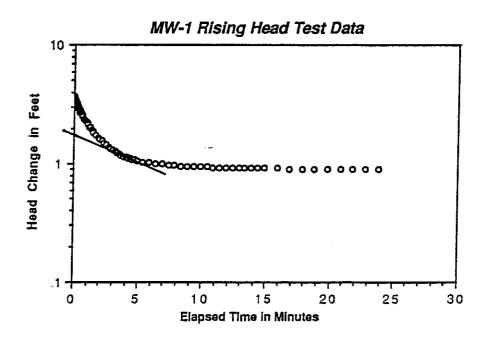
Notes:
1) Well pumped at 10.0 gpm for 15 minutes.
2) t = Elapsed total time since pump turned off; t' = Elapsed time since pump turned off

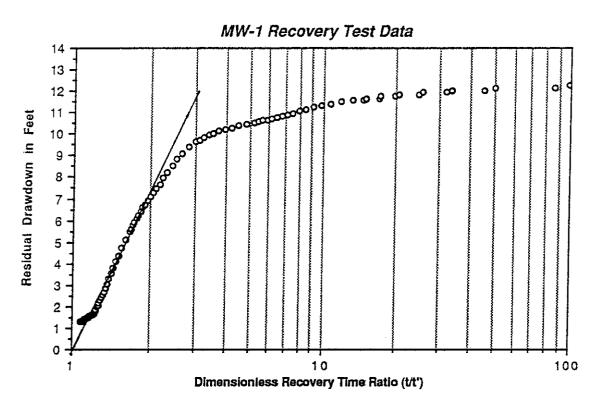
Elapsed Time (t) Depth to Water in Feet in Fee	vr
0.50 17.06 1.18 1.00 20.37 4.49 1.50 22.71 6.83 2.08 25.03 9.15 2.75 27.63 11.75 3.33 29.64 13.76 4.00 31.72 15.84 5.00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	vr
1.00 20.37 4.49 1.50 22.71 6.83 2.08 25.03 9.15 2.75 27.63 11.75 3.33 29.64 13.76 4.00 31.72 15.84 5.00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	
1.50 22.71 6.83 2.08 25.03 9.15 2.75 27.63 11.75 3.33 29.64 13.76 4.00 31.72 15.84 5.00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	
2.08 25.03 9.15 2.75 27.63 11.75 3.33 29.64 13.76 4.00 31.72 15.84 5.00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	
2.75 27.63 11.75 3.33 29.64 13.76 4.00 31.72 15.84 5.00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	
3.33 29.64 13.76 4.00 31.72 15.84 5.00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	
4.00 31.72 15.84 5.00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	
5 00 35.52 19.64 5.50 35.54 19.66 6.00 36.83 20.95	
5.50 35.54 19.66 6.00 36.83 20.95	
6.00 36.83 20.95	
8 00 40.33 24.45	
8.50 41.32 25.44	
9.00 42.08 26.20	
9,50 42,73 26,85	
10.00 43.33 27.45	
10,50 43,95 28,07	
11.00 44.52 28.64	
12.00 45.53 29.65	
13.00 46.39 30.51	
14.00 47.16 31.28	
15.00 47.89 32.01	
16.00 48.54 32.66	
17.00 49.08 33.20	
18.33 49.80 33.92	
19.00 50.04 34.16	
20.00 50.42 34.54 0.00 #DIV	V/0!
20.67 48.50 32.62 0.67 30.8	.85
21,.00 46,.89 31,.01 1,.00 21,.0	.00
2150 4476 2888 150 143	33
22.00 42.86 26.98 2.00 11.0	.00
22.50 41.08 25.20 2.50 9.00	00
2300 39.48 23.60 3.00 7.67	67
24.16 36.08 20.20 4.16 5.81	
24.50 35.26 19.38 4.50 5.44	
25.00 33.96 18.08 5.00 5.00	
25.50 32.93 17.05 5.50 4.64	
26.00 31.87 15.99 6.00 4.33	
26.50 30.86 14.98 6.50 4.08	
27.00 29.92 14.04 7.00 3.86	
28.00 28.31 12.43 8.00 3.50	
29.00 26.83 10.95 9.00 3.22	
30.00 25.54 9.66 10.00 3.00	
31.00 24.41 8.53 11.00 2.82	
32.00 23.41 7.53 12.00 2.67	
33.00 22.57 6.69 13.00 2.54	
34.00 21.83 5.95 14.00 2.43 35.00 21.13 5.25 15.00 2.33	
37.00 20.04 4.16 17.00 2.18 38.00 19.59 3.71 18.00 2.11	
40.00 18.81 2.93 20.00 2.00	
42.00 18.24 2.36 22.00 1.91	
44.00 17.79 1.91 24.00 1.83	
46.00 17.35 1.47 26.00 1.77	
48.00 17.06 1.18 28.00 1.71	
50.00 16.89 1.01 30.00 1.67	
52.00 16.62 0.74 32.00 1.63	
54.00 16.48 0.60 34.00 1.59	
56.00 16.33 0.45 36.00 1.56	
58.00 16.22 0.34 38.00 1.53	
60 08 16.13 0 25 40.08 1.50	
65.00 15.95 0.07 45.00 1.44	
67.00 15.89 0.01 47.00 1.43	
70.00 15.88 0.00 50.00 1.40	

Notes: 1) Well pumped at 8.0 gpm for 20 minutes.

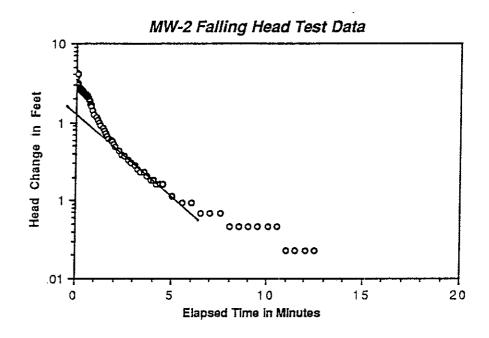
2) t = Elapsed total time since pump turned off; t' = Elapsed time since pump turned off.

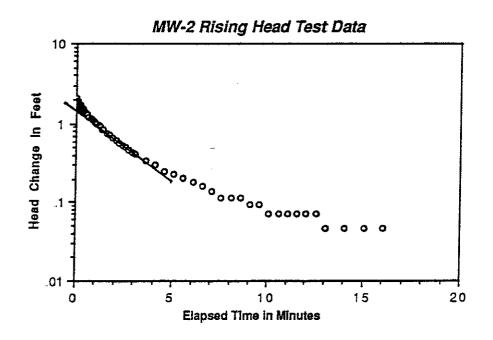


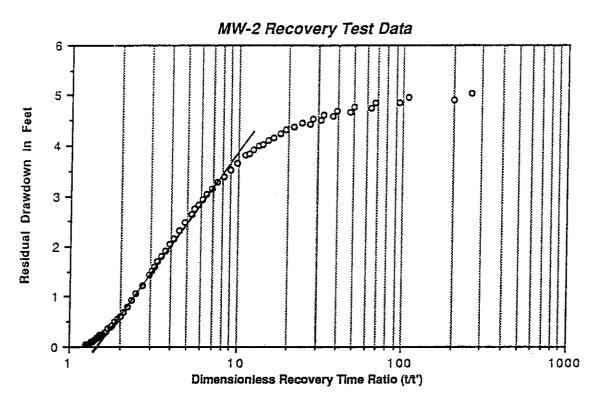




Notes: Well bailed at 1.2 gpm.
t = Total time since start of bailing.
t' = Time since bailing stopped.



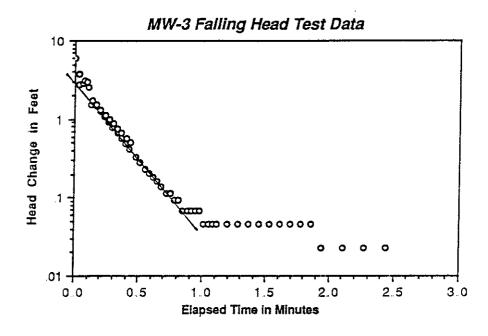


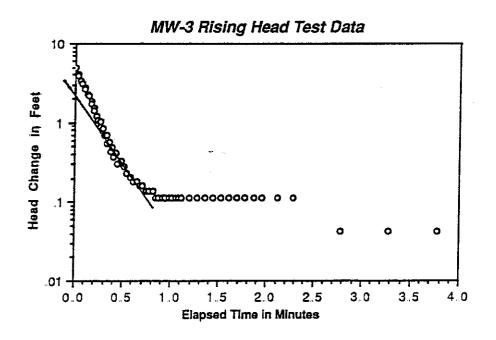


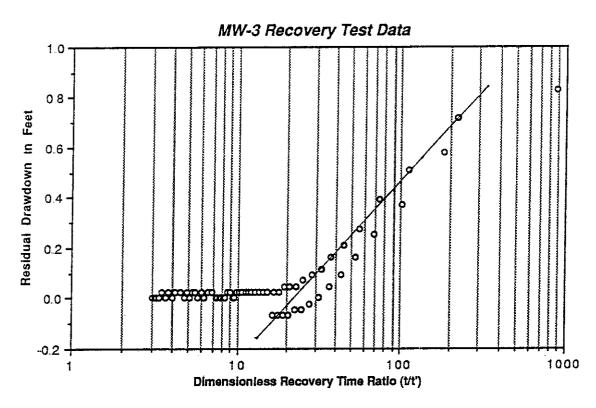
Notes: Well bailed at 0.67 gpm.

t = Total time since start of bailing.

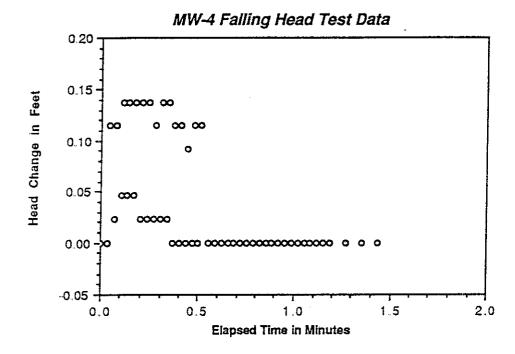
t* = Time since bailing stopped.



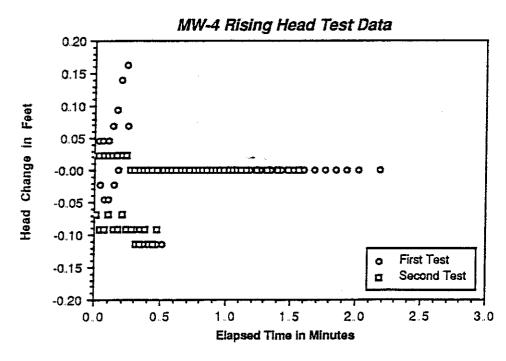




Notes: Well bailed at 1..1 gpm...
t = Total time since start of bailing...
t' = Time since bailing stopped...



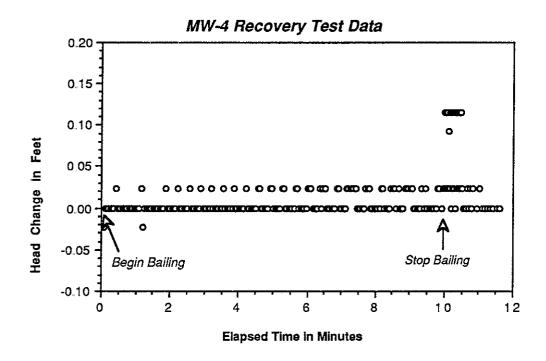
Notes: 1) Unable to analyze data.
2) Unable to plot head change data on log axis because of zero or negative values.



Notes: 1) Unable to analyze data.

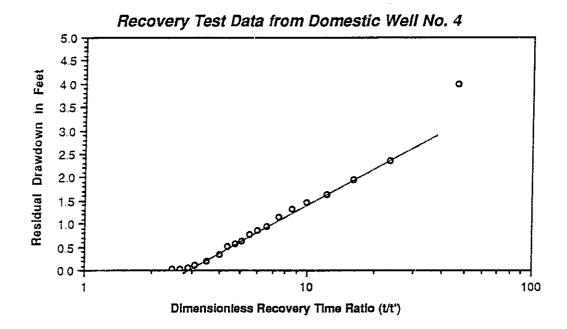
2) Unable to plot head change data on log axis because of zero or negative values.





Notes: 1) Unable to analyze data.

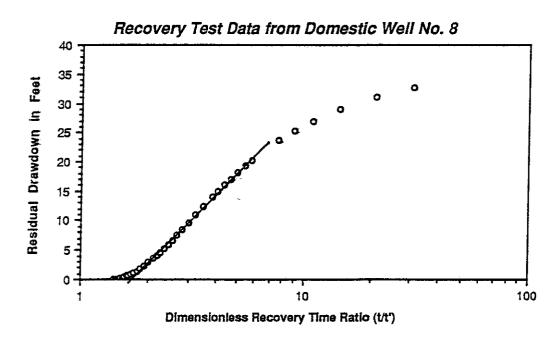
2) Unable to plot head change data on log axis because of zero or negative values.



Note: Well pumped at 10 gpm.

t = Total time since start of pumping...

t' = Time since pump shut off.



Note: Well pumped at 8 gpm.

t = Total time since start of pumping.

t' = Time since pump shut off.



APPENDIX D WELL LOG INVENTORY SOUTH KING COUNTY GROUNDWATER MANAGEMENT PROGRAM DATABASE

APPENDIX D WELL LOG INVENTORY SOUTH KING COUNTY GROUNDWATER MANAGEMENT PROGRAM DATABASE

This appendix contains the geologic well log reports and the well construction reports from the South King County Groundwater Management Plan database. Figure 12 displays the location of the majority of these wells, along with the approximate well depth and water level elevation at time of drilling.

The "local number" can be used to locate a particular well in the data base and on Figure 12. The local number is composed of three major terms. The first term is the township number, which provides a general north-south component to a well's location. In the case of Lake Sawyer, it is either "21N" (township 21 north) or "22N". The dividing line between these two townships is displayed on the upper right hand side of Figure 12.

The second term in the local number is the range, which provides an east-west component to a well's location. For the Lake Sawyer study, all wells are located in range 6E.

The first number of the final term is the section in which the well is located, within the specified township and range. Section numbers are included on Figure 12. Each section is alphabetically subdivided. Thus the letter component of this term provides further information on the location of the well within the section. The final number in this term is used to differentiate between wells in the same alphabetical subdivision of a section.

For example, the local number "22N/06E-32H01" specifies the following well location:

22N - Township 22 north;

06E - Range 6 east;

32 - Section 32 (within Township 22 north, Range 6 east);

H - Alphabetical subdivision within Section 32; and

01 - Number 1 well within subdivision H.

In this appendix the geologic well log reports are presented first, followed by the well construction reports. Within each of these two groups, the wells in Township 22N are presented first, followed by the wells in Township 21N. The wells in each township are arranged according to section number, starting with the lowest section and going to the highest.

The domestic wells used in the monitoring system could not always be correlated to a specific well number, as indicated in the following list:

Domestic Well Number	Local Number
1	9
1	<i>(</i>
2	?
3	?
4	21N/06E-04B07
5	21N/06E-03P02
6	?
7	?
8	?

GEOLOGIC WELL LOG REPORTS

trisjanjapinniikaattakkiitäs katt geologio well 100 Report täkk kantiikkinniikkiikkiikkiikkiikkiik

3115	10	472118122	033401	EOCAL NUMB	ER 22N:06E	-32A01	
LATII	TUDE	47 d 21 m	18 s	LO	INGITUDE	122 d 03 m	34 s
ALTII	TUDE	520.00		DATE OF CO	NSTRUCTION	02 / 15 / 19	77
CEPTH	OF WELL	145.00		DEP	TH OF HOLE	14500	
HATER	LEVEL	120,63		DAT	E MEASURED	02 / 22 / 19	77
SRILL	.ER	JOHNSON	CON	STRUCTION M	ETHOD A	TYPE OF FIN	ISH O
OHNER ID	SYSTEM	NAME			CONTACT		PHONE
HAM149	HAMME	R, LARRY					
	# I	NTVL(FT)	DESCRIPT	ION	±		
	2 : 3 :	23- 132	BROWN BROWN	GRAVEL AND ! HARDPAN BOUI	LDERS		
				WATER SAND I HARDPACKED I		AVEL	
	7 :	-					

	BITEID			47.21	18172	0341	5)i	LOCA	L MI	UHBER 22N:	06E-	32A02			
	LATITU	Œ		47 d	21 m	18	5			LONGITUDE		122	d 03	n 40	ē
	ALTITU	ΒE		51	0.00			DATE	0F	CONSTRUCT	ION	04 /	29 (1980	
	DEPTH	OF Y	ÆLL	13	8.00				į	EPTH OF H	OLE	138	.00		
	WATER	LEVE	EL.	10-	4.00				į	DATE MEASU	RED	9 5 /	01 /	1980	
	DRILLE	R		JOHN	30N		CON	STRUC	[OI	N METHOD A		TYPE	OF I	FINIS	H 0
OWN	ER ID	SYS	STEM	NAME						CONTA	CT				PHONE
000)87	DC	DGE,	, JACI	(
		INT		ATVL(8	T)	DES	CRIPT	ION							
		G1 F3 44 U3 4B	* * * * * * * * * * * * * * * * * * *	2 98- 115-	78 115 138	9	JURFAC JROWN JRAY H JROWN JRAY C	HARDPA ARDPAN WATER	I SF GRA	WEL					

*********************** **** GEGLOGIC WELL LOG REPORT **** ****************

OWNER ID SYSTEM	NAME	CONTACT	PH01
DRILLER	STORY/ARMSTR	CONSTRUCTION METHOD C TYPE OF A	FINISH S
JATER LEVEL	30 00	DATE MEASURED 03 / 12 /	1980
DEPTH OF WELL	100.00	DEPTH OF HOLE 275.00	
ALTITUDE	415.00	DATE OF CONSTRUCTION 02 / 18 /	1980
LATITUDE	47 d 21 m 24 s	LONGITUDE 122 a 04	a 22 s
SITEID	4721241220422)	1 LOCAL NUMBER 22N/05E-32C01	

CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

INTVL(FT) DESCRIPTION

INT

1	;	0- 25	GRAVEL AND BOULDERS
2	;	25- 29	DIRTY BROWN SANDY GRAVEL
3	;	29- 31	DIRTY GRAVEL, WATER BEARING
			CEMENTED GRAVEL WITH BOULDERS
		50~ 64	
6	:	64- 65	VERY DIRTY SAND, WATER BEARING
			DIRTY GRAVEL, WATER BEARING
			TIGHT DIRTY GRAVEL
9	1	77- 90	BROWN CLAY BOUND SRAVEL
			SAND AND GRAVEL, WATER BEARING
			TIGHT CEMENTED GRAVEL
12	E 2	103- 236	BLUE CLAY
13		236- 251	SILTY CLAY, WATER BEARING
14	:	251- 275	STICKY BLUE CLAY

BITEID	472124122044401	LOCAL NUMBER 22N/06E-	-32001
LATITUDE	47 d 21 m 24 s	LONGITUDE	122 d 04 m 44 s
ALTITUDE	420 " ÚÐ	DATE OF CONSTRUCTION	1 1
DEPTH OF W	ELL 36:00	DEPTH OF HOLE	000
WATER LEVE	L 21.00	DATE MEASURED	1 1
DRILLER	MAXWELL	CONSTRUCTION METHOD	TYPE OF FINISH
OWNER ID SYS	TEM NAME	CONTACT	PHOME

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

SITEI	0	472112122	033401	LOCAL N	IMBER 22N/061	E-32H01	
LATIT	JDE	47 d 21 m	12 s		LONGITUDE	122 d 0	3 m 36 s
ALTITU	UDE	50000		DATE OF	CONSTRUCTION	4 09 / 10	/ 1975
DEPTH	OF WE	LL 106.00		1	EPTH OF HOLE	105.00	
#ATER	LEVEL	84.00		Î	IATE MEASUREI	09 / 16	/ 1975
DRILLE	ER	JOHNSON	Cí	ONSTRUCTION	METHOD C	TYPE OF	FINISH P
f ID	SYSTI	EM NAME			CONTACT		PHONE
15	GREI	EN, KEN					
	INT	INTVL(FT)	DESCRI	PTION	***************************************		
	E-1 160 44 110	8- 72 72- 96 96- 106	BROWN BROWN	I HARDPAN I CLAY SHOC	XED WITH SRA	VEL	
	LATITE PLTITE PEPTH HATER PRILLE	ATITUDE PLTITUDE PETH OF WE HATER LEVEL PRILLER ID SYSTI S GREE INT 1: 2: 3: 4: 5:	ATITUDE 47 d 21 m ALTITUDE 500.00 ATER LEVEL 84.00 ATER LEVEL JOHNSON A ID SYSTEM NAME A INT INT INT INT 7 INTVL(FT) 1: 0- 8 2: 8- 72 3: 72- 76	ATITUDE 47 d 21 m 12 s NLTITUDE 500.00 DEPTH OF WELL 106.00 HATER LEVEL 84.00 DRILLER JOHNSON C TID SYSTEM NAME S GREEN, KEN INT 1 INTVL(FT) DESCRIPTION 2: 8- 72 BROWN 3: 72- 96 BROWN 4: 96-106 HARDF 5: -	ATTITUDE 47 d 21 m 12 s NUTTITUDE 500.00 DATE OF DEPTH OF WELL 106.30 I MATER LEVEL 84.00 I ORILLER JOHNSON CONSTRUCTION ORILLER JOHNSON CONSTRUCTION INT # INTVL(FT) DESCRIPTION 1 : 0 - 8 SANDY BROWN GRA 2 : 8 - 72 BROWN HARDPAN 3 : 72 - 96 BROWN CLAY SHOC 4 : 96 106 HARDPACKED BROWN 5 : -	ATTITUDE 47 d 21 m 12 s LONGITUDE NUTTITUDE 500.00 DATE OF CONSTRUCTION DEPTH OF WELL 106.00 DEPTH OF HOLD MATER LEVEL 84.00 DATE MEASURED DRILLER JOHNSON CONSTRUCTION METHOD C TID SYSTEM NAME CONTACT THE STATE OF BESCRIPTION 1: 0- 8 SANDY BROWN GRAVEL AND ROCK 2: 8- 72 BROWN HARDPAN 3: 72- 96 BROWN CLAY SHOCKED WITH GRAVEL, WAS 5: -	HATER LEVEL 84.00 DATE MEASURED 09 / 16 PRILLER JOHNSON CONSTRUCTION METHOD C TYPE OF TID SYSTEM NAME CONTACT S GREEN, KEN INT # INTVL(FT) DESCRIPTION 1 : 0- 8 SANDY BROWN GRAVEL AND ROCKS 2 : 8- 72 BROWN HARDPAN 3 : 72- 96 BROWN CLAY SHOCKED WITH GRAVEL 4 : 96- 106 HARDPACKED BROWN GRAVEL, WATER SEEP 5 : -

SITEI	5)	472041122	2040201
LATIT	JGE	47 d 20 s	n 41 s LONGITUDE 122 d 04 m 02 s
ALTIT	JDE	455.00	DATE OF CONSTRUCTION 02 / 20 / 1973
DEPTH	OF WELL	95.00	DEPTH OF HOLE 95.00
HATER	LEVEL	32,00	DATE MEASURED 02 / 23 / 1973
DRILLE	ER	NORTHWEST	CONSTRUCTION METHOD C TYPE OF FINISH P
OWNER ID	SYSTEM	NAME	CONTACT PHONE
9L0028	BLOOM	, RONALD	
	INT # I	NTVL(FT)	DESCRIPTION
	2: 3: 4: 5:	1- 30 30- 42 42- 56 56- 77	TOPSOIL SAND AND GRAVEL (TIGHT) BLUE GLACIAL TILL BROWN GLACIAL TILL BR TIGHT WATER BEARING SD & GR SILICA SANDSTONE WATER BEARING SAND GRAVEL

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SITEII	ļ	472)45122	040001 LOCAL NUMBER 22N:03E-32803
LATITL	JDE	47 d 20 m	45 s LONGITUDE 122 d 04 m 00 s
ALTITL	DE	45 5.00	DATE OF CONSTRUCTION 10 / 22 / 1971
DEPTH	OF WEL	i 87.00	DEPTH OF HOLE 87.00
WATER	LEVEL	37.00	DATE MEASURED 10 / 25 / 1971
DRILLE	IR .	NORTHWEST	CONSTRUCTION METHOD C TYPE OF FINISH O
OWNER ID	SYSTE	M NAME	CONTACT PHONE
262017	Ħ&L	KUSTOM KURE	MARVIN PERAULT 631-1237
	1NT	INTVL(FT)	DESCRIPTION
	2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0- 2	TOPSGIL
	1 2 : 3	0- 2 2- 11 11- 50	TOPSOIL SAND AND GRAVEL GLACIAL TILL BROWN
	1 2 : 3	0- 2 2- 11 11- 50	TOPSOIL SAND AND GRAVEL GLACIAL TILL BROWN
	and CA by agr	0- 2 2- 11 11- 50 50- 53	TOPSOIL SAND AND GRAVEL
	44 54 52 54 55	0- 2 2- 11 11- 50 50- 53 53- 75	TOPSOIL SAND AND GRAVEL SLACIAL TILL, BROWN SAND AND GRAVEL, CEMENTED

DRILLER	JOHNSON DRIL	CONSTRUCTION METHOD C TYPE OF FINISH P	
WATER LEVEL	33.00	DATE MEASURED / /	
DEPTH OF WELL	93. 00	DEPTH OF HOLE 0.00	
ALTITUDE	455.00	DATE OF CONSTRUCTION 09 / 19 / 1974	
LATITUBE	47 d 20 m 43 a	LONGITUDE 122 d 03 m 44 s	
SITEID	472)43122/33440)	L LOCAL NUMBER 22N/06E-32R01	

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

[H]

7	INIVLIFII	DESCRIPTION

į	:	0-	2	SURFACE
2	•	2-	14	LOOSE BROWN GRAVEL, ROCKS
3	;	14-	22	BROWN HARDPAN GRAVEL
4	;	22-	44	BROWN SANDY CLAY GRAVEL
5	;	44	58	SRAY HARDPAN
5	Ţ	58~	37	BROWN HARDPAN
7	;	87	38	WATER GRAVEL
8	:	88-	93	HARDPACKED BROWN GRAVEL
9	:	-		
10	;	-		
11	;	-		
12	;	-u		
13	;	-		
14	:	-		

SITEID	472125122025701	LOCAL NUMBER 22N/06E	-33001
LATITUDE	47 d 21 m 25 s	LONGITUDE	122 d 02 m 57 s
ALTETUDE	550.00	DATE OF CONSTRUCTION	09 / 25 / 1979
DEPTH OF WELL	159.00	CEPTH OF HOLE	159.00
NATER LEVEL	120.00	DATE MEASURED	10 / 04 / 1979
DRILLER	MUELLER CO.	NSTRUCTION METHOD C	TYPE OF FINISH S
OWNER ID SYSTEM	NAME	CONTACT	PHONE
NIE303 NIEDE	RKORN, P		
INT # [NTVL(FT) DESCRIP	TION	
	0- 20 SURFAI 20- 45 HARDPA		
	45- 50 SAND	TH DIVERN	
	50- 88 HARDP	AN GRAY	
	98- 140 HARDPA		
6 :	140- 159 SAND A	AND GRAVEL 25 GPM	

7: -

TIBLESIA PER ARTE LOG REPORT IN A LIBERTA PER ARTE LOG REPORT IN A LIBERTARIA PER ARTE LOG REPORT IN A LIBERTARIA PER ARTE LOG REPORT ARTE LOG

SITEID 472127120032401 LOCAL NUMBER 22N/06E-33D01

LATITUDE 47 d 21 m 27 s LONGITUDE 122 d 00 m 32 s

ALTITUDE 558 00 DATE OF CONSTRUCTION 01 / 01 / 1958

DEPTH OF WELL 175.00 DEPTH OF HOLE 0.00

WATER LEVEL 0.00 DATE MEASURED / /

DRILLER MYRL JOHNSON CONSTRUCTION HETHOD TYPE OF FINISH

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

dinternations and the LOG REPORT area constructed that areas and areas a

SITEID			4721	19122	0318	301	L004	L M	JMBER 2	2N/06E	-3300:	2			
LATITU	DΕ		47 d	21 a	18	9			LONGIT	UDE	122	d 03	g 18	3 5	
ALTITU	DE		53	500			DATE	0F	CONSTRI	UCTION	08 /	26 /	198()	
DEPTH (OF W	∉ELL	14	0.00				I	EPTH O	F HOLE	140	0.00			
WATER (LEVE	L	9	7.00				I	ATE ME	ASURED	08 /	28 /	1980)	
DRILLE	R		JOHN	SON		CO)	NSTRUC'	TION	I METHO) A	TYP	OF :	FINIS	6H 0	
ER ID	SYS	TEM	NAME						C0)	TACT				PHONE	
325	PE	DRIN	, RO	BERT											
			TVL()	FT)	DES	CRIPI	ION			- 40 July 40 July 40					
	2 3 4 5 6	; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;	3- 45- 48- 91- 117-	45 68 91 117 140	8 6 8	ROWN RAY E RAY H ROWN	HARDPA RAVEL IARDPAN HARDPA	i N S	RAVEL						
	LATITU ALTITU DEPTH WATER DRILLE	LATITUDE ALTITUDE DEPTH OF F WATER LEVE DRILLER ER ID SYS 325 PE INT 4 5 4 5	LATITUDE ALTITUDE DEPTH OF WELL WATER LEVEL DRILLER ER ID SYSTEM 325 PEDRIM INT # IN 1: 2: 3: 4: 5: 6:	LATITUDE 47 d ALTITUDE 53 DEPTH OF WELL 14 WATER LEVEL 9 DRILLER JOHN ER ID SYSTEM NAME 325 PEDRIN, RO. INT # INTVL(1) 1: 0- 2: 3- 3: 45- 4: 68- 5: 91- 6: 117-	LATITUDE 47 d 21 m ALTITUDE 535.00 DEPTH OF WELL 140.00 WATER LEVEL 97.00 DRILLER JOHNSON ER ID SYSTEM NAME 325 PEDRIN, ROBERT INT # INTVL(FT) 1: 0- 3 2: 3- 45 3: 45- 68 4: 68- 91 5: 91- 117	LATITUDE 47 d 21 m 18 ALTITUDE 535.00 DEPTH OF WELL 140.00 WATER LEVEL 97.00 DRILLER JOHNSON ER ID SYSTEM NAME 325 PEDRIN, ROBERT INT	LATITUDE 47 d 21 m 18 s ALTITUDE 535.00 DEPTH OF WELL 140.00 WATER LEVEL 97.00 DRILLER JOHNSON COM ER ID SYSTEM NAME 325 PEDRIN, ROBERT INT # INTVL(FT) DESCRIPT 1: 0- 3 SURFACT 2: 3- 45 BROWN 3: 45- 68 GRAY 67 4: 68- 91 GRAY 67 5: 91- 117 BROWN 6: 117- 140 GRAY 67	LATITUDE 47 d 21 m 18 s ALTITUDE 535.00 DATE DEPTH OF WELL 140.00 WATER LEVEL 97.00 DRILLER JOHNSON CONSTRUCT ER ID SYSTEM NAME 325 PEDRIN, ROBERT INT	LATITUDE 47 d 21 m 18 s ALTITUDE 535.00 DATE OF DEPTH OF WELL 140.00 I WATER LEVEL 97.00 I DRILLER JOHNSON CONSTRUCTION ER ID SYSTEM NAME 325 PEDRIN, ROBERT INT # INTVL(FT) DESCRIPTION 1 : 0- 3 SURFACE 2 : 3- 45 BROWN HARDPACKE 3 : 45- 68 GRAY GRAVEL 4 : 68- 91 GRAY HARDPAN 5 : 91- 117 BROWN HARDPAN S 6 : 117- 140 GRAY WATER SAND	LATITUDE 47 d 21 m 18 s LONGITO ALTITUDE 535.00 DATE OF CONSTRI DEPTH OF WELL 140 00 DEPTH OF WATER LEVEL 97.00 DATE MED DRILLER JOHNSON CONSTRUCTION METHOD ER ID SYSTEM NAME CON 325 PEDRIN, ROBERT INT # INTVL(FT) DESCRIPTION 1 : 0- 3 SURFACE 2 : 3- 45 BROWN HARDPACKED GRAVE 3 : 45- 68 GRAY GRAVEL 4 : 68- 91 GRAY HARDPAN 5 : 91- 117 BROWN HARDPAN GRAVEL 6 : 117- 140 GRAY WATER SAND AND GR	LATITUDE 47 d 21 m 18 s LONGITUDE ALTITUDE 535.00 DATE OF CONSTRUCTION DEPTH OF WELL 140.00 DEPTH OF HOLE WATER LEVEL 97.00 DATE MEASURED DRILLER JOHNSON CONSTRUCTION METHOD A ER ID SYSTEM NAME CONTACT INT # INTVL(FT) DESCRIPTION 1 : 0- 3 SURFACE 2 : 3- 45 BROWN HARDPACKED GRAVEL 3 : 45- 68 GRAY GRAVEL 4 : 68- 91 GRAY HARDPAN 5 : 91- 117 BROWN HARDPAN GRAVEL 6 : 117- 140 GRAY WATER SAND AND GRAVEL	LATITUDE 47 d 21 m 18 s LONGITUDE 122 ALTITUDE 535.00 DATE OF CONSTRUCTION 08 / DEPTH OF WELL 140.00 DEPTH OF HOLE 140 WATER LEVEL 97.00 DATE MEASURED 08 / DRILLER JOHNSON CONSTRUCTION METHOD A TYPE ER ID SYSTEM NAME CONTACT INT # INTVL(FT) DESCRIPTION 1 : 0- 3 SURFACE 2 : 3- 45 BROWN HARDPACKED GRAVEL 3 : 45- 68 GRAY GRAVEL 4 : 68- 91 GRAY HARDPAN 5 : 91- 117 BROWN HARDPAN GRAVEL 6 : 117- 140 GRAY WATER SAND AND GRAVEL	ALTITUDE 535.00 DATE OF CONSTRUCTION OB / 26 / DEPTH OF WELL 140.00 DEPTH OF HOLE 140.00 WATER LEVEL 97.00 DATE MEASURED OB / 28 / DRILLER JOHNSON CONSTRUCTION METHOD A TYPE OF ER ID SYSTEM NAME CONTACT 325 PEDRIN, ROBERT INT # INTVL(FT) DESCRIPTION 1 : 0- 3 SURFACE 2 : 3- 45 BROWN HARDPACKED GRAVEL 3 : 45- 68 GRAY GRAVEL 4 : 68- 91 GRAY HARDPAN 5 : 91- 117 BROWN HARDPAN GRAVEL 6 : 117- 140 GRAY WATER SAND AND GRAVEL	LATITUDE	LATITUDE

	BITEID	47211512202420	1 LOCAL NUMBER 22N/06E-33601	
	LATITUDE	47 d 21 m 15 s	LONGITUDE 122 d 02 m 42 s	
	ALTITUDE	480.00	DATE OF CONSTRUCTION 01 / 01 / 1900	
	DEPTH OF WELL	32.00	DEPTH OF HOLE 0.00	
	WATER LEVEL	9.71	DATE MEASURED 01 / 28 / 1963	
	GRILLER		CONSTRUCTION METHOD D TYPE OF FINISH	
OWN	IER ID SYSTEM	NAME	CONTACT PHONE	

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

	SITEID	47205412202210	1 LOCAL NUMBER 22N/05E	-33J01
	LATITUDE	47 d 20 m 54 s	LONGITUDE	122 d 02 m 21 s
	ALTITUDE	550.00	DATE OF CONSTRUCTION	05 / 11 / 1976
	DEPTH OF WELL	130 .00	DEPTH OF HOLE	130 , 00
	WATER LEVEL	104.00	DATE MEASURED	05 / 11 / 1976
	DRILLER	NW PUMP & DR	CONSTRUCTION METHOD A	TYPE OF FINISH
C	WNER ID SYSTEM	NAME	CONTACT	PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

INT

불	INTVL(FT)	DESCRIPTION	

1: 0- 2 TOPSOIL
2: 2- 27 BROWN TILL
3: 27- 78 BLUE TILL
4: 78- 99 BROWN TILL
5: 99- 110 BROWN CEMENTED SAND AND GRAVEL
6: 110- 130 WATER BEARING SAND AND GRAVEL

7: --

	SITEID	472)5812202200	1 LOCAL NUMBER 22N/06E	-33101
	LATITUDE	47 d 20 m 58 s	LONGITUDE	122 d 02 m 20 s
	ALTITUDE	58700	DATE OF CONSTRUCTION	09 / 03 / 1974
	CEPTH OF WELL	91.00	DEPTH OF HOLE	000
	WATER LEVEL	76 00	DATE MEASURED	09 / 03 / 1974
	DRILLER	JOHNSON DRLE	CONSTRUCTION METHOD C	TYPE OF FINISH P
OHN	ER ID SYSTEM	NAME	CONTACT	PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

Ī	N	Ţ	
	4		

ŧ	IN)VL(FI)	DESCRIFTION	

1: 0- 2 SURFACE
2: 2- 16 BROWN HARDPAN
3: 16- 56 SRAY HARDPAN GRAVEL
4: 56- 82 HARDPAN GRAVEL
5: 82- 83 WATER GRAVEL
6: 83- 91 BROWN HARDPAN

7: -

SITEID	47205912202210	LOCAL NUMBER 22N/06E-	-33302
LATITUDE	47 i 20 m 59 s	LONGITUDE	122 d 02 a 71 s
ALTITUDÉ	587.00	DATE OF CONSTRUCTION	11 / 26 / 1974
DEPTH OF WELL	135.00	DEPTH OF HOLE	0.00
WATER LEVEL	117.00	DATE MEASURED	11 / 26 / 1974
DRILLER	NORTHWEST PU	CONSTRUCTION METHOD C	TYPE OF FINISH
OWNER ID SYSTEM	NAME	CONTACT	PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

INT

#	INTVL(FT)	DESCRIPTION	

1: 0- 3 TOPSOIL

2: 3- 34 BROWN GLACIAL TILL

3: 34- 72 BLUE BLACIAL TILL 4: 72- 128 BROWN GLACIAL TILL

5: 128-135 WATER BEARING SAND AND GRAVEL

6 : -

7: -

	SITEI)	472053122	021601	LOCAL HUM	BER 22N:06E	-33304	
	LATIT	JDE	47 d 20 s	53 s	L	ONGITUDE	122 d 02 a	16 s
	ALTITU	JDE	550.00		DATE OF C	ONSTRUCTION	11 / 16 / 19	177
	DEPTH	OF WELL	160.00		DE	PTH OF HOLE	160.00	
	WATER	L.EVEL	110.00		DA	TE MEASURED	11 / 22 · 19	177
	DRILLE	ī R	NORTHWEST	CONS	TRUCTION I	METHOD A	TYPE OF FIN	ISH P
OHNE	ER ID	SYSTEM	NAME			CONTACT		PHONE
WAI4	41	STIAW	, DAN					
		INT # II	NTVL(FT)	DESCRIPTI	0N		n an air air an an an	
		2 : 3 : 4 :	0- 3 3- 51 51- 72 72- 120	BROWN T BLUE TI BROWN T	ILL LL ILL	MATER LAVE	- FO	
		J :	1207 140	DAUMM	ILL W/Ubb.	. WATER LAYE	GA.	

6: 143-160 BLUE TILL 7: -

SITEII)	47203912	2031501 LOCAL NUMBER 22N 06E-33N01
LATIT	JDE	47 d 20 a	m 39 s LONGITUDE 122 d 03 m 15 s
ALTITU	Œ	435,00	DATE OF CONSTRUCTION 02 / 18 / 1983
DEPTH	OF WELL	57.00	DEPTH OF HOLE 57.00
WATER	LEVEL	3,00	DATE MEASURED 02 / 18 / 1983
DRILLE	R	JOHNSON	CONSTRUCTION METHOD A TYPE OF FINISH O
OWNER ID	SYSTEM	NAME	CONTACT PHONE
BAS019	BASS		
	INT # [!	NTVL(FT)	DESCRIPTION
	Let 153 at 123	4- 8 8- 18 18- 28 28- 53	GRAVEL BROWN SANDY CLAY GRAY SAND AND GRAVEL GRAY SAND BROWN SILT GRAY WATER SAND AND GRAVEL

7: -

SITEID	47/2049122025701	LOCAL NUMBER 22N-06E-	33P
LATITUDE	47 d 20 m 49 s	LONGITUDE	122 d 02 m 57 s
ALTITUDE	525.00	DATE OF CONSTRUCTION	12 + 01 / 1977
DEPTH OF WELL.	7500	DEPTH OF HOLE	75.00
WATER LEVEL	45,25	DATE MEASURED :	11 / 17 / 1977
DRILLER	BURT WELL DR	CONSTRUCTION METHOD C	TYPE OF FINISH S
OWNER ID SYSTEM	NAME	CONTACT	PHONE

CWNER INFORMATION NOT AVAILABLE FOR THIS WELL

INT #	INTVL	FT) 	DESCRIPTION
i:	: Ů-	25	TILL
2 :	25-	47	LARGE GRAVEL AND SAND
3 :	47-	66	GRAVEL, SAND AND WATER
4	66-	75	BROWN SILTY SAND
5 :	-		
5 :			
7 :			

	SITEID	47204812202570	1 LOCAL NUMBER 22N/06E	-3 3 P02
	LATITUDE	47 d 20 m 48 s	LONGITUDE	122 d 02 m 57 s
	ALTITUDE	510.00	DATE OF CONSTRUCTION	01 / 03 / 1978
	DEPTH OF WELL	72.00	DEPTH OF HOLE	72,00
	WATER LEVEL	42.00	DATE MEASURED	12 / 18 : 1977
	DRILLER	BURT WELL DR	CONSTRUCTION METHOD C	TYPE OF FINISH S
OWNI	ER ID SYSTEM	NAME	CONTACT	PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

INT

#	INTVL(FT)	DESCRIPTION

1: 0- 7 BOULDERS

2: 7- 38 BROWN HARDPAN

3: 38- 65 LARGE GRAVEL, SAND AND WATER

4: 65- 72 BROWN HARDPAN

5;

6:

7:

atar GEOLOGIC WELL LOE REFORT that transprending well Loe Refort that

	BITEIS	47204812203030	1 - LOCAL MUMBER 22N 68E-	TUF03
	LATITUDE	47 d 20 m 46 s	LONGITUDE	122 d 03 m 03 s
	ALTITUDE	455.00	DATE OF CONSTRUCTION	1 1
	DEPTH OF WELL	0.00	DEPTH OF HOLE	695.00
	WATER LEVEL	0.00	DATE MEASURED	ı İ
	DRILLER	ARMSTRONG	CONSTRUCTION METHOD C	TYPE OF FINISH
01	WWER ID SYSTEM	NAME	CONTACT	PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

INT

#		,	INTVL(FT)	DESCRIPTION		
			* 100 C** C** T** T** T** T** T** T** T**			
	1	3	0- 18	BROWN CLAY WITH SAND AND BRAVEL		
	2	:	18- 33	BLUE GRAY CLAY W/SAND AND GRAVEL		
٠						
	4	:	47- 53	BLUE GRAY CLAY W/SAND AND GRAVEL		
	5	:	53- 72	BLUE SANDY CLAY WITH BRAVEL		
	ģ	:	72- 85	BROWN CLAY-BOUND SAND AND GRAVEL		
	Ĭ	;	85- 96	DARK BRWN CLAY-BOUND SD & GR		
	8	:	98- 140	BLUE GRAY SILTY SAND		
	7	;	140- 155	BLUE GRAY SANDY CLAY		
	10	:	155- 1 88	BLUE GRAY SILTY SAND W/SOME GRAVEL		
	11	:	188- 255	GRAY SLTY CLAY W/EEPTH		
	12	•	255~ 280	GRAY CLAY W/YERY FINE SAND		
				GRAY CLAY W/SAND AND SRAVEL		
	14	;	285- 296	GRAY SILTY SANDY CLAY		
				GRAY SANDY CLAY WITH SOME GR. HARD		
	16	:	315- 335	GRAY SILTY CLAY		
	17	:	335- 344	GRAY SANDY CLAY		
	18	:	344- 358	GRAY SILTY CLAY		
	19	ŧ	33 8 - 362	GRAY SILTY CLAY W/ABUNDANT GRAVEL		
;	20	;	362- 474	GRY SLTY CLY W/OCC. SRAVEL		
	21	•		GRY SILTY CLA/ AND GRAVEL		
,	22	:	475- 595	SRAY SILTY CLAY W/OCC. GRAVEL		
	23	ţ	5 95- 6 52	GRAY CLAY, HARD		
				GRAY SILTY CLY W/GRAVEL, GR INCREAS		
			672 - 676			
				GRAY SILTY VERY FINE SAND		
	_			SRAY SANDY SILT		
2	28	:	6 85- 695	GRAY SILT		

BEREATERERESSER SERVICE SERVIC

SITEI	D	4721031220	10201	LOCAL I	KUMBER 22N/04	E-34H01	
CATIT	1DE	47 d 21 m)3 =		LONGITUDE	122 d 01 m	02 s
ALTITI	UDE	500 00		DATE OF	CONSTRUCTIO	N 01 / 03 / 19	180
DEPTH	OF WELL	155.00			DEPTH OF HOL	E 155.00	
MATER	LEVEL	11900			DATE MEASURE	D 01 / 04 / 19	9 0
DRILLE	ī,	NORTHWEST	CON	STRUCTIO	IN METHOD A	TYPE OF FIN	19H O
DWNER ID	SYSTEM	NAME			CONTACT		PHONE
SVE409	SVERDA	NRSK√, DAVE					
	INT # IN	ITVL(FT) [)ESCRIPT	ION			
	2:	0- 32 32- 125 125- 147	BLUE T	ILL AND			

4: 147- 155 WATER BEARING SAND AND GRAVEL

5 : 6 : 7 :

SITEID	472_401220115)1	LGCAL NUMBER 22N+96E-	-34001
LATITUGE	47 d 20 m 40 s	LONGITUDE	122 d 01 m 15 s
ALTITUDE	575. 00	DATE OF CONSTRUCTION	06 / 20 - 1978
DEPTH OF N	ELL 98.00	DEPTH OF HOLE	98. 00
WATER LEVE	L 53.00	DATE MEASURED	06 / 20 / 1978
DR ILLER	NORTHWEST CO	NSTRUCTION METHOD A	TYPE OF FINISH O
OWNER ID SYS	TEM NAME	CONTACT	PHONE
LAS238 LA	SHER, DAVE		
INT #	INTVL(FT) DESCRIP	TION	
2 3	: 0- 2 TOPSD: : 2- 28 BROWN : 28- 77 Blue ' : 77- 98 Water	ŢILL	<u>1</u>

5: -6: -7: -

SITEID	472050122010001	LOCAL NUMBER 22N-06E-	-34 R 01
LATITUDE	47 d 20 m 50 s	LONGITUDE	122 d 01 m 00 s
ALTITUDE	550.00	DATE OF CONSTRUCTION	06 / 17 / 1980
DEPTH OF WE	LL 80.00	DEPTH OF HOLE	80.00
HATER LEVEL	40,00	DATE MEASURED	06 / 17 / 1980
DRILLER	NORTHWEST	CONSTRUCTION METHOD A	TYPE OF FINISH O
OWNER ID SYST	akan ma	CONTACT	PHONE
MAI257 MAI	ERS, LAURETTA		
INT \$	INTVL(FT) DESCR	IPTION	

1: 0- 1 TOPSOIL, BROWN

2: 1- 34 SAND AND GRAVEL, BROWN, DRY

3: 34- 55 TILL, BLUE, DAMP

4: 55- 80 WATER BEARING SAND AND GRAVEL

5 : -6 : -

7 : -

SIT	EID	1729	371220	010101	LOCAL	NUMBER 2	22N/08E-	-34802		
LAT	ITUDE	47 d	20 m	37 s		LONGIT	TUDE	122 d	01 a 01	s
ALT.	ITUDE	55	0.00		DATE O	F CONSTR	RUCTION	06 / 09	7 / 1983	
DEPT	TH OF I	4ELL 5	5.00			DEPTH O	IF HOLE	65. ()0	
WATE	ER LEVI	EL 3	0.00			DATE ME	ASURED	06 / 09	7 / 1983	
DRIL	LER	JOHN	SON	CON	ISTRUCTI	OHT3K KO	ID A	TYPE O	F FINIS	H 0
OWNER II) SYS	STEM NAME				CO	INTACT			PHONE
DRL092	DF	RLLEVICH,	VAL							432-0200
	1 N 1	INTVL(FT) 	DESCRIPT	TION					
	28454	: 0- : 3- : 40- : 65- : - : -	60 65	BROWN GRAY W	HARDPAN IATER GR					

Si	TEID	472003122015701	LOCAL NUMBER 21N/04E-	-03E01
LA	KTITUDE	47 d 20 a 03 s	LONGITUDE	122 d 01 m 57 s
AL	TITUDE	500.00	DATE OF CONSTRUCTION	01 / 01 / 1958
DΞ	PTH OF WELL	51.00	DEPTH OF HOLE	0.00
WA	ITER LEVEL	12.00	DATE MEASURED	01 / 01 / 1962
DR	HLLER	MYRL JOHNSON	CONSTRUCTION METHOD	TYPE OF FINISH
OWNER	ID SYSTEM	NAME	CONTACT	PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

į	JANER ID SYSTEM	NAME	CONTACT	PHONE
	GRILLER	SANDPOINT	CONSTRUCTION METHOD V	TYPE OF FINISH
	WATER LEVEL	0.00	DATE MEASURED	i = t
	DEPTH OF WELL	16.00	DEPTH OF HOLE	0.00
	ALTITUDE	495.00	DATE OF CONSTRUCTION	1 1
	LATITUDE	47 d 20 m 20 s	LONGITUDE	122 d 0 1 m 56 s
	SITEID	47202012201560	1 LOCAL NUMBER 21N/06E-	03E02

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

SITEID	47200412201390	1 LOCAL NUMBER 21N/06E-	-03L01
LATITUDE	47 d 20 m 04 s	LONGITUDE	122 d 01 a 39 s
4LTITUDE	550::00	DATE OF CONSTRUCTION	1 1
DEPTH OF WELL	107.00	DEPTH OF HOLE	0.00
WATER LEVEL	0.00	DATE MEASURED	<i>}</i>
ORILLER		CONSTRUCTION METHOD	TYPE OF FINISH
OWNER ID SYSTEM	NAME	CONTACT	PHONE

DONALD FURMAN

886-2264

SAWYERWOOD ESTATES W. S.

76460N

RESELECTE CONTRACTOR C

SITEID	47202112202070	1 LOCAL NUMBER 21N/06E	-03P02
LATITUDE	47 d 19 m 46 s	LONGITUDE	122 á 01 m 46 s
ALTITUDE	515.00	DATE OF CONSTRUCTION	1 1
DEPTH OF WELL	50.00	DEPTH OF HOLE	0.00
WATER LEVEL	30.34	DATE MEASURED	08 / 16 / 1962
DRILLER	DORSTEN	CONSTRUCTION METHOD	TYPE OF FINISH
OWNER ID SYSTEM	NAME	CONTACT	PHONÉ
56185A MORRIS	S BROTHERS		

SITEID 472030122023701 LOCAL NUMBER 21N/06E-04B01

LATITUDE 47 d 20 m 30 s LONGITUDE 122 d 02 m 37 s

ALTITUDE 500.00 DATE OF CONSTRUCTION / /

DEPTH OF WELL 50.00 DEPTH OF HOLE 0.00

WATER LEVEL 32.61 DATE MEASURED 08 / 16 / 1962

DRILLER XYRL JOHNSON CONSTRUCTION METHOD TYPE OF FINISH

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

HERRETERS OF WELL LOG REPORT FREE PROPERTY OF THE PROPERTY OF

SITEID	47202712202400	1 LOCAL NUMBER 21N/06E	-(4802
LATITUDE	47 d 20 m 27 s	LONGITUDE	122 d 0 2 m 40 s
ALTITUDE	500.00	DATE OF CONSTRUCTION	01 / 01 / 1951
DEPTH OF WELL	5000	DEPTH OF HOLE	000
WATER LEVEL	33.87	DATE MEASURED	08 / 15 / 1982
DRILLER	MYRL JOHNSON	CONSTRUCTION METHOD	TYPE OF FINISH

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

OWNER ID SYSTEM NAME

DISTIE	47203112202330	1 LOCAL NUMBER 21N/08E	-()4803
LATITUDE	47 d 20 m 27 s	LONGITUDE	122 d 02 m 45 s
ALTITUDE	50000	DATE OF CONSTRUCTION	09 / 01 / 1959
DEPTH OF WELL	54 . ÛÛ	DEPTH OF HOLE	0.00
HATER LEVEL	27., 47	DATE MEASURED	08 / 16 / 1962
DRILLER	MYRL JOHNSON	CONSTRUCTION METHOD	TYPE OF FINISH
OWNER ID SYSTEM	NAME	CONTACT	PHONE
LAD200 LADDEF	?AD		

SITEID	47203112202330	2 LOCAL NUMBER 21N CSE	-0480301
LATITUDE	47 d 20 m 27 s	LONGITUDE	122 d 62 m 45 s
ALTITUDE	500.00	DATE OF CONSTRUCTION	f = f
DEPTH OF WELL	65.00	DEPTH OF HOLE	0.00
WATER LEVEL	38.17	DATE MEASURED	08 / 12 / 1986
DRILLER		CONSTRUCTION METHOD	TYPE OF FINISH
DWNER ID SYSTEM	NAME	CONTACT	PHONE

LA0300

LADDERUND, LOIS

SITEID	472026122024501	LOCAL NUMBER 21N/06E-0	4804
LATITUDE	47 d 20 m 25 s	LONGITUDE	122 d 02 m 45 s
ALTITUDE	502.00	DATE OF CONSTRUCTION O	7 / 13 / 1961
DEPTH OF WELL	48.00	DEPTH OF HOLE	0.00
WATER LEVEL	28.00	DATE MEASURED 0	
DRILLER	JOHNSON	CONSTRUCTION METHOD C	TYPE OF FINISH P
OWNER ID SYSTEM	NAME	CONTACT	PHONE

1N:	!	INTVL(F	Τ)	DESCRIP	TION			 _
1 2		0- 28-	28 48		GRAVEL SAND AND	GRAVEL		
3	;	~						
4 5							•	
5	•	-						
7	;	-						

SITEID	47202812202420	1 LOCAL NUMBER 21N/06E	-04804
LATITUDE	47 d 70 m 32 s	LONGITUDE	122 á 02 a 39 s
ALTITUDE	500.00	DATE OF CONSTRUCTION	1 1
DEPTH OF WELL	8500	DEPTH OF HOLE	0.00
WATER LEVEL	43)0	DATE MEASURED	03 / 01 / 1971
DRILLER		CONSTRUCTION METHOD	TYPE OF FINISH O
OWNER ID SYSTEM	NAME	CONTACT	PHONE

EC0400

ECOLOGY, DEPT OF

	SITEIS	ļ	47202	25122	024101	LOCAL	NUMBE	R 21N/08	E-04805			
	LATITU	ÐΕ	47 ธ	20 g	27 s		i.ON	GITUDE	122	d 02 (n 46 s	٠
	ALTITU	DE	50(000		DATE	OF CON	STRUCTIO	IN 01 /	02 / 3	1976	
	DEPTH	OF WE	ILL 83	3.00			DEPT	H OF HOL	.E 83	.00		
	WATER	LEVEL	. 43	5.00			DATE	MEASURE	D 01 / 9	02 / 1	1976	
	DRILLE	R	JOHNS	BON	Cí	NSTRUCT	ION ME	THOO A	TYPE	OF F	INISH S	
OWNI	ER ID	SYST	EH NAME					CONTACT			PHONE	
416	508	KCY	ID 105					JUDITH	NELSON		631-	0565
		INT #	INTVL(F	T)	DESCRIF	TION		n en en 10 au en en en				
		C3 -5- 67 E3	0- 3- 20- 52- -	20 52	GRAVE	iL 	· ·	SRAVEL,				

	SITEID	4720301220238	801 LOCAL NUMBER 21N/06E-04B07	
	LATITUDE	47 d 20 m 30	s LONGITUDE 122 d 02 m 38	5
	ALTITUDE	500.00	DATE OF CONSTRUCTION 08 / 01 / 1956	
	DEPTH OF WELL	80.00	DEPTH OF HOLE 0.00	
	WATER LEVEL	40.00	DATE MEASURED 08 / 01 / 1956	
	ERILLER		CONSTRUCTION METHOD B TYPE OF FINISH	P
OWN	ER ID SYSTEM	NAME	CONTACT P	HONE

!	472025	122024501	LOCAL	NUMBER 21	I/06E-04B0	8	
IJΕ	47 d 2	0 a 33 s		LONGITUI)E 122	d 02	m 40 s
ĐE	500.	00	DATE 0	IF CONSTRUC	TION 12 /	13 /	1975
OF WE	L 85.	00		DEPTH OF	HOLE 8	500	
LEVEL.	43	00		DATE MEAS	URED 12 /	13 /	1975
2	JOHNSO	M (CONSTRUCTI	ON METHOD	A TYP	E OF F	INISH S
SYSTE	IM NAME			CONT	ACT		PHONE
KCHI	105			100	ITH NELSO	M.	6 31-0565
INT == 	INTVL(FT) DESCR	PTION				
3 4 5 6	3- : 20- 52- :	20 GRAV 52 BRN	EL HARDPAN G	RAVEL			
	DE D	DE 47 d 2 DE 500. OF WELL 85. LEVEL 43 R JOHNSO SYSTEM NAME KCMD 105 INT # INTVL(FT 1: 0- 2: 3- 3: 20- 4: 52- 6: -	DE 47 d 20 m 33 s DE 500.00 OF WELL 85.00 LEVEL 43 00 R JOHNSON 6 SYSTEM NAME KCWD 105 INT # INTVL(FT) DESCRITE 1: 0- 3 SURF 2: 3- 20 GRAV 3: 20- 52 BRN 4: 52- 85 WATE 5: - 6: -	### INTVL(FT) DESCRIPTION 1 : 0 - 3 SURFACE 2 : 3 - 20 GRAVEL 3 : 20 - 52 BRN HARDPAN G 4 : 52 - 85 WATER GRAVEL 5 : -	1	122 102 103 104 105 105 106 107 108	LEVEL. 43 00 DATE MEASURED 12 / 13 / R JOHNSON CONSTRUCTION METHOD A TYPE OF F SYSTEM NAME CONTACT KCWD 105 JUDITH NELSON INT # INTVL(FT) DESCRIPTION 1 : 0~ 3 SURFACE 2 : 3~ 20 GRAVEL 3 : 20~ 52 BRN HARDPAN GRAVEL 4 : 52~ 85 WATER GRAVEL 5 : ~ 6 : ~

SITEID	4720291220241	1 LOCAL NUMBER 21N/05E-04809	
LATITUDE	47 d 20 m 29 s	LONGITUDE 122 d 02 m 41 s	
ALTITUDE	534.00	DATE OF CONSTRUCTION 03 / 24 / 1980	
DEPTH OF WELL	87.00	DEPTH OF HOLE 92.00	
WATER LEVEL	38.00	DATE MEASURED 04 / 03 / 1980	
ORILLER	STORY/ARMSTR	CONSTRUCTION METHOD C TYPE OF FINISH	S
OWNER ID SYSTEM	NAKE	CONTACT PH	ONE

IN:		INTVL(F	T)	DESCRIPTION
í	:	() -	31	BRWN SD & GR #/BOULDERS, TIGHT
2	1	31-	62	BRWN SD & GR TO 8" LOOSE, SOME WTR
3	;	52 -	54	TIGHT SD & GR CONFINING LAYER
4	*	64-	70	CRS GRAVEL AND SAND
5	•	70-	78	CRSE GRAVEL, VERY LITTLE SAND
6	;	78-	82	CASE GRAVEL AND SAND
7	;	82-	70	GRAVEL AND SAND
8	:	90-	92	CLAYBOUND SD & GR. TAN BINDER
3	;	-		
10	;	-		
11	:			
12	ţ	-		
13	:			
14	ė	-		

	SITEID	47202912203000	1 LOCAL NUMBER 21N/06E	-04001
	LATITUDE	47 d 20 m 29 s	LONGITUDE	122 d 03 m 00 s
	ALTITU DE	495.00	DATE OF CONSTRUCTION	01 / 01 / 1963
	DEPTH OF WELL	67.00	DEPTH OF HOLE	0.00
	WATER LEVEL	35.47	DATE MEASURED	08 / 15 / 1962
	DRILLER	JOHNSON DRIL	CONSTRUCTION METHOD	TYPE OF FINISH
0#	NER ID SYSTEM	NAME	CONTACT	PHONE

OWNER ID SYSTEM	NAKE	CONTACT	PHONE
ORILLER	NORTHWEST PU	CONSTRUCTION METHOD	TYPE OF FINISH
WATER LEVEL	3700	DATE MEASURED	10 / 25 / 1971
DEPTH OF WELL	87 00	DEPTH OF HOLE	0.00
ALTITUDE	510.00	DATE OF CONSTRUCTION	10 / 25 / 1971
LATITUDE	47 d 20 a 29 s	LONGITUDE	122 d 03 m 12 s
SITEID	472029122031201	LOCAL NUMBER 21H/08E-	-04001

SITEID	472018132032401	LOCAL NUMBER 21N/06E-04E01	
LATITUDE	47 d 20 a 18 s	LONGITUDE 122	1 03 m 24 s
ALTITUDE	55000	DATE OF CONSTRUCTION 06	01 / 1975
DEPTH OF WELL	123.00	DEPTH OF HOLE 0	.00
MATER LEVEL	108.00	DATE MEASURED 06 / (01 / 1975
DRILLER	EVERGREEN DR	CONSTRUCTION METHOD C TYPE	OF FINISH
OWNER ID SYSTEM	NAME	CONTACT	PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

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7	INIVL(FI)	DESCRIPTION	

1	;	0- 5	TOPSOIL.
2	;	5- 58	GRAY HARDPAN
3	ŧ	58- 67	BROWN TILL COMPACT WITH ROCKS
4	ž	67- 71	DIRTY TIGHT GRAVEL
5	;	71- 83	GRAY SANDY CLAY
6	:	83- 96	GRAY HARD COMPACT SANDY SILT
7	ī	95- 103	GRAY CLAY
8	:	103- 123	W.B. SD&GR PROGRESSIVELY SET CLEANR
9	:	-	
10	:	~	
11	÷	-	
12	;	•	
13	ţ	-	
14	:	-	

SITEID	47202917202590	1 LOCAL NUMBER ZIN/065	-04F01
LATITUDE	47 d 20 m 29 s	LONGITUDE	122 d 02 m 39 s
ALTITUDE	515.00	DATE OF CONSTRUCTION	$\ell = \ell$
DEPTH OF WELL	47.00	DEPTH OF HOLE	0.00
WATER LEVEL	31.43	DATE MEASURED	08 / 15 / 1962
DRILLER	CLYDE DORSTE	CONSTRUCTION METHOD	TYPE OF FINISH
OWNER ID SYSTEM	NAME	CONTACT	PHONE

	SITEID	472017122023901	LOCAL NUMBER 21N/06E-	04601
	LATITUDE	47 d 20 m 17 s	LONGITUDE	122 d 02 m 39 s
	ALTITUDE	500.00	DATE OF CONSTRUCTION	11 / 21 / 1957
	DEPTH OF WELL	80.00	DEPTH OF HOLE	0.00
	WATER LEVEL	0.00	DATE MEASURED	l i
	DRILLER	MARTIN R LUT	CONSTRUCTION METHOD C	TYPE OF FINISH
OWNE	R ID SYSTEM	NAME	CONTACT	PHONE

*********************** **** GEOLOGIC WELL LOS REFORT **** 422212333277222333553275225

-1	·•:	•=====================================	interpretation of	meet grow	ಶ್ರೆಗ ಕ್ರಾಪ್ತಿಕ್ಕಾಗಿ ಪ್ರವಿಧ್ಯಾಸಕ್ಕೆ ಸಂಪ್ರಿಸಿಕ್ಕಾಗಿ ಪ್ರವಿಧ್ಯಾಸಿಕ್ಕಾಗಿ ಪ್ರವಿಧ್ಯಾಸಿಕ್ಕಾಗಿ ಪ್ರವಿಧ್ಯಾಸಿಕ್ಕಾಗಿ ಪ್ರವಿ ಪ್ರಶ್ನೆಗಳು	A4584	
51121	Ŀ	4:10011122	0 <u>7</u> 4531	LULAL AUN	958 Z1N/05E	-04101	
LATIT	UDE	47 d 20 m	01 s	Ĺ,	ONGITUDE	122 d 02 s	9 43 s
4LTIT	UDE	510.00	Ī	DATE OF C	ONSTRUCTION	10 / 26 / 3	1983
DEPTH	OF WELL	99.00		DE	PTH OF HOLE	99.00	
#ATER	LEVEL	4000		DA	TE MEASURED	10 / 26 / 1	1983
DRILLS	ER	NORTHWEST	CONST	RUCTION	METHOD A	TYPE OF FI	INISH O
OWNER ID	SYSTEM	NAME			CONTACT		PHONE
284277	TOLLES	ER, GENE					886-1176
	INT # II	ATVL(FT)	DESCRIPTIO	H			

1: 0- 2 TOPSDIL, BROWN, DRY

2- 11 GLACIAL TILL BROWN, DRY 2:

3: 11- 21 SAND AND OCC. GRAVEL, BROWN, DRY

4 : 21- 47 GLACIAL TILL, BLUE, DRY

5: 47- 99 WATER BEARING SAND AND GRAVEL

ó:

7 :

ε	10466D SMITH	, CLYDE	D. & A.	JOHNSON 986-1762
	NAMER ID SYSTEM	NAME	CONTACT	PHONE
	DRILLER	(CONSTRUCTION METHOD	TYPE OF FINISH
	WATER LEVEL	21.37	DATE MEASURED	07 / 23 / 1986
	DEPTH OF WELL	60.00	DEPTH OF HOLE	0.00
	ALTITUDE	510.00	DATE OF CONSTRUCTION	i = I
	LATITUDE	47 d 20 m 01 s	LONGITUDE	122 d 02 m 39 s
	SITEID	472001122023901	LOCAL NUMBER 21N/06E	-04894

SITEID	47194912202420	1 LOCAL NUMBER 21N:06E-04901	
LATITUSE	47 d 19 m 49 s	LONSITUDE 122 d 02 m 42	s
ALTITUDE	520.00	DATE OF CONSTRUCTION 01 / 19 / 1968	
DEPTH OF WELL	75.00	DEPTH OF HOLE 0.00	
WATER LEVEL	25.00	DATE MEASURED 01 / 19 / 1968	
DRILLER	STIMSON DRIL	CONSTRUCTION METHOD V TYPE OF FINISH	
OWNER ID SYSTEM	NAME	CONTACT P	HONE

SITEI	D	4719471220	023401	LOCAL NUMBE	R 21N:06E	-04002	
LATIT	JDE	47 d 19 m	47 s	LON	SITUDE	122 d 02 m	35 s
4LTITI	JDE	520.00		DATE OF CONS	STRUCTION	02 / 22 / 19	4 7
GEPTH	OF WELL	5900		DEPTH	OF HOLE	5900	
WATER	LEVEL	33.00		DATE	MEASURED	62 22 / 19	6 7
DRILLE	R	JOHNSON DR	ILL CON	STRUCTION MEI	'HOD C	TYPE OF FIN	ISH P
OWNER ID	SYSTEM	MAHE			CONTACT		PHONE
76452P	SAWYE	RWOOD WATER	HETEYS 1		DOUGLAS.	H. 45.	
	INT # I	NTVL(FT)	DESCRIPT	ION		9 49 al , 45 48 58 11 al	
	2 : 3 : 4 :	5- 17 17- 43 43- 59 59-	HARDPAI HARDPAI WATER S	L AND ROCKS N AND GRAVEL CKED SAND AND GAND AND GRAV GRAVEL	GRAVEL		

	BITEID	47195812202420	01 LOCAL NUMBER 21N/065-04803	
	LATITUDE	47 d 19 m 52 s	LONGITUDE 122 d 02 m 40 s	
	ALTITUDE	515.00	DATE OF CONSTRUCTION 09 / 18 / 1975	
	DEPTH OF WELL	52.00	DEPTH OF HOLE 52.00	
	#ATER LEVEL	25,00	DATE MEASURED 09 / 18 / 1975	
	DRILLER	JCHNSON	CONSTRUCTION METHOD C TYPE OF FINISH P	
OWN	ER ID SYSTEM	I NAME	CONTACT PHONE	
057	BENSC	IN, EDWARD		

SITEID	471952122022601	LOCAL NUMBER 21N/06E-04R01
LATITUDE	47 d 19 m 52 s	LONGITUDE 122 d 02 m 26 s
ALTITUDE	500.00	DATE OF CONSTRUCTION / /
DEPTH OF WELL	27.00	DEPTH OF HOLE 0.00
WATER LEVEL	15.17	DATE MEASURED 08 / 14 / 1962
DRILLER		CONSTRUCTION METHOD D TYPE OF FINISH

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

OWNER ID SYSTEM NAME

SITEID	47203312203380	I LOCAL NUMBER 21N/06E	-05A01
LATITUBE	47 d 20 m 33 s	LONGITUBE	122 d 03 m 39 s
ALTITUDE	525 00	DATE OF CONSTRUCTION	05 / 12 + 1974
GEPTH OF WELL	90.00	DEPTH OF HOLE	000
WATER LEVEL	5.00	DATE MEASURED	05 / 12 / 1974
DRILLER	JOHNSON DR C	CONSTRUCTION METHOD C	TYPE OF FINISH P
OWNER ID SYSTEM	NAME	CONTACT	PHONE

1141				
#		INTVL(F	T)	DESCRIPTION
				111 page 112 and 112 a
İ	1	0-	5	SURFACE
2	:	3-	15	BROWN HARDPAN
3	ļ	15-	46	BLUE CLAY
4	1	46-	90	FINE WATER SAND
5	*			
4	:	-		
7	÷	-		

SITEID 471917122043501 LOCAL NUMBER 21N/06E-08E01

LATITUDE 47 d 19 m 17 s LONGITUDE 122 d 04 m 35 s

ALTITUDE 530.00 DATE OF CONSTRUCTION 01 / 01 / 1957

DEPTH OF WELL 65.00 DEPTH OF HOLE 0.00

WATER LEVEL 8.00 DATE MEASURED / /

DRILLER HYRL JOHNSON CONSTRUCTION METHOD TYPE OF FINISH

OWNER ID SYSTEM NAME CONTACT PHONE

telegangan kanan an telegangan kanan an telegan an te

	SITELD	47192112204180	1 LOCAL NUMBER 21N:06E	-08F()1
	LATITUDE	47 d 19 m 21 s	LONGITUDE	122 d 04 m 18 s
	ALTITUDE	47000	DATE OF CONSTRUCTION	06 / 03 / 1982
	DEPTH OF WELL	119.00	DEPTH OF HOLE	119.00
	MATER LEVEL	0.00	DATE MEASURED	1 /
	DRILLER	STORY/DOEGE	CONSTRUCTION METHOD C	TYPE OF FINISH
04)	HER ID SYSTEM	NAME	CONTACT	PHONE
				*

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

INT

₩	TMIAT(LI)	DESTRILION

1: 0- 80 BROWN, GRY-BRWN GR W/VARYING MTR	X
2: 80-102 SRY SD & GR W/SLT MTRX, SOME WAT	ER
3: 102-116 SD, MED-F W/OCC. 9R	
4: 116-119 GRAY SANDY, SILTY, CLAY	
5 ; -	
<u> </u>	
7: -	

	SITEID	47193112203370	1 LOCAL NUMBER 21N/06E-08H01	
	LATITUDE	47 d 19 m 31 s	LGN6ITUDE 122 d 03 m 37 s	
	ALTITUDE	490.00	DATE OF CONSTRUCTION 09 / 09 / 1982	
	DEPTH OF WELL	12700	DEPTH OF HOLE 127.00	
	WATER LEVEL	4 00	DATE MEASURED 09 / 12 / 1982	
	DRILLER	STORY/DODGE	CONSTRUCTION METHOD C TYPE OF FINISH F	
OW	NER ID SYSTEM	NAME	CONTACT PHONE	

OHNER INFORMATION NOT AVAILABLE FOR THIS WELL

INT

#	INTVL(FT)	DESCRIPTION	

1		ψ- <u>8</u>	SRAVEL AND SAND, SILT MATRIX
2	:	8- 17	GRAY-BRWN SAND AND GRAVEL
3	;	17- 33	GRAY-BROWN TILL
4	:	33- 74	YELLOW-BRWN SANDY SLTY CLAY OCC. SR
5	;	74- 89	FINE SAND, SILT, CLAY
6	;	8 9 - 117	GRAY, COMPACT TILL
7	;	117- 127	GRY-BLK HUDDY, CRS SD GRADING TOCKY

SITEID 471945122022201 LOCAL NUMBER 21N/06E-09A01

LATITUDE 47 d 19 m 45 s LONGITUDE 122 d 02 m 22 s

ALTITUDE 500.00 DATE OF CONSTRUCTION / /

DEPTH OF WELL 27.00 DEPTH OF HOLE 0.00

WATER LEVEL 14.69 DATE MEASURED 08 / 14 / 1962

DRILLER MYRL JOHNSON CONSTRUCTION METHOD TYPE OF FINISH

DWNER ID SYSTEM NAME CONTACT PHONE

SITEID	4717381220224)1 LOCAL NUMBER 21N/06E	-07A02
LATITUDE	47 d 19 m 38	LONGITUDE	122 d 02 m 24 s
ALTITUDE	525.00	DATE OF CONSTRUCTION	1 1
DEPTH OF	WELL 39.00	DEPTH OF HOLE	0.00
WATER LEVI	EL 28.02	DATE MEASURED	08 / 16 / 1962
DRILLER	MRYL JOHNSN	CONSTRUCTION METHOD	TYPE OF FINISH
OWNER ID SYS	STEM NAME	CONTACT	PHONE

TERRARAMENTE ENTRE DE CONTENENT DE CONTENENT DE CONTENENT DE CONTENENT DE CONTENENT DE CONTENENT DE CONTENENT DE CONTENENT DE CONTENET DE

SITEID 461733122021401 LGCAL NUMBER 218706E-09H01

LATITUDE 47 d 19 m 33 s LGNSITUDE 122 d v2 m 14 s

ALTITUDE 515.00 DATE OF CONSTRUCTION / /

OCITIONE ATOMAA BUILD OF PROGRAMMA 1200

DEPTH OF WELL 48.00 DEPTH OF HOLE 0.00

WATER LEVEL 25.00 DATE MEASURED / /

DRILLER CONSTRUCTION METHOD TYPE OF FINISH

OWNER ID SYSTEM NAME CONTACT PHONE

	SITEID	47140012202040)1 LOCAL NUMBER 21N/06E-	-10E01
	LATITUDE	47 d 19 a 00 s	LONGITUDE	122 d 02 m 04 s
	ALTITUDE	500.00	DATE OF CONSTRUCTION	1 1
	DEPTH OF WELL	45. 00	DEPTH OF HOLE	0.00
	WATER LEVEL	26.00	DATE HEASURED	/ /
	DRILLER	NORTHWEST DR	CONSTRUCTION METHOD	TYPE OF FINISH
OWN	HER ID SYSTEM	NAME	CONTACT	PHONE

SITEID	47192712202040	1 LOCAL NUMBER 21N/06E-	·10E02
LATITUDE	47 d 19 m 27 s	LONGITUDE	122 d 02 m 04 s
ALTITUDE	510.00	DATE OF CONSTRUCTION	1 - 1
DEPTH OF WELL	42.00	DEPTH OF HOLE	0.00
WATER LEVEL	30.47	DATE MEASURED	08 / 14 / 1962
DRILLER		CONSTRUCTION METHOD	TYPE OF FINISH

CONTACT

PHONE

CHNER INFORMATION NOT AVAILABLE FOR THIS WELL

OWNER ID SYSTEM NAME

OWNER ID SYSTEM	NAME	CONTACT	PHONE
DRILLER		CONSTRUCTION METHOD	TYPE OF FINISH
WATER LEVEL	27.43	DATE MEASURED	08 / 14 / 1965
DEPTH OF WELL	42.00	DEPTH OF HOLE	0.00
ALTITUDE	510.00	DATE OF CONSTRUCTION	01 + 01 / 1952
LATITUBE	47 d 19 m 24 s	LONGITUDE	122 d 02 m 03 s
SITEID	47192412202030	1 LOCAL NUMBER 21N/06E	-10E03

SITEID	471924122015101	LOCAL NUMBER 21N/06E-10F01	
LATITUDE	47 d 19 m 24 s	LONGITUDE 122 d 01 m 51 s	
ALTITUDE	500.00	DATE OF CONSTRUCTION 01 / 01 / 1930	
DEPTH OF WELL	2200	DEPTH OF HOLE 0.00	
HATER LEVEL	20.00	DATE MEASURED / /	

ORILLER CONSTRUCTION METHOD V TYPE OF FINISH T

OWNER ID SYSTEM NAME CONTACT PHONE

SITEID	471826122022201	LOCAL NUMBER 21N/06E	-16J01
LATITUDE	\$7 d 18 m 20 s	LONGITUDE	122 d 02 m 26 s
ALTITUDE	525.00	DATE OF CONSTRUCTION	06 / 09 / 1976
DEPTH OF WELL	99.00	DEPTH OF HOLE	99.00
WATER LEVEL	55.00	DATE MEASURED	12 / 20 / 1976
DRILLER	JOHNSON DRIL CON	STRUCTION METHOD A	TYPE OF FINISH O
TYPE OF SEAL	C BOTTOM OF SE	AL 18 METHOD OF	DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS DEEPENED FROM 63 TO 99FT, 12/20/76

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
1	0.00	6300	4.00
2	6300	99.00	5.00

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
1	0.00	63.00	6. 00	9	0.000
2	0.00	99.00	6. 00	3	0.000

OPENINGS INFORMATION

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE (

TYPE OF POWER

HORSEPOWER 0.00

BITEIE	47182312202410	1 LOCAL NUMBER 21N/06E-	16k01
LATITUDE	47 d 18 m 23 s	LONGITUDE	122 d 02 m 41 s
ALTITUDE	540.00	DATE OF CONSTRUCTION	01 / 06 / 1984
DEPTH OF WELL	117 00	DEPTH OF HOLE	117.00
WATER LEVEL	93.00	DATE MEASURED	01 / 10 - 1984
DRILLER	JOHNSON	CONSTRUCTION METHOD A	TYPE OF FINISH O
TYPE OF SEAL	C BOTTOM OF	SEAL 18 METHOD OF	DEVELOPMENT A
DEVEL METHOD	A HOURS OF	DEVEL. 3	

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

POZZI, CARL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 117.00 5.00

** CASING INFORMATION **

BOTTOM DIAMETER INTERVAL TOP CASING CASING NUMBER CASING CASING CASIN6 MATERIAL THICKNESS 0.00 117.00 6.00 0.000 1

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS DENINGS

PUMP INFORMATION

0,00

TYPE OF LIFT

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

HORSEPOWER

SI	ITEID		47 1	19251.	22000	101	LOCA	L NI	imber 21	LN/04E	-11601	l		
LA	ATITUD	E	47	d 19	n 25	5			LONGIT	IDE	122	d 00	m 01	ş
AL	OUTIT.	Ε	É	40.0)		DATE	OF	CONSTRU	ICTION	03 /	11 /	1981	
DЕ	PTH O	F WEL	.L 1	.27.00)			Ð	EPTH OF	HOLE	127	00		
ЯΑ	ITER L	EYEL		50 ()()			Ď	ATE MEA	ISURED	03 /	11 /	1981	
DR	ILLER		JOH	NSON		COM	STRUCT	rion	METHOD	A	TYPE	OF F	INISH	p
OWNER	iD :	SYSTE	H NAM	E					CON	TACT			P	HONE
CHE045		CHEA	THAM,	RAY										
		INT #	INTVL	(FT)	288	CRIPT	10N							
		2: 3: 4: 5:	4- 12- 70- 127-	- 12 - 70 - 127 -	6		DECAYE ANDSTO	ΝE	OCK - Water					

SITEL	<u>:</u>	471925121	591801	LOCAL 1	NUMBER 21N/C	&E-11H01	
LATIT	3 0 E	47 d 19 m	25 з		LONGITUDE	121 d 59	7 n 48 s
ALTIT	JDE	815.00		DATE OF	CONSTRUCTI	ON 02 / 04 /	1980
CEPTH	OF WELL	240.00			DEPTH OF HO	LE 240.00	
FATER	LEVEL	40.00			DATE MEASUR	ED 02 - 04 /	1980
CRILLE	ir.	JOHNSON	CON	ISTRUCTIO	N METHOD A	TYPE OF	FINISH P
OWNER ID	SYSTEH	NAME			CONTAC	T	PHONE
REI342	REICHE	ERT, MATHE	Ą				
	INT	ITVL(FT)	DESCRIPT	10N			
	2 :	0- 3 3- 12 12- 48	BROWN	DECAYED	ROCK		

4: 48-215 WHITE SOLID ROCK

5: -7: -

5: 215- 240 WHITE SOLID ROCK - WATER BEARING

SITEID	47183412203020	1 LOCAL NUMBER 21N+05E-15F01	
LATITUDE	47 d 18 m 32 s	LONGITUDE 122 d 03 m	02 s
ALTITUDE	515.00	DATE OF CONSTRUCTION 06 / 24 / 1	980
DEPTH OF WELL	10300	DEPTH OF HOLE 103.00	
WATER LEVEL	43,00	DATE MEASURED 06 / 24 / 1	98 0
DRILLER	NORTHWEST PD	CONSTRUCTION METHOD A TYPE OF FI	NISH 0
OWNER ID SYSTEM	NAME	CONTACT	PHONE

INT #	INTVL(FT)	DESCRIPTION
2 5 4	2- 28 28- 57 57- 98	TOPSOIL, BROWN SAND AND GRAVEL, BROWN SILTY SAND AND GRAVEL, BLUE GLCL TILL, BL, SM-AWNT OF WAT.100" WATER BEARING SAND AND GRAVEL

**** GEOLOGIC WELL LOG REPORT ****

SITEID	47132012202220	1 LICAL NUMBER 21N:06E	-14101
LATITUDE	47 d 18 m 20 s	LONGITUDE	122 d 02 m 25 s
ALTITUDE	525.00	DATE OF CONSTRUCTION	06 / 09 / 1976
DEPTH OF HELL	99.00	DEPTH OF HOLE	99,00
GAFER LEVEL	65 00	DATE MEASURED	12 : 20 / 1976
ORILLER	JOHNSON DRIL	CONSTRUCTION METHOD A	TYPE OF FINISH O

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

OWNER ID SYSTEM NAME

ΙN	Ī			
#		INTVL(F	T)	DESCRIPTION
1	:	A	₹	SURFACE
				BROWN HARDPAN GRAVEL
-			_	GRAY BROWN GRAVEL
_	-	_		-
	•	• •		GRAY BROWN HARDPACKED GRAVEL
5	;	46-	47	WATER GRAVEL (20PM)
<u> </u>	:	47-	53	BROWN HARDPACKED BRAVEL, W. SEEPAGE
7	;	63-	79	BROWN HARDPAN GRAVEL
8	•	79	99	BROWN GRAVEL (WATER)
ģ	i	-		
10	į	-		
11	;	-		
12	•	-		
13	;	-		
14	;	-		

SITEII)	471 823 122	024101 LOCAL NUMBER 21N:06E-16K01	
LATITU	JDE	47 á 18 m	23 s LONGITUDE 122 d 02 m 41 s	
ALTITU	JDE	540.00	DATE OF CONSTRUCTION 01 / 06 / 1984	
DEPTH	OF WEI	L 117.00	DEPTH OF HOLE 117.00	
WATER	LEVEL	93.00	DATE MEASURED 01 / 10 / 1984	
DRILLE	R	JOHNSON	CONSTRUCTION METHOD A TYPE OF FINISH O	
OWNER ID	SYSTE	M NAME	CONTACT PHONE	
P0Z335	P022	I. CARL		
		- 1		
	INT		DESCRIPTION	

RESERVATE SERVE SE

	BITEI	3		47	191	:122)247)1	LOC	ń.	NUMBI	ER 21	N/06E	-15	Q()1				
	LATITU	IDE		47	d I	L8 a	14	Ę			L01	46ITL	IDE	1.	22	d 0:	2 a	47	3
	ALTITU	ЮE		Ş	525.	.00			DAT	Ξ 0	F COI	VSTRL	ICTION	1)4	1	15 /	19	181	
	DEPTH	OF	WEL	Ŀ	78.	.50					066	TH OF	HOLE		78	50			
	WATER	LEV	EL		42.	.00					DATE	HEA	SURED	04	1	15	/ 15	81	
	TRILLE	9		NOR	(TH)	EST		CON	ISTAUC	ΙΤΙ	ON HE	THOD	A	Ti	rPE	ÐΕ	FIN	ISH	Ð
OWNE	ER 10	SY	STE	M NAM	E							CON	TACT					P	HONE
AND(08	Al	NDE	RSON,	JO	Ε						-							
		IN.	-	INTVL	(FT	·)	DE3(CRIPT	ION										
		2 3 4 5 6	***		- - -	55	51	LACIA	L TIL	Ĺ,	DRY,	980	OFT WN, MI GRAVI		НА	Ches			

WELL CONSTRUCTION REPORTS

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

HAN149 HANNER, LARRY

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 145.00 6.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 1 0.00 145.00 6.00 S 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

PUMP INFORMATION

TYPE OF LIFT

DEPTH OF PUMP INTAKE (

TYPE OF POWER

BITEID	47211	212203400	1	LOCAL N	JMBER 22	2M7 06E	-32A02			
LATITUDE	47 d	21 m 18 s			LONGITU	JDE	122	d 03	m 40	5
ALTITUDE	510	.00		DATE OF	CONSTRU	ICTION	04 /	29 /	1980	
DEPTH OF WELL	138	,00		Ī	EPTH OF	HOLE	138	.00		
WATER LEVEL	104	00		I	ATE MEA	SURED	05 /	01 /	1980	
DRILLER	JOHNS	DN	CONS	TRUCTION	METHOD	A	TYPE	OF I	FINISH	0
TYPE OF SEAL	C	BOTTOM O	F SEAL	18	HETH	OD OF	DEVEL	OPME!	47	
DEVEL. METHOD		HOURS OF	DEVEL	3						
REMARKS										

CONTACT

PHONE

\$ \$	HOLE	INFORMATION	\$ \$

OWNER ID SYSTEM NAME

DODGE, JACK

000039

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
MUMBER	HOLE	HOLE	OF HOLE
i	0.00	138.00	6.00

** CASING INFORMATION **

INTERVAL	TOP	MOTTOE	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASIN6	MATERIAL	THICKNESS
<u>1</u>	0.00	138.00	6.00	S	0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID	472124122042201	LOCAL NUMBER 22N/06E	-32C01
LATITUDE	47 d 21 a 24 s	LONGITUDE	122 d 04 m 22 s
ALTITUDE	415.00	DATE OF CONSTRUCTION	02 / 18 / 1980
DEPTH OF WELL	100.00	DEPTH OF HOLE	27500
WATER LEVEL	30.00	DATE MEASURED	03 / 12 / 1980
DRILLER	STORY/ARMSTR CON	STRUCTION METHOD C	TYPE OF FINISH S
TYPE OF SEAL	B BOTTOM OF SE	AL 20 METHOD OF	DEVELOPMENT P
DEVEL. METHOD	P HOURS OF DEVI	EL.	
REMARKS	RAN PRESENT FOR DR.	ILLING AND TESTING	

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

OWNER ID SYSTEM NAME

HOLE INFORMATION

CONTACT

PHONE

INTERVAL TOP OF E NUMBER HOLE 1 0.00		IAMETER F HOLE 16.00
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** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
1	0.00	9400	10.00	5.73	0.000

** OPENINGS INFORMATION **

					DIAMETER OPENINGS		
NUMBER	SELITUM	3CC11UH	OLEM 1469	MHIENIHE	UFENINDS	UFCM1M03	OLCHIMBO
1	92.00	100.00	R	R	9.00	0.080	0.00

SITEID	472124122044401	LOCAL NUMBER 22N/06E	-32D01
LATITUDE	47 d 21 m 24 s	LONGITUDE	122 d 04 m 44 s
ALTITUDE	420.00	DATE OF CONSTRUCTION	f = f
DEPTH OF WELL	. 36,,00	DEPTH OF HOLE	00,0
WATER LEVEL	21.00	DATE MEASURED	1 1
DRILLER	MAXWELL CO	NSTRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF SE	EAL 0 METHOD OF	DEVELOPMENT

DEVEL METHOD

HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

CASIN6 INTERVAL TOP BOTTOM DIAMETER CASING NUMBER CASING CASING CASING MATERIAL THICKNESS i 0.00 0.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

TRACEPERSON REPORT SEASON REPO

	************	>>> >		
SITEID 41	72112122033601 U	OCAL NUMBER 22N/06E-32	H01	
LATITUDE 47	7 d 21 m 12 s	LONGITUDE 1	22 d 03 m 36 s	
ALTITUDE	500.00 Di	ATE OF CONSTRUCTION 09	/ 10 / 1975	
DEPTH OF WELL	105.00	DEPTH OF HOLE	106.00	
WATER LEVEL	84,00	DATE MEASURED 09	/ 16 / 1975	
DRILLER JO	HNSON CONSTI	RUCTION METHOD C T	YPE OF FINISH P	
TYPE OF SEAL C	BOTTOM OF SEAL	18 METHOD OF DE	VEL.OPHENT	
DEVEL. METHOD HOURS OF DEVEL. 3				
REMARKS				
OWNER ID SYSTEM NA	ĦΕ	CONTACT	PHONE	
GRE145 GREEN, K	EN			
	## HOLE INF	FORMATION ##		
INTERVAL TOP O NUMBER HOLE 1 0.0		OF HOLE		
	** CASING INF	TORMATION ##		
INTERVAL TOP NUMBER CASING		DIAMETER CASING CASING MATERIAL		

6.00 S

0.000

104.00

INTERVAL	TOP	BOTTOM	TYPE	TYPE	DIAMETER	HIDTH	LENGTH
NUMBER	SECTION	SECTION	OPENINGS	MATERIAL	OPENINGS	OPENINGS	OPENINGS
				_			
i	96 00	106.00	P		0.00	0.125	0.00

PUMP INFORMATION

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

0.00

TYPE OF POWER E

SITEID	472041122040201	LOCAL NUMBER 22N/06E	I-32 0 01
LATITUDE	47 d 20 m 41 s	LONGITUDE	122 d 04 m 02 s
ALTITUDE	455.00	DATE OF CONSTRUCTION	1 02 / 20 / 1973
DEPTH OF WELL	95.00	DEPTH OF HOLE	95.00
WATER LEVEL	32.00	DATE MEASURED	02 / 23 / 1973
DRILLER	NORTHWEST CON	STRUCTION METHOD C	TYPE OF FINISH P
TYPE OF SEAL	C BOTTOM OF SE	AL 20 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEVI	EL . :1	
REMARKS			
DANER ID SYSTEM	NAME	CONTACT	PHONE
BLOO28 BLOOM,	, RONALD		
	11 (IA) = 1	NUMBERS TERM LA	

11	HOLE	INFORMATION	\$ \$

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
i	0.00	95.00	6.00

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
i	0.00	95.00	6.00	S	0.000

** OPENINGS INFORMATION **

INTERVAL	TOP	BOTTOM	TYPE	TYPE	DIAMETER	WIDTH	LENGTH
NUMBER	SECTION	SECTION	OPENINGS	MATERIAL	OPENINGS	OPENINGS	OPENINGS
<u>1</u>	66.00	77.00	۶		0.00	0.025	2.00

PUMP INFORMATION

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID	472045127040001	LOCAL NUMBER 22N/06E-	-32003
LATITUDE	47 d 20 m 45 s	LONGITUDE	122 d 04 m 00 s

ALTITUDE 455.00 DATE OF CONSTRUCTION 10 / 22 / 1971

DEPTH OF WELL 87.00 DEPTH OF HOLE 87.00

WATER LEVEL 37 00 DATE MEASURED 10 / 25 / 1971

BRILLER NORTHWEST CONSTRUCTION METHOD C TYPE OF FINISH O

TYPE OF SEAL B BOTTOM OF SEAL 20 METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL. 2

REMARKS

OWNER LD SYSTEM NAME CONTACT PHONE

262017 M&L KUSTON KURE HARVIN PERAULT 631-1237

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 87.00 5.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 87.00 6.00 S 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID	472043122034401	LOCAL NUMBE	ER 22N/06E	-32R01	
LATITUDE	47 d 20 m 43 s	LON	MGITUDE	122 d 03 m 44	5
ALTITUDE	455.00	DATE OF COM	ISTRUCTION	09 / 19 / 1974	
DEPTH OF WE	LL 93.00	DEPT	TH OF HOLE	0.00	
WATER LEVEL	33.00	DATE	: MEASURED	1 1	
DRILLER	JOHNSON DRIL CO	NSTRUCTION ME	THOD C	TYPE OF FINISH	P
TYPE OF SEA	L C BOTTOM OF S	EAL 0	METHOD OF	DEVELOPMENT	
DEVEL. METH	OD HOURS OF DE	VEL.			

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

* HOLE INFORMATION **

INTERVAL	TOP OF	SOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
i	0.00	0.00	0.00

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
<u>i</u>	0.00	93.00	6.00		0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

* PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE

0

TYPE OF POWER

HORSEPOWER

0.00

SITEIC	}	47212512	120 25 701	LOCAL NUM	BER 22N/06	E-33001	
LATITU	IDE	47 d 21	n 25 s	Ĺ	ONSITUDE	122 🕯 01	2 m 57 s
ALTITU	IDE	550.00)	DATE OF C	ONSTRUCTIO	N 09 / 25 .	/ 1979
DEPTH	OF WELL	159.00)	DE	PTH OF HOLI	E 159.00	
WATER	LEVEL	120.00	,	DA	TE MEASUREI	0 10 / 04 /	1979
DRILLE	R	MUELLER	CON	STRUCTION	METHOD C	TYPE OF	FINISH S
TYPE O	f SEAL	8 80	ITTOM OF SE	AL. 18	METHOD OF	DEVELOPME	ENT B
DEVEL.	METHOD	B HO	URS OF DEVI	EL. 2			
REMARK	S						
OWNER ID	SYSTEM	NAME			CONTACT		PHONE
NIE303	NIEDER	KORN, P					
			## HOLE I	INFORMATION	11		
INTERVAL NUMBER 1	НО	0F LE .00		• DIA) • OF •	10LE		
			## CASING !	INFORMATION	<u> </u>		
•			BOTTOM CASING				
1	0.0	0	154.00	6.00	S	0.	000
		ţ	* OPENINGS	INFORMATIO	N \$\$		
			TYPE OPENINGS				
<u>:</u>	154.00	159.00	3	R	5.50	0.025	0.00
			11 PUMP IN	FORMATION	‡ ‡		
TYPE OF	F LIFT	S					
DEPTH (OF PUMP	INTAKE	0				
TYPE OF	POWER	ε					
	_						

0.50

HORSEPOWER

SITEID 472127120032401 LOCAL NUMBER 22N/06E-33D01

LATITUDE 47 d 21 m 27 s LONGITUDE 122 d 00 m 32 s

ALTITUDE 558.00 DATE OF CONSTRUCTION 01 / 01 / 1958

DEPTH OF WELL 175.00 DEPTH OF HOLE 0.00

WATER LEVEL 0.00 DATE MEASURED / /

DRILLER MYRL JOHNSON CONSTRUCTION METHOD TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING MUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 0.00 6.00 5 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS WATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID 472118122031801 LOTAL NUMBER 22N/06E-33002 LATITUDE 47 d 21 m 18 s LONGITUDE 122 d 03 m 18 s ALTITUDE 535.00 DATE OF CONSTRUCTION 08 / 26 / 1980 DEPTH OF WELL 140.00 DEPTH OF HOLE 140.00 MATER LEVEL 97.00 DATE MEASURED 08 / 28 / 1980 DRILLER JOHNSON CONSTRUCTION METHOD A TYPE OF FINISH O TYPE OF SEAL C BOTTOM OF SEAL 18 METHOD OF DEVELOPMENT DEVEL. METHOD HOURS OF DEVEL. 4

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

PED325 PEDRIN. ROBERT

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 140.00 6.00

CASING INFORMATION

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 140.00 6.00 S 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

 SITEID
 472115122024201
 LOCAL NUMBER 22N/06E-33601

 LATITUDE
 47 d 21 m 15 s
 LONGITUDE
 122 d 02 m 42 s

 ALTITUDE
 480.00
 DATE OF CONSTRUCTION 01 / 01 / 1900

 DEPTH OF WELL
 32.00
 DEPTH OF HOLE
 0.00

 WATER LEVEL
 9.91
 DATE MEASURED 01 / 28 / 1963

 DRILLER
 CONSTRUCTION METHOD D
 TYPE OF FINISH

 TYPE OF SEAL
 BOTTOM OF SEAL
 0
 METHOD OF DEVELOPMENT

DEVEL. METHOD

HOURS OF DEVEL.

REMARKS

DANER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 0.00 48.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

PUMP INFORMATION

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID	472054122022101	LOCAL NUMBER 22N/06E	-33J01
LATITUDE	47 d 20 m 54 s	LONGITUDE	122 d 02 m 21
ALTITUDE	550.00	DATE OF CONSTRUCTION	05 / 11 / 1976
DEPTH OF WELL	130.00	DEPTH OF HOLE	130.00
WATER LEVEL	104.00	DATE MEASURED	05 / 11 / 1976
DRILLER	NW PUMP & DR CON	STRUCTION METHOD A	TYPE OF FINISH
TYPE OF SEAL	B BOTTOM OF SE	AL 18 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL.	

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

3

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 130.00 6.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 130.00 6.00 S 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

PUMP INFORMATION

TYPE OF LIFT

DEPTH OF PUMP INTAKE

TYPE OF POWER

HORSEPOWER

0.00

SITEID 472058122022001 LOCAL NUMBER 22N/06E-33J01

LATITUDE 47 d 20 m 58 s LONGITUDE 122 d 02 m 20 s

ALTITUDE 587.00 DATE OF CONSTRUCTION 09 / 03 / 1974

DEPTH OF WELL 91.00 DEPTH OF HOLE 0.00

MATER LEVEL 76.00 DATE MEASURED 09 / 03 / 1974

DRILLER JOHNSON DRLG CONSTRUCTION METHOD C TYPE OF FINISH P

TYPE OF SEAL C BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

CASING INFORMATION

INTERVAL TOP BOTTOM DIAMETER CASING CASING HUHBER CASING CASING THICKNESS CASING MATERIAL 0.00 91.00 1 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

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SITEID 472059122022101 LOCAL NUMBER 22N/06E-33J02

LATITUDE 47 d 20 m 59 s LONGITUDE 122 d 02 m 21 s

ALTITUDE 587.00 DATE OF CONSTRUCTION 11 / 26 / 1974

DEPTH OF WELL 135.00 DEPTH OF HOLE 0.00

WATER LEVEL 117.00 DATE MEASURED 11 / 26 / 1974

DRILLER NORTHWEST PU CONSTRUCTION METHOD C TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL 0 METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

DANER ID SYSTEM NAME

CONTACT

PHONE

DWNER INFORMATION NOT AVAILABLE FOR THIS WELL

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER MUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 135.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

31 PUMP INFORMATION 11

TYPE OF LIFT U

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

SITEID 472053122021601 LOCAL NUMBER 22N/06E-33J04

LATITUDE 47 d 20 m 53 s LONGITUDE 122 d 02 m 16 s

ALTITUDE 550.00 DATE OF CONSTRUCTION 11 / 16 / 1977

DEPTH OF WELL 160.00 DEPTH OF HOLE 160.00

WATER LEVEL 110.00 DATE MEASURED 11 / 22 / 1977

DRILLER NORTHWEST CONSTRUCTION METHOD A TYPE OF FINISH P

TYPE OF SEAL Z BOTTOM OF SEAL 18 METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL 1

REMARKS SEAL IS BENTONITE AND CEMENT

OWNER ID SYSTEM NAME CONTACT PHONE

WAI441 WAITE, DAN

* HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 160.00 6.00

CASING INFORMATION

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 160.00 6.00 S 0.000

OPENINGS INFORMATION

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

1 120.00 143.00 P 6.00 0.375 3.00

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID	472039122031501	LOCAL NUMBER 22N/06E	-33N01
LATITUDE	47 d 20 m 39 s	LONGITUDE	122 d 03 m 15 s
ALTITUDE	435.,00	DATE OF CONSTRUCTION	02 / 18 / 1983
DEPTH OF WELL	57.00	DEPTH OF HOLE	57.00
WATER LEVEL	3.00	DATE MEASURED	02 / 18 / 1983
ORILLER	JOHNSON CON	STRUCTION METHOD A	TYPE OF FINISH O
TYPE OF SEAL	C BOTTOM OF SE	AL 18 METHOD OF	DEVELOPMENT B
DEVEL. METHOD	B HOURS OF DEVI	EL. 4	
REMARKS			
OWNER ID SYSTEM	NAME	CONTACT	PHONE

‡ ‡	HOLE	INFORMATION	ţţ

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INTERVAL	TOP OF	BOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
1	0.00	57.00	6.00

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
1	0.00	57.00	6.00	S	0.000

OPENINGS INFORMATION

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

T/PE OF POWER E

		********		********	
SITEI] 4	72049122025701	LOCAL NUMS	BER 22N/06E-33	5P
LATIT	JDE 47	7 d 20 m 4 9 s	L	ONGITUDE 1	122 d 02 m 57 s
ALTITI	ΙΒΈ	52500	DATE OF CO	INSTRUCTION 12	! / 01 / 1977
DEPTH	OF WELL	75.00	DEF	TH OF HOLE	75.00
WATER	LEVEL	45.25	DAT	E MEASURED 11	. / 17 / 1977
DRILLE	IR BL	IRT WELL DR C	ONSTRUCTION H	ETHOD C T	YPE OF FINISH S
TYPE 0	IF SEAL G	BOTTOM OF	SEAL 22	METHOD OF DE	VELOPMENT
DEVEL.	METHOD	HOURS OF D	EVEL.		
REMARKS LOC: 700FT N, 450 W DF S1/4 COR SEC 33					
OWNER ID	SYSTEM NA	ME		CONTACT	PHONE
OWNER INFO	RMATION NO	T AVAILABLE FO	R THIS WELL		
		\$\$ HOLE	E INFORMATION	‡ ‡	
	HOLE	F 80TTOM Hole 0 75.00	OF H	OLE	
	** CASING INFORMATION **				
		BOTTOM CASING			
*	0.00	51.00	16.00		0.000
		## OPENING	S INFORMATION	<u> </u>	
INTERVAL	TOP BO	ITTOM TYPE	TYPE	DIAMETER WI	DTH LENGTH

** PUMP INFORMATION **

S

NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

16.00

15.00

0.125

0.000

0.00

0.00

TYPE OF LIFT

<u>i</u>

2

DEPTH OF PUMP INTAKE

51.00

66.00

65.00

75.00

TYPE OF POWER

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		1514	131111111	3145313333	,,,,,,,,,,		
SITEI	Ď	472048120	2025701	LOCAL NUMS	3ER 22N/068	-33PI)2	
LATIT	.ade.	47 d 20 a) 48 s	11	NGITUDE	122 d 00	2 m 57 g
ALTIT	UDE	510.00		DATE OF CO	INSTRUCTION	1 01 / 03 /	/ 1978
DEPTH	OF WELL	72.00		DEF	TH OF HOLE	72.00	
WATER	LEVEL	42.00		PAG	E MEASUREI	12 / 18 /	/ 1977
DRILL	ER	BURT WELL	. DR CON	STRUCTION A	ETHOD C	TYPE OF	FINISH S
ТУРЕ	OF SEAL	6 807	TOM OF SE	AL 22	METHOD OF	DEVELOPME	ENT
DEVEL	. METHOD	HOL	IRS OF DEV	Ei	-		
REMAR	kS	LOC: 650F	T N, 450F	T W OF S1/4	COR SEC 3	3	
OWNER ID	SYSTEM	NAME			CONTACT		PHONE
OWNER INF	OWNER INFORMATION NOT AVAILABLE FOR THIS WELL						
			## HOLE	INFORMATION	*		
	HC	LE	HOLE	F DIAM OF H 16	OLE		
		1	* CASING	INFORMATION	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		
				DIAMETER CASING			
<u>i</u>	0.0	0	50.00	16.00		0.	000
** OPENINGS INFORMATION **							
INTERVAL NUMBER	TOP SECTION	BOTTOM SECTION	TYPE OPENINGS	TYPE MATERIAL		WIDTH OPENINGS	
1 2	50.00 65.00	65.00 72.00	Ş	R	14.00 16.00	0.125 0.000	0.00 0.00

\$\$ PUMP INFORMATION \$\$

TYPE OF LIFT

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

********************* #### WELL CONSTRUCTION REPORT

SITEID	472046122030301	LOCAL NUMBER 22N/06E	-33P03
LATITUDE	47 d 20 m 46 s	LONGITUDE	122 d 03 m 03 s
ALTITUDE	455.00	DATE OF CONSTRUCTION	I = I
DEPTH OF WELL	. 0.00	DEPTH OF HOLE	695.00
WATER LEVEL	0.00	DATE MEASURED	1 1
DRILLER	ARMSTRONG CO	NSTRUCTION METHOD C	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF SI	EAL O METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DE	VEL.	

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 16.00 1 0.00 375.00

** CASING INFORMATION **

TOP INTERVAL BOTTOM DIAMETER CASING CASING HUMBER CASING CASING CASING MATERIAL THICKNESS

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

ISTIGRATURA KARAMATARA
SITEID	472103122010201	LOCAL NUMBER 22N/05E	-34H01
LATITUDE	47 d 21 m 03 s	LONGITUDE	122 d 01 m 02 s
ALTITUDE	600.00	DATE OF CONSTRUCTION	01 / 03 / 1980
DEPTH OF WELL	155.00	DEPTH OF HOLE	155.00
WATER LEVEL	119.00	DATE HEASURED	01 / 04 / 1980
DRILLER	NORTHWEST CON	STRUCTION METHOD A	TYPE OF FINISH O
TYPE OF SEAL	B BOTTOM OF SE	AL 18 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL. 1	
REMARKS			
OWNER ID SYSTEM	NAME	CONTACT	PHONE
SVE409 SVERDI	ARSKY, DAVE		

** NATE THEORDHITON *	11	HOLE	INFORMATION	\$ \$
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INTERVAL	TOP OF	BOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
1	0.00	155.00	6.00

* CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
1	0.00	155.00	6.00	S	0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID	472040122011501	LOCAL NUMBER 22N/06E	-34901
LATITUDE	47 d 20 m 40 s	FOWETINDE	122 d 01 m 15 s
ALTITUDE	575.00	DATE OF CONSTRUCTION	06 / 20 / 1978
DEPTH OF WELL	. 98.00	DEPTH OF HOLE	98. 00
WATER LEVEL	53.00	DATE MEASURED	06 / 20 / 1978
DRILLER	NORTHWEST COM	ISTRUCTION METHOD A	TYPE OF FINISH O
TYPE OF SEAL	B BOTTOM OF SE	AL 18 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL. i	

REHARKS

OWNER ID SYSTEM NAME

CONTACT

PHGNE

LAS238 LASHER, DAVE

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER

NUMBER HOLE HOLE OF HOLE

1 0.00 98.00 6.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 1 0.00 98.00 6.00 S 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS DPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE

TYPE OF POWER E

11111111111111111111111111111111 *** WELL CONSTRUCTION REPORT **** *************************

SITEID	472050122010001	LOCAL NUMBER 22N/06	E-34R01
LATITUDE	47 đ 20 m 50 s	LONGITUDE	122 d 01 m 00 s
ALTITUDE	550.00	DATE OF CONSTRUCTION	N 06 / 17 / 1980
DEPTH OF WELL	00,08	DEPTH OF HOLI	80, 00
WATER LEVEL	40.00	DATE MEASURE	0 06 / 17 / 1980
DRILLER	NORTHWEST CO	HSTRUCTION METHOD A	TYPE OF FINISH O
TYPE OF SEAL	8 BOTTOM OF S	EAL 18 METHOD OF	F DEVELOPMENT
DEVEL. METHO) HOURS OF DE	VEL. 1	
REMARKS			

OWNER ID SYSTEM NAME CONTACT

PHONE

MAI257 MAIERS, LAURETTA

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER HOLE NUMBER HOLE OF HOLE 1 0.0080.00 5.00

* CASING INFORMATION *

INTERVAL TOP BOTTOM DIAMETER CASING CASING MUMBER CASING CASING CASING MATERIAL THICKNESS 0.00 80.00 6..00 0.000

11 OPENINGS INFORMATION 11

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAXE 0

٤ TYPE OF POWER

SITEID	472037122010101	LOCAL NUMBER 22N/06E-34R02
LATITUDE	47 d 20 m 37 s	LONGITUDE 122 d 01 m 01 s
ALTITUDE	550.00	DATE OF CONSTRUCTION 06 / 09 / 1983
DEPTH OF WELL	. 65.00	DEPTH OF HOLE 65.00
WATER LEVEL	30.00	DATE MEASURED 06 / 09 / 1983
DRILLER	JOHNSON CO	INSTRUCTION METHOD A TYPE OF FINISH O
TYPE OF SEAL	C BOTTOM OF S	EAL 18 METHOD OF DEVELOPMENT
DEVEL. METHOD	HOURS OF DE	VEL. 3

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

DRL092 DRLLEVICH, VAL 432-0200

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 65.00 6.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING. CASING NUMBER CASING CASING CASING MATERIAL THICKNESS ţ 0.00 65.00 6.00 S 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

31.	TEID	4720)3122()1570)1	LOCAL	ΝIJ	MBER 21N.)6E	-03E)[
LA	TITUDE	47 d	20 m	03 s	.			LONGITUDE		122	Z d 01	a 57	7 3
ΑL	TITUDE	500	0.00			DATE (3F (CONSTRUCT	I ON	01 .	/ 01 /	1958	}
DEi	TH OF WELL	5:	1,00				D	EPTH OF H	JLE		0.00		
¥A]	TER LEVEL	17	2.00				Di	ATE MEASUR	RED	01	01 /	1962	
DR.	ILLER	MYRL.	JOHNS	ON	CONS	TRUCT:	ON	METHOD		TYF	e of	FINIS	Н
TYF	PE OF SEAL		BOTT	OM O	F SEAI	- ()	HETHOD	ΩF	DEVE	LOPHE	NT	
DE\	/EL. METHOD		HOUR	is of	DEVE	:							

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTO₩ DIAMETER CASING CASING NUMBER CASING CASING CASIN6 MATERIAL THICKNESS 1 0.00 0.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

LATITUDE 47 d 20 m 20 s LONGITUDE 122 d 01 m 56 s

ALTITUDE 495.00 DATE OF CONSTRUCTION / /

DEPTH OF WELL 16.00 DEPTH OF HOLE 0.00

WATER LEVEL 0.00 DATE MEASURED / /

DRILLER SANDPOINT CONSTRUCTION METHOD V TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

* HOLE INFORMATION **

INTERVAL TOP GF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL. TOP HOTTOS DIAMETER CASING **CASING** NUMBER CASING CASING CASING MATERIAL THICKNESS 0.00 1 0.00 4.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT P

DEPTH OF PUMP INTAKE (

TYPE OF POWER E

SITEID 472004122013901 LOCAL NUMBER 21N/06E-03L01

LATITUDE 47 d 20 m 04 s LONGITUDE 122 d 01 m 39 s

ALTITUDE 550.00 DATE OF CONSTRUCTION / /

DEPTH OF WELL 107.00 DEPTH OF HOLE 0.00

WATER LEVEL 0.00 DATE MEASURED / /

DRILLER CONSTRUCTION METHOD TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

76460N SAWYERWOOD ESTATES W. S. DOMALD FURMAN 886-2264

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 0.00 0.00 0.00 0.00

OPENINGS INFORMATION

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

11 PUMP INFORMATION 11

TYPE OF LIFT

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

SITEID	472021122020701	LOCAL NUMBER 21N/06E	E-03 P 02
LATITUDE	47 d 19 m 46 s	LONGITUDE	122 d 01 m 46 s
ALTITUDE	515.00	DATE OF CONSTRUCTION	1 / /
DEPTH OF WELL	. 50.00	DEPTH OF HOLE	0.00
HATER LEVEL	30.,34	DATE MEASURED	08 / 16 / 1962
DRILLER	DORSTEN CON	ISTRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF SE	CAL O METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL.	

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

56185A MORRIS BROTHERS

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

CASING INFORMATION

TOP INTERVAL BOTTON DIAMETER CASING CASING HUMBER CASIN6 CASING CASING MATERIAL THICKNESS 1 -0.62 0.00 6.00 0.000

OPENINGS INFORMATION

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

TITALITATION REPORT TOTAL TRANSPORT TOTAL TRANSPORT TOTAL TRANSPORTED TO THE PROPERTY OF THE P

 SITEID
 472030122023701
 LOCAL NUMBER 21N/06E-04801

 LATITUDE
 47 d 20 m 30 s
 LONGITUDE
 122 d 02 m 37 s

 ALTITUDE
 500.00
 DATE OF CONSTRUCTION / /
 /

 DEPTH OF WELL
 50.00
 DEPTH OF HOLE
 0.00

 WATER LEVEL
 32.61
 DATE MEASURED 08 / 16 / 1962

 DRILLER
 MYRL JOHNSON
 CONSTRUCTION METHOD
 TYPE OF FINISH

TYPE OF SEAL

DEVEL, METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME

CONTACT

BOTTOM OF SEAL O METHOD OF DEVELOPMENT

PHONE

DANER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 3.00 0.00 6.00 S 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID 472027122024001 LDCAL NUMBER 21N/06E-04B02

LATITUDE 47 d 20 m 27 s LONGITUDE 122 d 02 m 40 s

ALTITUDE 500.00 DATE OF CONSTRUCTION 01 / 01 / 1961

DEPTH OF WELL 50.00 DEPTH OF HOLE 0.00

WATER LEVEL 33.87 DATE MEASURED 08 / 15 / 1962

DRILLER MYRL JOHNSON CONSTRUCTION METHOD TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

DANER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

* HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING MUMBER CASING CASING CASING MATERIAL THICKNESS -0.30 1 0.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

* PUMP INFORMATION **

TYPE OF LIFT 3

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

	*********	*****	
SITEID	472031122023301	LOCAL NUMBER 21N/05E-04B03	
LATITUDE	47 d 20 m 27 s	LONGITUDE 122 d 02	2 m 45 s
ALTITUDE	500.00	DATE OF CONSTRUCTION 09 / 01	1959
DEPTH OF WELL	L 54.00	DEPTH OF HOLE 0.00	
WATER LEVEL	27.47	DATE MEASURED 08 / 16 /	1962
DRILLER	MYRL JOHNSON COM	NSTRUCTION HETHOD TYPE OF	FINISH
TYPE OF SEAL	BOTTOM OF SE	EAL O METHOD OF DEVELOPME	TM
DEVEL. METHO	D HOURS OF DEV	ÆL.	
REMARKS	WELL DEEPENED DUE	TO WATER LEVEL DECLINES	
OWNER ID SYSTEM	1 NAME	CONTACT	PHONE
LAD200 LADDE	ERAD		
	** HOLE	INFORMATION ##	
NUMBER 1	OP OF BOTTOM O HOLE HOLE 0.00 54.00	OF HOLE	
	** CASING	INFORMATION **	

es austua Talannuurtan se

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
1	0.00	0.00	6.00		0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

******************** *** WELL CONSTRUCTION REPORT *** *********************

SITEID 472031122023302 LOCAL NUMBER 21N/06E-04B03D1 LATITUDE 47 d 20 m 27 s LONGITUDE 122 d 02 m 45 s ALTITUDE 500.00 DATE OF CONSTRUCTION / / DEPTH OF WELL 65.00 DEPTH OF HOLE 0.00 WATER LEVEL 38.17 DATE MEASURED 08 / 12 / 1986 DRILLER CONSTRUCTION METHOD TYPE OF FINISH TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

LAD300 LADDERUND, LOIS

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER HOL.E NUMBER HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP MOTTON DIAMETER CASING CASING CASING MATERIAL THICKNESS MUMBER CASING CASING 0..00 0.00 1 0.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE

TYPE OF POWER

HORSEPOWER

0.00

SITEID	472026122024501	LOCAL NUMBER 21N/06E-	-04804
LATITUDE	47 d 20 m 26 s	LONGITUDE	122 d 02 m 45 s
ALTITUDE	502.00	DATE OF CONSTRUCTION	07 / 13 / 1951
DEPTH OF WELL	48.,00	DEPTH OF HOLE	0.00
WATER LEVEL	28.00	DATE MEASURED	07 / 13 / 1961
DRILLER	JOHNSON C	ONSTRUCTION METHOD C	TYPE OF FINISH P
TYPE OF SEAL	BOTTOM OF	SEAL 0 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF D	EVEL.	

REMARKS

CHNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

BOTTOM DIAMETER CASING CASING INTERVAL TOP CASING MATERIAL THICKNESS MUMBER CASING CASING 0.00 48.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

11 PUMP INFORMATION 11

TYPE OF LIFT

DEPTH OF PUMP INTAKE C

TYPE OF POWER

************************ #### WELL CONSTRUCTION REPORT #### *****************

SITEID 472028122024201 LOCAL NUMBER 21N/06E-04B04 LONGITUDE 122 d 02 m 39 s LATITUDE 47 d 20 m 32 s ALTITUDE 500.00 DATE OF CONSTRUCTION / / DEPTH OF WELL 85.00 DEPTH OF HOLE 0.00 WATER LEVEL 43.00 DATE MEASURED 03 / 01 / 1971 DRILLER CONSTRUCTION METHOD TYPE OF FINISH D TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD

HOURS OF DEVEL.

REMARKS

SCHEDULE YEAR WAS

OWNER ID SYSTEM NAME

CONTACT

PHONE

EC0400 EC0LOSY, DEPT OF

** HOLE INFORMATION **

TOP OF INTERVAL BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE <u>1</u> 0.00 0.00 0.00

11 CASING INFORMATION 11

INTERVAL TOP BOTTOM DIAMETER CASING CASING CASINS CASING MATERIAL THICKNESS NUMBER CASING 1 0.00 0.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

PUMP INFORMATION

TYPE OF LIFT

TYPE OF POWER

DEPTH OF PUMP INTAKE 0

HORSEPOWER

0.00

SIT	TEID	47202512	2024101	LOCAL NUN	BER 21N/068	E-04805		
LA	TITUDE	47 d 20 s	n 27 s	Li	ONGITUDE	122 d 02	2 m 46 s	
AL.1	TITUDE	500.00		DATE OF C	ONSTRUCTION	1 01 / 02 /	1976	
DEF	TH OF WELL	. 83.00		DE	TH OF HOLE	83.00		
WAT	ER LEVEL	43.00		DA	TE MEASURED	01 / 02 /	1976	
DRI	ILLER	JCHNSON	CON	STRUCTION 1	1ETHOD A	TYPE OF	FINISH S	
TYF	E OF SEAL	B 80°	FTOM OF SE	AL 20	HETHOD OF	DEVELOPME	ENT	
DEV	IEL. METHOD	ног	JRS OF DEVI	EL.				
REA	IARKS							
OHNER I	D SYSTEM	NAME			CONTACT		PHONE	
416508	k CMD	105			JUDITH	NELSON	631-05 65	j
			## HOLE	INFORMATION	11			
THTEDU	AL TO	e ue	א אחדדתם	= ntak	IETED			
NUMBE	R H	OLE OLE	HOLE		iole			
1		0,00	83.00	10	0.00			
		Ĭ	t CASING	INFORMATION	1 33			
INTERV				DIAMETER				
WALLE	R CAS	ING L	ASING	CASING	na i en 1	HL INIC	CCINA	
1	0.	00	73.00	10.00	S	0.	000	
		‡ į	OPENINGS	INFORMATIO	N II			
	AL TOP R SECTION							
1	73.00	83.00	S	R	10.00	0.040	0.00	
	** PUMP INFORMATION **							
TYP	TYPE OF LIFT							
DEP	DEPTH OF PUMP INTAKE 0							
TYP	e of power							

0.00

HORSEPOWER

SITEID	472030122023801	LOCAL NUMBER 21N/06E	-04807
LATITUDE	47 d 20 m 30 s	LONGITUDE	122 d 02 m 38 s
ALTITUDE	500.00	DATE OF CONSTRUCTION	08 / 01 / 1956
DEPTH OF WELL	. 80,00	DEPTH OF HOLE	0.00
WATER LEVEL	40.00	DATE MEASURED	08 / 01 / 1956
DRILLER	CON	ASTRUCTION METHOD B	TYPE OF FINISH P
TYPE OF SEAL	Z BOTTOM OF SE	SAL 0 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL.	

OWNER ID SYSTEM NAME

REMARKS

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

* HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

67= RUBBER COMPRESSED

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 1 0.00 80.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT C

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

YARARISERRARIA ARAKATAN KANTAN
SITEID	472025122024501	LOCAL NUMB	ER 21N/06E	-04808	
LATITUDE	47 d 20 m 33 s	LO	NGITUDE	122 d 02	2 m 40 s
ALTITUDE	50000	DATE OF CO	NSTRUCTION	1 12 / 13 /	1975
DEPTH OF WELL	85.00	DEP	TH OF HOLE	85.00	
WATER LEVEL	43.00	DATI	E MEASURED	12 / 13 /	1975
DRILLER	JOHNSON COM	STRUCTION H	ETHOD A	TYPE OF	FINISH S
TYPE OF SEAL	B BOTTOM OF SE	AL 20	METHOD OF	DEVELOPME	INT
DEVEL. METHOD	HOURS OF DEVI	EL.			
REMARKS					
OWNER ID S/STEM	NAME		CONTACT		PHONE
416508 KCWD	105		HTIQUL	NELSON	631-0565
	## HOLE	INFORMATION	ŢŢ		
INTERVAL TO	P OF BOTTOM OF	F DIAMS	ETER		
	OLE HOLE 0.00 85.00	OF 40 10.	JLE .00		
	## CASING	INFORMATION	design of the second se		
INTERVAL TO	P ROTTOM	DIAMETER	CASIN	s cas	TNG
NUMBER CAS					
1 0.	00 75.00	10.00	5	0.	000
	t* OPENINGS	INFORMATION			
	BOTTOM TYPE SECTION OPENINGS				
1 75.00	85.00 S	8	10.00	0.040	0.00
	## PUMP IN	FORMATION :	:		
TYPE OF LIFT	T				
DEPTH OF PUMP	INTAKE 0				
TYPE OF POWER					

900.00

HORSEPOWER

	1200	504000001104	· ASSI SILI		
51:10	4720	29122024101	LUCAL NUME	BER 21N:06E-04	·B07
LATITU	DE 47 d	20 a 29 s	FC	MGITUDE 1	22 d 02 m 41 s
ALTITU	DE 53	6.00	DATE OF CO	NSTRUCTION 03	/ 24 / 1980
DEPTH (OF WELL 8	700	DEP	TH OF HOLE	92.00
WATER !	LEVEL 3	8.00	DAT	E MEASURED 04	/ 03 / 1980
DRILLE	R STOR	Y/ARMSTR CO	NSTRUCTION M	ETHOD C T	YPE OF FINISH S
TYPE OF	F SEAL 6	BOTTOM OF S	EAL 20	METHOD OF DE	VELOPMENT S
DEVEL.	METHOD S	HOURS OF DE	VEL. 4		
REMARKS RAN PRESENT FOR DRILLING AND TESTING					
DWNER ID	SYSTEM NAME			CONTACT	PHONE
OWNER INFOR	MATION NOT	WAILABLE FOR	THIS WELL		
		## HOLE	INFORMATION	‡ ‡	
	HOLE	BOTTOM (HOLE 20.00	OF H	JLE	
		** CASING	INFORMATION	11.41	
		DATTON	DIAMETER	SASTNS	racime
	TOP Casing			MATERIAL	

OPENINGS INFORMATION

NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

TYPE

R

DIAMETER

16.00

HIDTH

0.150

LENGTH

0.00

TYPE

R

BOTTOM

69.00

INTERVAL TOP

64.00

1

CERTIFICATION REPORT AND TRACTION REPORT AND T

SITEID	472029122030001	LOCAL NUMBER 21N/06E-	-04001
LATITUDE	47 d 20 m 29 s	LONGITUDE	122 d 03 m 00 s
ALTITUDE	495.00	DATE OF CONSTRUCTION	01 / 01 / 1963
DEPTH OF WELL	67.00	DEPTH OF HOLE	0.00
WATER LEVEL	35.47	DATE MEASURED	08 / 15 / 1962
DRILLER	JOHNSON DRIL COM	STRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF SE	AL 0 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL.	

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

TOP MOTTOR DIAMETER CASING CASING INTERVAL MATERIAL THICKNESS NUMBER CASING CASING CASING 0.00 6.00 0.000 1.75 1

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

S	ITEID	47202912203120	1 LOCAL NUMBER 21N/06E	-04D0 <u>1</u>
Ĺ	ATITUDE	47 d 20 m 29 s	LONGITUDE	122 d 03 m 12 s
Αl	TITUDE	510.00	DATE OF CONSTRUCTION	10 / 25 / 1971
DE	EPTH OF WELL	87.00	DEPTH OF HOLE	0,00
WA	YTER LEVEL	37.00	DATE MEASURED	10 / 25 / 1971
DF	RILLER	NORTHWEST PU	CONSTRUCTION METHOD	TYPE OF FINISH

TYPE OF SEAL B BOTTOM OF SEAL 0 METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
i	0.00	0.00	0.00

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
HUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
1	0.00	87.00	6.00		0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

PUMP INFORMATION

TYPE OF LIFT

DEPTH OF PUMP INTAKE

.

TYPE OF POHER

HORSEPOWER

0.00

SITEID	472018122032401 L0	CAL NUMBER 21N/06E	-04E01
LATITUDE	47 d 20 m 18 s	LONGITUDE	122 d 03 m 24 s
ALTITUDE	550.00 DA	TE OF CONSTRUCTION	06 / 01 / 1975
DEPTH OF WELL	123 00	DEPTH OF HOLE	0.00
WATER LEVEL	106.00	DATE MEASURED	06 / 01 / 1975
DRILLER	EVERGREEN DR CONSTR	UCTION METHOD C	TYPE OF FINISH
TYPE OF SEAL	B BOTTOM OF SEAL.	O METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEVEL.		

REMARKS

DANER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 123.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

SITEID	472029122025901	LOCAL NUMBER 21N/06E	-04F0 <u>1</u>
LATITUDE	47 d 20 m 29 s	LONGITUDE	122 d 02 m 59 s
ALTITUDE	515.00	DATE OF CONSTRUCTION	1 1
DEPTH OF WELL	47.00	DEPTH OF HOLE	0.00
WATER LEVEL	31.43	DATE MEASURED	08 / 15 / 1962
DRILLER	CLYDE DORSTE CON	STRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOH OF SE	AL 0 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL.	

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

* HOLE INFORMATION **

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
HUMBER	HOLE	HOLE	OF HOLE
1	0.00	0.00	0.00

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASIN6	CASING	CASING	MATERIAL	THICKNESS
1	-0.33	0.00	7.00		0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE C

TYPE OF POWER E

163444445347334444133433144144444 1914 WELL CONSTRUCTION REPORT 1444 1444444444444444444444444444444

SITEID	472017122023901	LOCAL NUMBER 21N/06E	-04601
LATITUDE	47 d 20 m 17 s	LONGITUDE	122 d 02 m 39 s
ALTITUDE	500.00	DATE OF CONSTRUCTION	11 / 21 / 1957
DEPTH OF WELL.	8000	DEPTH OF HOLE	0.00
WATER LEVEL	000	DATE MEASURED	I = I
DRILLER	MARTIN R LUT CO	INSTRUCTION METHOD C	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF S	SEAL 0 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DE	EVEL.	
mevadud			

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
MUMBER	HOLE	HOLE	OF HOLE
<u>i</u>	0.00	0.00	000

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASINS	CASING	MATERIAL	THICKNESS
1	0.00	0.00	6.00		0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

PUMP INFORMATION

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

SITEID 472001122024301 LOCAL NUMBER 21N/06E-04K01

LATITUDE 47 d 20 m 01 s LONGITUDE 122 d 02 m 43 s

ALTITUDE 510.00 DATE OF CONSTRUCTION 10 / 26 / 1983

DEPTH OF WELL 99.00 DEPTH OF HOLE 99.00

WATER LEVEL 40.00 DATE MEASURED 10 / 26 / 1983

DRILLER MORTHWEST CONSTRUCTION METHOD A TYPE OF FINISH O

TYPE OF SEAL B BOTTOM OF SEAL 18 METHOD OF DEVELOPMENT B

DEVEL METHOD B HOURS OF DEVEL. 1

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

284277 TOLLBER, GENE 386-1176

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 99.00 6.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 0.00 1 79.00 6.00 0.000 S

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

5

SITEID	472001122023901 LBCAL NUMBER 21N 05E	-04804
LATITUDE	47 d 20 m 01 s LONGITUDE	122 d 02 m 39
ALTITUDE	510.00 DATE OF CONSTRUCTION	1 1
DEPTH OF WELL	60.00 DEPTH OF HOLE	0,00
WATER LEVEL	21.37 DATE MEASURED	07 / 23 / 1986
DRILLER	CONSTRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF SEAL. O METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEVEL.	

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

80465D SMITH, CLYDE D. & A. JOHNSON 896-1762

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER
NUMBER HOLE HOLE OF HOLE
1 0.00 0.00 0.00

** CASING INFORMATION **

TOP DIAMETER INTERVAL BOTTOM CASING CASING CASING MATERIAL THICKNESS NUMBER CASING CASING 1 0.00 0.00 0.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE

TYPE OF POWER

SITEID 471949122024201 LOCAL NUMBER 21N/06E-04001

LATITUDE 47 d 19 m 49 m 49 m LONGITUDE 122 d 02 m 42 m

ALTITUDE 520.00 DATE OF CONSTRUCTION 01 / 19 / 1968

DEPTH OF WELL 75.00 DEPTH OF HOLE 0.00

WATER LEVEL 25.00 DATE MEASURED 01 / 19 / 1968

DRILLER STIMSON DRIL CONSTRUCTION METHOD V TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

11 CASING INFORMATION 11

INTERVAL TOP MOTTOR DIAMETER CASING CASING NUMBER CASING. CASING CASING MATERIAL THICKNESS 0.00 1 0.00 6,00 0.000

OPENINGS INFORMATION

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE

TYPE OF POWER

		****	*******	*******			
SITEI	D	47194712	2023401	LOCAL NUMS	BER 21N/06	E-04 0 02	
LATIT	UDE	47 d 17 s	47 s	1.0	ONGITUDE	122 d 0	2 a 35 s
ALTIT	UDE -	520.00		DATE OF CO	INSTRUCTION	N 02 / 22	/ 1967
DEPTH	OF WELL	5900		DEF	TH OF HOLE	59.00	
WATER	LEVEL	33.00		DAT	E MEASURES	02 / 22	/ 1967
DRILL	ER	JOHNSON D	ALL CON	STRUCTION N	ETHOD C	TYPE OF	FINISH P
TYPE	OF SEAL	Z B01	TOM OF SE	AL 0	METHOD OF	DEVELOPM	ENT
DEVEL	. METHOD	HOL	IRS OF DEVI	EL.			
REMAR	KS						
OWNER ID	SYSTEM	NAME			CONTACT		PHONE
764629	SAWYER	RNOOD WATE	R SYSTEM		DOUGLAS	, H. & S.	
			## HOLE	INFORMATION	žţ.		
	HC	ILE	HOLE	F DIAM OF H	OLE		
		Ì	t Casing !	INFORMATION	ý Ý A		
				DIAMETER CASING			
<u>i</u>	0.0		59.00	6.00	S	θ.	.000
		**	OPENINGS	INFORMATIO	1 1		
				TYPE MATERIAL			

PUMP INFORMATION

59.00 P 0.00

0.125

3.00

TYPE OF LIFT

1

DEPTH OF PUMP INTAKE 0

49.00

TYPE OF POWER

SITEID	471958122024201	LOCAL NUMBER 21N/06E	-04 <u>0</u> 03
LATITUDE	47 d 19 m 52 s	LONGITUDE	122 d 02 m 40 s
ALTITUDE	515.00	DATE OF CONSTRUCTION	09 / 18 / 1975
DEPTH OF WELL	52.00	DEPTH OF HOLE	52.00
WATER LEVEL	2500	DATE MEASURED	09 / 18 / 1975
DRILLER	JOHNSON CON	STRUCTION METHOD C	TYPE OF FINISH P
TYPE OF SEAL	C BOTTOM OF SE	AL 18 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL. 3	
REMARKS			

05750H BENSON, EDWARD

OWNER ID SYSTEM NAME

****** HOLE INFORMATION ******

CONTACT

PHONE

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 52.00 6.00

** CASING INFORMATION **

INTERVAL TOP BOTTON DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 0..00 52.00 1 6.00 0.000

OPENINGS INFORMATION

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

1 42.00 52.00 P 0.00 0.000 0.00

PUMP INFORMATION

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID 471952122022601 LOCAL NUMBER 21N/06E-04R01

LATITUDE 47 d 19 m 52 s LONGITUDE 122 d 02 m 26 s

ALTITUDE 500.00 DATE OF CONSTRUCTION / /

DEPTH OF WELL 27.00 DEPTH OF HOLE 0.00

WATER LEVEL 15.17 DATE MEASURED 08 / 14 / 1962

DRILLER CONSTRUCTION METHOD D TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL 0 METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL,

** HOLE INFORMATION **

PHONE

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 0.00 0.00 0.00

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

************************ **** WELL CONSTRUCTION REPORT **** *********************

SITEID 472033122033801 LDCAL NUMBER 21N/06E-05A01 LATITUDE 47 d 20 m 33 s

LONGITUDE 122 d 03 m 38 s

ALTITUDE 525.00 DATE OF CONSTRUCTION 05 / 12 / 1974

DEPTH OF WELL 90.00

DEPTH OF HOLE 0.00

WATER LEVEL

5.00

DATE MEASURED 05 / 12 / 1974

DRILLER

JOHNSON OR C CONSTRUCTION METHOD C TYPE OF FINISH P

TYPE OF SEAL C

BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD

HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL NUMBER

1

INTERVAL

TOP OF HOLE 0.00

TOP

BOTTOM OF HOLE 0.00

DIAMETER OF HOLE

0.00

11 CASING INFORMATION 11

NUMBER CASING

BOTTOM CASING

DIAMETER CASING

CASING MATERIAL

CASING THICKNESS

0.00 1

70..00

6.00

0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM

TYPE

TYPE

DIAMETER WIDTH LENGTH

NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE

0

TYPE OF POWER

HORSEPOWER

0.00

517EID 471917122043501 LOCAL NUMBER 21N/06E-08E01

LATITUDE 47 d 19 m 17 s LONGITUDE 122 d 04 m 35 s

ALTITUDE 530.00 DATE OF CONSTRUCTION 01 / 01 / 1957

DEPTH OF WELL 65.00 DEPTH OF HOLE 0.00

WATER LEVEL 8.00 DATE MEASURED / /

DRILLER MYRL JOHNSON CONSTRUCTION METHOD TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP SOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 1 0.00 0.00 6.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID 471921122041801 LOCAL NUMBER 21N/06E-08F01

LATITUDE 47 d 19 m 21 s LONGITUDE 122 d 04 m 18 s

ALTITUDE 470.00 DATE OF CONSTRUCTION 06 / 03 / 1982

DEPTH OF WELL 119.00 DEPTH OF HOLE 119.00

WATER LEVEL 0.00 DATE MEASURED / /

DRILLER STORY/DODGE CONSTRUCTION METHOD C TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS R&N PRESENT FOR DRILLING

DWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER MUMBER HOLE HOLE OF HOLE 1 0.00 119.00 8.00

CASING INFORMATION

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

TEXESTRATES AND TRUE TO THE TERM OF THE TEXE ATTEMPT A

SIT	EIŪ	471931122	033701	LOCAL NUMB	ER 21N/06E	-08H01	
LAT	ITUDE	47 d 19 m	31 s	LO	NGITUDE	122 d 03	a 37 s
ALT	ITUDE	490.00		DATE OF CO	NSTRUCTION	09 / 09 /	1982
DEP	TH OF WELL	127.00		DEP	TH OF HOLE	127.00	
TAK	ER LEVEL	4.00		DAT	E MEASURED	09 / 12 /	1982
DRI	LLER	STORY/DOD	GE CONS	STRUCTION M	ETHOD C	TYPE OF	FINISH F
TYP	E OF SEAL	B BOT	TOM OF SEA	AL 9	METHOD OF	DEVELOPME	MT
DEV	EL. METHOD	нои	RS OF DEVE	īL.			
REM	ARXS	RAN PRESE	NT FOR ORI	ILLING AND	TESTING		
OWNER I	D SYSTEM	MAME			CONTACT		PHONE
OWNER I	NFORMATION	NOT AVAIL	ABLE FOR T	THIS WELL			
			## HOLE	(NFORMATION	‡ ‡		
	AL TOS			T DIAM			
1		0.00	20.00				
		į	‡ CASING I	(NFORMATION	4. 4.		
				DIAMETER			
NUMBE	R CAS	146 C	ASIN6	CASING	MAIEKI	AL IHIL	ecjina.
1	0.0)0 i	27.00	8.00	5	0.	000
		11	OPENINGS	INFORMATIO	**		
	AL TOP R SECTION			TYPE MATERIAL			

p

12.00 17.00

8.00

0.000

0.00

SITEID 471945122022201 LOCAL NUMBER 21N/06E-09A01

LATITUDE 47 d 19 m 45 s LONGITUDE 122 d 02 m 22 s

ALTITUDE 500.00 DATE OF CONSTRUCTION / /

DEPTH OF WELL 27.00 DEPTH OF HOLE 0.00

WATER LEVEL 14.69 DATE MEASURED 08 / 14 / 1962

DRILLER MYRL JOHNSON CONSTRUCTION METHOD TYPE OF FINISH

TYPE OF SEAL. BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 1 00,0 0.00 5.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT P

DEPTH OF PUMP INTAKE O

TYPE OF POWER E

SITEID	471938122022401	LOCAL NUMBER 21N/06E-	-0 9A 02
LATITUDE	47 d 19 m 38 s	LONGITUDE	122 d 02 m 24 s
ALTITUDE	525.00	DATE OF CONSTRUCTION	/ /
DEPTH OF WELL	39 00	DEPTH OF HOLE	0.00
HATER LEVEL	28.02	DATE MEASURED	08 / 15 / 1962
DRILLER	MRYL JOHNSN CC	DMSTRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF S	GEAL O METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DE	EVEL.	

REMARKS

OWNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

HOLE INFORMATION

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE 0F HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING MATERIAL THICKNESS

1 -0.30 0.00 5.00 0.000

\$\$ OPENINGS INFORMATION \$\$

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

******************* 3344 WELL CONSTRUCTION REPORT \$344 ********************

SITEID	46193312202140	1 LOCAL NUMBER 21N/06E-	-09H01
LATITUDE	47 d 19 m 33 s	LONGITUDE	122 d 02 m 14 s
ALTITUDE	515.00	DATE OF CONSTRUCTION	1 1
DEPTH OF WELL	48. 00	DEPTH OF HOLE	0.00
WATER LEVEL	25.00	DATE MEASURED	<i>i</i>
DRILLER		CONSTRUCTION METHOD	TYPE OF FINISH

TYPE OF SEAL

BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL. METHOD

HOURS OF DEVEL.

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
1	0.00	0.00	0.00

** CASING INFORMATION **

INTERVAL	TOP	BOTTOM	DIAMETER	CASING	CASING
NUMBER	CASING	Casing	CASIN S	MATERIAL	THICKNESS
1	0.00	0.00	0.00		0.000
<u>1</u>	0.00	4.44	v_*vv		0.000

** OPENINGS INFORMATION **

INTERVAL TOP TYPE BOTTOM TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE

TYPE OF POWER Ξ

INTRACTURE CONSTRUCTION REPORT AND INTEREST AND A WELL CONSTRUCTION REPORT AND ADDRESS OF THE PROPERTY OF THE

SITEID 471400122020401 LOCAL NUMBER 21N:08E-10E01

LATITUDE 47 d 19 m 00 s LONGITUDE 122 d 02 m 04 s

ALTITUDE 500.00 DATE OF CONSTRUCTION / /

DEPTH OF WELL 45.00 DEPTH OF HOLE 0.00

WATER LEVEL 25.00 DATE MEASURED / /

DRILLER NORTHWEST OR CONSTRUCTION METHOD TYPE OF FINISH

TYPE OF SEAL BOTTOM OF SEAL O METHOD OF DEVELOPMENT

DEVEL, METHOD HOURS OF DEVEL.

REMARKS

ONNER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

t* HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING MUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 0.00 6.00 0.00

* OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

********************* **** WELL CONSTRUCTION REPORT **** **********************

SITEID	471927122020401	LOCAL NUMBER 21N/06E	-10 E 02
LATITUDE	47 d 19 a 27 s	LONGITUDE	122 d 02 m 04 s
ALTITUDE	510.00	DATE OF CONSTRUCTION	I = I
DEPTH OF WELL	42.00	DEPTH OF HOLE	0.00
WATER LEVEL	30.67	DATE MEASURED	08 / 14 / 1962
DRILLER	CON	ISTRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF SE	TAL 0 METHOD OF	DEVELOPMENT
DEVEL. METHOD	HOURS OF DEV	EL.	

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS 1 0.00 0.00 0.00 0.000

OPENINGS INFORMATION

INTERVAL TOP MOTTOR TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF FUMP INTAKE 0

E

TYPE OF PONES

HORSEPOWER

0.33

SITEID	471924122026301 LOCAL NUMBER 21N/06	E-10E03
LATITUDE	47 d 19 m 24 s LONGITUDE	122 d 02 m (3 s
ALTITUDE	510.00 DATE OF CONSTRUCTIO	N 01 / 01 / 1952
DEPTH OF WELL	42.00 DEPTH OF HOL	E 0.00
WATER LEVEL	27.43 DATE MEASURE	D 08 / 14 / 1965
DRILLER	CONSTRUCTION METHOD	TYPE OF FINISH
TYPE OF SEAL	BOTTOM OF SEAL O METHOD O	F DEVELOPMENT
DEVEL. METHOD	HOURS OF DEVEL.	
REMARKS		

Remarks

OWNER ID SYSTEM NAME CONTACT PHONE

DWNER INFORMATION NOT AVAILABLE FOR THIS WELL.

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER NUMBER HOLE HOLE OF HOLE 1 0.00 0.00 0.00

11 CASING INFORMATION 11

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.30 0.00 5.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH HUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT J

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

DEVEL. METHOD HOURS OF DEVEL.

REMARKS

DANER ID SYSTEM NAME CONTACT PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER

NUMBER HOLE HOLE OF HOLE

1 0.00 0.00 0.00

** CASING INFORMATION **

INTERVAL TOP BOTTOM DIAMETER CASING CASING NUMBER CASING CASING CASING MATERIAL THICKNESS

1 0.00 0.00 2.00 0.000

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT P

DEPTH OF PUMP INTAKE 0

TYPE OF PONER E

sitiattiitiitiitiitiitiitiiti siti Well construction report tit sitittiitiitiitiitiitii

SITEID	471925.	122000101	LOCAL NUMB	BER 21% 066	E-11601	
LATITUD	E 47 d 1	7 a 25 s	Ĺ(INGITUDE	122 d 00) n 01 s
ALTITUD	E 640.)))	DATE OF CO	ONSTRUCTION	03 / 11 /	1981
DEPTH 0	F WELL. 127.0) 0	DEF	PTH OF HOLE	127.00	
WATER LEVEL 50.00 DATE MEASURED 03 / 11 / 1981						
DRILLER	JOHNSO1	i con	STRUCTION N	METHOD A	TYPE OF	FINISH P
TYPE OF	SEAL C	BOTTOM OF SEA	AL 18	METHOD OF	DEVELOPME	ENT B
DEVEL.	METHOD B	HOURS OF DEVE	EL. 3			
REMARKS						
OWNER ID	SYSTEM NAME			CONTACT		PHONE
CHEO&5	CHEATHAM. RAY					
	·	## HOLE I	INFORMATION	 		
INTERVAL NUMBER	TOP OF HOLE		F DIAM OF H			
1		40.00				
2	40.00	127.00	4	.00		
		is CASING 1	MFORMATION			
INTERVAL	TOP	MOTTOE	DIAMETER	CASIN	6 CAS	ING
NUMBER	CASING	CASING	CASING	MATERI	AL THIC	KNESS
	0.00					000
2	0.00	12700	4,00	5	0.	000
		tt OPENINGS	INFORMATIO	N 13		
	TOP BOTTON					
<u>+</u>	80.00 120.0	() F		0.00	0,000	0.00
		11 PUMP IN	FORMATION	**		
TYPE OF	LIFT	\$				
DEPTH OF	F PUMP INTAKE	()				
TYPE OF	POWER	Ξ				

0.75

HORSEPOWER

SITEID	471834122030201	LOGAL NUMBER 21N/06E	-16F01
LATITUDE	47 d 18 m 32 s	LONGITUDE	122 d 03 m 02 s
ALTITUDE	515.00	DATE OF CONSTRUCTION	05 / 24 / 1980
DEPTH OF WELL	103.00	DEPTH OF HOLE	103.00
WATER LEVEL	4300	DATE MEASURED	06 / 24 / 1980
DRILLER	NORTHWEST PD COM	ISTRUCTION METHOD A	TYPE OF FINISH O
TYPE OF SEAL	8 BOTTOM OF SE	TAL 18 METHOD OF	DEVELOPMENT
DEVEL METHOD	HOURS OF DEV	EL.	

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

OWNER INFORMATION NOT AVAILABLE FOR THIS WELL

* HOLE INFORMATION **

INTERVAL	TOP OF	BOTTOM OF	DIAMETER
NUMBER	HOLE	HOLE	OF HOLE
<u>‡</u>	0.00	103.00	4.00

CASING INFORMATION

INTERVAL	TOP	80TTOM	DIAMETER	CASING	CASING
NUMBER	CASING	CASING	CASING	MATERIAL	THICKNESS
11	0.00	103.00	6.00	S	0.000

** OPENINGS INFORMATION **

INTERVAL TOP 80TTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT

DEPTH OF PUMP INTAKE 0

TYPE OF POWER

613718	47181412202	4701	LOCAL NO	MBER 21N/06E	-16001	
LATITUDE	47 d 19 m 1	.4 5		LONGITUDE	122 d 02 m 47	5
ALTITUDE	525.00		DATE OF	CONSTRUCTION	04 / 15 / 1981	
DEPTH OF WELL	78.50		Ð	EPTH OF HOLE	7850	
WATER LEVEL	42.00		D	ATE MEASURED	04 / 15 / 1981	
DRILLER	NORTHWEST	CON	STRUCTION	METHOD A	TYPE OF FINIS	H O
TYPE OF SEAL	80770	H OF SE	AL 0	METHOD OF	DEVELOPMENT B	
DEVEL. METHOD	B HOURS	OF DEVI	EL. 1			

REMARKS

OWNER ID SYSTEM NAME

CONTACT

PHONE

ANDOOS ANDERSON, JOE

** HOLE INFORMATION **

INTERVAL TOP OF BOTTOM OF DIAMETER
NUMBER HOLE HOLE
1 0.00 78.50 6.00

** CASING INFORMATION **

CASING DIAMETER CASING INTERVAL TOP MOTTOR CASING CASING MATERIAL THICKNESS NUMBER CASING 78.50 6.00 0.000 1 -1.50

** OPENINGS INFORMATION **

INTERVAL TOP BOTTOM TYPE TYPE DIAMETER WIDTH LENGTH NUMBER SECTION SECTION OPENINGS MATERIAL OPENINGS OPENINGS

** PUMP INFORMATION **

TYPE OF LIFT S

DEPTH OF PUMP INTAKE 0

TYPE OF POWER E

SITEID	47.	1925121594801	LOCAL NUM	BER 21N/06	E-11H01	
LATITU	IDE 47	d 19 m 25 s	i. i.	ONGITUDE	121 d 5	9 a 48 s
ALTITU	DE 8	115.00	DATE OF C	ONSTRUCTIO	N 02 / 04	/ 1980
DEPTH	OF WELL	24000	DE	PTH OF HOL	E 240.00	
⊭ATER	LEVEL	40.00	DA	TE MEASURE	0 02 / 04	/ 1980
DRILLE	R JOH	INSON CO	NSTRUCTION	METHOD A	TYPE OF	FINISH P
TYPE 0	F SEAL C	BOTTOM OF S	EAL 18	METHOD O	F DEVELOPM	ENT B
DEVEL.	METHOD B	HOURS OF DE	VEL.			
REMARK	5					
OWNER ID	SYSTEM NAM	E		CONTACT		PHONE
REI342	REICHERT,	MATHEW				
		 ≱‡ HOLE	INFORMATION	į		
INTERVAL	TOP OF	MOTTOR	OF DÍAN	1ETER		
NUMBER 1	HOLE 0.00	HOLE	UF 1	10LE 5.00		
2	0.00 40.00			1.00		
			INFORMATION	i ii		
		** 0.:22712	en annin a			
		BOTTOM CASING				
1	0.00	60.00	6.00		0.	.000
2	0.00	240.00	4.00		0	.000
		11 OPENINGS	S INFORMATIO	N 11		
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TYPE OF POWER

HORSEPOWER

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APPENDIX E LAKE SAWYER SEPTIC LEACHATE SURVEY ENTRANCO ENGINEERS, INC.



ENTRANCO ENGINEERS. INC.

LAKE WASHINGTON PARK BUILDING (206) 827-1300 5808 LAKE WASHINGTON BOULEVARD N E KIRKLAND WA 98033

June 27, 1989

Mr. Tom Noyes Hart Crowser, Inc. 1910 Fairview Avenue East Seattle, Washington 98102-3699

Re: Lake Sawyer Septic Leachate Survey Field Survey Report Entranco Project No. 89047-60

Dear Tom:

This letter constitutes our field report for the performance of a septic leachate survey of selected shorelines along Lake Sawyer. The letter report describes methods, results, and reasons for premature termination of the survey.

Methods

A K-V Associates Model 15 septic leachate detector was used to perform the leachate survey. The detector is a portable fluorometer which operates at a fixed wavelength responsive to fluorescence of human urine and laundry whiteners. The unit was calibrated using open water from Lake Sawyer as a source for background adjustment and standard dilutions. A one percent urine solution was used as the calibration standard; a field measurement comparable to the response from a one percent solution is generally used as the threshold value strongly indicative of an inadequately treated wastewater plume. Backgrounds were adjusted to 0.10 on the output meter (range 0.0 to 1.0), with one percent standards generating responses of 0.38 to 0.58 operating at a signal multiplier of 4. Unit calibration was generally repeated on an hourly frequency throughout the survey.

The survey was conducted along approximately 1.1 miles of the eastern shoreline of the lake (see attached map), with the operator sweeping the intake wand of the instrument along a 2 meter swath in front while wading the shallow (0.5-1.2 meter) littoral. Detector measurements were compared to concurrent chloride ion probe measurements on a reach by reach basis along the shoreline. The survey was conducted during June 12-13 until leachate detector electronic problems forced a termination of the survey.

Results

There was an absence of conclusive septic leachate plume activity observed along the shorelines surveyed. No activity indicative of concentrated effluent strength plumes was discovered. There were some indications of slight fluorometric response (from a background of 0.10 to readings of 0.14 - 0.18) which were generally corroborated by independent chloride measurements. However, these were well below the threshold one percent response level and occurred along relatively broad reaches of the shoreline.

Mr. Tom Noyes June 27, 1989 Page Two

Observed bank cuts and outcrops indicated that indigenous soils along the surveyed reach were glacial outwash gravels and cobble. Soil conditions, field results, and past experience under similar conditions suggest that wastewater is either being adequately treated, or, if inadequately treated, is undergoing sufficient vertical and/or lateral dispersion from the drainfield that plumes either: (1) disperse in a broad uniform reach at plume concentrations too low for positive detection; or (2) subtend the shoreline and erupt at a deeper littoral location beyond the reach of the detector.

Results therefore suggest that, at least for the shoreline reach surveyed, the most appropriate strategy for characterization of nutrient loadings from on-site wastewater systems would not be by site specific plume monitoring but by a randomized shallow well point monitoring of groundwater along influent reaches of the shoreline.

Instrument Performance

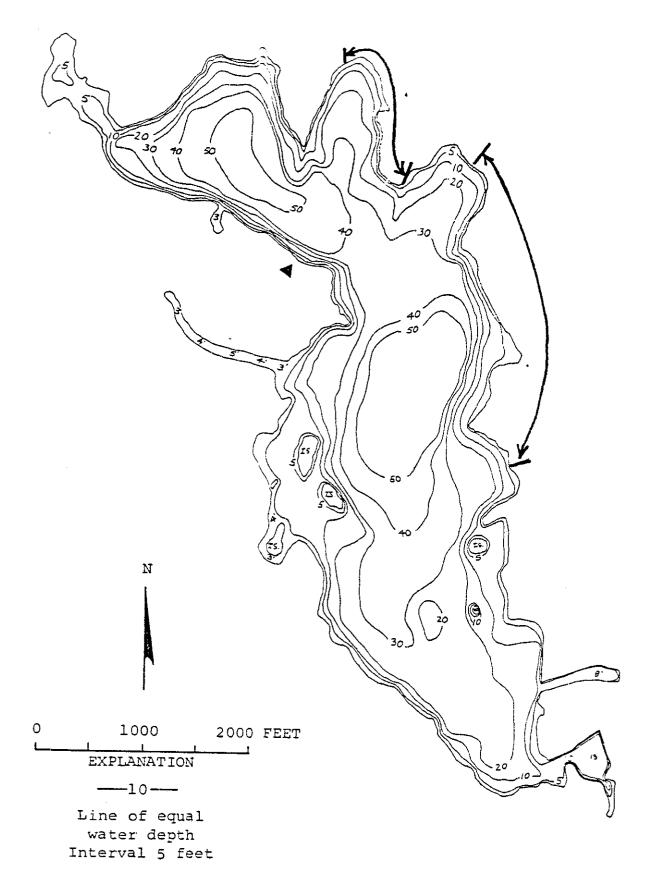
While the survey was prematurely terminated due to electronic difficulties with the leachate detector, it should be emphasized that the detector operated in a very sensitive and stable manner during the period it was operational. Based on our experience, we believe that the absence of effluent strength activity indicated by the detector during its period and reach of performance was valid.

Field failure of the leachate detector appeared to be attributed to heat fatigue of a heat resistor or the light source after a period of operation. K-V Associates was unable to identify and correct the problem when the unit was returned to their laboratory. Further repair efforts were deferred when it became apparent that the survey would not be able to be completed in time to assist in the location of shoreline monitoring wells.

While the potential for field survey delays due to instrument repair was clearly identified prior to the survey, we sincerely regret that we were not able to perform the entire survey to meet Hart Crowser's technical objectives and schedule constraints. We attempted to take all reasonable measures to minimize the potential for this occurrence. We hope that what information was obtained will assist you in the successful completion of the project.

Sincerely,

George S. Edwards, P.E.



Sawyer Lake, King County. From U.S. Geological Survey, June 6, 1973.



TRANSIT

NOTEBOOK NO. 301

FB 1399

LAKE SAWYER SEPTIC LEACHATE SURVEY

a product of

J. L. DARLING CORPORATION TACOMA, WASHINGTON 98421 U.S.A.

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DATE	PAGE	E X DESCRI	PTION	Late Sanger	4/17/89 11/1
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:					

Plue house, white trim awning on left New brown cedar, solonium - wa cabin, sliders in front, fieldstino walls seasult or wind soct in dal Map JW 3-21-1-1 Reige house, brown trim, pellous green out to umbrela Large cedar house, bushouse hummoch yellow house, ligh an bent, chain link fence & compre, gazabo Cal bration 10:19

Backgound .08

190 Du (4x) 36 From house, of balcones, drine bet Tan - green, order sied, blue war own whywich just usible among. Small, old brown wagning to white

~ 616 Large dark brown set in a back, shed rear water junger gym real shall, buck of for About set back cement stairing.

Her, sundy gray, on flat ground

rear water.

~620 New, gray-bace wholve aways, Sunstante in this section has Sulvacant lot (180ks like a parti 5mall old garage i 695 White Warren from wood porch 5chindeldecter sign, red dock 696 Small yellow, 5 dways on the Far Large, new, sand & beige, brick stop +

and para spit (are /

flower formaners)

	G/17 3/399
41 Swing on point large sandy house	
(Reading stone elevated approved to	8:00 Calibration
(Reading sight relevated appeared to	Background = 0.1
	170 - 0.35
Calibration 1:55 pm	8:10 map nw3-21-4
4x power	8:19 Lot 412 0.06 to 0.1
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46 large 2 story gray up concrete	Skip 3 Lots - 64-00 1
embrandment, Bock by dwine, board	8:36 Lot 76 Recallente = 0.08
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unt worldn't react to 1% STD	9:00 Lot # 2 Henry an point.
* Quit For Day Inst Down *	9:00 Lot # 2 Honor on point. O. (8 Took SAmplus than back to Background
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6/13	3/399
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8:11 LT 10 0,08	2 0.06 20 0.1
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	& Gray House
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	58

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<u> </u>	lean point	F	
1001	96 Calibration		
10:39	Lor 429 Gal		
-	170 0.15 of water New 100	1.1	
	Survey terminated for day due to operational problems wy leachate detector	6	
	leachate detector		

APPENDIX F MONITORING NETWORK ELEVATION SURVEY

APPENDIX F MONITORING NETWORK ELEVATION SURVEY

The monitoring wells' and wellpoints' top-of-casing elevations and the staff gage elevations were surveyed to an accuracy of 0.01 foot by Meridith, Inc. The U.S. Coastal and Geodetic Survey Benchmark Z253 (shown on Figure 2) with an elevation of 564.684 feet, relative to the USC&GS datum, was used as the reference elevation for this survey. After the elevation data were available for the monitoring wells and the wellpoints, Hart Crowser surveyed the top of casing elevations for the eight domestic wells used in the monitoring network, using the surveyed wellpoints as reference elevations. In addition, the elevations of wellpoints no. 1 and no. 2 were resurveyed due to inconsistent water level data. The original wellpoint elevations were confirmed. Meridith's elevation survey report is included in this appendix.

LAKE SAWYER ELEVATION DATA (REVISED 2/12/90)

<u>Point</u>	<u>Description</u> <u>E</u>	<u>levation (FT)</u>
BM Z253	3.5" brass cap in concrete abutment by the intersection of the Burlington Northern Railroad and Highway 169 marked USC&GS Z253, 1944	564.684
TBM A	PK nail on the centerline of driveway and the edge of 288th at the top of a steep downhill slope	605.532
MW 4	Top of PVC pipe	54929
TBM B	Spike in base of power pole at the intersection of 292nd and 232nd	544.558
TBM C	Spike in base of power pole No. 28850 at 28859 229th	528.998
WP 1	Top of PVC pipe	521.44
WP 2	Top of PVC pipe	521.74
WP 3	Top of PVC pipe	522.69
WP 4	Top of PVC pipe	520.56
WP 5	Top of PVC pipe	520.64
WP 6	Top of PVC pipe	521.13
TBM D	Spike in base of tree, 10 feet from road sign, at the intersection of 298th and 232nd	570.385
WP 7	Top of PVC pipe	520.68
TBM E	Spike in base of power pole No. 30067 at 30067 232nd S.E.	546.124
WP 8	Top of PVC pipe	520.90
TBM F	Spike in base of power pole on 232nd between the Falk and Morris homes	546.957
WP 9	Top of PVC pipe	521.82
WP 10	Top of PVC pipe	521.75
TBM G	Spike in base of 30" cedar approximately 300 feet east of gate at intersection of logging roads	552.200
MW 1	Top of PVC pipe	53564

<u>Point</u>	Description	Elevation (FT)
TBM H	Spike in base of 24" fir tree on West Edge Road near the southeast corner of Lake Sawyer	527.,128
SG 1	Rock Creek staff gage elevation at the 2.01 foot reading on the gauge (current WS)	519.58
TBM I	Spike in base of power pole No. 6 at the end of S.E. 312th	557.474
MW 3	Top of PVC pipe	560.84
SG 2	Etton's dock staff gage elevation on top of gage, 0.24'above reading 10.12	521 . 73
WP 11	Top of PVC pipe	520.91
твм ј	Spike in base of power pole at the intersection of S.E. 312th and 230th Place S.E.	556.033
твм к	Spike in base of power pole at the intersection of Lake Sawyer Road and S.E. 312th	543.442
TBM L	Spike in base of power pole at the intersection of Lake Sawyer Road and S.E. 307th	543 672
MW 2	Top of PVC pipe	57246
твм Еуе	Top of eye bolt in edge of dock 6.65 feet east of of Well Point No. 12	519.379
WP 12	Top of PVC pipe	519.55
TBM M	Spike in base of power pole at the intersection of Lake Sawyer Road and S.E. 304th Place	537.586
TBM Nail	Spike in base of power pole No. B59E2 at 30404 225th S.E.	543.639
WP 13	Top of PVC pipe	521.93
TEM N	West head bolt of fire hydrant at 29745 224th S.E.	524.129
SG 3	Top of fence post downstream from spillway	517.68
SG 4	Top of staff gage at spillway 0.98' above the 3.0 mark on the gage	520.51
TBM 0	North head bolt of fire hydrant at the intersection of S.E. 296th and 224th S.E.	538.269

Lake Sawyer Elevation Data (** **) Page 3

<u>Point</u>	Description	Elevation (FT)
WP 14	Top of PVC pipe	519.38
TBM P	Southwest head bolt of fire hydrant No. 33 approximately 300 feet north of fire station	532567
TBM Q	Spike in base of power pole at the intersection of Lake Sawyer Road and 216th S.E.	526.351
TBM R	Spike in base of power pole approximately 1,000 feet east of intersection of 216th and 292nd S.E.	525.079
WP 15	Top of metal pipe	520 . 27

Appendix 6.1b. Sediment core data (continued).

	Upper Horizon	Lower		Average Dry	Total Pb-210	Unsupported Pb-210	Stable Pb	Total P	Total N	70
tation	Depth	Depth	Solids	Density	(dpm/g)	(dpm/g)	(ppm DW)	(% DW)	(X DW)	(% DI
	(cm)	(cm)	*	(gDW/cm3)	dry wt.	dry wt.		(,,),,	(,,, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	• • • • • • • • • • • • • • • • • • • •
:=====						=======================================				
4	0	1	3.40	0.0410	2673 2073	2630 2890		0221 0182	113 122	13 12
4	1	2	4.86	00532	2933 2881	28.,38 28.,38		0182	1.04	11
4	2	3	604	00607	31.02	30,59		0.111	0.99	12
4	3	4	693	0.0751		29 ₄ 51		0,117	106	13
4	4	5	762	0.0867	29.94 71.05					12
4	5	6	802	00717	31.05	3062		0.108	103	
4	6	7	8.50	0.0936	27,,64	27,21		0.097	104	13
4	7	8	9.02	0.0931	22.92	2249		0090 0088	0.99 1.00	12 12
4	8	9	9.75	0.0977	24.,67	2424 2290		0.085	0.94	12
	9	10	1027	01225	2333					12
4	10	11	11.25	0,1162	23,41	22.98		0084	0.,94 0.,94	12
4	11	12	11.32	01225	21.27	20.84		0.083		
4	12	13	11.29	0.1283	18.96	18,53		0086	0.94	12
4	13	14	11.03	0.1214	19.14	18.71		0085	0.94	12
4	14	15	11.49	01139	13.59	1316		0078	086	12
4	15	16	1128	0.1249	14.715	1429		0074	089	13
4	16	17	10.07	0.1058	13.09	1266		0,070	092	14
4	17	18	8.,95	0.0948	10.2	9.77		0.067	091	14
4	18	19	9,,66	0 . 1058	6.26	583		0.068	100	15
4	19	20	9.53	01012	529	486		0073		
4	20	21	9.12	01006						
4	21	. 22	9 49	00902	4.63	420		0070	1.,14	17
4	22	23	9.24	0.1029						
4	23	24	9.92	01040	3.47	304		0.071	1.26	18
4	24	25	956	01000						
4	25	26	972	01023	3.32	289		0.072	127	18
4	26	27	908	0.0954						
4	27	28	9,56	00983	3.62	319		0074	125	19
4	28	29	8.90	0.0931						
4	29	30	9 64	01006	2,91	2.48		0075	126	19.
4	30	31	892	00965						
4	31	32	9.41	0.0948	2.86	243		0.075	130	19
4	32	33	864	00867						
4	33	34	9.05	0.0890	289	2.46		0.075	1.34	20.
4	34	35	8,46	0.,0665	•					
4	35	36	896	0.1052	. 2,,02	1.59		0077	136	20.
4	36	37	8.74	0.0792						
4	37	38	917	0.0988	168	125		0.,082	134	20.
4	38	39	8.58	0.0861						
4	39	40	9,19	01029	172	1.,29		0.082	133	19.
4	40	41	8.72	0.0908	· · · · -					
4	41	42	9.05	0.0884	151	108				
4	42	43	8.62	0.0925						
·		44	9.20	0.0896	1.03	0.,60				
4	43	45	8.61	0.0855	1.03	0.,00				
4	44		9,16	0.0867	145	102				
4	45	46		0.0837	1,43	1.02	•			
4	46	47	8.60 9.35	00954	112	069				
4	47	48		00934	1.12	0,09				
4	48	49	868							
4	49	50	8.80	0,0954						
4	50	51	8 84	0.0977						
4	51	52	8,98	0,0873						
4	52	53	8,99	0.0954						
4	53	54	8 89	00994						
- 4	54	55	9.04	00913						
4	55	56	919	0.0977						
4	56	57	9.46	0.0890						
4	57	58	9.43	0.0960						
4	58	59	9.50	0.0873	±					
4	59	60	1012	01075	0.43					

Appendix G.1a. Sediment core data.

Ctation	Upper	Lower Horizon	Percent	Average Dry	Total Pb-210	Unsupported Pb-210	Stable Pb	Total P	Total N	TOO
SCALION	Depth	Depth	Solids	Density	(dpm/g)	(dpm/g)	(ppm DW)	(% DW)	(% DW)	(% DW)
	(cm)	(cm)	*	(gDW/cm3)	dry wt.	dry wt.			********	
====== 3	 0	:======== 1	419	0.0486	25 01	23.66		0,,123	1.24	14.2
3	ĭ	ż	536	0.,0566	25.59	24.24		0.,166	1.33	13.9
3	ż	3	5.,96	0.0642	30.87	2 9 .,52		0129	1.27	13.8
3333333333333333333	3	4	6.62	0.0728	31.96	30.,61		0.117	1.,18	14.,9
3	4	5	6.93	0.0728	2669	25 , 34	32.3	0098	132	15.3
3	5	6	7.15	00746	2525	2390	463	0.107	117	15.6
₹	6	7	7.34	0.0832	22.70	21.35	311	0100	119	16.,2
3	7	8	773	00798	22.00	20,65	257	0098	113	152
~	8	9	814	0.0960	16.63	1528	28.9	0.,099	118	16.1
3	9	10	823	0.0798	1712	1577	24.6	0.115	117	16.2
~	10	11	8.91	0,,0925	17, 23	1588	26.4	0.092	1,14	15.5
~	11	12	9.54	0.1127	15,49	1414	21.2	0.096	108	15.,7
~	12	13	8.69	0.0902	13.54	1219	27.8	0.090	1.13	15.6
ž	13	14	8.80	0.0902	11.40	10 .05	23.9	0.083	103	161
7	14	15	8.58	01029	9.36	8.01	28.1	0.083	1.25	17.5
4	15	16	8.40	0.0850	7.76	6.41	24.3	0.082	117	178
7	16	17	7.49	0.0815	7.98	663	217	0.075	129	17.6
7	17	18	8.50	0.0977	5.61	426	154	0.054	128	19.0
2	18	19	8.89	0.0942	6.85	550	3.8	0085	132	19.5
3	19	20	9.10	0.0983	6.40	5,,05	13.1	0082	1,26	18.9
		21	9.43	0.0965	0.40	J.,UJ	10.5	0085	1,26	18.8
3	20	21	9,43 9,15	01064	5.73	438	15.5	0.005	1 4 20	10.0
3	21	23	9, 13	00954	7.13	7,30	9.1	0080	128	19.2
3 3 3 3 3	22		8.88	0 0867	509	374	11.3	0.,000	1,,20	,,,,
3	23	24		0.0960	J., U9	3.14	9.7	0082	1,29	20.0
3	24	25	9.09	0.0908	539	4.04	101	0.002	i a E 7	20.0
3	25	26	8.76		3"37	4,04		0.,082	143	20.5
3	26	27	8,54	00896	7 61	2 14	72 77	0.002	1.43	20.5
3	27	28	8.44	0.0861	3.51	2.16		0.007	1,40	20.6
3 3	28	29	861	0.0902	7 20	4.0/	78	0.083	1,40	20.6
3	29	30	826	00838	329	194	86			
3	30	31	NA	0.0838		4.00				
3	31	32	825	0.0838	3.33	198	8		4 77	24.0
3	32	33	8,40	0.,0861		4	5.6	0.081	137	21.0
3	33	34	8,21	0 0867	2.70	135	43			
3 3 3 3	34	35	NA	0.0850					•	
3	. 35	36	821	0.0832	314	179	6.6		4	1
3	36	37	860	00827			4.9	0089	152	21.4
3	37	38	809	00867	183	048	3.3			
, 3	38	39	874	0.0850			45	0085	1.52	22.0
3	39	40	836	00873	1.81	046	42			
3 3 3	40	41	8.42	0.0798			42	0088	150	21.9
3	41	42	7.84	00838	1 99	0.64	22			
.3	42	43	8.32	00769		•	42			
3	43	44	8.31	00844	2.07	072	3.3			
3 -	44	45	8.,71	00896			3.4			
3	45	46	8.46	0.0873	1.65	030	35			
3 3 3	46	47	1303	01347		•				
3	47	48	8 23	0.,0821	1.,57	0.22				
3	. 48	49	861	0.0844						
3	49	50	862	00855						
. 3	50	51	8.68	0.0827						
3	51	52	8.32	00850						
3	52	53	8,50	00832						
3	53	54	8.39	00867						
3	54	55	8.43	00838				0090	1.54	21.7
₹	55	56	8.68	00884						
3 3 3 3 3 3 3 3 3 3 3	56	57	8.68	00884						
3	57	58	8.68	0.0931						
_	58	59	9.05	00919						
3										

LAKE SAWYER ZOOPLANKTON

Copepods

DIAPTOMUS OREGONENSIS (Skistodiaptomus oregonensis)
EPISCHURA NEVADENSIS
CYCLOPS BICUSPIDATUS Thomasii
ORTHOCYCLOPS MODESTUS

Cladocera

DAPHNIA PULICARIA (could be D. pulex need adult males to be sure)
DAPHNIA THORATA
DAPHNIA GALEATA

DIAPHANOSOMA BIRGEI (D.leuchtenbergianum)

BOSMINA LONGIROSTRIS

KERATELLA COCHLEARIS

CERIODAPHNIA LACUSTRIS CERIODAPHNIA RETICULATA

CHYDORUS SPHAERICUS (Littoral species)

Rotifera

KERATELLA EARLINAE KERATELLA QUADRATA KELLICOTTIA LONGISPINA KELLICOTTIAL BOSTONIENSIS POLYARTHRA VULGARIS GROUP CONOCHILUS UNICORNIS CONOCHILUS HIPPOCREPIS TRICHOCERCA CYCLINDRICA TRICHOCERCA ROUSELETTI SYNCHAETA PECTINATA SYNCHAETA SPECIES SYNCHAETA STYLATA GASTROPUS STYLIFER POMPHOLOX COMPLANATA ASPLANCHNA PRIODONTA HEXARTHRA MIRA FILINIA SPP COLLOTHECA PELAGICA NOTHOLCA SQUAMULA NOTHOLCA ACUMINATA MONOSTYLA (LITTORAL) ASCHOMORPHA OVALIS (CHROMOGASTER OVALIS) ASCOMORPHELLA VOLVOCICOLA (preditory on volvox)

CILIATE PROTOZOA